APPENDIX X: TRAFFIC IMPACT ANALYSIS	

Final EFSEC Application for Site Certification

Horse Heaven Wind Farm

# **Traffic Impact Analysis**

Horse Heaven Wind Farm Benton County, Washington EFSEC Docket Number: EF-210011

Submitted to:

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Prepared for:

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## **EXECUTIVE SUMMARY**

Tetra Tech has reviewed the potential traffic impacts associated with the proposed peak construction activity of the Horse Heaven Wind Farm, LLC project (the Project) to be located in the Horse Heaven Hills area on unincorporated land in Benton County, Washington. Horse Heaven Wind Farm, LLC (Applicant) is proposing to construct and operate a renewable energy generation facility that would have a maximum grid injection capacity of up to 1,150 megawatts for a combination of wind and solar facilities, battery energy storage systems (BESS), and other Project components, including underground and overhead electrical collection lines, underground communication lines, new Project substations, access roads, operations and maintenance facilities, and meteorological towers.

At its closest point, the Project would be located approximately four miles south/southwest of the City of Kennewick and the larger Tri-Cities urban area, along the Columbia River. The Project's Lease Boundary (approximately 72,428 acres) incorporates all of the parcels for which the Applicant has executed a lease to construct the wind turbines, solar arrays, and associated facilities. The Project's Wind Energy Micrositing Corridor encompasses 11,850 acres within the Lease Boundary and consists of the areas where the turbines and supporting facilities would be sited during the final design. The Applicant seeks authorization for an original scope of up to 244 turbine locations and a maximum of three solar arrays, with all possible turbine locations and solar arrays cumulatively reviewed to analyze potential resource impacts.

There are two options for the wind turbines currently under consideration (Option 1 and Option 2). The final number and location of turbines within the proposed Wind Energy Micrositing Corridor would reflect the final engineering design, model selection, and any additional avoidance and mitigation identified in the Final Environmental Impact Statement (FEIS). The specific model used would depend on the commercial availability and technology at the time of construction. The number of turbines would not exceed 244, and the maximum turbine height (at blade tip) would not exceed 671 feet. The Draft Environmental Impact Statement (DEIS) previously prepared for the Project assumed that the road disturbance associated with Turbine Option 1 and Turbine Option 2 would be identical.

As currently proposed, the Project would be built in two construction phases (Phase 1 and Phase 2). Phase 1 construction could generate power via wind and solar. Phase 1 could also include a BESS capable of storing energy. Phase 2 construction has two alternatives (Phase 2a and Phase 2b). Phase 2a could consist of the construction of both wind and solar facilities. The Applicant's Phase 2a scenario also includes the construction of a BESS. Phase 2b could increase power generation via the construction of additional wind turbines, but construction would not include a BESS or solar array.

During peak construction of each phase, a typical day would include the transportation of workers, materials, and movement of heavy equipment. Numerous on-site roadways are proposed to support the Project as well as two laydown areas. The laydown areas would facilitate the delivery and assembly of materials and equipment. They would also serve as parking areas for the construction workforce. An additional tower transfer yard will be used to support the delivery of tower components prior to construction.

Preliminary transportation assessments were previously included in the December 2022 DEIS and February 2021 (as revised December 2022) updated Application for Site Certification (ASC) for the Project. Pending approval of the Site Certification Agreement (SCA), the Applicant now anticipates beginning construction of the first phase of the Project in late 2024 or early 2025 with commercial operation approximately 11 months following start of construction. A second phase of the Project would begin

construction in 2025 and begin operation in 2026. The construction schedule would be revised according to the actual approval of the Site Certification Agreement and implementation of commercial agreements for power purchase, and a copy provided to EFSEC at least 60 days prior to the start of construction or as required under the SCA.

Based on comments received on the DEIS, EFSEC has requested that a more detailed Traffic Impact Analysis be provided the Project.

A summary of the study methodology and key findings is presented below.

#### **Study Methodology**

The study methodology for the detailed Traffic Impact Analysis was developed in consultation with representatives from the Washington Department of Transportation (WSDOT) South Central Region at a traffic scoping meeting for the Project that was held virtually on June 7, 2023. The purpose of the meeting was to discuss the initial assumptions documented in the Project Traffic Scoping Letter (TSL) dated May 24, 2023, which identified key aspects of the traffic study methodology including the study area roadways and intersections to be reviewed, consideration of other possible area developments and background traffic growth, and anticipated vehicle trip generation and trip distribution characteristics of the Project and the analysis required to evaluate the potential Project-related traffic impacts.

The study evaluates existing and future traffic operations (with and without the proposed Project) at 29 existing intersections, and the proposed site driveways serving two laydown areas and 10 roadway segments for key roadways serving the Project site. The study provides a detailed analysis of roadway and intersection capacity during the weekday morning and weekday evening peak hours, when the combination of existing traffic on the surrounding area roadways and new traffic associated with peak construction activity of the Project would be greatest.

The 2023 Existing traffic volumes were developed based on intersection Turning Movement Counts (TMCs) collected at the 29 existing study area intersections on Tuesday June 13<sup>th</sup> and Wednesday June 14<sup>th</sup> 2023. Construction hours of operation are anticipated to generally occur from 7:00 AM to 7:00 PM. For the purposes of the study, the TMCs were collected at the study area intersections from 5:00 AM to 8:00 AM and 4:00 PM to 8:00 PM to establish the weekday morning and evening commuter peak hour conditions. In addition, Automatic Traffic Recorder (ATR) counts collected over a 24-hour weekday period in June 2023 for eight study area roadways. The ATR data collected as part of this study was then supplemented with daily and peak period traffic count data for two additional study area roadways obtained from the WSDOT permanent traffic count stations located on Route 221 and I-82 in the vicinity of the Project site.

As part of the assessment of existing traffic conditions, Tetra Tech conducted extensive field observations, which included photos of the study area intersections, and dash cam video footage along all of the principal roadways in the vicinity of the Project site. Field observations were used to document existing roadway and intersection lane geometry, posted speed limits and traffic control. Tetra Tech obtained crash data from WSDOT for the most recent five-year period available to identify possible existing traffic safety deficiencies at the study area intersections.

Tetra Tech contacted WSDOT and Benton County planning staff to identify what, if any, planned roadway improvements (implemented by others) would be constructed either before or during the construction of the Horse Heaven Wind Farm Project. Based on these discussions, there are currently no planned projects

that would result in changes in future traffic operations at the study area roadways and intersections within the anticipated time frame of the construction of the proposed Project.

As currently proposed, the Project would be constructed in two phases, which would occur in successive 11-month periods, with Phase 1 beginning in late 2024/early 2025. As a conservative measure, the future design year chosen for analysis in the TIA was the year 2025 for Phase 1 and the year 2026 for Phase 2.

The 2023 Existing peak hour traffic volumes were projected to the future design years of 2025 and 2026, by which time the Project's peak construction activity is expected to occur. The 2023 Existing traffic volumes were grown by 1.0 percent per year for the two design year forecast periods (2025 and 2026) and the traffic volumes adjusted to reflect the 2025 No Build (Without Project) and 2026 (Without Project) conditions.

It is expected that Phase 1 will begin construction on the eastern side of Interstate 82 (I-82). As Phase 1 construction continues, it is expected that peak construction activity will move westward across the Project lease area. To be conservative with our analysis, peak construction activity for Phase 1 was analyzed travelling to Laydown Yard 1 and Laydown Yard 2. It is expected that Phase 2 will only occur on the western side of the Project lease area. Peak construction activity for Phase 2 was only analyzed heading to Laydown Yard 2.

The traffic increases associated with the Project's peak construction activity during Phase 1 were then added to the 2025 No Build (Without Project) weekday peak hour traffic volumes to reflect the 2025 Phase 1 Build peak hour traffic volumes, and the traffic increases associated with Phase 2 were added to the 2026 No Build (Without Project) peak hour traffic volumes to reflect the 2026 Build (With Phase 2 Peak Construction) conditions.

Roadway and intersection capacity analyses were then conducted at each of the study intersections for the following scenarios to identify potential existing and projected traffic deficiencies near the Project site:

- 2023 Existing AM/PM Peak Hours
- 2025 No Build (Without Project) AM/PM Peak Hours
- 2026 No Build (Without Project) AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #1 AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #2 AM/PM Peak Hours
- 2026 Phase 2 Build to Laydown Yard #2 AM/PM Peak Hours

#### **Future Site-Generated Traffic**

The Project will consist of three stages: construction, O&M, and decommissioning. The highest volume of site-related trips will occur during the peak construction phase of the Project. Consequently, the traffic impact analysis in this report is based on the vehicle trip generation associated with the peak construction phase workforce levels. Preliminary vehicle trip generation estimates were included in the Project's ASC. These estimates were further refined as part of the previously prepared TSL to inform the study area and be used as the basis for analysis in the TIA.

Construction of the proposed energy facility is expected to include site grading, construction of temporary access roads, wind turbine and solar panel installation, inspections, and equipment deliveries. It is anticipated that, at peak operations, the site could experience construction workforce levels of up to 467 construction workers at one time. There are no public transportation services in the vicinity of the Project site that are anticipated to be used for the Project. Therefore, for the purposes of this assessment, it was assumed that no construction workers would use public transit to access the site. However, it is anticipated that some construction workers would arrive and depart the site together (carpooling). For purposes of this assessment, it was assumed that the average vehicle would have 1.25 occupants to represent carpooling to/from the site.

Construction hours of operation are assumed to generally be 7:00 AM to 7:00 PM with the majority of construction workers arriving prior to 7:00 AM and departing after 7:00 PM. Since the peak hours of the adjacent street traffic are expected to occur sometime during the peak commuting periods of 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM, it is expected that the majority of construction workers would arrive and depart the site outside of the typical weekday morning and weekday evening commuter peak hours of the adjacent streets. However, to present a conservative assessment of potential traffic increases associated with the Project, it is assumed that all of the construction workers would arrive during the weekday morning peak hour and depart during the weekday evening peak hour. The proposed Project is expected to generate approximately 1,498 weekday daily workforce trips and 374 weekday peak hour workforce trips during Phase 1 peak construction activity.

The peak construction activities are currently anticipated to occur for only a portion of each the two successive 11-month periods. The remainder of the construction period is anticipated to generate fewer vehicle trips. The supporting trip generation calculations and assumptions for the proposed Project's peak construction workforce levels are provided in Section 3.2.1

There are currently two alternatives being considered for Phase 2 of the Project construction. Table 7 presents a summary of the trip generation estimates for the proposed Project's peak construction workforce activities by phase. The more conservative trip generation of the two Phase 2 alternatives (Phase 2a) will be used for the TIA analyses. Phase 2a peak construction activity is expected to generate approximately 1,376 weekday daily workforce trips and 344 weekday peak hour workforce trips.

#### **Project Trip Distribution Patterns**

Tetra Tech developed separate Project trip distribution patterns for the Project's peak construction activities at the two proposed laydown areas. The trip distribution patterns were developed based on consideration of the effective population within the anticipated employment workforce (approximate two-hour drivetime zone) and the currently proposed location of the laydown area for each phase.

#### **Intersection Capacity Analysis**

To quantify potential traffic impacts associated with the proposed development, Tetra Tech conducted intersection capacity analyses at key intersections near the Project site for the following scenarios.

- 2023 Existing AM/PM Peak Hours
- 2025 No Build (Without Project) AM/PM Peak Hours
- 2026 No Build (Without Project) AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #1 AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #2 AM/PM Peak Hours
- 2026 Phase 2 Build to Laydown Yard #2 AM/PM Peak Hours

The capacity analyses indicate that all study area intersections and roadways operate well below capacity at level-of-service (LOS) C or better operations during the weekday peak hours. Therefore, there is ample capacity at the study area intersections and roadways to support the peak construction operations associated with the Project.

#### **Traffic Safety Analysis**

Tetra Tech conducted a traffic safety analysis for each of the study area intersections based on procedures outlined in the Safety Analysis Guide published by the Washington State Department of Transportation (WSDOT) using the Highway Safety Manual (HSM) spreadsheet tool, version 9.1 (<a href="http://safetyperformance.org/tools/">http://safetyperformance.org/tools/</a>). Tetra Tech obtained Crash Data from WSDOT for the study area roadways and intersections for the most recent five-year period available. Highway Safety Manual (HSM) predictive methods were used to analyze safety at each of the study area intersections in accordance with WSDOT guidelines using Transportation Research Board (TRB) Federal Highway Administration (FHWA) IHSDM software.

The HSM analysis indicates that four of the study area intersections, listed below, could potentially be improved with safety improvements:

- Route 221 at Sellards Road
- Route 221 at Route 14
- Route 14 at S. Plymouth Road
- Webber Canyon Road and Badger Road

The HSM spreadsheet tool was used to calculate the Predicted Crash Frequency and the Expected Crash Frequency for each intersection to determine which intersections had potential for traffic safety improvement. Tetra Tech analyzed the effectiveness of potential safety mitigation measures to address existing safety deficiencies, including the provisions of additional lane geometry (separate left-turn or right-turn lanes on the main road approaching the intersection) and providing street lighting where it is not currently provided.

The HSM spreadsheet tool indicates that the provision of separate left-turn and right-turn lanes and street lighting (where not currently provided) could potentially reduce the crash frequency at these intersections. An auxiliary lane warrant analysis to determine whether or not installation of a turn lane at these four locations would be warranted. Two of the study intersections (Route 14 at S. Plymouth Road and Route 221 at Route 14) meet the traffic volume threshold for implementation of additional turn lanes under existing traffic volume conditions (independent of the Project construction activity). The left-turn lane warrant was not met for any of the analysis scenarios.

In addition, the available Stopping Sight Distance (SSD) for vehicles approaching the high crash frequency intersections is well in excess of the required SSD for the posted speed limit. This indicates that drivers approaching the intersection will have more than sufficient view of potential turning traffic at the intersection to either stop or adjust their speed to avoid a collision.

The proposed Project is not anticipated to materially impact future traffic operations or safety at any of the study area intersections. However, as part of the proposed Project, the Applicant will develop a comprehensive Construction Traffic Management Plan (CTMP) for each phase of the construction to alert drivers of potential increased construction traffic and turning activities at key intersections throughout the study area. The Applicant will also work with the general contractor (once selected) to identify truck haul routes to and from the construction laydown areas to avoid sensitive areas and minimize impacts to local agricultural activity along the study area roadways, to the extent possible. In addition, the Applicant will prepare a Traffic Safety Plan (TSP) for the Project-related construction traffic activity in accordance with WSDOT standards. A detailed description of the draft TSP is provided in the subsequent sections of this report.

#### **Travel Demand Management Measures**

There are currently no public transportation services in the vicinity of the proposed laydown areas. However, the Applicant commits to implementing a Transportation Demand Management (TDM) program to reduce automobile travel and traffic impacts associated with the Project construction. Potential TDM measures include the following:

- Encourage carpooling among the construction workforce
- Provide a transportation coordinator who can match workers for carpooling
- Explore the feasibility of providing shuttle bus service between the proposed laydown areas and the construction sites
- Explore the feasibility of coordinating a Vanpool service from select locations within the Tri-Cities area.

#### **Conclusions**

The proposed Project is anticipated to generate negligible traffic once constructed. During the peak construction phases of the Project, a workforce of up to 467 construction workers is anticipated. Tetra Tech has evaluated the potential traffic impacts associated with this temporary increase in traffic at 29 existing study area intersections and the proposed site driveways at the two laydown areas. The analysis indicates that ample capacity is available within the study area to support the peak construction workforce levels associated with the Project with all intersections operating well below capacity at LOS C or better operations.

The safety analysis conducted as part of this study indicates that four of the existing study intersections would benefit from potential safety mitigation measures to address existing safety deficiencies. An auxiliary lane warrant analysis indicates that two of the study intersections (Route 14 at S. Plymouth Road and Route 221 at Route 14) currently meet the traffic volume threshold for implementation of separate right-turn lanes under existing traffic volume conditions (independent of the Project construction activity). However, the intersection capacity analysis indicates that these intersections will continue to operate well below capacity during all phases of Project construction. In addition, the available sight distances at the high crash rate intersections are well above the required SSD, indicating that motorists approaching these locations will have more than sufficient view of the potential turning movements at the intersections to stop or adjust their speeds as needed to avoid a potential collision.

Given the temporary nature of the potential traffic increases associated with Project construction, no additional turn lanes are recommended at these intersections by the Applicant as part of the proposed Project at this time. However, the Applicant will develop a comprehensive CTMP, for each

phase of the construction, to alert drivers of potential increased construction traffic and turning activities at key intersections throughout the study area. The Applicant will also work with the general Balance of Plant, or construction contractor (once selected) to identify truck haul routes to and from the construction laydown areas to avoid sensitive areas and minimize impacts to local agricultural activity along the study area roadways, to the extent possible. In addition, the Applicant will prepare a TSP for the Project-related construction activity in accordance with WSDOT standards. The Applicant will also work with the contractor to develop a TDM program to minimize reliance on single-occupant vehicles and reduce single-occupancy vehicle trips to and from the site.

Based on the analyses presented in this report and upon implementation of the recommended CTMP and TSP, the potential traffic increases associated with the proposed Project can be safely accommodated at the laydown area driveways and existing study area intersections and roadways with no significant impact to future traffic operations on the surrounding area roadway network.

## 1.0 INTRODUCTION

#### 1.1 PROJECT DESCRIPTION

Horse Heaven Wind Farm, LLC (Applicant) is proposing to construct and operate the Project in unincorporated Benton County, Washington, within the Horse Heaven Hills area. The Project would consist of a renewable energy generation facility that would have a maximum grid injection capacity of up to 1,150 megawatts for a combination of wind and solar facilities, battery energy storage systems (BESS), and other Project components, including underground and overhead electrical collection lines, underground communication lines, new Project substations, access roads, operations and maintenance facilities, and meteorological towers.

At its closest point, the Project would be located approximately four miles south/southwest of the City of Kennewick and the larger Tri-Cities urban area, along the Columbia River. Figure 1 shows the current Project Lease Boundary and Project vicinity. The Project's Lease Boundary (approximately 72,428 acres) incorporates all of the parcels for which the Applicant has executed a lease to construct the turbines, solar arrays, and associated facilities. The Project's Wind Energy Micrositing Corridor encompasses 11,850 acres within the Lease Boundary and consists of the areas where the turbines and supporting facilities would be sited during the final design. The Applicant seeks authorization for an original scope of up to 244 turbine locations and a maximum of three solar arrays, with all possible turbine locations and solar arrays cumulatively reviewed to analyze potential resource impacts.

There are two options for the wind turbines (Option 1 and Option 2). The final number and location of turbines within the proposed Wind Energy Micrositing Corridor would reflect the final engineering design, model selection, and any additional avoidance and mitigation identified in the Final Environmental Impact Statement (FEIS). The specific model used would depend on the commercial availability and technology at the time of construction. The number of turbines would not exceed 244, and the maximum turbine height (at blade tip) would not exceed 671 feet. The Draft Environmental Impact Statement (DEIS) previously prepared for the Project assumed that the road disturbance associated with Turbine Option 1 and Turbine Option 2 would be identical.

As currently proposed, the Project would be built in two construction phases (Phase 1 and Phase 2). Phase 1 construction could generate power via wind and solar. Phase 1 could also include a BESS capable of storing energy. Phase 2 construction has two alternatives (Phase 2a and Phase 2b). Phase 2a could consist of the construction of both wind and solar facilities. The Applicant's Phase 2a scenario also includes the construction of a BESS. Phase 2b could increase power generation via the construction of additional wind turbines, but construction would not include a BESS or solar arrays.

During peak construction of each phase, a typical day would include the transportation of workers, materials, and movement of heavy equipment. Numerous on-site roadways are proposed to support the Project as well as two laydown areas as shown in Figure 1. The laydown areas would facilitate the delivery and assembly of materials and equipment. They would also serve as parking areas for the construction workforce. An additional tower transfer yard will be used to support the delivery of tower components prior to construction.

#### 1.2 STUDY METHODOLOGY

The traffic study methodology was developed in consultation with representatives from the Washington Energy Facility Site Evaluation Council (EFSEC) and the Washington Department of Transportation (WSDOT) South Central Region at a traffic scoping meeting for the Project that was held virtually on June 7, 2023. The purpose of the meeting was to discuss the assumptions from the Traffic Scoping Letter (dated May 24, 2023), which identified key aspects of the traffic study including the study area roadways and intersections to be reviewed, consideration of other possible area developments and background traffic growth, and analysis required to evaluate the potential Project-related traffic impacts. The Traffic Scoping Letter is included in Appendix A.

This Traffic Impact Analysis (TIA) provides a detailed analysis of existing and future traffic operations (both with and without the proposed Project) during the weekday morning and weekday evening peak hours at the study area intersections (including the proposed site driveways at the two laydown areas) identified through consultation with EFSEC and WSDOT officials.

This study was conducted in three phases. The first phase involved an inventory of existing traffic conditions in the vicinity of the site. As part of the existing conditions assessment, peak period traffic counts were collected in June 2023. A field visit was conducted to inventory roadway and intersection geometries and traffic control as well as to observe the general operational characteristics for each of the study area intersections. WSDOT crash data was also reviewed.

The second phase of the study builds upon the data collected in the first phase and establishes the framework for evaluating potential traffic impacts associated with the Project. The following scenarios were developed to analyze these impacts.

- 2023 Existing AM/PM Peak Hours
- 2025 No Build (Without Project) AM/PM Peak Hours
- 2026 No Build (Without Project) AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #1 AM/PM Peak Hours
- 2025 Phase 1 Build to Laydown Yard #2 AM/PM Peak Hours
- 2026 Phase 2 Build to Laydown Yard #2 AM/PM Peak Hours

In the third phase of this study, the existing and projected future traffic operations at each of the study intersections were analyzed to identify potential traffic operational deficiencies and, if warranted, potential improvements to mitigate the Project's peak construction traffic impacts.

#### 2.0 EXISTING CONDITIONS

The effective evaluation of potential transportation impacts associated with the Project requires a thorough understanding of the existing traffic conditions on the roadways and intersections in the vicinity of the Project site. The existing conditions assessment consists of an inventory of the roadway and intersection geometries and traffic control devices; projection of peak period traffic volumes; field observations; safety analysis; review of pedestrian, bicycle; and analysis of existing traffic operations.

#### 2.1 STUDY AREA ROADWAYS

The site is located within unincorporated Benton County, in an area known as the Horse Heaven Hills. The Project site is bisected by regional travel routes, the majority of which are owned and operated by the Washington Department of Transportation (WSDOT) and Benton County. The Project-generated construction traffic will travel to and from the site via the following key study area roadways.

**Interstate 82.** Interstate 82 (I-82) is classified as a US Interstate Highway and is under WSDOT jurisdiction. It generally runs east to west with two travel lanes in each direction between Exit 82 and Exit 113. I-82 runs north to south from Exit 113 to Exit 131. The posted speed limit is 70 miles per hour (mph) between exit 82 and exit 131. I-82 is a rural interstate. Interchanges provide access to nearby communities and cities including Prosser, Benton City, Richland, and Kennewick.

**WSDOT Route 221.** Route 221 is classified as a rural minor arterial and is under WSDOT jurisdiction. Within the study area, Route 221 provides one travel lane in each direction. The posted speed limit along Route 221 in the study area is 65 mph. Land use along this roadway is primarily agricultural with some residential and commercial uses near the intersection with Route 14.

**WSDOT Route 397.** Route 397 is classified as an urban minor arterial and is WSDOT jurisdiction. Within the study area, Route 397 provides one travel lane in each direction. The posted speed limit along Maple Street in the study area is 60 mph. Limited commercial and residential activity is found along Route 397 within the study area. Route 397 is signed as a truck route, providing access between I-82 and industrial uses in Finley and south Kennewick.

**Bofer Canyon Road.** Bofer Canyon Road is classified as a rural local access roadway and is under local jurisdiction (Benton County). Within the study area, Bofer Canyon Road provides one travel lane in each direction. Bofer Canyon Road generally runs in a north-south direction and runs parallel to I-82. The posted speed limit along North Avenue is 50 mph. Land uses along Bofer Canyon Road in the vicinity of the Project is limited to agriculture.

**Locust Grove Road.** Locust Grove Road is classified as a rural minor collector under local jurisdiction (Benton County). Within the study area, Locust Grove Road typically consists of one travel lane in each direction. Locust Grove Road runs in an east-west direction and has a generally straight alignment. The posted speed limit along Locust Grove Road is 50 mph. Land uses along Locust Grove Road include agriculture, religious and commercial uses.

**Sellards Road.** Sellards Road is classified as a rural major collector under local jurisdiction (Benton County). Within the study area, Sellards Road typically consists of one travel lane in each direction. Sellards Road runs in an east-west direction and has a generally straight alignment. The posted speed limit along Sellards Road is 50 mph. Land uses along Sellards Road consist of nearly all agricultural uses.

#### 2.2 STUDY AREA INTERSECTIONS

The study area intersections chosen for detailed analysis were determined in consultation with the Washington Energy Facility Site Evaluation Council (EFSEC) and WSDOT. All study area intersections chosen are unsignalized. The study area intersections are shown in Figure 1 and are listed below:

- Wine Country Road at I-82 NB Ramps
- Wine Country Road at I-82 SB Ramps



- Wine Country Road at Route 22
- Route 22 at Route 221 and Paterson Avenue
- Route 221 at County Well Road
- Route 221 at Cemetery Road
- Route 221 at Sellards Road
- Webber Canyon Road at County Well Road
- Cemetery Road at Travis Road
- S. Plymouth Road at Locust Grove Road
- Route 221 at Route 14
- Route 14 at S. Plymouth Road
- Route 14 at I-82 SB Ramps
- Route 14 at I-82 NB Ramps
- Coffin Road at I-82 SB Ramps
- Coffin Road at I-82 NB Ramps
- Coffin Road at Bofer Canyon Road
- Bofer Canyon Road at Beck Road
- Locust Grove Road at I-82 SB Ramps
- Locust Grove Road at I-82 NB Ramps
- Route 397/Locust Grove Road at Bofer Canyon Road
- Route 397 at S. Owens Road
- Route 397 at S. Nine Canyon Road
- Route 397 at S. Finley Road
- Nine Canyon Road at Kirk Road
- Nine Canyon Road at Beck Road
- Nine Canyon Road at Coffin Road
- Locust Grove Road at S. Clodfelter Road
- Webber Canyon Road at Badger Road

The existing lane geometry and traffic control at each of the study area intersections is documented in the capacity analysis provided in Appendix I of this report and detailed for the key intersections below.

Wine Country Road at I-82 Northbound Ramps. Wine Country Road intersects the I-82 northbound ramps to form a three-way, unsignalized intersection. The Wine Country Road eastbound approach consists of a single through/right lane. The Wine Country Road westbound approach consists of a through lane and a left-turn lane. The I-82 Ramp northbound approach consists of a single shared left/right-turn lane and is under STOP control. There are currently no sidewalks or crosswalks at the intersection. Land uses adjacent to the intersection include agriculture and single-family homes.

Wine Country Road at I-82 Southbound Ramps. Wine Country Road intersects the I-82 southbound ramps to form a three-way, unsignalized intersection. The Wine Country Road eastbound approach consists of a single through/right lane. The Wine Country Road westbound approach consists of a through lane and a left-turn lane. The I-82 Ramp northbound approach consists of a single shared left/right-turn

lane and is under STOP control. There are currently no sidewalks or crosswalks at the intersection. Land adjacent to the intersection remains undeveloped.

Route 22 at Wine Country Road and Chapman Lane. Route 22 intersects Wine Country Road from the south and Chapman Lane from the north to form a four-way, unsignalized intersection. The Wine Country Road eastbound approach to the intersection consists of one left-turn lane and a shared through/right lane. The Wine Country Lane westbound approach consists of one left-turn lane and a shared through/right lane. The Route 22 northbound approach consists of a shared left/through lane and a right-turn lane. The northbound approach is under STOP control. The Chapman Lane southbound approach is opposite the Route 22 northbound approach and provides a single general-purpose lane. Chapman Lane is under STOP control. Sidewalks are located on the south side of Wine Country Road west of the intersection. Adjacent land uses are commercial (Wineries, Assisted Living Center).

Route 22 at Route 221 and Paterson Avenue. Route 221 intersects Route 22 from the east and Paterson Avenue from the west to form a four-way, unsignalized intersection. The Paterson Avenue eastbound approach to the intersection consists of a single general-purpose lane. The Paterson Avenue and Route 221 approaches are under STOP control. The Route 221 westbound approach consists of a single general-purpose lane. The Route 22 northbound approach consists of one left-turn lane and a shared through/right-turn lane. The Route 22 southbound approach one left-turn lane and a shared through/right-turn lane. There are currently no sidewalks or crosswalks at the intersection. Adjacent land uses to the intersection consist of commercial and residential (single family home) uses.

**Route 221 at County Well Road.** County Well Road approaches Route 221 from the northeast to form a three-way, unsignalized intersection. The Route 221 southeast and northwest approaches each consist of a single general-purpose lane. The County Well Road southwest approach consists of a single general-purpose lane and is under STOP control. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**Route 221 at Cemetery Road.** Route 221 intersects Cemetery Road from the north and south to form a four-way, unsignalized intersection. The Cemetery Road eastbound and westbound approaches consist of a single lane and are under STOP control. The Route 221 northbound and southbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**Route 221 at Sellards Road.** Route 221 intersects Sellards Road from the north and south to form a fourway, unsignalized intersection. The Sellards Road eastbound and westbound approaches consist of a single lane and are under STOP control. The Route 221 northbound and southbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**Webber Canyon Road at County Well Road.** County Well Road approaches Webber Canyon Road from the southwest to form a three-way, unsignalized intersection. The Webber Canyon Road southeast and northwest approaches each consist of a single general-purpose lane. The County Well Road northeast approach consists of a single general-purpose lane and is under STOP control. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**Cemetery Road at Travis Road.** Travis Road intersects Cemetery Road from the north and south to form a four-way, unsignalized intersection. The Cemetery Road eastbound and westbound approaches each consist of a single travel lane under STOP control. The Travis Road northbound and southbound

approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**S. Plymouth Road at Locust Grove Road.** Locust Grove Road intersects Plymouth Road from the east to form a three-way, unsignalized intersection. The Locust Grove Road westbound approach consists of a single lane and is under STOP control. The Plymouth Road northbound and southbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Agricultural land uses surround the study intersection.

**Route 221 at Route 14.** Route 221 and Paterson Road intersect Route 14 from the north and south to form a four-way, unsignalized intersection. The Route 14 eastbound approach consists of a single lane general-purpose lane. The Route 14 westbound approach consists of a shared left-turn/through lane and a right-turn lane. The Paterson Road northbound approach consists of a single general-purpose lane under STOP control. The Route 221 southbound approach consists of a single general-purpose lane under STOP control. There are currently no sidewalks or crosswalks at the intersection. Commercial and residential land uses are present north of the intersection.

Route 14 at S. Plymouth Road. S. Plymouth Road intersects Route 14 from the north and south to form a four-way unsignalized intersection. The S. Plymouth Road northbound and southbound approaches each consist of a single general-purpose lane and operate under STOP control. The Route 14 eastbound and westbound approaches each consist of a left-turn lane and a shared thru/right-lane. There are currently no sidewalks or crosswalks at the intersection. Commercial and residential land uses are present north of the intersection.

**Route 14 at I-82 Southbound Ramps.** The I-82 southbound on- and off-ramps intersect Route 14 from the north and south to form a four-way unsignalized intersection. The southbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Route 14 eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Route 14 at I-82 Northbound Ramps.** The I-82 northbound on- and off-ramps intersect Route 14 from the north and south to form a four-way unsignalized intersection. The northbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Route 14 eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Coffin Road at I-82 Southbound Ramps.** The I-82 southbound on- and off-ramps intersect Coffin Road from the north and south to form a four-way unsignalized intersection. The southbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Coffin Road eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Coffin Road at I-82 Northbound Ramps.** The I-82 northbound on- and off-ramps intersect Coffin Road from the north and south to form a four-way unsignalized intersection. The northbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Coffin Road eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Coffin Road at Bofer Canyon Road.** Bofer Canyon Road intersects Coffin Road from the north and south to form a four-way unsignalized intersection. The Bofer Canyon Road approaches each consist of a single general-purpose lane and operate under STOP control. The Coffin Road eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is partially used for agriculture.

**Bofer Canyon Road at Beck Road.** Bofer Canyon Road intersects Beck Road from the north and south to form a four-way unsignalized intersection. The Bofer Canyon Road approaches each consist of a single general-purpose lane and operate under STOP control. The Beck Road eastbound and westbound approaches each consist of a single general-purpose lane and operate under STOP control. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is partially used for agriculture.

Locust Grove Road at I-82 Southbound Ramps. The I-82 southbound on- and off-ramps intersect Locust Grove Road from the north and south to form a four-way unsignalized intersection. The southbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Locust Grove Road eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Locust Grove Road at I-82 Northbound Ramps.** The I-82 northbound on- and off-ramps intersect Locust Grove Road from the north and south to form a four-way unsignalized intersection. The northbound off-ramp approach consists of a single general-purpose lane and operate under STOP control. The Locust Grove Road eastbound and westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

Route 397/Locust Grove Road at Bofer Canyon Road. Bofer Canyon Road intersects Locust Grove Road/Route 397 from the north and south to form a four-way unsignalized intersection. The Bofer Canyon Road approaches each consist of a single general-purpose lane and operate under STOP control. The Locust Grove Road eastbound and Route 397 westbound approaches each consist of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is partially used for agriculture.

**Route 397 at S. Owen Road.** S. Owens Road intersects Route 397 from the north to form a three-way unsignalized intersection. The S. Owens Road southbound approach consists of a left-turn lane and a right-turn lane and operates under STOP control. The Route 397 eastbound approach consists of a left-turn lane and a through travel lane. The Route 397 westbound approach consists of a single general-purpose lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

Route 397 at S. Nine Canyon Road. S. Nine Canyon Road intersects Route 14 from the north and south to form a four-way unsignalized intersection. The S. Nine Canyon Road northbound approach consists of a shared left-turn/through lane and a channelized right-turn lane all of which operate under STOP control. The S. Nine Canyon Road southbound approach consists of a single general-purpose travel lane which operates under STOP control. The Route 397 eastbound and westbound approaches each consist of a left-turn lane and a shared thru/right lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Route 397 at S. Finley Road.** S. Finley Road intersects Route 14 from the south to form a three-way unsignalized intersection. The S. Finley Road northbound approach consists of a single general-purpose travel lane which operates under STOP control. The Route 397 westbound approach consists of a left-turn lane and a through travel lane. The Route 397 eastbound approach consists of a shared through/right-turn lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection remains undeveloped.

**Nine Canyon Road at Kirk Road.** Kirk Road intersects Nine Canyon Road from the east to form a three-way unsignalized intersection. The Kirk Road westbound approach consists of a single general-purpose travel lane which operates under STOP control. The Nine Canyon Road northbound and southbound approaches each consist of a single general-purpose travel lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is used for agricultural purposes.

Nine Canyon Road at Beck Road. Beck Road intersects Nine Canyon Road from the west to form a three-way unsignalized intersection. The Beck Road eastbound approach consists of a single general-purpose travel lane which operates under STOP control. The Nine Canyon Road northbound and southbound approaches each consist of a single general-purpose travel lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is used for agricultural purposes.

Nine Canyon Road at Coffin Road. Coffin Road intersects Nine Canyon Road from the west to form a three-way unsignalized intersection. The Coffin Road eastbound approach consists of a single general-purpose travel lane which operates under STOP control. The Nine Canyon Road northbound and southbound approaches each consist of a single general-purpose travel lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is used for agricultural purposes.

Locust Grove Road at S. Clodfelter Road. A four-way unsignalized intersection is created by the junction of S. Clodfelter Road (from the north and west), Locust grove from the east and C Williams Road from the south. The S. Clodfelter Road southbound approach and the C Williams northbound approach each consist of a single travel lane and operate under STOP control. The eastbound S. Clodfelter Road approach and the westbound Locust Grove Road approach each consist of a single general-purpose travel lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is used for agricultural purposes.

**Webber Canyon Road at Badger Road.** Badger Road intersects Webber Canyon Road from the northeast to form a three-way unsignalized intersection. The Badger Road southwest approach consists of a single general-purpose travel lane which operates under STOP control. The Webber Canyon Road northwestbound and south-eastbound approaches each consist of a single general-purpose travel lane. There are currently no sidewalks or crosswalks at the intersection. Land surrounding the intersection is used for agricultural purposes.

#### 2.3 EXISTING TRAFFIC VOLUMES

Peak period intersection turning movement counts (TMCs) and automatic traffic recorder (ATR) counts were collected in collected on June 13<sup>th</sup> and June 14<sup>th</sup> 2023 in the vicinity of the Project site. Existing continuous count stations along Route 221 and I-82 under the jurisdiction of WSDOT were used to supplement collected traffic count data.

# 2.3.1 Daily Traffic Volumes

ATR counts were conducted along following roadway segments within the study area:

- 1. I-82 (north of Coffin Road)
- 2. Route 397 (west of Nine Canyon Road)
- 3. Route 221 (south of Sellards Road)
- 4. Bofer Canyon Road (north of Coffin Road)
- 5. Nine Canyon Road (south of Route 397)
- 6. Locust Grove Road (between Nicosin Road and I-82)
- 7. Travis Road (north of Sellards Road)
- 8. Plymouth Road (north of Route 14)
- 9. Sellards Road (between Route 221 and Tyack Road)
- 10. Badger Canyon Road (north of Sellards Road)

The ATR data collected provides the basis the Roadway Segment Analysis to be discussed in Section 4.2 A summary of existing daily traffic volumes by direction is provided in Table 1. The ATR traffic volume data is provided in Appendix B.

Table 1. Weekday Daily Traffic Volume Summary

		Weekday ADT	AM Peak Hour	PM Peak Hour
Location		(vpd) <sup>1</sup>	(vph) <sup>2</sup>	(vph)
I-82	Northbound	12,650	1,056	767
(North of Coffin Road)	Southbound	12,345	<u>236</u>	<u>1,420</u>
	Total	24,995	1292	2,187
Route 397	Eastbound	670	42	54
(West of Nine Canyon Road)	Westbound	<u>696</u>	<u>48</u>	<u>51</u>
	Total	1,366	90	105
Route 221	Northbound	1,298	32	200
South of Sellards Road)	Southbound	<u>1,359</u>	<u>180</u>	<u>84</u>
	Total	2,657	212	284
Bofer Canyon Road	Northbound	25	0	5
(North of Coffin Road)	Southbound	<u>14</u>	<u>1</u>	<u>0</u>
	Total	39	1	5
Nine Canyon Road	Northbound	120	1	14
(South of Route 397)	Southbound	<u>133</u>	<u>18</u>	<u>10</u>
	Total	253	19	24
Locust Grove Road	Eastbound	349	12	51
(Between Nicosin Road and I-82)	Westbound	<u>242</u>	<u>36</u>	<u>10</u>
	Total	591	48	61
Travis Road	Northbound	402	13	55
(North of Sellards Road)	Southbound	<u>335</u>	<u>35</u>	<u>20</u>
	Total	737	48	75
S. Plymouth Road	Northbound	675	21	55
(North of Route 14)	Southbound	<u>677</u>	<u>52</u>	<u>60</u>
	Total	1,352	73	115
Sellards Road	Eastbound	566	28	55
(Between Route 221 and Tyack Road)	Westbound	<u>482</u>	<u>40</u>	<u>41</u>
Noauj	Total	1,048	68	96
Badger Canyon Road	Northbound	43	2	8
(North of Sellards Road)	Southbound	<u>25</u>	<u>1</u>	<u>1</u>
	Total	68	3	9

Based on automatic traffic recorder counts collected on June 13th, 2023. 1vpd = vehicles per day 2vph = vehicles per hour



The ATR traffic volume data is provided in Appendix B.

#### 2.3.2 Peak Hour Traffic Volumes

The combined critical peak demand periods of Project-related construction traffic and adjacent street traffic will occur during the weekday morning and weekday evening midday peak hours. The TMC data was collected during the typical weekday from 5:00 AM to 8:00 AM and 4:00 PM to 8:00 PM. The turning movement count data sheets are provided in Appendix B.

#### 2.4 PEDESTRIAN AND BICYCLE ACCOMMODATIONS

Within the study area, limited pedestrian and bicycle infrastructure is provided. Sidewalks exist only on the south side of Wine Country Road west of Route 22. There are no marked or signalized pedestrian crossings within the study area. There are currently no dedicated bike lanes or shared bike lanes (sharrows) along study area routes. The Tri-Cities Bicycle Map, created by the Benton-Franklin Council of Governments, recommends routes which use study area roadways including Route 397, Bofer Canyon Road, S. Clodfelter Road and Travis Road. During construction, roadways should remain open and accessible for all roadway users.

#### 2.5 TRAFFIC SAFETY ANALYSIS

Crash data for the study area intersections was obtained from WSDOT via the crash request center for the most recent five-year period available (2018 through 2022). Crash data summaries received from WSDOT included severity, type of crash, weather conditions, road surface condition, time, and date.

ITE Million Entering Vehicles (MEV) Method. In this analysis crashes are measured based on frequency per million entering vehicles (MEV). This ratio is a function of the average daily traffic entering the intersection and the annual frequency of accidents. This method of analysis is used to identify areas that need further review. A typical review threshold for accidents at an intersection is 1.00 accidents per MEV. Peak Hour TMCs collected as part of this study were grown to estimate entering average daily traffic volumes. A summary of the accident data for the intersections within the study area are shown in Table 2. The crash data, if any, and crash rate calculations for each study intersection are provided in Appendix C. A brief description of the crash history for the five-year study period reviewed for each of the study area intersections is provided below.

$$Rate\ per\ MEV = \frac{number\ of\ crashes\ in\ five\ years\ \times\ 1\ million}{PM\ Peak\ Hour\ Volume\ \times\ PM\ Peak\ Hour\ Factor\ (0.09)\times365\times5\ years}$$

Crashes resulting in fatalities were reported at the following intersections in the study area: Route 221 at Sellards Road & Route 14 at S. Plymouth Road. The fatality at Route 221 and Sellards Road occurred on August 8th, 2021, at approximately 8:15 PM due to a motorist failing to stop while heading westbound on Sellards Road causing an angle collision. The fatality at Route 14 and S. Plymouth Road occurred on July 22<sup>nd</sup>, 2022 at approximately 4:30 AM due to an angle collision when a motorist failed to stop while travelling on Plymouth Road northbound.

Table 2. Crash Data Summary (2018-2022)

	Wine Country Road at			Route 221 at			Webber Canyon Road at Cemetery Road S. Plymouth Road at				Route 14 at			
	I-82 Northbound Ramps	I-82 Southbound Ramps	Route 22	Route 22 and Paterson Ave	County Well Road	Cemetery Road S	ellards Road	County Well Road	Travis Road	S. Clodfelter Road	Route 221	S. Plymouth Road	I-82 Southbound Ramps	I-82 Northbound Ramps
Year	_				_	_		_	_					
2018	0	1	1	1	0	0	1	0	0	0	3	4	0	0
2019	1	2	4	3	0	0	2	0	1	0	3	1	0	0
2020	0	0	1	2	0	0	2	0	0	0	2	2	2	0
2021	0	1	0	2	0	0	3	0	0	0	0	2	1	0
2022	<u>2</u>	<u>1</u>	<u>1</u>	1	<u>0</u>	<u>0</u>	<u>4</u>	<u>0</u>	1	1	<u>2</u>	<u>3</u>	<u>0</u>	<u>0</u>
Total	3	5	7	9	0	0	12	0	2	1	10	12	3	0
Type	4	0	0	_	0	0	0	0	0	0	0	0	4	0
Angle	1	0	2	5	0	0	6	0	0	0	2	2	1	0
Rear-end	1 0	1	3 0	0	0 0	0	2	0 0	0 0	0	ı	6	0	0
Head-on Sideswipe	0	0	1	0	0	0	1	0	0	0	0 0	0	0	0
Single Vehicle	1	4	1	3	0	0	2	0	2	1	7	3	2	0
Other/Unknown	1	4 0	0	_	_					1	1	1		0
Total	<u>0</u> <b>3</b>	<u>0</u> <b>5</b>	<u>0</u> <b>7</b>	<u>0</u> <b>9</b>	<u>0</u> <b>0</b>	<u>0</u> <b>0</b>	<u>0</u> <b>12</b>	<u>0</u> <b>0</b>	<u>0</u> <b>2</b>	<u>0</u>	<u>0</u> <b>10</b>	<u>0</u> <b>12</b>	<u>0</u> <b>3</b>	<u>0</u> <b>0</b>
lotai	3	3	,	3	U	· ·	12	U	2	•	10	12	3	U
Severity		0	0	0	0	0	0	0	0	_	0		0	2
No Apparent Injury	2	3	3	8	0	0	9	0	2	1	8	4	3	0
Suspected Minor Injury	0	1	0	0	0	0	0	0	0	0	1	1	0	0
Possible Injury	1	0	3	0	0 0	0	0	0 0	0	0	1	6	0	0
Suspected Serious Injury	0	0	1	0		0	1	0	0	0	0	0	0	0
Fatality	0	1	0	-	0	0	0		0	0	0	1	0	0
<u>Unknown</u> <b>Total</b>	<u>0</u> <b>3</b>	<u> </u> 5	<u>0</u> <b>7</b>	<u>0</u> <b>9</b>	<u>0</u> <b>0</b>	<u>0</u> <b>0</b>	<u>0</u> <b>12</b>	<u>0</u> <b>0</b>	<u>0</u> <b>2</b>	<u>0</u>	<u>0</u> 10	<u>0</u> <b>12</b>	<u>0</u> <b>3</b>	<u>0</u> <b>0</b>
lotai	3	5	,	9	U	U	12	U	2	1	10	12	3	U
Weather	_	_		_	_	_		_						
Clear	0	2	0	3	0	0	4	0	1	0	2	5	1	0
Cloudy	2	1	5	6	0	0	7	0	1	0	7	7	0	0
Rain	0	1	1	0	0	0	0	0	0	0	0	0	0	0
Snow	0	0	1	0	0	U	0	0	0	0	Ü	0	2	0
Fog or Smog or Smoke	1	1	0	U	0	U	1	0	0	1	0	0	0	0
Other/Unknown	0	<u>0</u>	<u>0</u>	0	0	0	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>0</u>	0	0
Total	3	5	1	9	0	0	12	0	2	1	10	12	3	U
Time														
7am to 9am	1	0	0	1	0	0	0	0	0	0	3	0	1	0
9am to 4pm	2	1	4	6	0	0	5	0	0	0	4	4	0	0
4pm to 6pm	0	0	1	1	0	0	2	0	0	1	0	2	1	0
6pm to 12am	0	0	1	0	0	0	2	0	1	0	2	3	1	0
12am to 7am	<u>0</u>	4	<u>1</u>	1	<u>0</u>	0	<u>3</u>	<u>0</u>	1	<u>0</u>	<u>1</u>	<u>3</u>	<u>0</u>	<u>0</u>
Total	3	5	7	9	0	0	12	0	2	1	10	12	3	0
Crash Rates	0.31	0.33	0.41	0.76	0.00	0.00	1.84	0.00	1.23	0.34	1.61	1.03	0.28	0.00

<sup>1)</sup> Based on crash data obtained from WSDOT's online crash data request portal.



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Table 2 (Continued) Crash Data Summary (2018-2022)

		Coffin Road at	t	Bofer Canyon Road at	Locust Gro	ve Road at		Route	397 at		Nir	ne Canyon Roa	d at	Locust Grove Road at	Webber Canyon Roa at
	I-82 Southbound Ramps	I-82 Northbound Ramps	Bofer Canyon Road	Beck Road	I-82 Southbound Ramps	I-82 Northbound Ramps	Bofer Canyon Road	S. Olympia Street	S. Nine Canyon Road	S. Finley Road	Kirk Road	Beck Road	Coffin Road	S. Clodfelter Road	
'ear															
018	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
019	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1
020	0	0	0	0	0	1	1	0	0	0	0	0	0	1	0
021	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
022	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
otal	0	0	0	0	0	3	1	0	2	1	0	0	0	1	1
ype															
ngle	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
ear-end	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0
ead-on	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
deswipe	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ngle Vehicle	0	0	0	0	0	2	1	0	1	1	0	0	0	1	1
her/Unknown	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
tal	0	0	0	0	0	3	1	0	2	1	0	0	0	1	1
everity															
Apparent Injury	0	0	0	0	0	2	0	0	2	1	0	0	0	0	0
spected Minor Injury	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
ssible Injury	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
spected Serious Injury	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
tality	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
nknown	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	1	<u>0</u>
otal	0	0	0	0	0	3	1	0	2	1	0	0	0	1	1
eather															
ear	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0
oudy	0	0	0	0	0	1	0	0	2	0	0	0	0	0	1
ain	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ow	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
g or Smog or Smoke	0	0	0	0	0	2	1	0	0	0	0	0	0	0	0
ner/Unknown	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	0
otal	0	0	0	0	0	3	1	0	2	1	0	0	0	1	1
me															
am to 9am	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0
m to 4pm	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
m to 6pm	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0
m to 12am	0	0	0	0	0	2	0	0	0	0	0	0	0	0	1
am to 7am	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>1</u>	<u>0</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	0	0
otal	0	<u></u>	<u></u>	0	0	<del>_</del> 3	<del>-</del> 1	0	<u>-</u>	1	<u></u>	0	0	1	1
rash Rates	0.00	0.00	0.00	0.00	0.00	0.43	0.16	0.00	0.59	0.50	0.00	0.00	0.00	0.54	0.25

<sup>1)</sup> Based on crash data obtained from WSDOT's online crash data request portal.



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# 2.5.1 Highway Safety Manual Safety Performance Summary

Tetra Tech conducted a traffic safety analysis for each of the study area intersections based on procedures outlined in the Safety Analysis Guide published by WSDOT using the Highway Safety Manual (HSM) spreadsheet tool, version 9.1 (<a href="http://safetyperformance.org/tools/">http://safetyperformance.org/tools/</a>). The HSM tool is used to calculate the Predicted Crash Frequency and the Expected Crash Frequency.

The Predicted Crash Frequency is developed based on consideration of the intersection geometry (number of lanes approaching the intersection including turn lanes, if present), intersection layout (angle of the intersecting streets), provision (or absence of) street lighting, and average annual daily traffic (AADT) volumes at the intersection. The Predicted Crash Frequency represents the average safety performance experienced at intersections with similar design characteristics and traffic volumes, expressed in crashes per year.

The Expected Crash Frequency is developed based on the same factors as above but also considers the average reported crash frequency at the intersection (over the past five years). This includes an assessment of the severity of crashes (i.e. crashes involving personal injury and/or fatalities versus crashes that result in property damage only). This analysis is considered a more reliable metric of existing or actual average crash performance, measured in crashes per year.

The Predicted Crash Frequency for each intersection is then subtracted from the Expected Crash Frequency to identify intersections with the highest potential for safety improvement. A net difference greater than zero indicates intersections that are likely to experience more crashes than typical intersections with similar roadway characteristics and traffic volumes, and highest potential to reduction of fatal and serious injury crashes and return the greatest benefit for the cost of a safety Project. The HSM crash frequency calculations are provided in Appendix C of this report. A comparison of the Predicted Crash Frequency and Expected Crash Frequency for each of the study area intersection are presented in Tables 3 and 4.

Table 3. Highway Safety Manual Predictive/Expected Safety Performance Summary

		Crash Frequency (Crashes/Year)						
Intersection	Crash Severity	Predicted (Geometry/ Volume)	Expected (Geometry/Volume/ Crash History)	Potential for Improvement				
W	Fatal & Injury	0.4	0.3	0.0				
Wine Country Road at I-82	PDO	0.6	0.4	0.0				
Northbound Ramps	Total	1.0	0.7	0.0				
W. O. J. D. J. J. O.	Fatal & Injury	0.4	0.4	0.0				
Wine Country Road at I-82	PDO	0.6	0.6	0.0				
Southbound Ramps	Total	1.0	1.0	0.0				
W. O. J. D. J. J. D. J.	Fatal & Injury	1.6	0.8	0.0				
Wine Country Road at Route	PDO	2.1	1.0	0.0				
22	Total	3.7	1.8	0.0				
D 1 00 1D 1 001 1	Fatal & Injury	1.0	0.8	0.0				
Route 22 at Route 221 at	PDO	1.3	1.1	0.0				
Paterson Ave	Total	2.2	1.9	0.0				
D / 20/ 10 / 14/ II	Fatal & Injury	0.1	0.1	0.0				
Route 221 at County Well	PDO	0.2	0.1	0.0				
Road	Total	0.3	0.2	0.0				
D	Fatal & Injury	0.0	0.0	0.0				
Route 221 at Cemetery	PDO	0.1	0.0	0.0				
Road	Total	0.1	0.1	0.0				
	Fatal & Injury	0.6	0.9	0.3				
Route 221 at Sellards Road	PDO	0.8	1.2	0.3				
	Total	1.5	2.1	0.6				
	Fatal & Injury	0.1	0.0	0.0				
Webber Canyon Road at	PDO	0.1	0.1	0.0				
County Well Road	Total	0.1	0.1	0.0				
0 1 5 1 1 7 1	Fatal & Injury	0.0	0.0	0.0				
Cemetery Road at Travis	PDO	0.1	0.1	0.0				
Road	Total	0.1	0.1	0.0				
C. Dharacath David Atlanta	Fatal & Injury	0.1	0.1	0.0				
S. Plymouth Road at Locust Grove Road	PDO	0.1	0.1	0.0				
Giove Road	Total	0.2	0.2	0.0				
	Fatal & Injury	0.7	0.8	0.1				
Route 221 at Route 14	PDO	1.0	1.1	0.1				
	Total	1.7	1.9	0.2				

Table 3. (Continued) Highway Safety Manual Predictive/Expected Safety Performance Summary

		s/Year)		
Intersection	Crash Severity	Predicted (Geometry/ Volume)	Expected (Geometry/Volume/ Crash History)	Potential for Improvement
	Fatal & Injury	0.6	0.7	0.2
Route 14 at S. Plymouth	PDO	0.7	1.0	0.2
Road	Total	1.3	1.7	0.4
5 / // // 60 6 // /	Fatal & Injury	0.7	0.4	0.0
Route 14 at I-82 Southbound	PDO	1.0	0.5	0.0
Ramps	Total	1.7	1.0	0.0
	Fatal & Injury	0.9	0.3	0.0
Route 14 at I-82 Northbound	PDO	1.1	0.3	0.0
Ramps	Total	2.0	0.6	0.0
	Fatal & Injury	0.1	0.1	0.0
Coffin Road at I-82	PDO	0.1	0.1	0.0
Southbound Ramps	Total	0.2	0.1	0.0
	Fatal & Injury	0.1	0.0	0.0
Coffin Road at I-82	PDO	0.1	0.1	0.0
Northbound Ramps	Total	0.1	0.1	0.0
	Fatal & Injury	0.1	0.0	0.0
Coffin Road at Bofer Canyon	PDO	0.1	0.1	0.0
Road	Total	0.1	0.1	0.0
	Fatal & Injury	0.0	0.0	0.0
Bofer Canyon Road at Beck	PDO	0.0	0.0	0.0
Road	Total	0.0	0.0	0.0
	Fatal & Injury	0.5	0.2	0.0
Locust Grove Road at I-82	PDO	0.8	0.3	0.0
Southbound Ramps	Total	1.3	0.5	0.0
	Fatal & Injury	0.9	0.4	0.0
Locust Grove Road at I-82	PDO	0.7	0.3	0.0
Northbound Ramps	Total	2.0	1.0	0.0
<b>5</b>	Fatal & Injury	0.2	0.2	0.0
Route 397 at Locust Grove	PDO	1.1	0.6	0.0
Road at Bofer Canyon Road	Total	0.5	0.4	0.0
D / 007 / 0 01	Fatal & Injury	0.2	0.1	0.0
Route 397 at S. Olympia	PDO	0.3	0.2	0.0
Street	Total	0.6	0.2	0.0

Table 3. (Continued) Highway Safety Manual Predictive/Expected Safety Performance Summary

		Crash Frequency (Crashes/Year)					
Intersection	Crash Severity	Predicted (Geometry/ Volume)	Expected (Geometry/Volume/ Crash History)	Potential for Improvement			
Davita 207 at C. Nina	Fatal & Injury	0.2	0.2	0.0			
Route 397 at S. Nine Canyon Road	PDO	0.3	0.1	0.0			
Carryon Road	Total	0.4	0.4	0.0			
	Fatal & Injury	0.0	0.1	0.0			
Route 397 at S. Finley Road	PDO	0.2	0.2	0.0			
	Total	0.1	0.1	0.0			
Nine Osman Basel at Kida	Fatal & Injury	0.0	0.0	0.0			
Nine Canyon Road at Kirk Road	PDO	0.1	0.1	0.0			
Road	Total	0.0	0.0	0.0			
Nine Course Book of Book	Fatal & Injury	0.0	0.0	0.0			
Nine Canyon Road at Beck Road	PDO	0.0	0.0	0.0			
Roau	Total	0.0	0.0	0.0			
Nine Course Dead at Coffin	Fatal & Injury	0.1	0.0	0.0			
Nine Canyon Road at Coffin Road	PDO	0.1	0.1	0.0			
Roau	Total	0.1	0.1	0.0			
	Fatal & Injury	0.1	0.1	0.0			
Locust Grove Road at S. Clodfelter Road	PDO	0.2	0.2	0.0			
Ciouleilei Roau	Total	0.3	0.3	0.0			
Walthan Camuan Dag Lat	Fatal & Injury	0.2	0.3	0.1			
Webber Canyon Road at Badger Road	PDO	0.3	0.5	0.2			
Daugei Noau	Total	0.5	0.8	0.3			

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Table 4. Highway Safety Manual Predictive/Expected Safety Performance Summary – Key Locations

		Crash Frequency (Crashes/Year)							
Intersection	Crash Severity	Predicted (Geometry/Volume)	Expected (Geometry/Volume/ Crash History)	Potential for Improvement					
	Fatal & Injury	0.6	0.9	0.3					
Route 221 at Sellards Road	PDO	0.8	1.2	0.3					
	Total	1.5	2.1	0.6					
	Fatal & Injury	0.7	0.8	0.1					
Route 221 at Route 14	PDO	1.0	1.1	0.1					
	Total	1.7	1.9	0.2					
	Fatal & Injury	0.6	0.7	0.2					
Route 14 at S. Plymouth Road	PDO	0.7	1.0	0.2					
	Total	1.3	1.7	0.4					
	Fatal & Injury	0.2	0.3	0.1					
Webber Canyon Road at Badger Road	PDO	0.3	0.5	0.2					
	Total	0.5	8.0	0.3					

As shown in Table 4, the HSM analysis indicates that four of the study area intersections have higher Expected Crash Frequencies than average for intersections with similar design features and traffic volumes with potential for improvement. These include the following locations:

- Route 221 at Sellards Road
- Route 221 at Route 14
- Route 14 at S. Plymouth Road
- Webber Canyon Road at Badger Road

The next step in the traffic safety review is to evaluate potential measures to reduce the Predicted and Expected Crash Frequencies at the four high crash frequency intersections. A summary of the potential safety improvements at the high crash frequency locations is presented in Figure 33 (as discussed in more detail in Section 4.4). The potential reduction in the Predicted and Expected Crash Frequencies associated with each of the potential traffic safety mitigation measures at these intersections is summarized in Table 5.

 Table 5.
 Predicted Effectiveness of Potential Safety Improvements

	2023 Existing Results		Add Street Lighting		Add Right-Turn Lanes		Add Left-Turn Lanes		Overall Mitigation	
Roadway/Direction	Predicted Average Crash Frequency	Expected Average Crash Frequency	Predicted Average Crash Frequency	Expected Average Crash Frequency	Predicted Average Crash Frequency	Expected Average Crash Frequency	Predicted Average Crash Frequency	Expected Average Crash Frequency	Predicted Average Crash Frequency	Overall Mitigation Expected Average Crash Frequency
Route 221 at Sellards Road	1.5	2.1	1.3	2.0	1.1	1.8	8.0	1.6	0.5	1.2
Route 221 at Route 14	1.7	1.9	N/A	N/A	1.5	1.8	0.9	1.5	0.8	1.3
Route 14 at S. Plymouth Road	1.3	1.7	N/A	N/A	1.0	1.5	N/A	N/A	1.0	1.5
Webber Canyon Road at Badger Road	0.5	0.8	0.4	0.7	0.4	0.7	0.3	0.6	0.2	0.5

N/A - Mitigation measures are already in place at these locations and were therefore not considered as potential improvements.



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# 2.5.2 Sight Distance Analysis

Tetra Tech reviewed the available sight distance at the four study area intersections identified in the safety analysis to determine whether or not insufficient sight distance may be adversely affecting the safety at these intersections. The available sight distance was determined based on procedures outlined in *A Policy On Geometric Design of Highways and Streets*, published by the American Association of State Highway and Transportation Officials (AASHTO). Tetra Tech then compared the available sight distance to the required Stopping Sight Distance (SSD) for the anticipated travel speeds for vehicle traveling past the site.

85<sup>th</sup> percentile travel speed data was available from WSDOT (Continuous Count Station: P17) along Route 221. At all other locations the posted speed limit was used in the Sight Distance Analysis. A summary of the available and required SSD at the four study area intersections identified in the safety analysis is presented in Table 6.

Table 6. Sight Distance Summary

Intersection	Assumed Speed (mph)	Approx. Grade	AASHTO Desirable (feet) <sup>1</sup>	Estimated Distance (feet)	Meets AASHTO Desirable				
Route 221 at Sellards Road									
Stopping Distance									
From the North	72 <sup>2</sup>	-1.1	780	1000+	Yes				
From the South	71 <sup>2</sup>	1.2	730	1000+	Yes				
Route 14 at Route 221									
Stopping Distance									
From the East	65 <sup>3</sup>	-0.5	650	1000+	Yes				
From the West	65 <sup>3</sup>	0.9	635	1000+	Yes				
Washington Street/Proposed Southerly Site Driveway									
Stopping Distance					_				
From the East	55 <sup>3</sup>	-0.3	495	1000+	Yes				
From the West	55 <sup>3</sup>	0.6	490	1000+	Yes				
Route 16/Truck Access Driveway									
Stopping Distance									
From the North	40 <sup>3</sup>	-0.5	305	1000+	Yes				
From the South	50 <sup>3</sup>	-0.3	430	650+	Yes				

<sup>&</sup>lt;sup>1</sup>Obtained from A Policy On Geometric Design of Highways and Streets, 2018 Edition, published by the American Association of State Highway and Transportation Officials (Exhibit 3-1) for the assumed travel speeds for required stopping sight distance and desirable intersection sight distance based on roadway grades.

As shown in Table 6, the available sight distance at each of the study area intersections exceeds the minimum AASHTO-required stopping sight distance for the assumed travel speed. The sight distance calculations are included in Appendix D.

<sup>&</sup>lt;sup>2</sup> Assumed Speeds at Route 221 & Sellards Road is the Observed 85% speed.

<sup>&</sup>lt;sup>3</sup> Assumed Speed are the Posted Speed Limit.

## 3.0 FUTURE CONDITIONS

To determine future traffic demands on the study area roadways, the 2023 Existing weekday morning and weekday evening peak hour traffic volumes were projected to the future design years of 2025 for Phase 1 and 2026 for Phase 2. Independent of the proposed Project, the future No Build (Without Project) traffic volumes are assumed to include all existing traffic as well traffic increases resulting from general background traffic growth and other planned development Projects in the vicinity of the site.

The following section of the report provides a detailed description of the development of the future peak hour traffic projections and other factors influencing the future traffic conditions in the vicinity of the Project site.

#### 3.1 FUTURE NO BUILD CONDITIONS

The future No Build (Without Project) condition establishes the basis for evaluating the transportation impacts associated with the proposed Project. The No Build condition includes the effects of general area growth, other planned development Projects and planned transportation improvements expected to be completed by the Design Year of 2025 for Phase 1 and 2026 for Phase 2.

To establish the future 2025 and 2026 No Build traffic volumes, the 2023 Existing condition traffic volumes were projected to the 2025 and 2026 design years, by which time the Project is expected to be under construction. Traffic growth is primarily a function of changes in motor vehicle use and expected land development in the region. To predict a rate at which traffic on the roadways in the vicinity of the site can be expected to grow during the two- and three-year forecast period (2023 to 2025 and 2023 to 2026), both historic traffic growth and planned area developments were examined. A discussion of the development of the future No Build (Without Project) conditions is provided below.

# 3.1.1 General Background Traffic Growth

A general background growth rate was applied to the 2023 Existing condition traffic volumes based on a review of the Washington Department of Transportation (WSDOT) permanent count station data and in consultation with WSDOT. The permanent count station P17, Route 221 indicates declining traffic volumes since 2018 and the permanent count station Location P09, I-82 indicates no growth in traffic volumes since 2021. However, to provide a conservative assessment, an annual growth rate of 1.0 percent per year was assumed for this study. This is consistent with the growth rate recommendations provided by WSDOT. The background traffic growth rate data is provided in Appendix E.

# 3.1.2 Background Development

Other planned area developments could also result in increased traffic on the surrounding area roadways. Tetra Tech coordinated with WSDOT and Benton County planning staff to determine background development projects approved or under construction that my need to be considered in the development of future year traffic volumes. The County and State did not identify any additional background development projects within the study area.

## 3.1.3 Planned Roadway Improvements

Based on consultation with Benton County, County Well Road is slated for reconstruction as part of the Transportation Improvement Program. Country Well Road is planned to be reconstructed from a gravel roadway to an all-weather paved road with improved guard rails and drainage over the course of three phases. Phase 1 is planned for 2024 and phases 2 and 3 are planned for 2026 and 2027, respectively. Phase 1 will reconstruct County Well Road between Route 221 and McBee Road. Phase 2 will occur between McBee Road and Clodius Road. Phase 3 will occur between Clodius Road and Travis Road.

No other major planned roadway improvements within the study area need to be considered for the 2026 No Build conditions.

#### 3.1.4 Future 2025/2026 No Build Traffic Volumes

The 2023 Existing condition peak hour traffic volumes were grown by 1.0 percent per year over the two-year study horizon to establish the 2025 No Build (Without Project) traffic volumes. Similarly, the 2023 Existing condition peak hour traffic volumes were grown by 1.0 percent per year over the three-year study horizon to establish the 2026 No Build (Without Project) traffic volumes. The 2025 No Build weekday morning and weekday evening peak hour traffic volume networks are illustrated in Figures 5 and 6, respectively. The 2026 No Build weekday morning and weekday evening peak hour traffic volume networks are illustrated in Figures 7 and 8, respectively.

#### 3.2 FUTURE BUILD CONDITIONS

To assess the potential transportation-related impacts associated with the Project's peak construction activities, the overall travel demands were determined based on proposed site access as well as the anticipated trip generation, travel mode split, trip distribution and trip assignment. The Project's travel demand was then added to the future 2025 and 2026 No Build traffic volumes (without the Project) to develop the future 2025 and 2026 Build condition traffic volumes (with the Project's peak construction activity). A discussion of the development of the future Build condition is provided below.

# 3.2.1 Project-Generated Trips

The Project will consist of three stages: construction, O&M, and decommissioning. The highest volume of site-related trips will occur during the peak construction phase of the Project. Therefore, the Traffic Impact Analysis (TIA) will be based on the vehicle trip generation associated with the peak construction phase workforce levels.

Preliminary vehicle trip generation estimates were included in the Project's Application for Site Certification (ASC). These estimates were further refined as part of the Project Traffic Scoping Letter (TSL) to inform the study area and be used as the basis for analysis in the TIA. Construction of the proposed energy facility is expected to include grading, panel installation, inspections, and equipment deliveries.

Peak construction activities are currently anticipated to occur for only a portion of each the two successive 11-month periods. To account for peak construction activities, the anticipated average daily workforce levels (374 workers on site per day) was multiplied by a factor of 1.25. Therefore, it is anticipated that at peak operations, the site could experience construction workforce levels of up to 467 construction workers at one time. Construction hours of operation are assumed to generally be 7:00 AM to 7:00 PM with the majority of construction workers arriving prior to 7:00 AM and departing after 7:00 PM. Since the peak hours of the adjacent street traffic are expected to occur sometime during the peak commuting periods of 7:00 AM to

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9:00 AM and 4:00 PM to 6:00 PM, it is expected that the majority of construction workers would arrive and depart the site outside of the typical weekday morning and weekday evening commuter peak hours of the adjacent streets.

However, to present a conservative assessment of potential traffic increases associated with the Project, it is assumed that all of the construction workers would arrive during the weekday morning peak hour and depart during the weekday evening peak hour. The remainder of the construction periods is anticipated to generate fewer vehicle trips.

There are no public transportation services in the vicinity of the Project site that are anticipated to be used for the Project. Therefore, for the purposes of this assessment, it was assumed that no construction workers would use public transit to access the site. Additionally, it is anticipated that some construction workers would arrive and depart the site together (carpooling). For purposes of this assessment, it was assumed that the average vehicle would have 1.25 occupants to represent carpooling to/from the site.

There are currently two alternatives being considered for Phase 2 of the Project construction. Table 7 presents a summary of the trip generation estimates for the proposed Project's peak construction workforce activities by phase. The more conservative trip generation of the two Phase 2 alternatives (Phase 2a) will be used for the TIA analyses.

Table 7. Construction Trip Generation Summary<sup>1</sup>

		Phase 1			Phase 2a			Phase 2b	
Time Period/ Direction	Workforce <sup>2</sup>	Trucks <sup>3</sup>	Total	Workforce <sup>4</sup>	Trucks⁵	Total	Workforce <sup>6</sup>	Trucks <sup>7</sup>	Total
				We	ekday Daily				
Enter	748	250	998	688	200	888	660	206	866
Exit	748	250	998	688	200	888	660	206	866
Total	1,496	500	1,996	1,376	400	1,776	1,320	412	1,732
				Weekday I	Morning Peak Ho	ur			
Enter	374	13	387	344	10	354	330	11	341
Exit	0	13	13	0	10	10	0	11	11
Total	374	26	400	344	20	364	330	22	352
				Weekday I	Evening Peak Ho	ur			
Enter	0	13	13	0	10	10	0	11	11
Exit	374	13	387	344	10	354	330	11	341
Total	374	26	400	344	20	364	330	22	352

<sup>1)</sup> Based on the Horse Heaven Wind Farm Updated ASC (December 2022).

<sup>2)</sup> Used ASC estimated maximum Phase 1 peak period worker vehicle trips of 374. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.

<sup>3)</sup> Used ASC estimated maximum Phase 1 250 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hours.

<sup>4)</sup> Used ASC estimated maximum Phase 2a peak period worker vehicle trips of 344. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.

<sup>5)</sup> Used ASC estimated maximum Phase 2a 200 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hours.

<sup>6)</sup> Used ASC estimated maximum Phase 2b peak period worker vehicle trips of 330. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.

<sup>7)</sup> Used ASC estimated maximum Phase 2b 206 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hour.

### 3.2.2 Trip Distribution

Tetra Tech has developed separate trip distribution patterns for the Project's peak construction activities at the two proposed laydown areas. The trip distribution patterns were developed based on consideration of the effective population within the anticipated employment workforce (approximate two-hour drivetime zone) and the currently proposed location of the laydown area for each phase.

For purposes of this study, it is assumed that construction workers will come from within a draw area of an approximately two-hour drivetime zone due to the demand for skilled laborers onsite. To estimate the distribution patterns for the construction phases, cordon lines were drawn around the site and the cities and towns within the draw area were each assigned a route (or routes, if more than one seemed appropriate) to travel to/from the site. The routes were determined based on travel patterns during peak commuting periods. The populations of each of the cities or towns within the draw area was determined using available US Census population data and used as a method of "weighting" the trip distribution of each of the likely routes to the site. Cities and towns with a less than 30-minute drive to the site were weighted fully. As the travel time to the site increased, its effective population decreased. Populations greater than two-hour drivetimes from the site were considered to not have an effect on the model. The calculated trip distribution patterns are shown in Table 8.

Table 8. Trip Distribution Summary

	Laydov	n Yard 1	Laydow	n Yard 2
Roadway/Direction	Entering Distribution	Exiting Distribution	Entering Distribution	Exiting Distribution
Sellards Road to/from the West	1%	1%	1%	1%
Route 221 to/from the West	0%	0%	18%	18%
Webber Canyon Road to/from North	0%	0%	17%	17%
S. Clodfelter Road to/from North	0%	0%	21%	21%
I-82 to/from North	52%	52%	1%	1%
Route 395 to/from North	25%	25%	22%	22%
S. Olympia Street to/from North	5%	5%	3%	3%
Route 397 to/from North	1%	1%	1%	1%
Bofer Canyon Road to/from South	16%	16%	0%	0%
S. Plymouth Road to/from South	0%	0%	16%	16%
Route 221 to/from South	<u>0%</u>	<u>0%</u>	<u>0%</u>	<u>0%</u>
Total	100%	100%	100%	100%

In general, the analysis indicates that the majority of Project trips will arrive/depart the site from the north. The entering and exiting Project construction workforce trip distribution patterns to Laydown Yard 1 are shown in Figures 9 and 10, respectively. The entering and exiting Project construction workforce trip distribution patterns to Laydown Yard 2 are shown in Figures 11 and 12, respectively. The distribution analysis is included in Appendix F.

The Project trips associated with the two construction phases were then assigned to the study area roadway network based on the Project distribution patterns presented in Figures 9 through 12. The resulting construction traffic volumes are presented in Figures 13 through 18.

Figure 13 - Phase 1 to Laydown Yard #1 Weekday Morning Peak Hour Trips

- Figure 14 Phase 1 to Laydown Yard #1 Weekday Evening Peak Hour Trips
- Figure 15 Phase 1 to Laydown Yard #2 Weekday Morning Peak Hour Trips
- Figure 16 Phase 1 to Laydown Yard #2 Weekday Evening Peak Hour Trips
- Figure 17 Phase 2 to Laydown Yard #2 Weekday Morning Peak Hour Trips
- Figure 18 Phase 2 to Laydown Yard #2 Weekday Evening Peak Hour Trips

## 3.2.3 Build (With Project) Peak Hour Traffic Volumes

The new trips associated with the proposed Project were then added to the 2025 Phase 1 and 2026 Phase 2 No Build (Without Project) traffic volumes. The resulting 2025 Build (Phase 1) and 2026 Build (Phase 2) weekday morning and weekday evening peak hour traffic volumes are presented in Figures 19 through 24.

- Figure 19 2025 Build Phase 1 to Laydown Yard #1 Weekday Morning Peak Hour
- Figure 20 2025 Build Phase 1 to Laydown Yard #1 Weekday Evening Peak Hour
- Figure 21 2025 Build Phase 1 to Laydown Yard #2 Weekday Morning Peak Hour
- Figure 22 2025 Build Phase 1 to Laydown Yard #2 Weekday Evening Peak Hour
- Figure 23 2026 Build Phase 2 to Laydown Yard #2 Weekday Morning Peak Hour
- Figure 24 2026 Build Phase 2 to Laydown Yard #2 Weekday Evening Peak Hour

# 3.2.4 Construction Traffic Impacts to High Frequency Crash Intersections

Based on a detailed review of the anticipated trip generation and trip distribution patterns associated with each phase of construction, the proposed Project is not anticipated to result in any new traffic at the four high frequency crash locations identified above until the later stage of Phase I (for the Phase 1 construction sites located on the west side of I-82 served by Laydown Yard 2). The Project is also expected to result in minor additional traffic increases at the four high frequency crash locations during Project construction Phase 2, albeit with fewer traffic increases than during Project construction Phase 1. Before construction begins, advanced warning signage should be implemented throughout the study area alerting motorists to expect additional truck activity. Figure 35 shows a draft Temporary Construction Warning Signage Plan which includes potential advanced warning signage near the four intersections with high crash frequency. The final Temporary Construction Warning Signage Plan will be prepared in coordination with WSDOT as part of the Applicant's Construction Traffic Management Plan (CTMP).

#### 3.2.4.1 Auxiliary Lane Warrant Analysis

Tetra Tech conducted Auxiliary Lane Warrant analyses at the four high frequency crash locations for the following conditions based on the WSDOT Design Manual. The weekday morning and evening commuter peak hour traffic volumes for the following conditions were estimated for the following conditions:

- 2023 Existing
- Future 2025 Build Phase I to Laydown Yard 2 (worst-case construction traffic volume condition)

The traffic volume inputs and resulting left-turn and right-turn lane warrant analyses for the 2023 Existing and Projected 2025 Build Phase I to Laydown Yard 2 weekday morning and weekday evening commuter peak hours conditions are provided in Appendix G. A summary of the Auxiliary Lane Warrant analysis is presented in Table 9.

 Table 9.
 Auxiliary Turn Lane Warrant Analysis

				Veekday Morning Pea		44.54	Webber 6	
	Route Sellard	221 at		e 221 at ite 14		e 14 at		nyon Road at
						outh Road		er Road
	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southboun
Left-Turn Lane Warranted? <sup>1</sup>	No	No	No	No	No	No	N/A	No
Right-Turn Lane Warranted? <sup>2</sup>	No	No	No	Yes	No	No	No	N/A
				eekday Afternoon Pe				
		221 at ls Road		221 at ite 14		e 14 at outh Road		nyon Road at er Road
	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southboun
Left-Turn Lane Warranted? <sup>1</sup>	No	No	No	No	No	No	N/A	No
Right-Turn Lane Warranted? <sup>2</sup>	No	No	No	No	No	Yes	No	N/A
		2021 Phase	1 to Laydown Yard	2 - Weekday Morning	Peak Hour			
		221 at ls Road		221 at ite 14		e 14 at outh Road		nyon Road at er Road
	Northbound	Southbound	Eastbound	Westbound	Northbound	Southbound	Eastbound	
Left-Turn Lane Warranted? <sup>1</sup>	No	No	No	No	No	No	N/A	No
Right-Turn Lane Warranted? <sup>2</sup>	No	No	No	Yes	No	Yes	No	N/A
		2025 Phase	e 1 to Laydown Yard 2	- Weekday Afternoon	Peak Hour			
		221 at ls Road		221 at ite 14		e 14 at outh Road		nyon Road at er Road
	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southboun
Left-Turn Lane Warranted? <sup>1</sup>	No	No	No	No	No	No	N/A	No
Right-Turn Lane Warranted? <sup>2</sup>	No	No	No	No	No	Yes	No	N/A

Notes:



<sup>1)</sup> Left-Turn Lane Warrant Analysis Based on WSDOT Design Manual 22-01.21 Chapter 1310 Exhibit 7.

<sup>2)</sup> Right-Turn Lane Warrant Analysis Based on WSDOT Design Manual 22-01.21 Chapter 1310 Exhibit 19.

<sup>3)</sup> N/A = Not Applicable

As shown in Table 9, the 2023 Existing and Projected 2025 Build Phase I to Laydown Yard 2 weekday commuter peak hour traffic volumes at the intersections of Route 221 and Sellards Road, Webber Canyon Road and Badger Road, fall below the minimum traffic volume warrant thresholds for installation of either a separate left-turn or right-turn lane. The Auxiliary Lane Warrant Analysis also indicates that the intersection of Route 221 and Route 14 currently satisfies the minimum traffic volume warrant threshold for the consideration of a right-turn lane on the Route 14 westbound approach to the intersection with Route 221, during the weekday morning peak hour condition. This intersection currently provides a short right-turn lane on the Route 14 westbound approach, with no deceleration lane provided. However, the proposed Project is not expected to result in any additional vehicles trips to the intersection during either the weekday morning peak hour or weekday afternoon commuter peak hour. Consequently, no geometric improvements are currently proposed at these intersections as part of the Horse Heaven Wind Farm Project.

The Auxiliary Lane Warrant Analysis indicates that the intersection of Route 14 and S. Plymouth Road currently satisfies the minimum traffic volume warrant threshold for the consideration of a right-turn lane on the Route 14 westbound approach to the intersection with S. Plymouth Road during the weekday morning peak hour. The Project construction is expected to add approximately 63 additional westbound right-turn movements during the weekday morning peak hour and 4 additional westbound right-turn movements during the weekday evening peak hour.

The proposed Project is not anticipated to materially impact future traffic operations or safety at any of the study area intersections. The right-turn lane warrants listed above are met under existing conditions independent of the proposed Project. In addition, as discussed previously in this report, the available Stopping Sight Distance (SSD) for vehicles approaching the high crash frequencies intersections is well in excess of the required SSD for the posted speed limit. This indicates that drivers approaching the intersection will have more than sufficient view of potential turning traffic at the intersection to either stop or adjust their speed to avoid a collision.

As part of the proposed Project, the Applicant will develop a comprehensive CTMP for each phase of the construction, to alert drivers of potential increased construction traffic and turning activities at key intersections throughout the study area. The Applicant will also work with the general contractor (once selected) to identify truck haul routes to and from the construction laydown areas to avoid sensitive areas and minimize impacts to local agricultural activity along the study area roadways, to the extent possible. In addition, the Applicant will prepare a Traffic Safety Management Plan for the Project-related construction activity in accordance with WSDOT standards. A detailed description of the draft Traffic Safety Plan (TSP) is provided in the subsequent sections of this report.

#### 4.0 TRAFFIC OPERATIONS ANALYSIS

In previous sections of this report, the quantity (volume) of traffic on the study area roadways was described. The following section describes the quality of traffic flow on the study area roadways and intersections for the given traffic demands. As a basis for this assessment, road segment and intersection capacity analyses were conducted for the 2023 Existing, 2025 and 2026 No Build (Without Project), 2025 Build (Phase 1 To Laydown Yard 1), 2025 Build (Phase 1 to Laydown Yard 2), and 2026 Build (Phase 2 to Laydown Yard 2) weekday morning and weekday evening peak hour traffic conditions.

The roadway segment analysis has conducted for 10 key roadway segments within the study area using the Highway Capacity Software (HCS 2022) based on the Highway Capacity Manual, 6<sup>th</sup> Edition. The intersection capacity analysis was conducted for the 29 existing intersections and two proposed site driveways serving the laydown yards using for the Build (with Project) conditions Synchro 11 Intersection Capacity and Traffic Simulation Software. The detailed capacity analysis worksheets for the key roadway segments and study area intersections are provided in Appendix H and Appendix I, respectively. A discussion of the evaluation criteria and a summary of the results of the roadway segment and intersection capacity analyses are presented below.

#### 4.1 METHODOLOGY

Level-of-service (LOS) is a term used to describe the quality of traffic flow on roadways or at intersections. It is an aggregate measure of travel delay, driver convenience and safety based on a comparison of a roadway facility's capacity relative to the traffic demands. Operating levels of service are reported on a scale of A to F, with A representing the best operating conditions (with little or no vehicle delay) and F representing the worst operating conditions (with long delays). The capacity analyses for roadway segments and unsignalized study intersections were based on the *Highway Capacity Manual (HCM)* 6<sup>th</sup> *Edition*. The LOS criteria for roadway segments and unsignalized intersections are presented in Table 10.

Table 10 Intersection Level of Service Criteria

	Average Delay per Vehicle (Seconds)	Density (Veh/lane/mile)
Level of Service <sup>1</sup>	Unsignalized Intersections	Roadway Segments
Α	≤10.0	0-11
В	10.1 to 15.0	11-18
С	15.1 to 25.0	18-26
D	25.1 to 35.0	26-35
E	35.1 to 50.0	35-45
F	>50.0	>45

Source: Transportation Research Board Highway Capacity Manual (HCM) 2000 (signalized)/6<sup>th</sup> Edition (unsignalized) <sup>1</sup>If the v/c is greater than 1.0, than the level-of-service designation is LOS F, regardless of delays (HCM 6<sup>th</sup> Edition only)

The results of the roadway segment analysis for the weekday morning and weekday afternoon peak hour conditions is presented in Tables 11 and 12, respectively, The results of the unsignalized intersection capacity analysis for the weekday morning and weekday evening peak conditions are presented in Tables

13, and 14 hours, respectively. A brief discussion of the results of the roadway segment and intersection capacity analyses is presented in the following sections of this report.

#### 4.2 ROADWAY SEGMENT CAPACITY ANALYSIS RESULTS

As shown in Tables 11 and 12, the roadway segment capacity analysis indicates that all of the study area roadways segments currently operate well below capacity at LOS B or better operations, and will continue operate well below capacity for all pashes of the Project construction. All Roadway Segments analyzed are expected to operate at LOS A during all analysis scenarios, except for I-82 (north of Coffin Road). This roadway segment will operate at LOS A in the northbound direction and LOS B in the southbound direction during all analysis scenarios. Based on the detailed analysis, the proposed Project will not result in any capacity constraints at the study area roadways and, ample capacity is available to support peak construction operations associated with the Project.

Table 11. Segment Capacity Analysis Summary – Weekday AM Peak Hour

		2023	Existin	g Condi	tions	2025	No Buil	d Cond	itions	2026	No Buil	d Cond	itions	2025	Phase ′ Yaı	l to Lay d 1	down	2025		l to Lay	down	2026		2 to Lay rd 2	down
			tion 1 /EB)		tion 2 /WB)	Direc (NB			tion 2 /WB)	Direc (NB			tion 2 WB)	Direc (NB		Direc (SB/		Direc (NB	tion 1 /EB)	Direc (SB/		Direc (NB			tion 2 /WB)
		Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS
1	Sellards Road – between Route 221 and Tyack Road	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.6	Α	0.1	Α	0.6	Α	0.1	Α
2	Travis Road - North of Sellards Road	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.8	Α	0.0	Α	0.7	А
3	Badger Canyon Road – North of Sellards Road	0.0	Α	0.0	Α	0.0	Α	0.0	А	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	А
4	Plymouth Road – North of Route 14	0.0	Α	0.2	Α	0.0	Α	0.2	Α	0.0	Α	0.2	Α	0.0	Α	0.2	Α	0.3	Α	0.2	Α	0.3	Α	0.2	А
5	Bofer Canyon Road – North of Coffin Road	0.0	Α	0.0	Α	0.0	Α	0.0	А	0.0	Α	0.0	Α	0.1	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	А
6	Locust Grove Road – between Nicosin Road and I-82	0.0	Α	0.1	Α	0.0	Α	0.2	Α	0.0	Α	0.2	Α	0.0	Α	0.2	Α	0.0	Α	1.6	Α	0.0	Α	1.4	А
7	Route 397 – West of Nine Canyon Road	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α
8	Nine Canyon Road – South of Route 397	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α
9	Route 221 – South of Sellards Road (From WSDOT Website)* Data used 6/5/23	0.0	А	0.6	А	0.0	Α	0.6	А	0.0	А	0.7	A	0.0	А	0.6	Α	0.0	А	0.6	А	0.0	А	0.7	А
10	I-82 – North of Coffin Road (From WSDOT Website)	8.4	Α	2.4	Α	8.6	Α	2.5	Α	8.7	Α	2.5	Α	8.6	Α	2.5	Α	8.6	Α	2.5	Α	8.7	Α	2.5	А



Table 12. Segment Capacity Analysis Summary – Weekday PM Peak Hour

		2023	Existin	g Condi	tions	2025	No Buil	d Cond	itions	2026	No Bui	ld Cond	itions	2025	Phase 1 Yar		down	2025		l to Lay	down	2026		2 to Lay	down
		Direc (NB	tion 1 /EB)	Direc (SB/	tion 2 WB)	Direc (NB	tion 1 /EB)		tion 2 /WB)	Direc (NB			tion 2 WB)	Direc (NB			tion 2 WB)	Direc (NB	tion 1 /EB)	Direc (SB/		Direc (NB			tion 2 /WB)
		Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS	Veh Den	LOS
1	Sellards Road – between Route 221 and Tyack Road	0.1	Α	0.1	Α	0.1	Α	0.1	А	0.2	Α	0.1	Α	0.1	Α	0.1	Α	0.2	Α	0.7	Α	0.2	Α	0.7	А
2	Travis Road - North of Sellards Road	0.2	Α	0.0	Α	0.2	Α	0.0	А	0.2	Α	0.0	Α	0.2	Α	0.0	Α	0.7	Α	0.0	Α	0.7	Α	0.0	А
3	Badger Canyon Road – North of Sellards Road	0.0	Α	0.0	Α	0.0	Α	0.0	А	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	А	0.0	Α	0.0	Α	0.0	А
4	Plymouth Road – North of Route 14	0.2	Α	0.2	Α	0.2	Α	0.2	А	0.2	Α	0.2	Α	0.2	Α	0.2	Α	0.2	А	0.6	Α	0.2	Α	0.6	А
5	Bofer Canyon Road – North of Coffin Road	0.0	Α	0.0	Α	0.0	Α	0.0	А	0.0	Α	0.0	Α	0.0	Α	0.1	Α	0.0	А	0.0	Α	0.0	Α	0.0	А
6	Locust Grove Road — between Nicosin Road and I-82	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.1	Α	0.0	Α	0.8	Α	0.0	Α	0.8	Α	0.0	Α
7	Route 397 – West of Nine Canyon Road	0.1	Α	0.1	Α	0.1	Α	0.1	А	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	Α	0.1	А
8	Nine Canyon Road – South of Route 397	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α	0.0	Α
9	Route 221 – South of Sellards Road (From WSDOT Website)* Data used 6/5/23	0.8	А	0.2	Α	0.8	Α	0.2	А	0.8	Α	0.2	А	0.8	А	0.2	А	0.8	А	0.2	А	0.8	Α	0.2	А
10	I-82 – North of Coffin Road (From WSDOT Website)	6.9	Α	12.2	В	7.0	Α	12.4	В	7.1	Α	12.6	В	7.0	Α	12.4	В	7.0	А	12.4	В	7.1	Α	12.6	В



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#### 4.3 UNSIGNALIZED INTERSECTION CAPACITY ANALYSIS RESULTS

As shown in Tables 13 and 14, the intersection capacity analysis indicate that all of the study area intersections currently operate well below capacity (LOS C or better) and will continue to operate well below capacity for all phases of the Project construction. The proposed Project will result in a maximum increase in delay of approximately two seconds per vehicle, except for Bofer Canyon Road and Locust Grove Road/Route 397. This Intersection is expected to have an increase in delay of approximately 8.1 seconds during the weekday evening peak hour for 2025 Build Phase 1 to Laydown Yard 1 condition. The capacity analysis for the study area intersections indicate that ample capacity is available to support peak construction operations associated with the Project. In addition, the proposed site driveways serving Laydown Yard 1 and Laydown Yard 2 are also expected to operate at LOS B or better for all construction scenarios.

Unsignalized Intersection Capacity Analysis Summary – Weekday AM Peak Hour Table 13.

			2023 E	Existing			2025 N	o Build			2026 N	lo Build		20	)25 Build · Laydow	- Phase ′ n Yard 1	l to	20	25 Build - Laydow			20	26 Build · Laydow	– Phase 2 n Yard 2	
Intersection	Movement	v/c <sup>1</sup>	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
I-82 Northbound Ramps &	NB L/R	0.25	11.4	В	25	0.18	10.5	В	17.5	0.18	10.5	В	17.5	0.18	10.5	В	17.5	0.18	10.5	В	17.5	0.18	10.5	В	17.5
Wine Country Road	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.01	7.7	Α	0	0.00	7.6	Α	0	0.00	7.6	Α	0	0.00	7.6	Α	0	0.00	7.6	Α	0	0.00	7.6	А	0
	WB T	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
I-82 Southbound Ramps &	NB L/R	0.25	13.0	В	25	0.22	12.4	В	20	0.22	12.4	В	20	0.22	12.5	В	20	0.32	13.8	В	35	0.31	13.7	В	32.5
Wine Country Road	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0
	WB L	0.01	8.0	Α	0	0.00	7.9	Α	0	0.00	7.9	А	0	0.00	8.0	Α	0	0.00	7.9	Α	0	0.00	7.9	Α	0
	WB T	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Wine Country Road &	NB L/T	0.06	14.8	В	5	0.05	13.9	В	5	0.05	14.0	В	5	0.05	13.9	В	5	0.06	16.0	С	5	0.06	15.9	С	5
Chapman Lane	NB R	0.25	10.6	В	25	0.22	10.3	В	22.5	0.23	10.3	В	22.5	0.24	10.4	В	22.5	0.23	10.3	В	22.5	0.23	10.3	В	22.5
	EB L	0.00	7.5	Α	0	0.00	7.4	Α	0	0.00	7.4	А	0	0.00	7.4	Α	0	0.00	7.4	Α	0	0.00	7.4	Α	0
	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.10	8.2	Α	7.5	0.09	8.1	Α	7.5	0.09	8.1	Α	7.5	0.09	8.1	Α	7.5	0.13	8.3	Α	12.5	0.13	8.3	Α	10
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.01	18.2	С	0	0.01	16.8	С	0	0.01	16.8	С	0	0.01	17.0	С	0	0.01	19.5	С	0	0.01	19.3	С	0
Route 22 &	NB L	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	А	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0
Paterson Road/Route 221	NB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	EB L/T/R	0.09	12.4	В	7.5	0.07	11.7	В	5	0.07	11.8	В	5	0.07	11.8	В	5	0.08	13.1	В	7.5	0.09	13.1	В	7.5
	WB L/T/R	0.13	10.8	В	12.5	0.11	10.4	В	10	0.11	10.4	В	10	0.11	10.5	В	10	0.13	11.0	В	10	0.13	10.9	В	10
	SB L	0.08	8.2	Α	7.5	0.07	8.1	Α	5	0.07	8.1	Α	5	0.07	8.1	Α	5	0.12	8.3	Α	10	0.12	8.3	Α	10
	SB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Route 221 &	NB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
County Well Road	WB L/R	0.00	10.2	В	0	0.00	9.9	Α	0	0.00	9.9	Α	0	0.00	9.9	Α	0	0.00	10.3	В	0	0.00	10.3	В	0
	SB L/T	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	Α	0
Route 221 &	NB L/T/R	0.00	0.0	А	0	0.00	0.0	А	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	А	0
County Well Road	EB L/T/R	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
-	WB L/T/R	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
	SB L/T/R	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
						-100	0	- *			3.0	1		2.00	3.0				0	, ,			3.0		

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Unsignalized Intersection Capacity Analysis Summary – Weekday AM Peak Hour Table 13. (Continued)

			2023 E	Existing			2025 No	o Build			2026 N	o Build		20	25 Build - Laydow		1 to	20	25 Build - Laydow		1 to	20	25 Build - Laydow		2 to
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
Route 221 & Sellards Road	NB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	EB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.01	10.8	В	0	0.01	12.3	В	0	0.01	12.2	В	0
	WB L/T/R	0.09	11.1	В	7.5	0.06	10.3	В	5	0.06	10.4	В	5	0.06	10.4	В	5	0.08	11.7	В	7.5	0.08	11.6	В	5
	SB L/T/R	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.07	7.6	Α	5	0.07	7.6	Α	5
Webber Canyon Road &	NB L/T/R	0.00	7.3	А	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.4	Α	0	0.00	7.4	Α	0
County Well Road	EB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Travis Road &	NB L/T/R	0.00	0.0	А	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0
Cemetery Road	EB L/T/R	0.00	9.0	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0	0.00	9.2	Α	0	0.00	9.2	Α	0
	WB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.00	7.2	А	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0
S. Plymouth Road &	NB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Locust Grove Rad	WB L/R	0.08	8.9	Α	5	0.06	8.8	Α	5	0.06	8.8	Α	5	0.06	8.8	Α	5	0.09	10.6	В	7.5	0.09	10.4	В	7.5
	SB L/T	0.00	7.4	А	0	0.00	7.4	Α	0	0.00	7.4	Α	0	0.00	7.4	Α	0	0.10	7.8	Α	7.5	0.09	7.8	Α	7.5
Paterson Road/Route 221 &	NB L/T/R	0.00	11.2	В	0	0.00	10.7	В	0	0.00	10.7	В	0	0.00	10.7	В	0	0.00	10.7	В	0	0.00	10.7	В	0
Route 14	EB L	0.02	8.6	Α	0	0.01	8.4	Α	0	0.01	8.4	Α	0	0.01	8.4	Α	0	0.01	8.4	Α	0	0.01	8.4	Α	0
	EB T/R	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0
	WB T/R	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.13	10.1	В	10	0.10	9.7	Α	7.5	0.10	9.8	Α	7.5	0.10	9.7	Α	7.5	0.10	9.7	Α	7.5	0.10	9.8	Α	7.5
S. Plymouth Road &	NB L/T/R	0.02	10.0	В	2.5	0.02	9.5	А	0	0.02	9.6	Α	0	0.02	9.5	Α	0	0.02	9.6	Α	2.5	0.02	9.7	А	2.5
Route 14	EB L	0.00	10.0	Α	0	0.00	9.5	Α	0	0.00	9.5	Α	0	0.00	9.5	Α	0	0.00	9.8	Α	0	0.00	9.8	Α	0
	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.23	15.5	С	22.5	0.15	13.0	В	12.5	0.15	13.1	В	12.5	0.15	13.0	В	12.5	0.16	13.5	В	15	0.16	13.5	В	15
I-82 Southbound Off-Ramp	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	А	0
& Route 14	WB L/T	0.01	8.2	Α	0	0.01	8.2	Α	0	0.01	8.2	Α	0	0.01	8.2	Α	0	0.01	8.2	Α	0	0.01	8.2	Α	0
	SB L/T/R	0.19	11.9	В	17.5	0.14	10.9	В	12.5	0.15	11.0	В	12.5	0.14	10.9	В	12.5	0.16	11.5	В	15	0.16	11.5	В	15

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Table 13. (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday AM Peak Hour

			2023 E	Existing			2025 N	o Build			2026 N	lo Build		20	)25 Build · Laydow	– Phase ′ n Yard 1	1 to	20	25 Build · Laydow			20	25 Build · Laydow		
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
I-82 Northbound On-Ramp	NB L/T/R	0.42	11.4	В	52.5	0.35	10.7	В	40	0.35	10.7	В	40	0.35	10.7	В	40	0.41	11.3	В	50	0.41	11.3	В	50
& Route 14/McNary	EB L/T	0.01	7.4	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
I-82 Southbound Off-Ramp	EB T/R	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
& Coffin Road	WB L/T	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0
	SB L/T/R	0.03	8.8	Α	2.5	0.02	8.7	Α	2.5	0.02	8.7	А	2.5	0.02	8.7	Α	2.5	0.02	8.7	Α	2.5	0.02	8.7	Α	2.5
I-82 Northbound On-Ramp	NB L/T/R	0.02	8.6	Α	2.5	0.01	8.5	A	0	0.01	8.5	A	0	0.07	8.7	Α	5	0.01	8.5	А	0	0.01	8.5	Α	0
& Coffin Road	EB L/T	0.00	0.00	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0
Bofer Canyon Road &	NB L/T/R	0.00	8.8	Α	0	0.00	8.7	A	0	0.00	8.7	A	0	0.00	9.6	A	0	0.00	8.7	Α	0	0.00	8.7	Α	0
Coffin Road	EB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.04	7.3	Α	2.5	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.00	8.9	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0	0.00	8.8	Α	0
I-82 Southbound Off-Ramp	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0
& Locust Grove Road	WB L/T	0.10	7.6	Α	7.5	0.08	7.5	Α	5	0.08	7.5	Α	5	0.08	7.5	Α	5	0.08	7.5	А	5	0.08	7.5	Α	5
	SB L/T/R	0.21	11.7	В	20	0.14	10.4	В	12.5	0.14	10.5	В	12.5	0.59	17.7	С	97.5	0.22	10.3	В	20	0.22	10.4	В	20
I-82 Northbound Off-Ramp	NB L/T/R	0.06	10.3	В	5	0.04	9.9	Α	2.5	0.04	9.9	А	2.5	0.06	12.5	В	5	0.04	10.0	В	2.5	0.04	10.0	В	2.5
& Locust Grove Road	EB L/T	0.02	8.4	Α	2.5	0.01	8.3	Α	0	0.01	8.3	Α	0	0.01	8.3	Α	0	0.02	8.3	Α	0	0.02	8.3	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0
Bofer Canyon Road &	NB L/T/R	0.00	10.9	В	0	0.00	10.4	В	0	0.00	10.4	В	0	0.02	12.3	В	2.5	0.00	10.5	В	0	0.00	10.5	В	0
Locust Grove Road	EB L/T/R	0.00	8.1	Α	0	0.00	8.0	Α	0	0.00	8.0	Α	0	0.00	8.0	Α	0	0.00	8.0	Α	0	0.00	8.0	Α	0
/Route397	WB L/T/R	0.00	7.4	Α	0	0.00	7.4	Α	0	0.00	7.4	Α	0	0.02	8.1	Α	2.5	0.00	7.4	А	0	0.00	7.4	Α	0
	SB L/T/R	0.01	10.5	В	0	0.01	10.2	В	0	0.01	10.2	В	0	0.01	10.7	В	0	0.01	10.3	В	0	0.01	10.3	В	0
Route 397 &	EB L	0.01	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	А	0	0.01	7.3	A	0	0.00	7.3	А	0	0.00	7.3	А	0
S. Olympia Street	EB T	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB R	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0
	SB L	0.14	9.1	Α	12.5	0.12	9.0	Α	10	0.12	9.0	Α	10	0.14	9.1	Α	12.5	0.13	9.1	Α	10	0.13	9.1	Α	10

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Table 13. (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday AM Peak Hour

			2023 E	ixisting			2025 No	Build			2026 N	o Build		20	25 Build Laydow	– Phase on Yard 1		20	25 Build - Laydow			20	25 Build · Laydow	– Phase 2 n Yard 2	
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
S. Nine Canyon Road &	NB L/T/R	0.00	8.6	Α	0	0.00	8.4	Α	0	0.00	8.4	Α	0	0.00	8.4	Α	0	0.00	8.4	Α	0	0.00	8.4	Α	0
Route 397	EB L	0.01	7.4	Α	0	0.01	7.4	Α	0	0.01	7.4	Α	0	0.01	7.4	Α	0	0.01	7.4	Α	0	0.01	7.4	Α	0
	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.00	8.3	Α	0	0.00	8.2	Α	0	0.00	8.2	Α	0	0.00	8.2	Α	0	0.00	8.2	Α	0	0.00	8.2	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.07	9.3	Α	5	0.05	9.0	Α	5	0.05	9.0	Α	5	0.05	9.1	Α	5	0.05	9.1	Α	5	0.05	9.1	Α	5
S. Finley Road & Route 397	NB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	А	0
· · · · · · · · · · · · · · · · · · ·	EB T/R	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
	WB L	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0	0.01	7.3	Α	0
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
O Nine Common Band 6	1 ID = 1/D	0.00	0.0	Δ.					_											_	_				
S. Nine Canyon Road &  E. Kirk Road	NB T/R WB L/R	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
E. NIIK NOAU	SB L/T	0.00	0.0	A	0	0.00	0.0	A A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
	SB L/T	0.00	0.0	, ,	J	0.00	0.0	A	U	0.00	0.0	A	U	0.00	0.0	A	U	0.00	0.0	A	U	0.00	0.0	A	U
S. Nine Canyon Road &	NB L/T	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Beck Road	EB L/R	0.00	8.7	Α	0	0.00	8.6	Α	0	0.00	8.6	Α	0	0.00	8.6	Α	0	0.00	8.6	Α	0	0.00	8.6	Α	0
	SB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Nine Canyon Road	NB L/T	0.00	7.6	Α	0	0.00	7.6	A	0	0.00	7.6	А	0	0.00	7.6	А	0	0.00	7.6	А	0	0.00	7.6	А	0
& Coffin Road	EB L/R	0.02	8.5	Α	0	0.01	8.5	A	0	0.01	8.5	A	0	0.01	8.5	A	0	0.01	8.5	A	0	0.01	8.5	A	0
	SB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
S. Clodfelter Road &	NB L/T/R	0.01	9.1	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.0	Α	0	0.01	9.9	Α	0	0.01	9.8	Α	0
Locust Grove Road	EB L/T/R	0.01	7.6	Α	0	0.00	7.6	Α	0	0.00	7.6	Α	0	0.00	7.6	А	0	0.01	7.8	Α	0	0.01	7.8	А	0
	WB L/T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.04	8.8	Α	2.5	0.03	8.8	Α	2.5	0.03	8.8	Α	2.5	0.03	8.8	Α	2.5	0.13	9.8	Α	10	0.12	9.7	Α	10
Webber Canyon Road &	NB T/R	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Badger Road	WB L/R	0.03	8.9	Α	2.5	0.02	8.7	Α	2.5	0.02	8.7	Α	2.5	0.02	8.7	A	2.5	0.02	8.8	Α	2.5	0.03	8.8	Α	2.5
	SB L/T	0.01	7.4	Α	0	0.01	7.4	A	0	0.01	7.4	A	0	0.01	7.4	A	0	0.01	7.4	A	0	0.01	7.4	A	0
						0.0.		, ,		0.0.		**		0.01		- 11		0.0.		, ,		0.0.			
Bofer Canyon Road	NB L/T/R	0.00	6.9	Α	0	0.00	6.9	Α	0	0.00	6.9	Α	0	0.06	7.0	Α	5	0.00	6.9	Α	0	0.00	6.9	Α	0
& Beck Road	EB L/T/R	0.00	6.9	Α	0	0.00	6.9	Α	0	0.00	6.9	Α	0	0.00	7.9	Α	0	0.00	6.9	Α	0	0.00	6.9	Α	0
	WB L/T/R	0.01	8.3	Α	0	0.01	8.2	Α	0	0.01	8.2	Α	0	0.03	9.3	Α	2.5	0.01	8.2	Α	0	0.01	8.2	Α	0
	SB L/T/R	0.01	8.0	Α	0	0.01	8.0	Α	0	0.01	8.0	Α	0	0.46	12.3	В	60	0.01	8.0	Α	0	0.01	8.0	Α	0

<sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout) <sup>2</sup>Delay = Average delay per vehicle (seconds) <sup>3</sup>LOS = Level of Service <sup>4</sup>95<sup>th</sup> percentile queue (feet)

Table 13. (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday AM Peak Hour

			2023 E	xisting			2025 N	o Build			2026 N	o Build		20	25 Build - Laydow			20	25 Build - Laydow			20		– Phase 2 n Yard 2	
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
Beck Road at	NB L/R													0.02	9.7	Α	2.5	0.00	0.0	Α	0	0.00	0.0	Α	0
Laydown Yard 1	EB T/R	In	tersection E	Does Not E	xist	In	tersection E	oes Not E	xist	Ir	ntersection D	oes Not E	xist	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L/R	•												0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0
Locust Grove Road at	EB L/T													0.00	0.0	Α	0	0.15	8.2	Α	12.5	0.14	8.1	Α	12.5
Laydown Yard 2	WB T/R	In	tersection E	Does Not E	xist	In	tersection [	oes Not E	Exist	Ir	ntersection D	oes Not E	xist	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/R													0.00	0.0	Α	0	0.02	11.3	В	2.5	0.02	10.7	В	0

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)

Unsignalized Intersection Capacity Analysis Summary – Weekday PM Peak Hour Table 14.

			2023 E	Existing			2025 No	<b>Build</b>			2026 N	lo Build		20	)25 Build · Laydow	– Phase ′ n Yard 1	1 to	20	25 Build · Laydow			20	25 Build · Laydow	– Phase 2 n Yard 2	
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
I-82 Northbound Ramps &	NB L/R	0.41	13.8	В	50	0.36	12.7	В	40	0.36	12.8	В	40	0.38	13.0	В	45	0.37	13.2	В	42.5	0.37	13.2	В	42.5
Wine Country Road	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.03	7.8	Α	2.5	0.03	7.7	Α	2.5	0.03	7.7	Α	2.5	0.03	7.7	Α	2.5	0.03	7.9	Α	2.5	0.03	7.9	А	2.5
	WB T	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
I-82 Southbound Ramps &	NB L/R	0.30	17.4	С	30	0.28	16.6	С	27.5	0.28	16.8	С	27.5	0.28	16.9	С	27.5	0.30	18.0	С	32.5	0.31	18.0	С	32.5
Wine Country Road	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0
	WB L	0.01	8.3	Α	0	0.01	8.3	Α	0	0.01	8.3	Α	0	0.01	8.3	Α	0	0.01	8.4	Α	0	0.01	8.4	Α	0
	WB T	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0
Wine Country Road &	NB L/T	0.40	31.5	D	45	0.35	27.7	D	37.5	0.36	28.1	D	40	0.38	29.9	D	42.5	0.36	28.0	D	37.5	0.36	28.3	D	40
Chapman Lane	NB R	0.31	11.7	В	32.5	0.29	11.4	В	30	0.29	11.4	В	30	0.29	11.4	В	30	0.36	12.1	В	40	0.36	12.1	В	40
	EB L	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	EB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	WB L	0.18	8.5	Α	17.5	0.17	8.4	Α	15	0.17	8.4	Α	15	0.18	8.5	Α	17.5	0.17	8.4	Α	15	0.17	8.4	Α	15
	WB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/T/R	0.04	26.4	D	2.5	0.04	23.9	С	2.5	0.04	24.3	С	2.5	0.04	25.1	D	2.5	0.04	26.8	D	2.5	0.04	27.1	D	2.5
Route 22 &	NB L	0.01	7.5	Α	0	0.01	7.5	Α	0	0.01	7.5	Α	0	0.01	7.5	Α	0	0.01	7.5	Α	0	0.01	7.5	А	0
Paterson Road/Route 221	NB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
	EB L/T/R	0.14	15.9	С	12.5	0.12	15.0	С	10	0.13	15.3	С	10	0.12	15.3	С	10	0.13	15.8	С	10	0.14	16.0	С	12.5
	WB L/T/R	0.36	13.1	В	40	0.33	12.5	В	35	0.33	12.6	В	35	0.33	12.7	В	35	0.42	13.8	В	52.5	0.42	13.8	В	52.5
	SB L	0.10	8	Α	7.5	0.09	8.0	Α	7.5	0.10	8.0	Α	7.5	0.09	8.0	Α	7.5	0.10	8.0	Α	7.5	0.10	8.0	Α	7.5
	SB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
Route 221 &	NB T/R	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0
County Well Road	WB L/R	0.01	10.3	В	0	0.01	10.0	В	0	0.01	10.0	В	0	0.01	10.0	В	0	0.01	10.5	В	0	0.01	10.4	В	0
	SB L/T	0.00	7.7	Α	0	0.00	7.7	Α	0	0.00	7.7	Α	0	0.00	7.7	Α	0	0.00	7.8	Α	0	0.00	7.8	Α	0
Route 221 &	NB L/T/R	0.00	0	Α	0	0.00	0.0	A	0	0.00	0.0	А	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	Α	0
County Well Road	EB L/T/R	0.00	12.9	В	0	0.00	12.2	В	0	0.00	12.2	В	0	0.00	12.2	В	0	0.00	13.1	В	0	0.00	13.0	В	0
	WB L/T/R	0.00	0	Α	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0
	SB L/T/R	0.00	0	Α	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	0.0	Α	0

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Table 14 (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday PM Peak Hour

2023 Existing						2025 No Build					2026 N	o Build		20	)25 Build · Laydow	– Phase on Yard 1	1 to	20	25 Build - Laydow			2025 Build – Phase 2 to Laydown Yard 2				
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	
Route 221 & Sellards Road	NB L/T/R	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	
	EB L/T/R	0.01	11.7	В	0	0.01	11.3	В	0	0.01	11.3	В	0	0.01	11.3	В	0	0.01	11.4	В	0	0.01	11.4	В	0	
	WB L/T/R	0.11	10.3	В	10	0.09	10.0	В	7.5	0.09	10.0	В	7.5	0.10	10.2	В	7.5	0.18	10.5	В	17.5	0.18	10.4	В	15	
	SB L/T/R	0.04	8.1	Α	2.5	0.03	8.0	Α	2.5	0.03	8.0	Α	2.5	0.03	8.0	Α	2.5	0.04	8.0	Α	2.5	0.03	8.0	Α	2.5	
Webber Canyon Road &	NB L/T/R	0.00	7.3	Α	0	0.00	7.3	А	0	0.00	7.3	Α	0	0.00	7.3	А	0	0.00	7.3	Α	0	0.00	7.3	Α	0	
County Well Road	EB L/T/R	0.01	9	А	0	0.01	8.9	Α	0	0.01	8.9	Α	0	0.01	8.9	А	0	0.01	9.2	Α	0	0.01	9.2	Α	0	
	SB L/T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
Travis Road &	NB L/T/R	0.00	7.3	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.2	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	
Cemetery Road	EB L/T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
	WB L/T/R	0.00	0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
	SB L/T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
S. Plymouth Road &	NB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	A	0	0.00	0	Α	0	
Locust Grove Road	WB L/R	0.02	9.7	Α	2.5	0.02	9.5	Α	2.5	0.02	9.5	Α	2.5	0.02	9.3	Α	2.5	0.24	10.2	В	22.5	0.22	10.1	В	20	
	SB L/T	0.02	7.6	Α	2.5	0.02	7.5	А	2.5	0.02	7.5	Α	2.5	0.02	7.5	А	2.5	0.02	7.6	Α	2.5	0.02	7.6	А	2.5	
Paterson Road/Route 221 &	NB L/T/R	0.04	0.4		0	0.00	0.4		0	0.00	0.4		0	0.00	0.4	Δ.		0.00	0.4	Δ.	0	0.00	0.4			
Route 14	EB L	0.01	9.4 7.8	A	5	0.00	9.1 7.7	A	2.5	0.00	9.1	A	2.5	0.00	9.1 7.7	A	2.5	0.00	9.1 7.7	A	2.5	0.00	9.1 7.7	A	2.5	
	EB T/R	0.00	0	Α Α	0	0.04	0	A	0	0.04	0	A	0	0.04	0	A	0	0.04	0	A	0	0.04	0	A	2.5	
	WB L	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	
	WB T/R	0.00	0	A	0	0.00	0	A	0	0.00	0.0	A	0	0.00	0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	
	SB L/T/R	0.29	12.9	В	30	0.21	11.4	В	20	0.21	11.4	В	20	0.21	11.4	В	20	0.21	11.4	В	20	0.21	11.4	В	20	
S. Plymouth Road &	NB L/T/R	0.06	44.0	В	5	0.05	40.7	В	5	0.05	40.7	В	5	0.05	40.7	В	5	0.05	10.7	В	5	0.05	40.7	В	5	
Route 14	EB L	0.00	11.3	А	0	0.05 0.01	10.7 7.9		0	0.03	10.7 7.9	A	0	0.03	10.7 7.9	А	0	0.05 0.01	7.9	_	0	0.03	10.7 7.9	А	0	
Troute 11	EB T/R	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	
	WB L	0.03	8.1	Α	2.5	0.02	8.0	A	2.5	0.02	8.0	A	2.5	0.02	8.0	Α	2.5	0.02	8.0	A	2.5	0.02	8.0	A	2.5	
	WB T/R	0.00	0	A	0	0.02	0.0	A	0	0.02	0.0	A	0	0.02	0.0	A	0	0.02	0.0	A	0	0.02	0.0	A	0	
	SB L/T/R	0.26	17.7	С	25	0.20	15.2	C	17.5	0.20	15.3	С	17.5	0.20	15.2	С	17.5	0.35	17.8	С	37.5	0.34	17.7	С	37.5	
I-82 Southbound Off-Ramp	EB T/R	0.00	0	А	0	0.00	0	A	0	0.00	0	А	0	0.00	0	А	0	0.00	0	Α	0	0.00	0	А	0	
& Route 14	WB L/T	0.00	0	A	0	0.00	8.2	A	0	0.00	8.2	A	0	0.00	8.2	A	0	0.00	8.4	A	0	0.00	8.4	A	0	
	SB L/T/R	0.00	10	В	7.5	0.01	9.8	A	5	0.01	9.9	A	5	0.01	9.8	A	5	0.01	9.9	A	5	0.01	9.9	A	5	

<sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Table 14 (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday PM Peak Hour

	2023 Existing						2025 N	o Build			2026 N	lo Build		20	25 Build Laydow	– Phase on Yard 1	1 to	20	25 Build - Laydow		1 to	2025 Build – Phase 2 to Laydown Yard 2				
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	
I-82 Northbound On-Ramp	NB L/T/R	0.29	12.3	В	30	0.27	12.0	В	27.5	0.27	12.0	В	27.5	0.27	12.0	В	27.5	0.27	12.0	В	27.5	0.28	12.1	В	27.5	
& Route 14/McNary	EB L/T	0.06	7.4	Α	5	0.06	7.4	Α	5	0.06	7.4	Α	5	0.06	7.4	Α	5	0.06	7.4	Α	5	0.06	7.4	Α	5	
	WB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
I-82 Southbound Off-Ramp	EB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
& Coffin Road	WB L/T	0.01	7.4	Α	0	0.01	7.4	Α	0	0.01	7.4	Α	0	0.05	7.5	Α	2.5	0.01	7.4	Α	0	0.01	7.4	Α	0	
	SB L/T/R	0.03	8.7	А	2.5	0.02	8.6	Α	2.5	0.02	8.6	А	2.5	0.03	9.1	Α	2.5	0.02	8.6	Α	2.5	0.02	8.6	Α	2.5	
I-82 Northbound On-Ramp	NB L/T/R	0.02	8.5	Α	2.5	0.01	8.5	Α	0	0.01	8.5	Α	0	0.01	8.5	Α	0	0.01	8.5	Α	0	0.01	8.5	A	0	
& Coffin Road	EB L/T	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.4	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	
	WB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	А	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
Bofer Canvon Road &	NB L/T/R	0.02	9	Α	2.5	0.02	8.9	A	0	0.02	8.9	А	0	0.02	9.3	Α	2.5	0.02	8.9	Α	0	0.02	8.9	А	0	
Coffin Road	EB L/T/R	0.00	7.2	A	0	0.00	7.2	Α	0	0.00	7.2	A	0	0.00	7.2	A	0	0.00	7.2	A	0	0.00	7.2	A	0	
	WB L/T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	A	0	0.00	0.0	Α	0	
	SB L/T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.06	8.6	Α	5	0.00	0.0	Α	0	0.00	0.0	Α	0	
I-82 Southbound Off-Ramp	EB T/R	0.00	0	Δ.	0	0.00	0	Δ.	0	0.00	0	Δ.	0	0.00	0	Δ.		0.00	0	Δ.	0	0.00	0	Δ.	0	
& Locust Grove Road	WB L/T	0.00	7.5	A	2.5	0.00	7.5	Α	2.5	0.00	7.5	A	0	0.00	7.5	A	2.5	0.00	7.7	A	2.5	0.00	7.7	A	0	
a Locust Grove Road	SB L/T/R	0.02	7.5	В	12.5	0.02	10.0	A B	12.5	0.02	10.0	В	2.5 12.5	0.02	10.1	В	12.5	0.03	10.8	A B	15	0.02	10.7	В	2.5	
	OB ENTITE	0.13	10.1	Ь	12.5	0.14	10.0	ь	12.5	0.14	10.0	ь	12.3	0.13	10.1	Б	12.3	0.10	10.6	ь	13	0.10	10.7	ь	13	
I-82 Northbound Off-Ramp	NB L/T/R	0.16	10	В	15	0.15	9.8	Α	12.5	0.16	9.9	Α	12.5	0.16	10.0	В	15	0.16	10.1	В	15	0.16	10.1	В	15	
& Locust Grove Road	EB L/T	0.03	7.7	Α	2.5	0.03	7.7	Α	2.5	0.03	7.7	Α	2.5	0.04	8.5	Α	2.5	0.09	7.9	Α	7.5	0.09	7.8	Α	7.5	
	WB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
Bofer Canyon Road &	NB L/T/R	0.01	10.3	В	0	0.01	10.1	В	0	0.01	10.2	В	0	0.55	18.2	С	85	0.01	10.2	В	0	0.01	10.3	В	0	
Locust Grove Road	EB L/T/R	0.00	7.4	Α	0	0.00	7.4	Α	0	0.00	7.4	А	0	0.00	7.4	А	0	0.00	7.4	Α	0	0.00	7.4	Α	0	
/Route397	WB L/T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0	0.00	7.7	А	0	0.00	0.0	Α	0	0.00	0.0	А	0	
	SB L/T/R	0.01	9.4	А	0	0.01	9.3	Α	0	0.01	9.3	А	0	0.01	9.3	А	0	0.01	9.3	А	0	0.01	9.3	А	0	
D + 007.0	<b>ED</b> 1																									
Route 397 &	EB L	0.12	7.7	Α	10	0.11	7.7	Α	10	0.12	7.7	Α	10	0.13	7.8	Α	10	0.12	7.7	Α	10	0.12	7.8	Α	10	
S. Olympia Street	EB T	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	
	WB T/R	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	
	SB R	0.05	12.9	В	5	0.05	12.6	В	5	0.05	12.7	В	5	0.05	13.2	В	5	0.05	13.0	В	5	0.05	13.1	В	5	
<sup>1</sup> v/c = Volume to capacity ratio	SB L	0.07	9	Α	5	0.07	9.0	Α	5	0.08	9.0	Α	5	0.07	9.0	Α	5	0.07	9.0	Α	5	0.07	9.0	Α	5	

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)



Table 14 (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday PM Peak Hour

			2023 E	Existing			2025 No	<b>Build</b>			2026 N	o Build		20	025 Build Laydow	- Phase n Yard 1		202	25 Build · Laydow			2025 Build – Phase 2 to Laydown Yard 2				
Intersection	Movement	v/c¹	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	
S. Nine Canyon Road &	NB L/T/R	0.04	9.4	Α	2.5	0.03	9.1	Α	2.5	0.03	9.1	Α	2.5	0.03	9.1	Α	2.5	0.03	9.1	Α	2.5	0.03	9.1	Α	2.5	
Route 397	EB L	0.04	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	0.03	7.5	Α	2.5	
	EB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
	WB L	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	
	WB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
	SB L/T/R	0.03	8.9	Α	2.5	0.02	8.8	Α	2.5	0.03	8.8	Α	2.5	0.02	8.8	Α	2.5	0.02	8.8	Α	2.5	0.03	8.8	Α	2.5	
S. Finley Road & Route 397	NB L/T/R	0.04	9.1	Α	2.5	0.03	8.8	Α	2.5	0.03	8.8	Α	2.5	0.03	8.8	Α	2.5	0.03	8.8	А	2.5	0.03	8.8	А	2.5	
	EB T/R	0.00	0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	0.00	0.0	A	0	
	WB L	0.00	7.3	A	0	0.00	7.3	A	0	0.00	7.3	A	0	0.00	7.3	A	0	0.00	7.3	A	0	0.00	7.3	A	0	
	WB T/R	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	0.00	0	A	0	
					-		-		-		-						-		-							
S. Nine Canyon Road &	NB T/R	0.00	0	A	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
E. Kirk Road	WB L/R SB L/T	0.00	8.7	A	0	0.00	8.6	A	0	0.00	8.6	A	0	0.00	8.6	A	0	0.00	8.6	A	0	0.00	8.6	A	0	
	SD L/I	0.00	0	A	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
S. Nine Canyon Road &	NB L/T	0.00	0	А	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
Beck Road	EB L/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
	SB T/R	0.00	0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
Nine Canyon Road	NB L/T	0.02	7.2	А	2.5	0.01	7.2	A	0	0.01	7.2	А	0	0.01	7.2	А	0	0.01	7.2	А	0	0.01	7.2	А	0	
& Coffin Road	EB L/R	0.02	8.9	Α	2.5	0.01	8.8	Α	0	0.01	8.8	Α	0	0.01	8.8	Α	0	0.01	8.8	Α	0	0.01	8.8	Α	0	
	SB T/R	0.00	0	Α	0	0.00	0	Α	0	0.00	0	А	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
S. Clodfelter Road &	NB L/T/R	0.00	9.7	А	0	0.00	9.5	Α	0	0.00	9.5	А	0	0.00	9.5	Α	0	0.00	12.0	В	0	0.00	11.7	В	0	
Locust Grove Road	EB L/T/R	0.02	7.4	А	2.5	0.02	7.4	Α	2.5	0.02	7.4	Α	2.5	0.02	7.4	Α	2.5	0.07	7.5	А	5	0.07	7.5	Α	5	
	WB L/T/R	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.3	Α	0	0.00	7.5	Α	0	0.00	7.5	Α	0	
	SB L/T/R	0.02	8.9	Α	2.5	0.02	8.8	Α	2.5	0.02	8.8	Α	2.5	0.02	8.9	Α	2.5	0.03	9.6	Α	2.5	0.02	9.6	Α	2.5	
Webber Canyon Road &	NB T/R	0.00	0	Α	0	0.00	0	A	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	0.00	0	Α	0	
Badger Road	WB L/R	0.07	9.2	А	5	0.06	9.0	Α	5	0.06	9.0	Α	5	0.06	9.0	Α	5	0.06	9.4	Α	5	0.06	9.3	Α	5	
	SB L/T	0.04	7.5	А	2.5	0.04	7.5	А	2.5	0.04	7.5	А	2.5	0.04	7.5	А	2.5	0.04	7.6	А	2.5	0.04	7.6	А	2.5	
Bofer Canyon Road	NB L/T/R	0.00	6.6	A	0	0.00	6.6	Α	0	0.00	6.6	Α	0	0.01	7.2	Α	0	0.00	6.6	Α	0	0.00	6.6	Α	0	
& Beck Road	EB L/T/R	0.00	7.1	А	0	0.00	7.1	Α	0	0.00	7.1	Α	0	0.00	7.5	Α	0	0.00	7.1	Α	0	0.00	7.1	Α	0	
	WB L/T/R	0.00	6.3	Α	0	0.00	6.3	Α	0	0.00	6.3	Α	0	0.38	8.6	Α	45	0.00	6.3	Α	0	0.00	6.3	Α	0	
	SB L/T/R	0.01	7.6	Α	0	0.01	7.6	Α	0	0.01	7.6	Α	0	0.02	8.5	Α	2.5	0.01	7.6	Α	0	0.01	7.6	Α	0	

<sup>1</sup>v/c = Volume to capacity ratio <sup>2</sup>Delay = Average delay per vehicle (seconds) <sup>3</sup>LOS = Level of Service <sup>4</sup>95<sup>th</sup> percentile queue (feet)



## Table 14 (Continued) Unsignalized Intersection Capacity Analysis Summary – Weekday PM Peak Hour

2023 Existing						2025 No Build				2026 No Build				20	25 Build - Laydow			20	25 Build · Laydow			2025 Build – Phase 2 to Laydown Yard 2			
Intersection	Movement	v/c <sup>1</sup>	Delay <sup>2</sup>	LOS <sup>3</sup>	95 <sup>th</sup> Q <sup>4</sup>	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q	v/c	Delay	LOS	95 <sup>th</sup> Q
Beck Road at	NB L/R														10.8	В	45	0.00	0.0	Α	0	0.00	0.0	А	0
Laydown Yard 1	EB T/R	In	tersection D	oes Not E	xist	Intersection Does Not Exist				Intersection Does Not Exist			0.00	0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	Α	0	
	WB L/R								0.00				0.0	Α	0	0.00	0.0	Α	0	0.00	0.0	А	0		
Locust Grove Road at	EB L/T					Intersection Does Not Exist								0.07	7.3	Α	5	0.10	8.3	Α	7.5	0.00	7.3	Α	0
Laydown Yard 2	WB T/R	In	tersection D	oes Not E	xist					Ir	ntersection L	Does Not E	xist	0.02	7.1	Α	2.5	0.00	0.0	Α	0	0.00	0.0	Α	0
	SB L/R												0.00	0.0	Α	0	0.42	9.8	Α	52.5	0.36	10.8	В	42.5	

<sup>&</sup>lt;sup>1</sup>v/c = Volume to capacity ratio (no v/c reported for roundabout)

<sup>2</sup>Delay = Average delay per vehicle (seconds)

<sup>3</sup>LOS = Level of Service

<sup>4</sup>95<sup>th</sup> percentile queue (feet)

#### 4.4 TRUCK HAUL ROUTES - OVERSIZED LOADS

The construction of the proposed wind and solar facility will require large vehicle deliveries for a variety of materials that may include concrete, solar panels, earth materials, building materials, etc. It is anticipated that the majority of construction truck trips to and from the Project site will follow the same general trip distribution patterns described for the workforce Project trips identified earlier in the study.

In addition to the standard sized truck deliveries associated with Project construction, it is anticipated that oversized truck loads will also be needed to deliver wind turbine components and some larger components of the proposed solar arrays. It is anticipated that that all oversized load deliveries would originate to/from the south on Interstate 82 (I-82). Tetra Tech has identified potential truck haul routes between the site parcels and the regional roadway system for these larger oversized delivery vehicles. The potential entering and exiting truck haul routes for the Project oversized load trucks are shown in Figures 30 and 31, respectively.

It is also anticipated that, due to manufacturer constraints, wind turbine tower components will be needed to be delivered and stored on site prior to the construction of the turbines. As currently proposed, the tower transfer yard would be located on the south side of Locus Grove Road, just west of I-82. The specific oversized truck load routes for tower components destine to the tower transfer yard are shown on Figures 32 and 33.

The specific type of oversized vehicle(s) needed to support these additional construction deliveries are highly dependent on the turbine manufacturer and selected transport logistics company. The manufacturer and transport logistics company have not yet been identified. A detailed evaluation of potential truck haul routes for these oversized wind turbine deliveries will be conducted once a manufacturer is chosen.

Additionally, the Applicant commits to preparing a Construction Transportation Management Plan (CTMP) prior to beginning construction to include temporary construction warning signage.

#### 5.0 TRAFFIC MITIGATION

As discussed in previous sections of this report, the proposed Project is not anticipated to result in capacity constraints at any of the roadway segments and study area intersections evaluated in the study. Tetra Tech has identified safety improvements to be implemented as part of the proposed Project to address existing safety deficiencies and off-set the potential traffic increases associated with the proposed Project.

#### 5.1 TRAFFIC SAFETY PLAN

The draft Traffic Safety Plan (TSP) will assist in developing the CTMP. The TSP will supply information about the future impacts to the transportation network due to the peak construction of the Project. These impacts are expected to occur within and around the Project boundary during the construction phases of the Project. This document will provide recommendations to enhance traffic safety during construction.

# 5.1.1 Objectives and Strategies

The objective of traffic safety management plan is:

- To avoid interrupting normal traffic flow. This flow varies depending on the time (rush hours, shift change) and location of the Project entrances (cars, trucks, deliveries).
- For the management of traffic flow allowing Project daily activities to be conducted in a safe manner.
- To protect all road users by defining the type of required traffic safety equipment. The equipment
  to be used will be, but not limited to, truck route signage, detour route signage, barricades, traffic
  lights, safety cones, fences, closed road signs.
- To enclose and separate the walking/driving paths on site while construction work is being performed.

The Project has multiple strategies to ensure a smooth traffic flow in and out the Project construction zone. These plans will include the following:

- Ensuring construction tasks will be conducted sequentially to the extent practicable to lessen any
  impacts as far as schedule or cost.
- Detailed safety Project procedure will be issued with the involvement and approval of both Project and safety management, taking into consideration the entrance to the Project by both equipment and personnel.
- All entry and exit movements to and from traffic streams shall be in accordance with the Project's core process and Environmental Safety and Health Plan (ES&H).
- Design the Project Driveways to include the proper number of traffic lanes and traffic control infrastructure to accommodate the traffic flow and provide safe exit onto the local roadway system.
- To the extent practicable, schedule deliveries to the site to occur during off-peak construction worker commuting times.

All traffic management works, and control devices shall be in accordance with Washington State and Federal Law for construction and improvement of roads. The safety manager and supervisors will work to implement the safety rules on site by keeping a close supervision of all personnel. They will take all reasonable measures to prevent accident or injury during the construction of the plant. All approved procedures and managements practices should be applied and closely implemented.

The final TSP will be finalized in coordination with the Washington State Department of Transportation (WSDOT) and Energy Facility Site Evaluation Council (EFSEC) and will include the following elements to be developed in consideration of local and federal regulations and potential impacts to the surrounding communities:

- Identification of a safety management team responsible for overseeing Project safety. Key personnel will have their roles and responsibilities outlined. The safety management team will routinely conduct safety inspections. Should any safety hazards be identified, the safety management team will implement the appropriate corrective measures in a timely fashion.
- Assessment of potential wind and solar construction site hazards and identification of measures to prevent and mitigate these hazards.
- Develop a training program in coordination with the contractor covering all aspects of safety. All
  construction staff will be required to undergo safety training prior to working at the Project
  construction site. Supplemental safety training will be offered throughout the duration of Project
  construction. The training program will encourage workers to report safety concerns and
  recommendations as well as promoting a culture of safety and open communication.
- Identification of required personal protective equipment (PPE) for wind and solar construction activities. Protocols for the proper use, maintenance and replacement of PPE will be established prior to construction.

- Development of an Emergency Response Plan (ERP) establishing protocols for evacuations, medical emergencies as well as fire and severe weather incidents occurring in the vicinity of the Project construction sites.
- Development of incident reporting and investigation protocol for reporting and documenting safety hazards.

## **5.1.2 Construction Traffic Management Plan**

A CTMP will be developed in consultation with the WSDOT and Benton County public works staff, as appropriate, prior to construction. The CTMP will follow WSDOT Design Manual 22-01.21 Chapter 1010. A CTMP is a key element in addressing known work zone safety and mobility impacts.

The CTMP may include Temporary Traffic Control (TTC) Components such as:

- · Lane closures or lane shifts
- Traffic Control devices
- Pavement markings
- · Changeable message signs
- Temporary signals

Transportation Systems Management and Operations (TSMO) is the second component to a TMS. Key TSMO components may include Work Zone Safety Management strategies such as:

- Positive protective devices
- · Speed limit reductions
- Automated flagger assistance devices
- · Radar speed display signs

The final element of a CTMP to be considered are Public Information and Awareness Strategies, which may include, but are not limited to, the following strategies:

- Public Awareness Strategies such as Brochures or mailers, press releases, paid advertisements, and Project website (consider providing information in other languages if appropriate).
- Motorist Information Strategies such as Highway advisory radio (HAR), changeable message signs, and transportation management center (TMC).

# 5.2 TRANSPORTATION DEMAND MANAGEMENT (TDM) PROGRAM

The Applicant also commits to implementing a Transportation Demand Management (TDM) program to reduce automobile travel and traffic impacts associated with the Project construction. Potential TDM measures include the following:

- Encourage carpooling among the construction workforce
- Provide a transportation coordinator who can match workers for carpooling
- Explore the feasibility of providing shuttle bus service between the proposed laydown areas and the construction sites
- Explore the feasibility of coordinating a Vanpool service from select locations within the Tri-Cities area.

## **6.0 CONCLUSIONS**

Tetra Tech has evaluated the potential traffic impacts associated with the temporary increase in traffic associated with peak construction operations for the Project at 29 existing study area intersections and the proposed site driveways at the two laydown areas. The analysis indicates that ample capacity is available within the study area to support the peak construction workforce levels associated with the Project with all intersections operating well below capacity at LOS C or better operations.

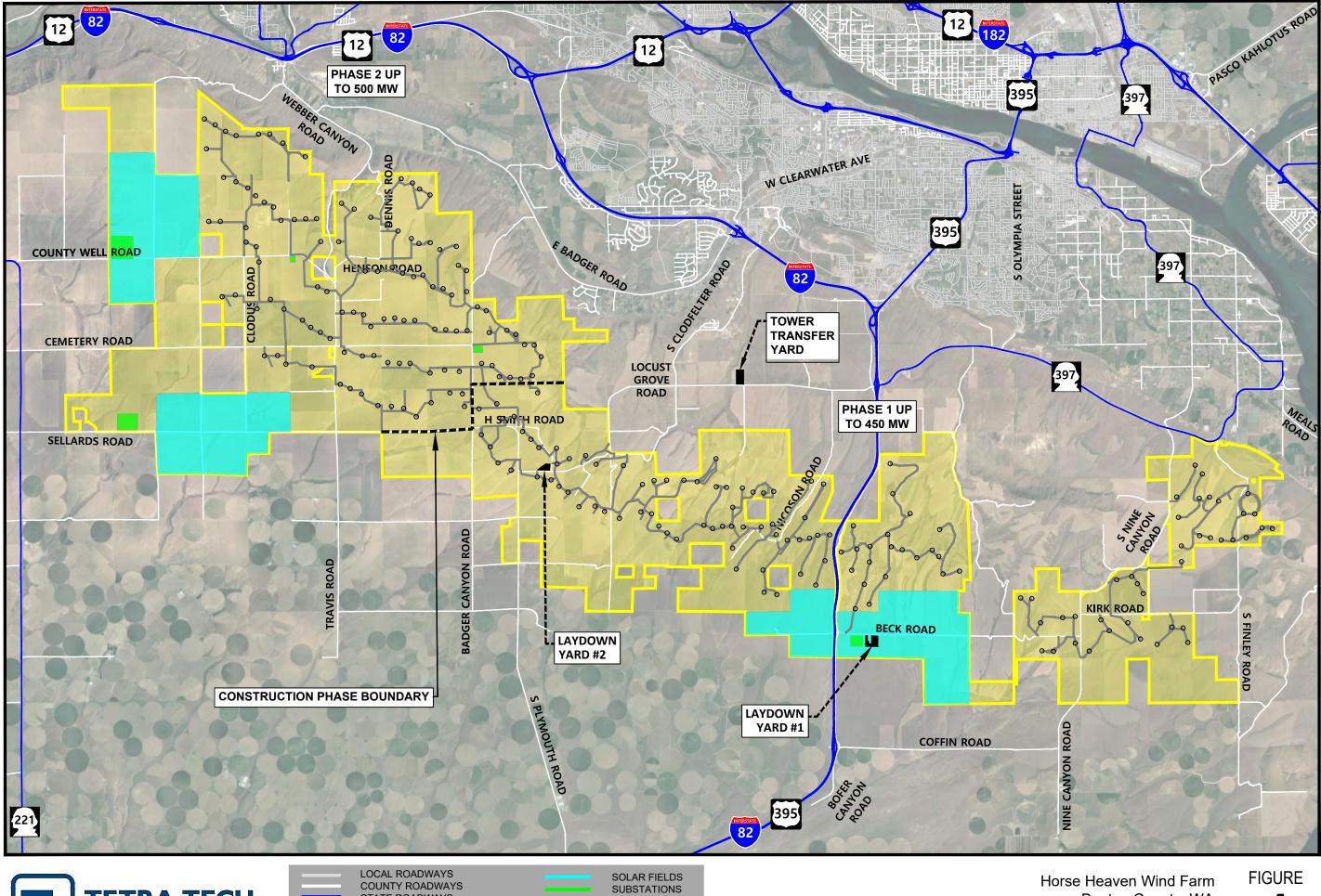
The safety analysis conducted as part of this study indicates that four of the existing study intersections would benefit from potential safety mitigation measures to address existing safety deficiencies. An auxiliary lane warrant analysis indicates that two of the study intersections (Route 14 at S. Plymouth Road and Route 221 at Route 14) currently meet the traffic volume threshold for implementation of separate right-turn lanes under existing traffic volume conditions (independent of the Project construction activity). However, the intersection capacity analysis indicates that these intersections will continue to operate well below capacity during all phases of the Project construction. In addition, the available sight distance at the high crash rate intersections is well above the required Stopping Sight Distance (SSD), indicating that motorists approaching these locations will have more than sufficient view of the potential turning movements at the intersections to stop or adjust their travel speeds as needed to avoid a potential collision.

Given the temporary nature of the of the potential traffic increases associated with Project construction, no additional turns are recommended at these intersections by the Applicant as part of the proposed Project at this time. However, the Applicant is committed to developing a comprehensive Construction Traffic Management Plan (CTMP), for each phase of the construction, to alert drivers of potential increased construction traffic and turning activities at key intersections throughout the study area. The Applicant will also work with the general contractor (once selected) to identify truck haul routes to and from the construction laydown areas avoid sensitive areas and minimize impacts to local agricultural activity along the study area roadways, to the extent possible. In addition, the Applicant will prepare a Traffic Safety Plan for the Project-related construction activity in accordance with the Washington State Department of Transportation (WSDOT) standards.

The Applicant will also work with the contractor to develop a Travel Demand Management (TDM) program to minimize reliance on single-occupant vehicles and reduce single occupancy vehicle trips to and from the site.

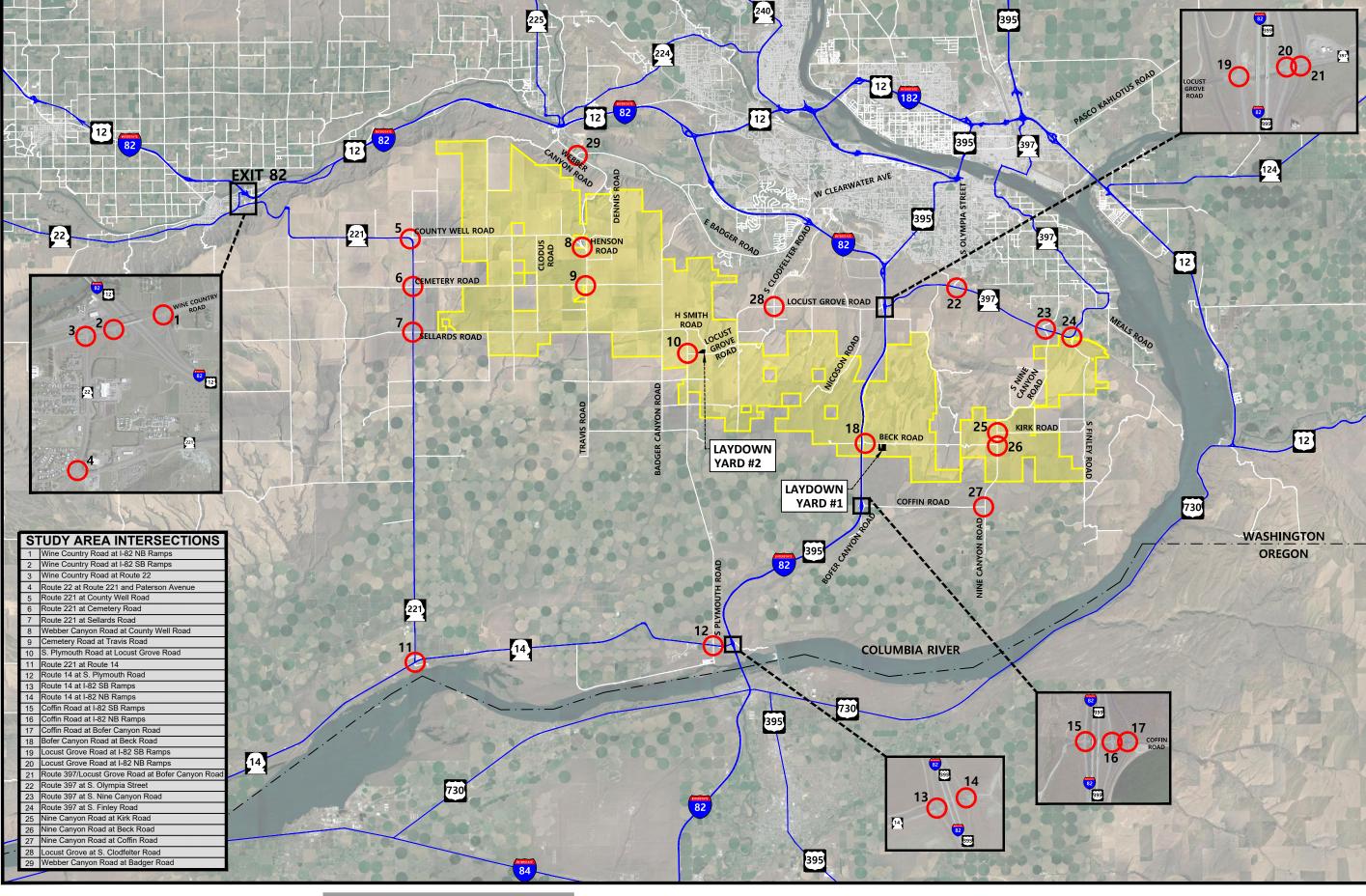
# **FIGURES**



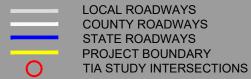


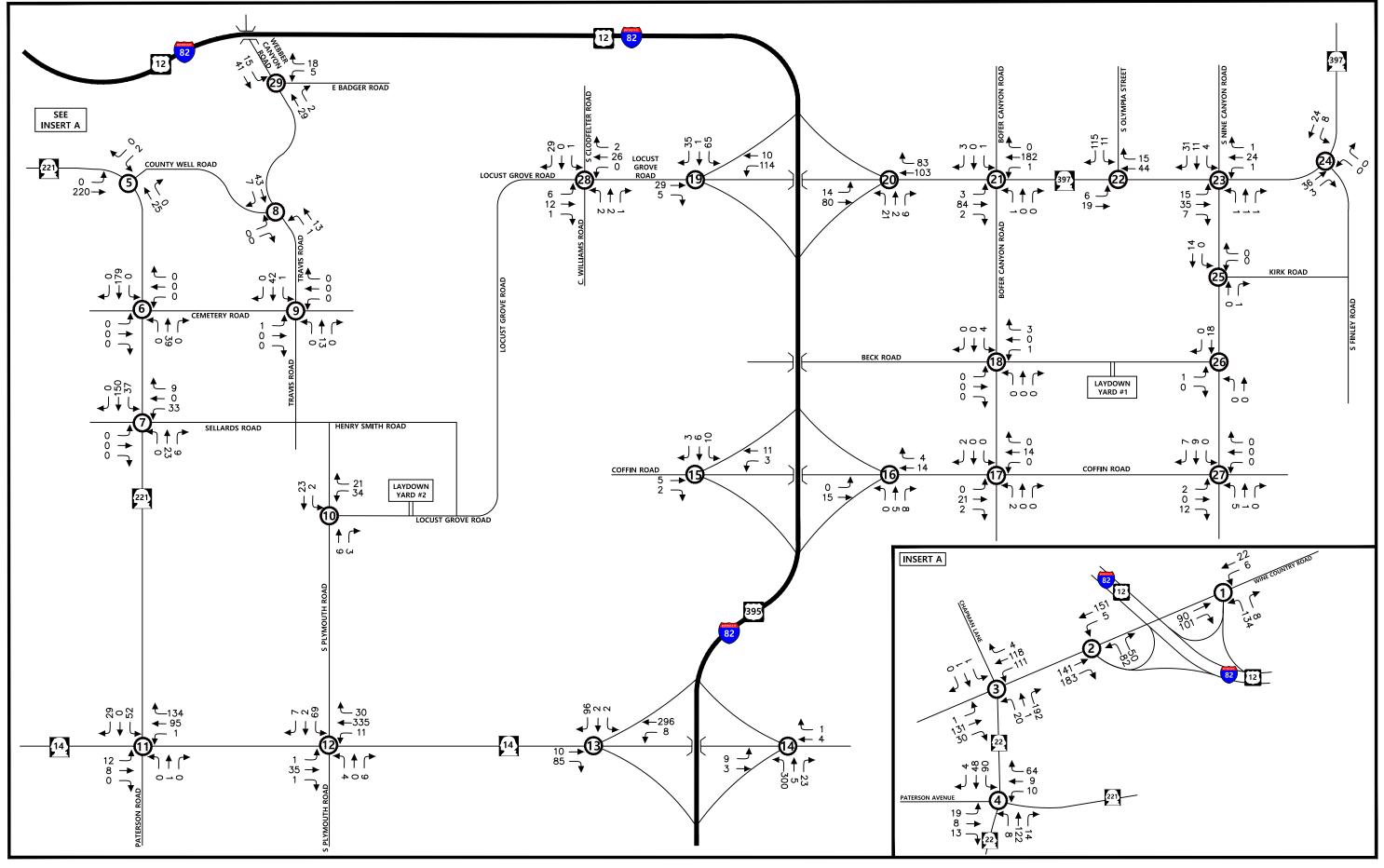


PROPOSED INTERNAL ROADWAYS



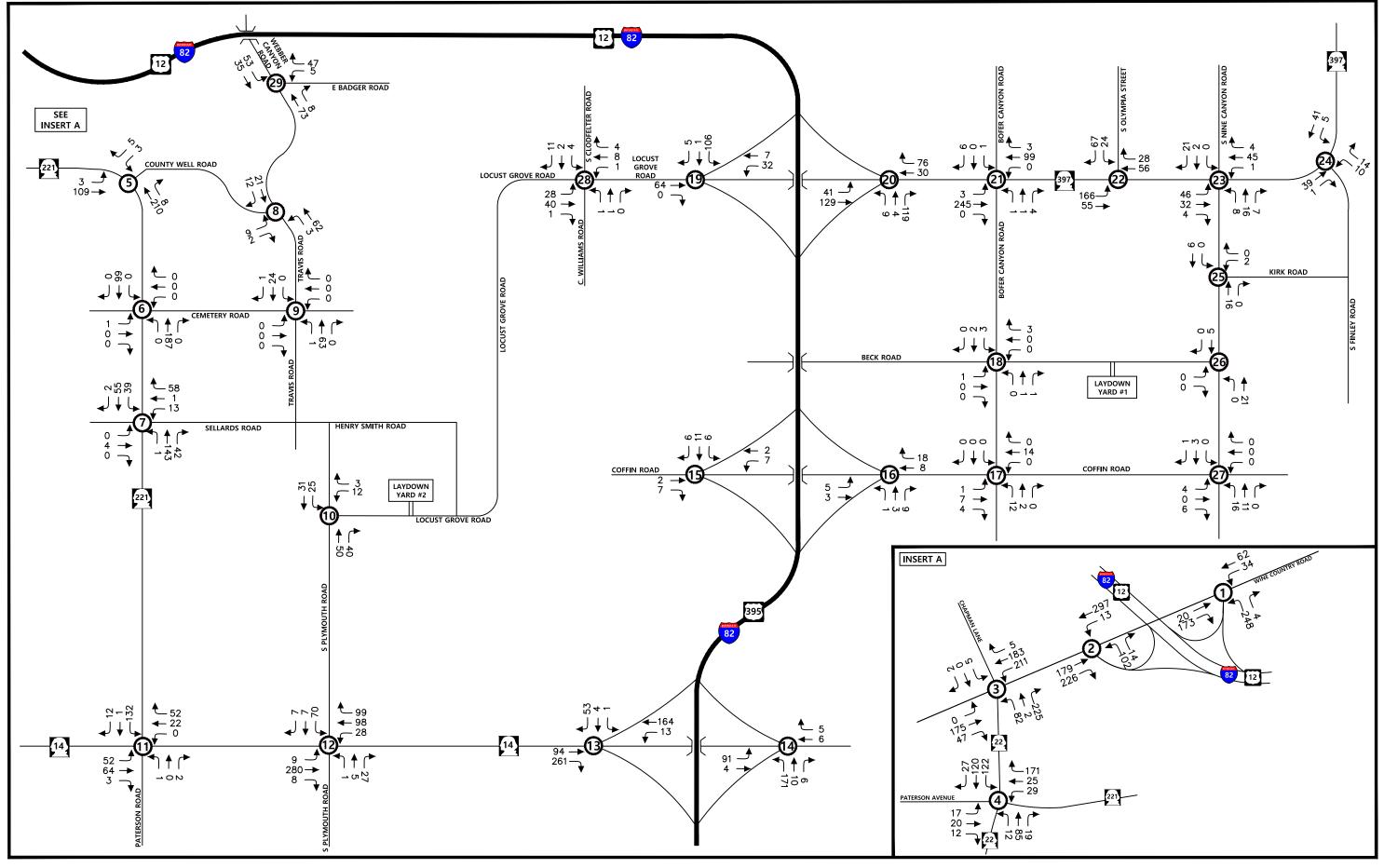






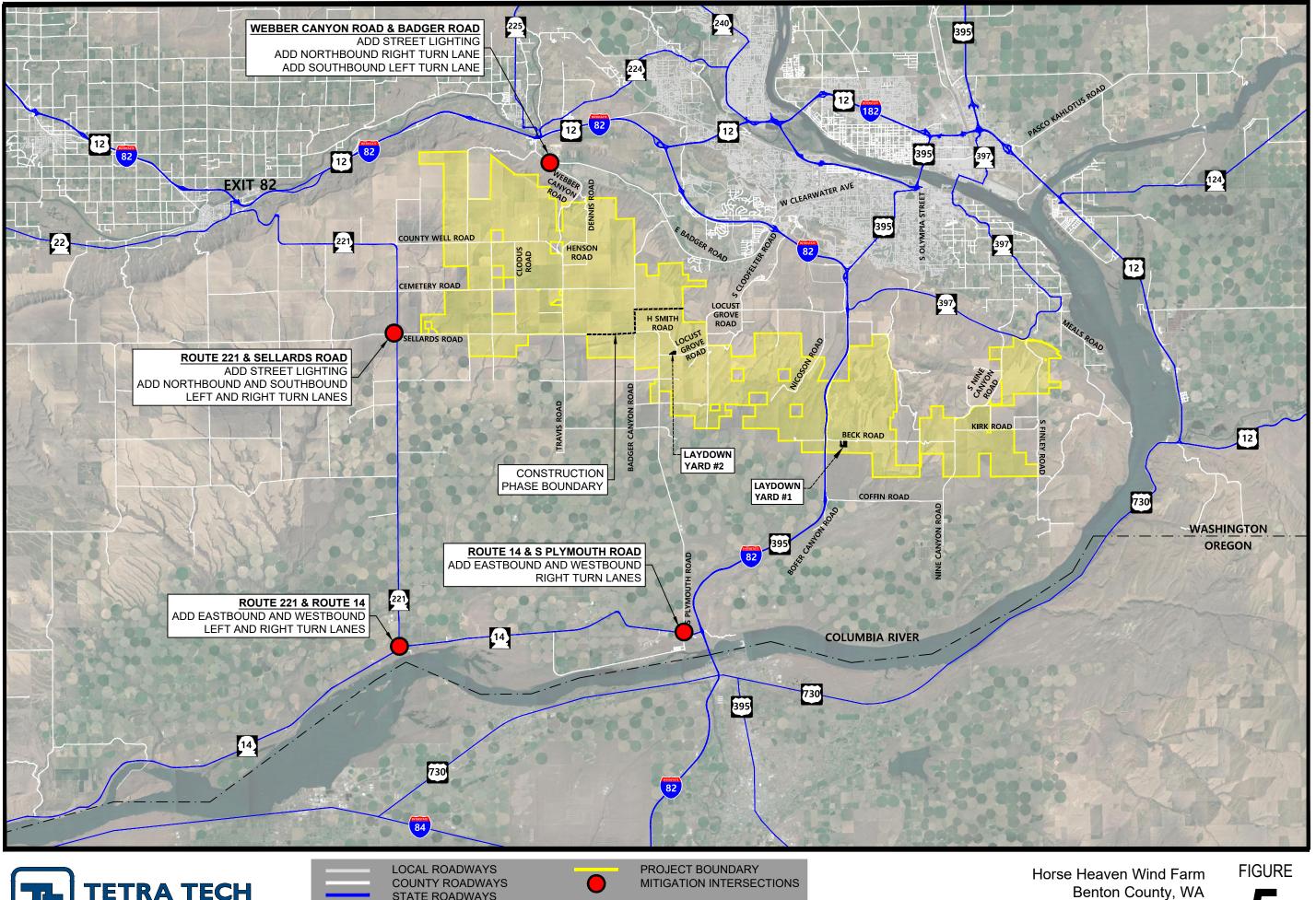


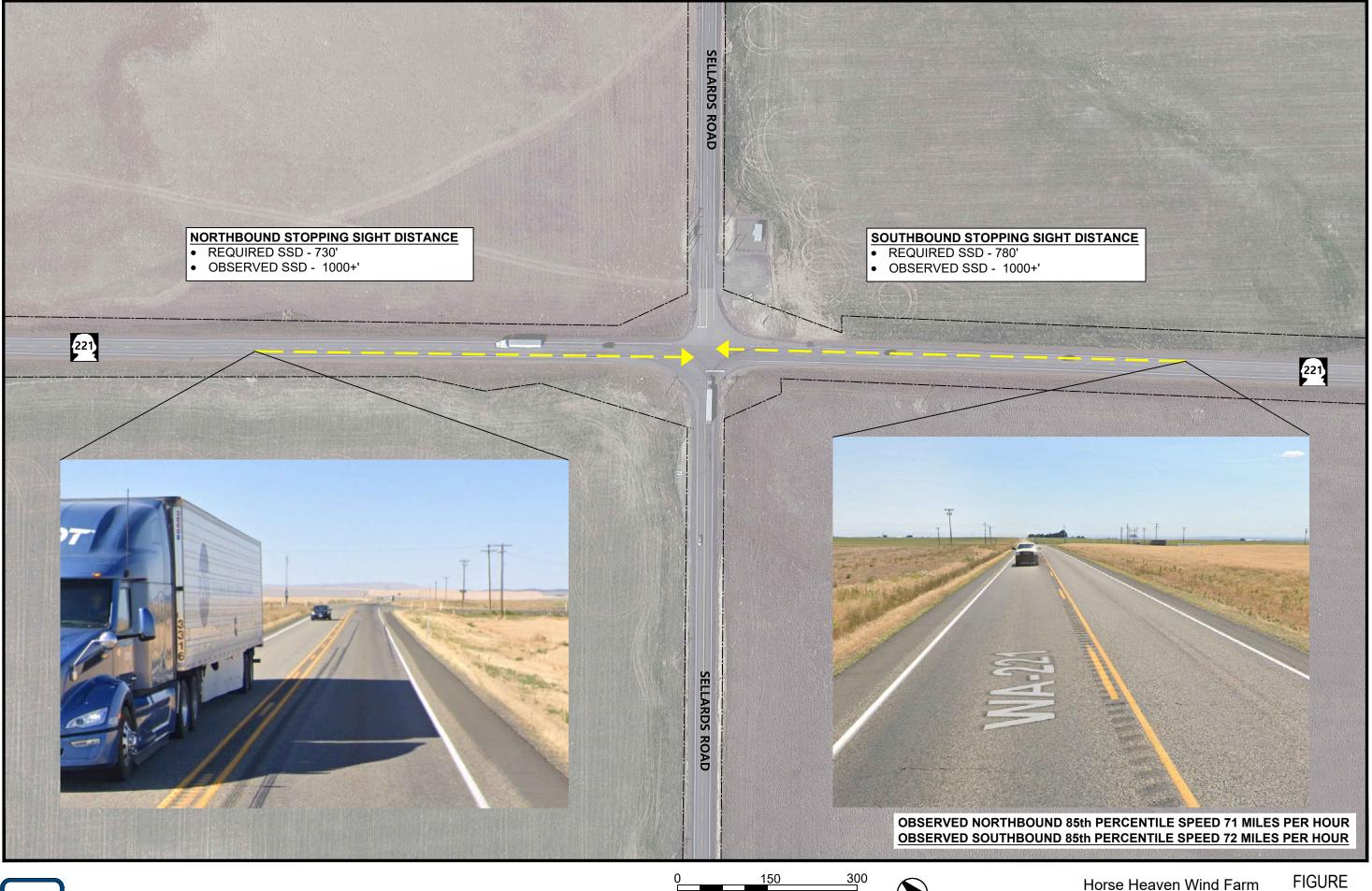




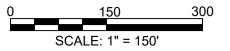


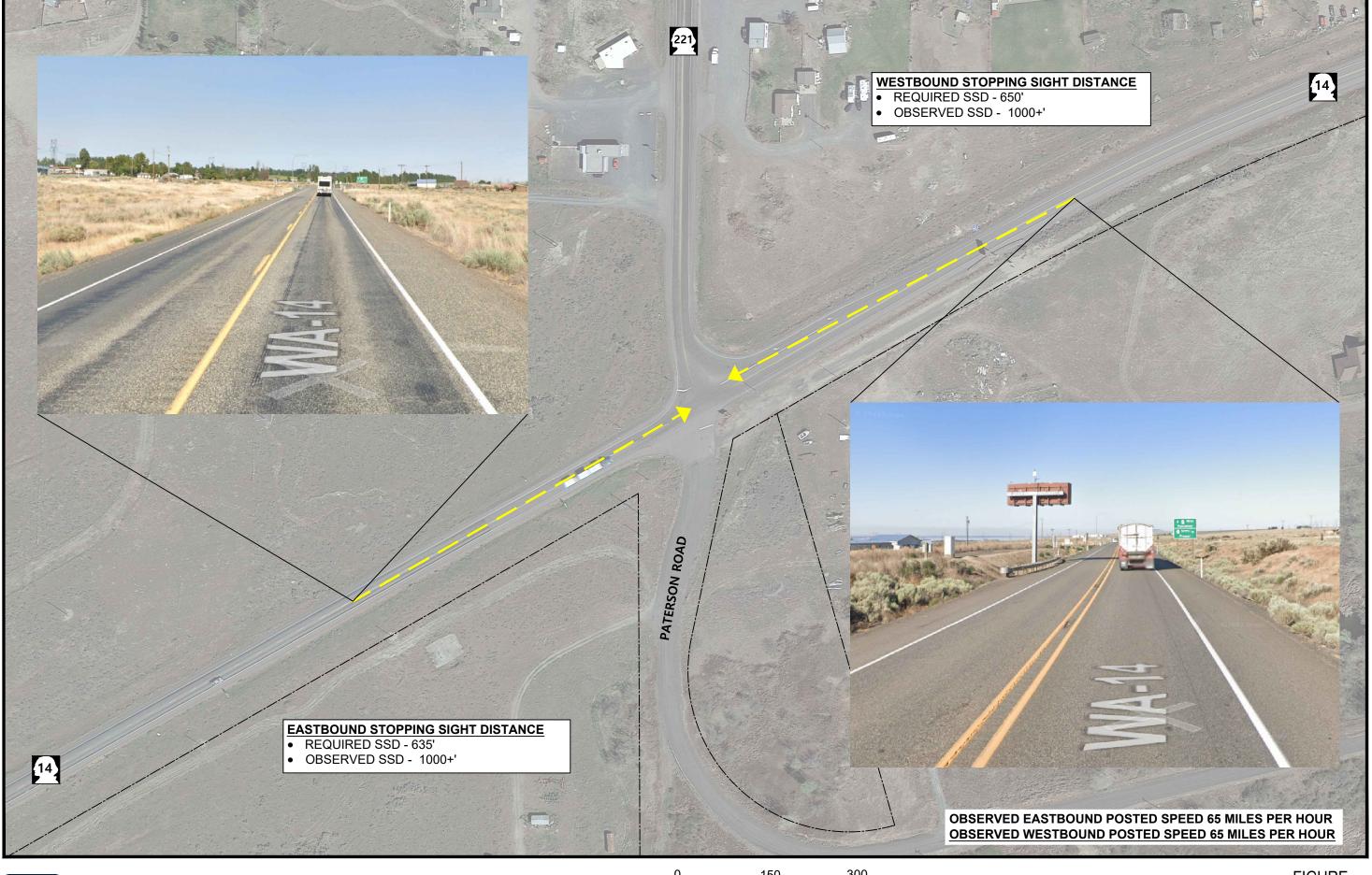














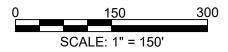
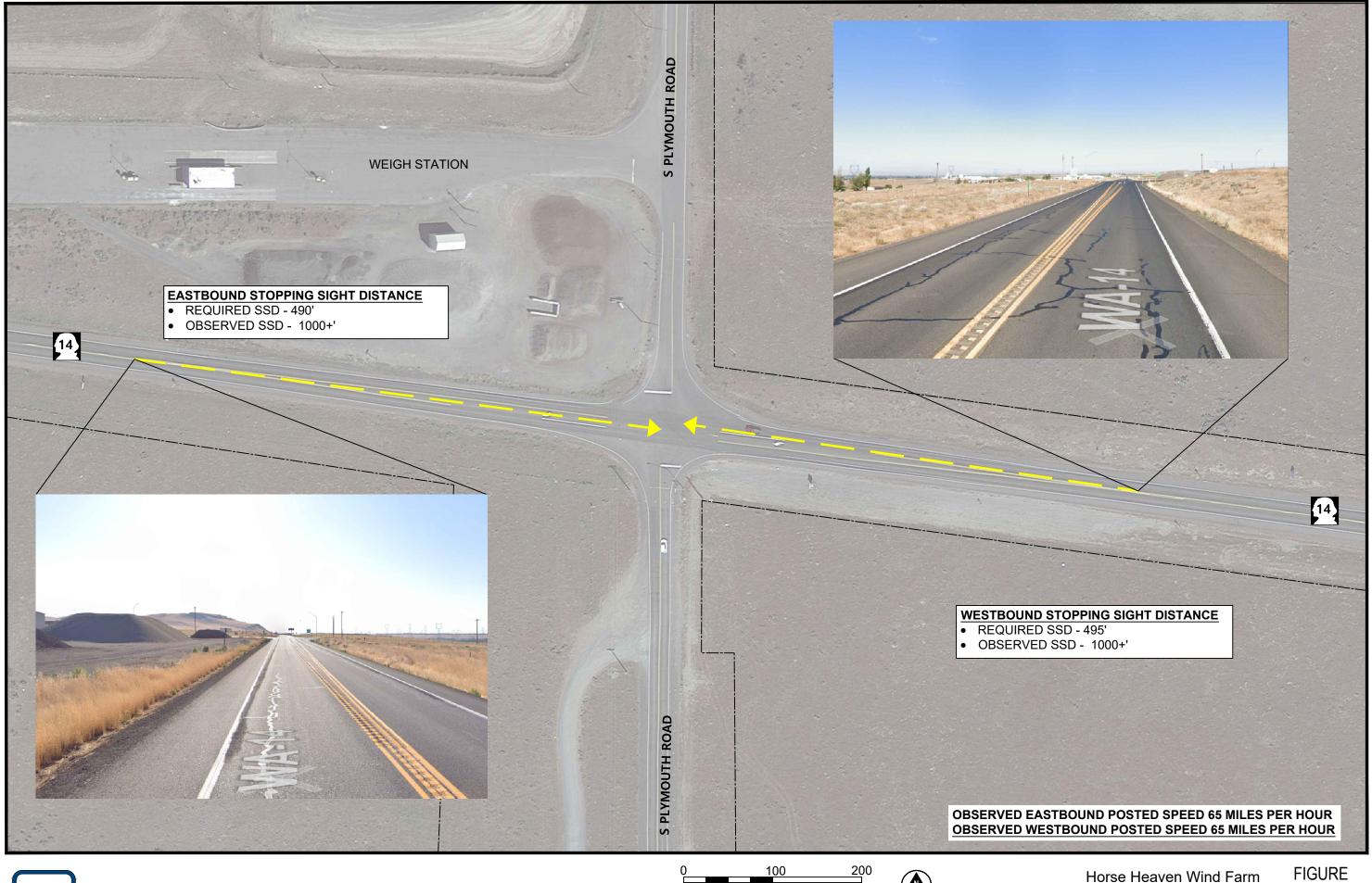
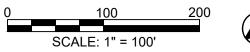
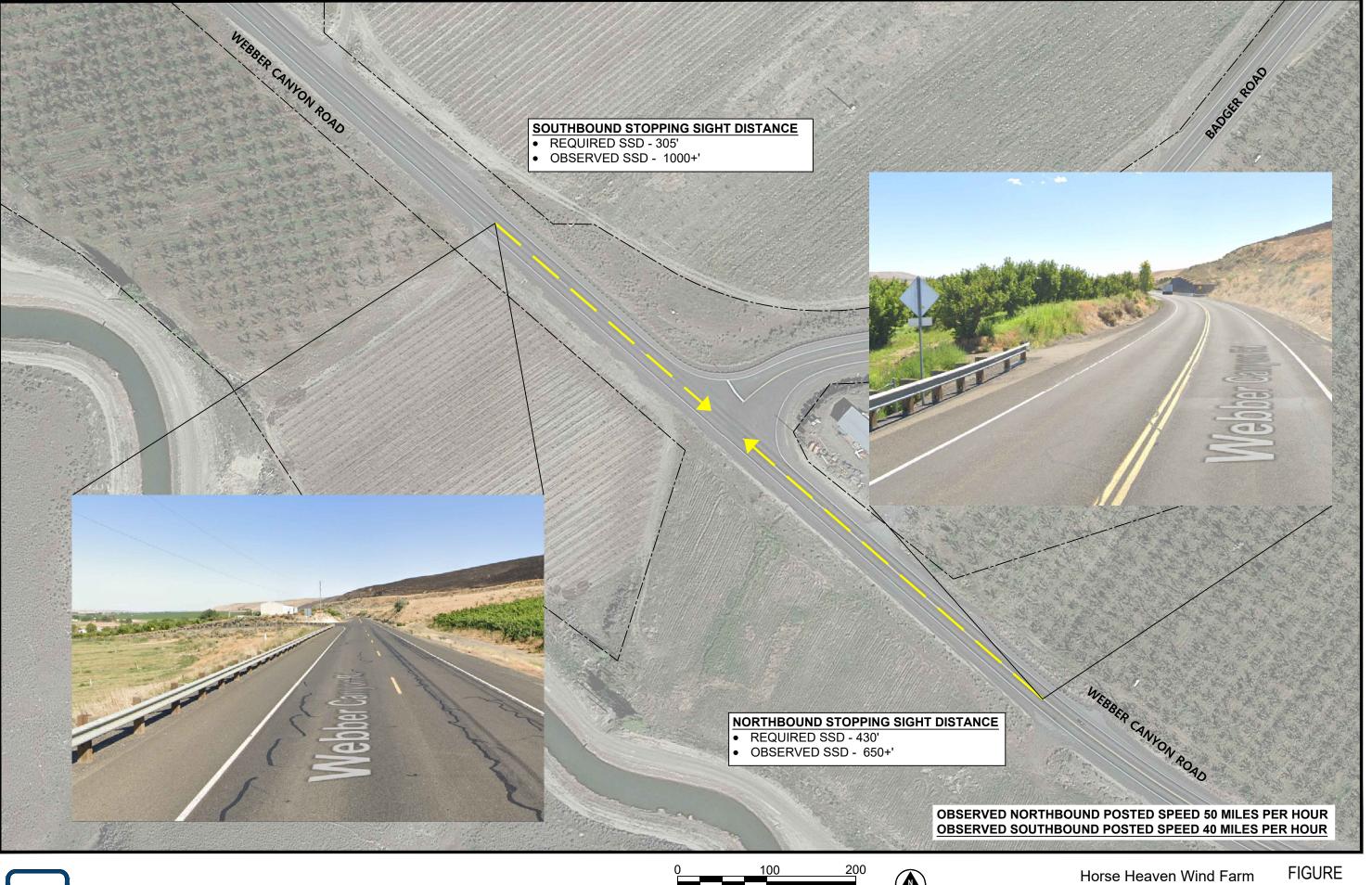


FIGURE 7

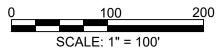




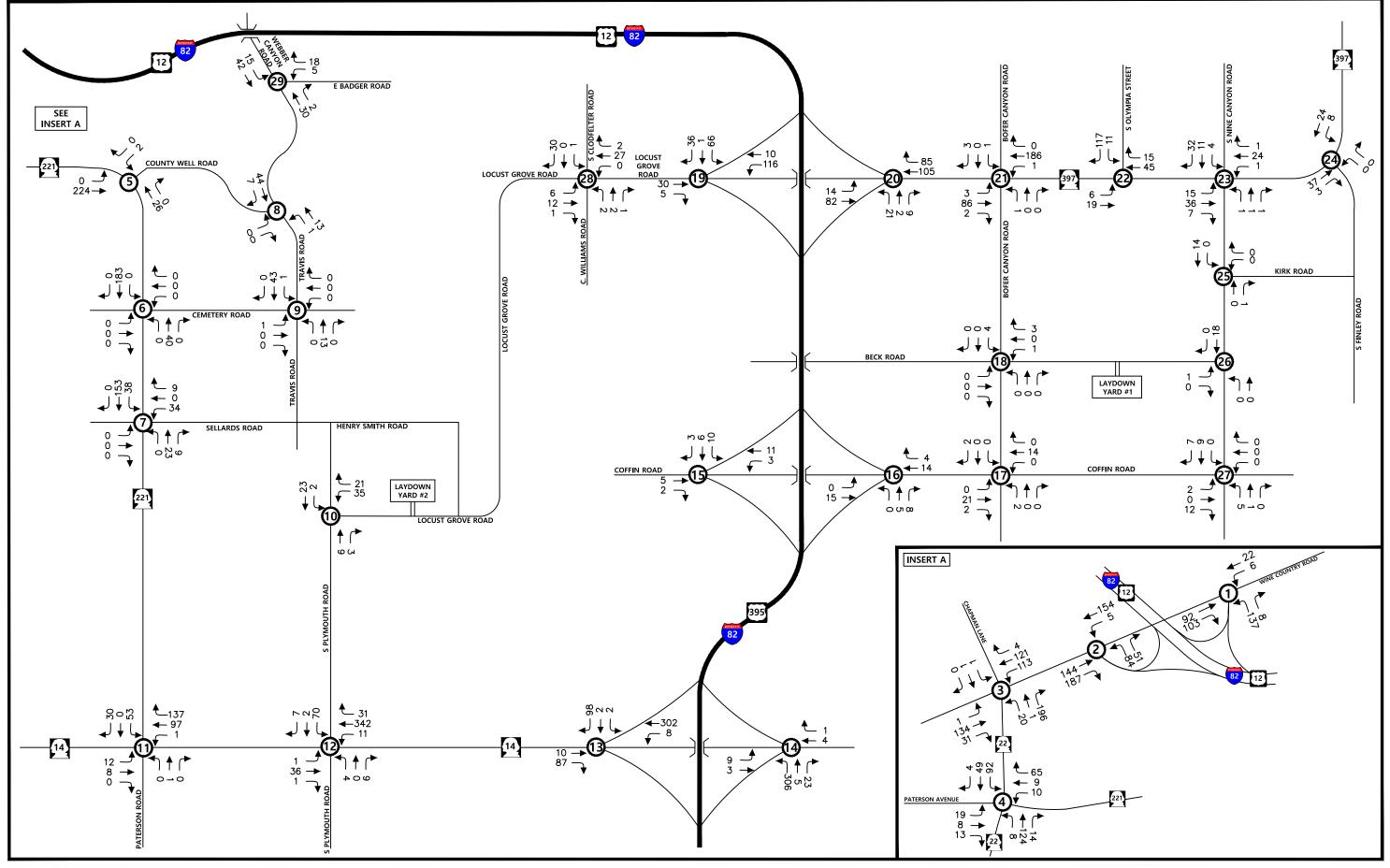






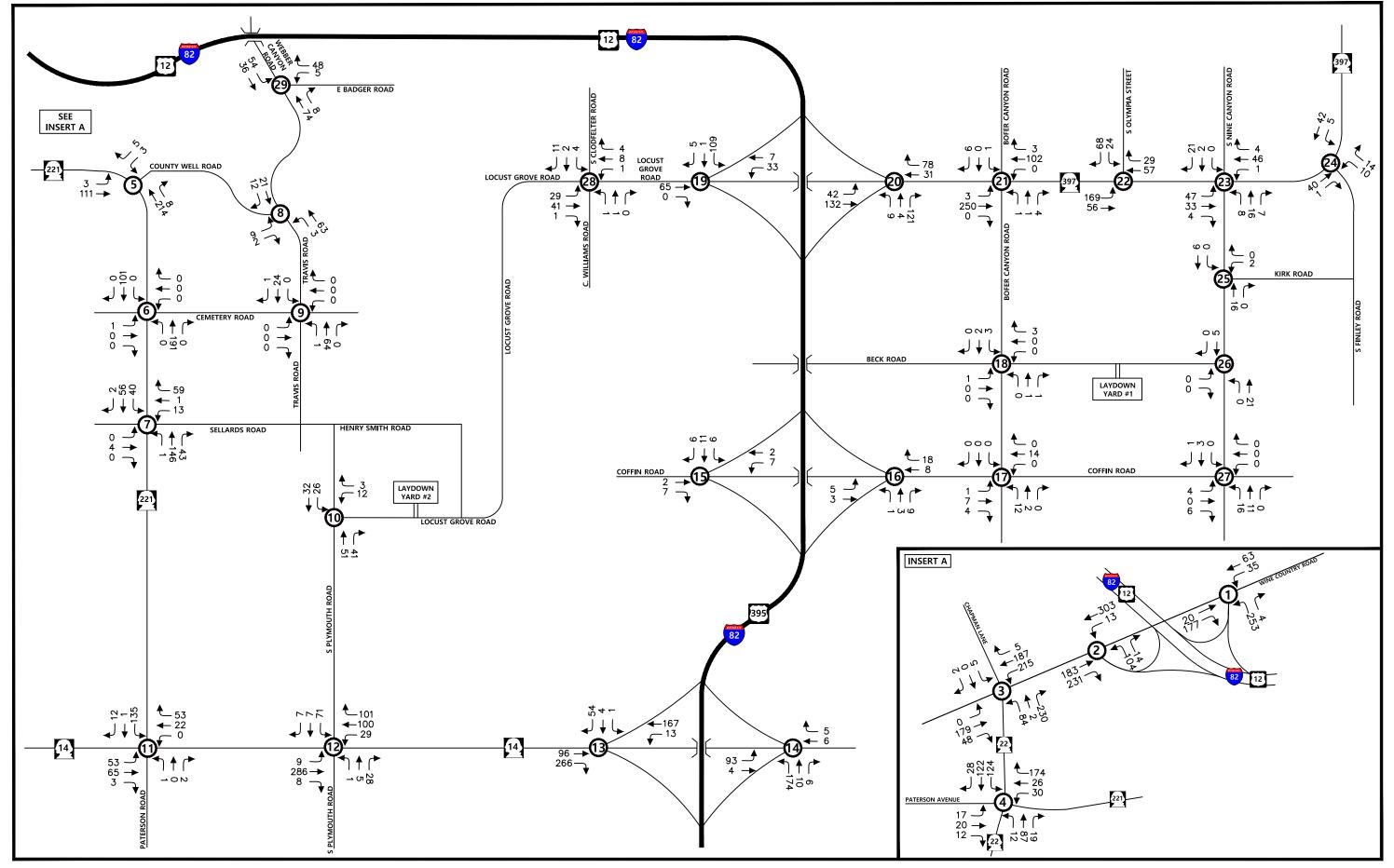


Benton County, WA



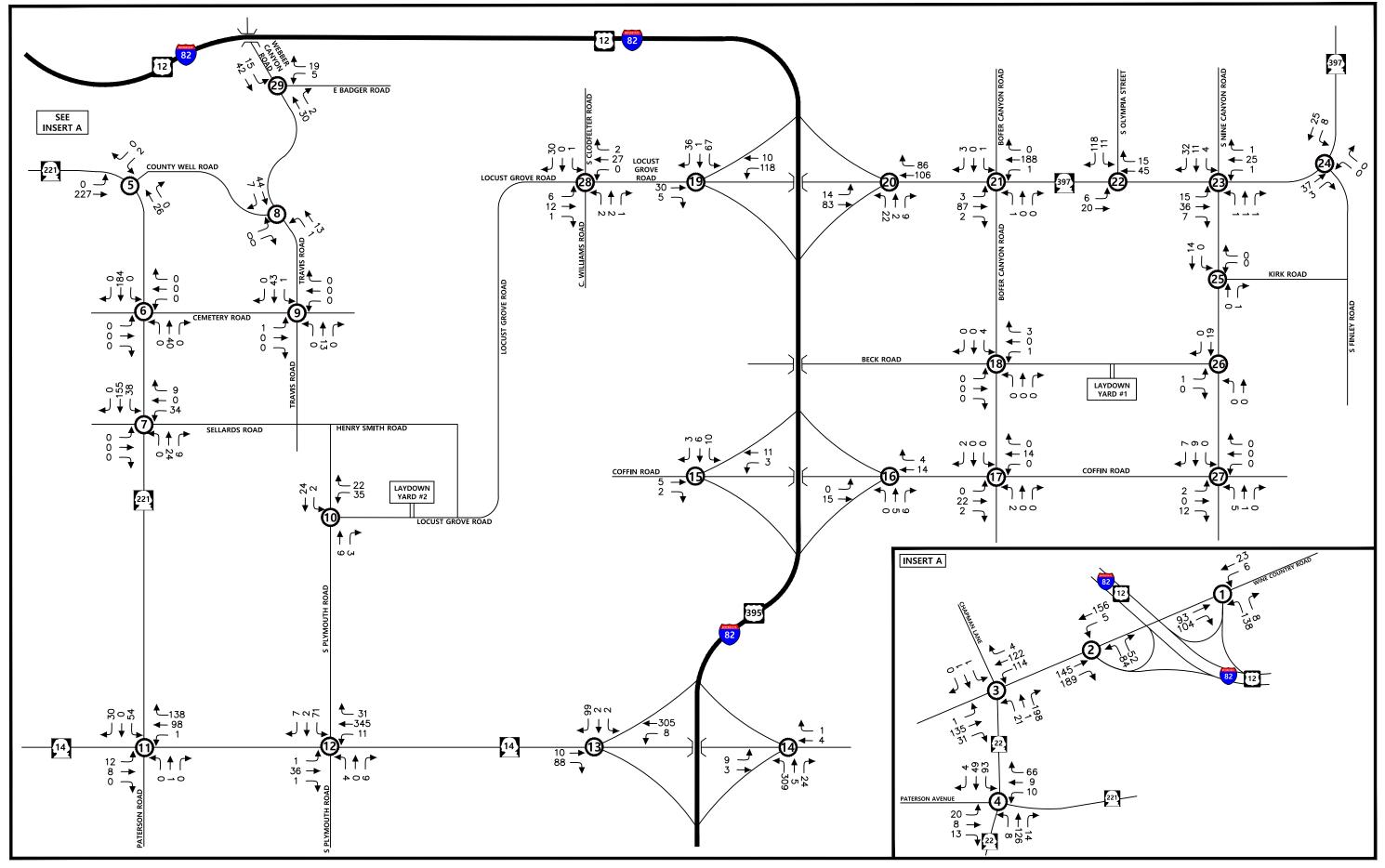






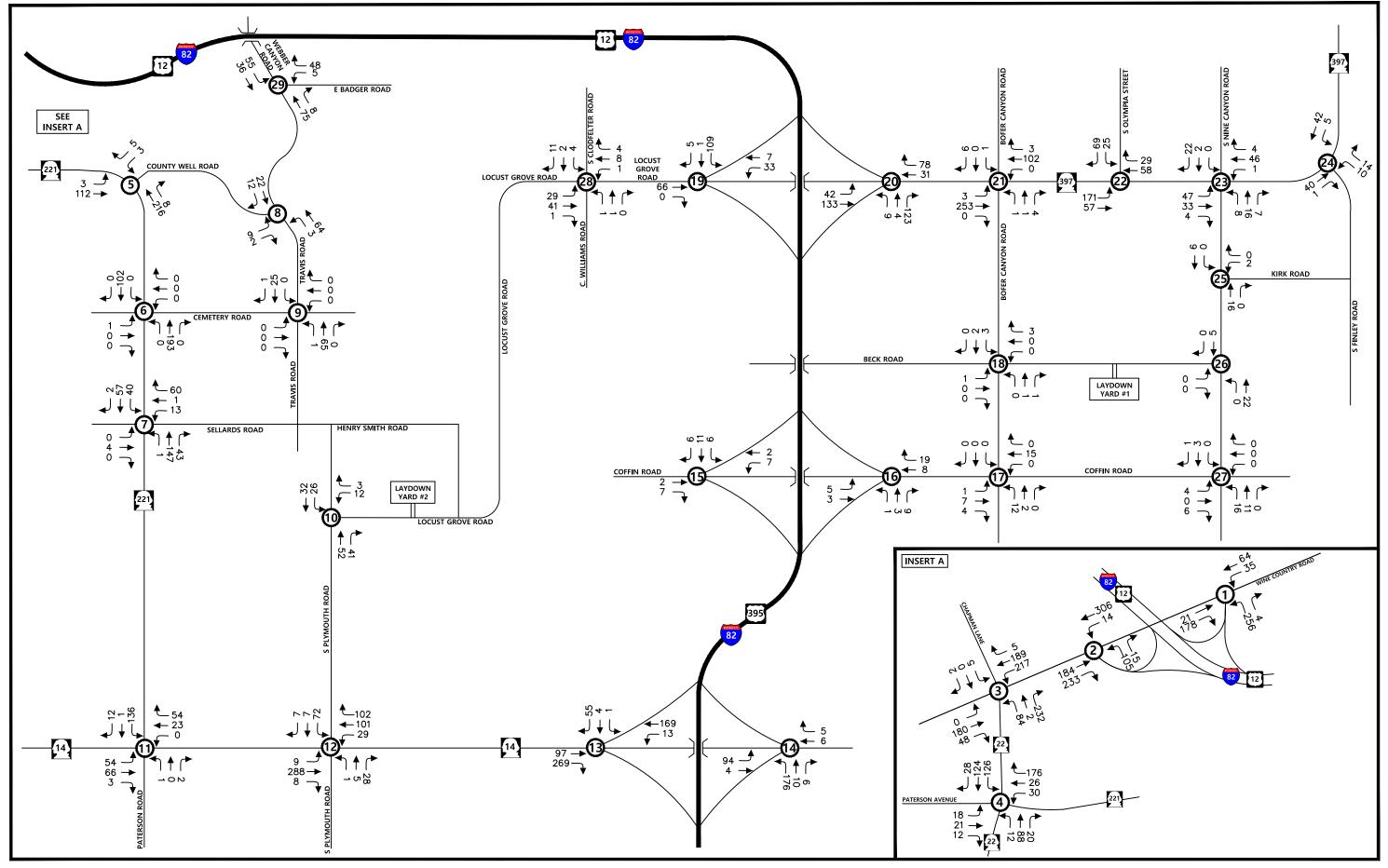






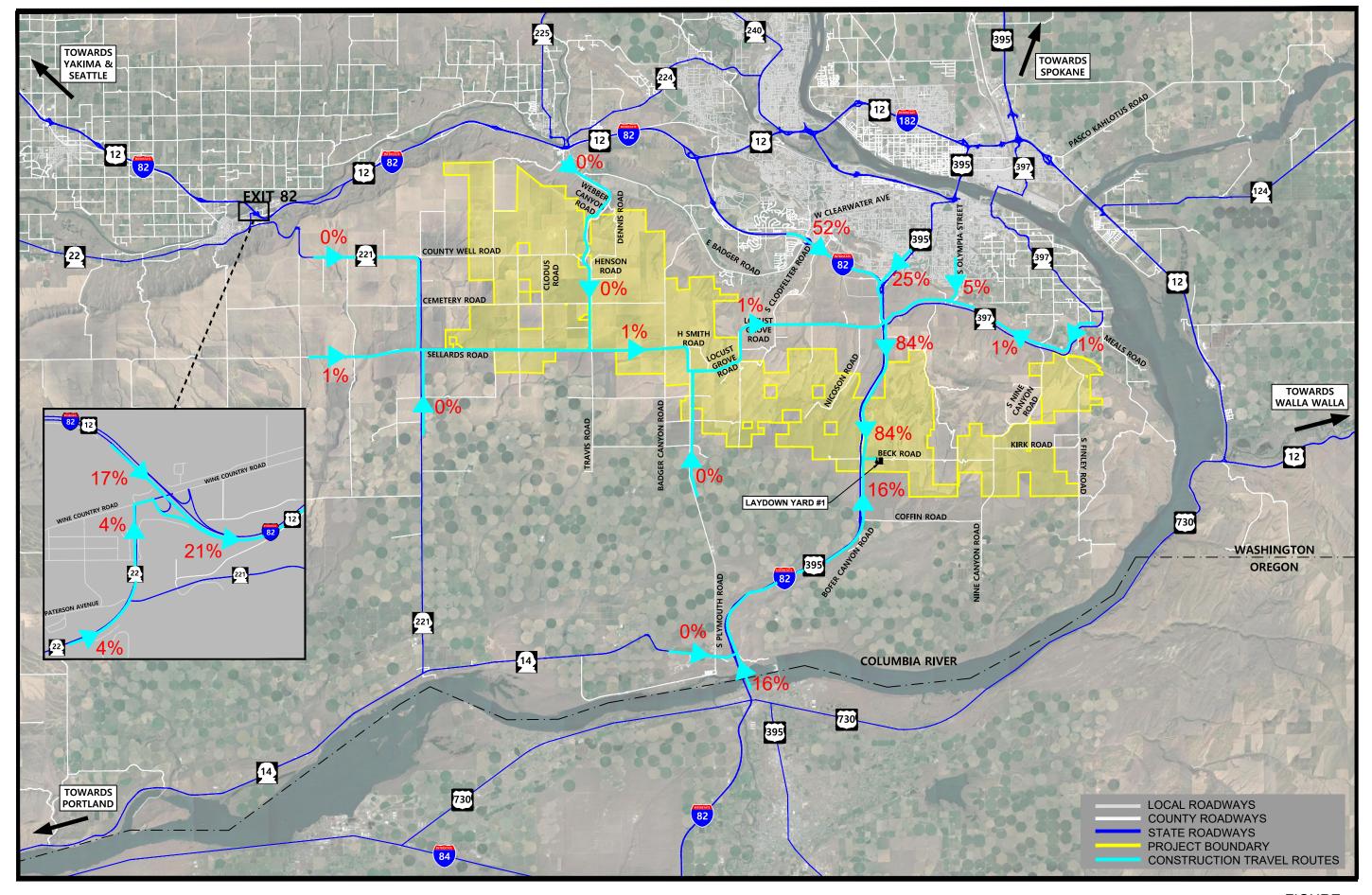




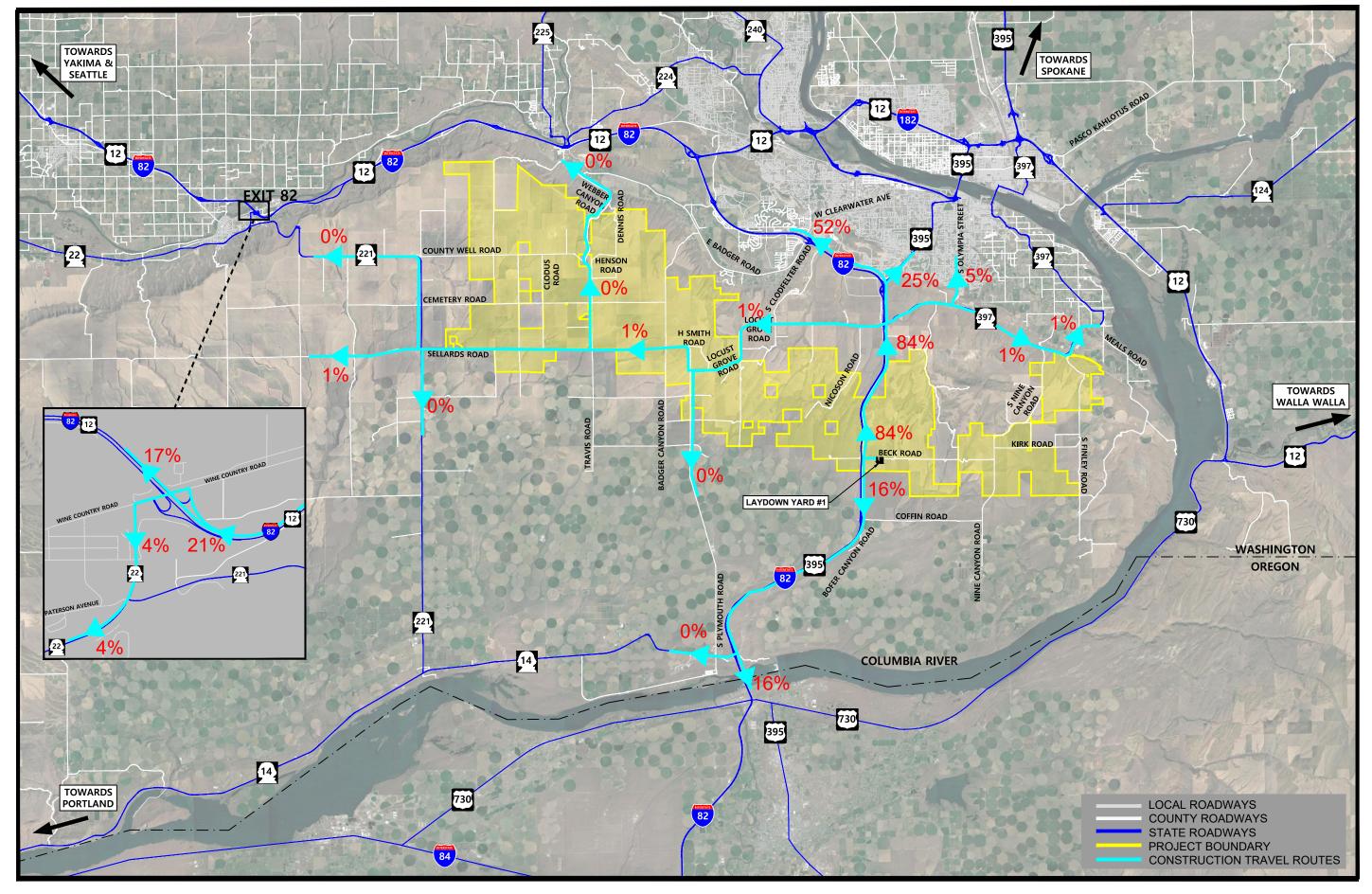




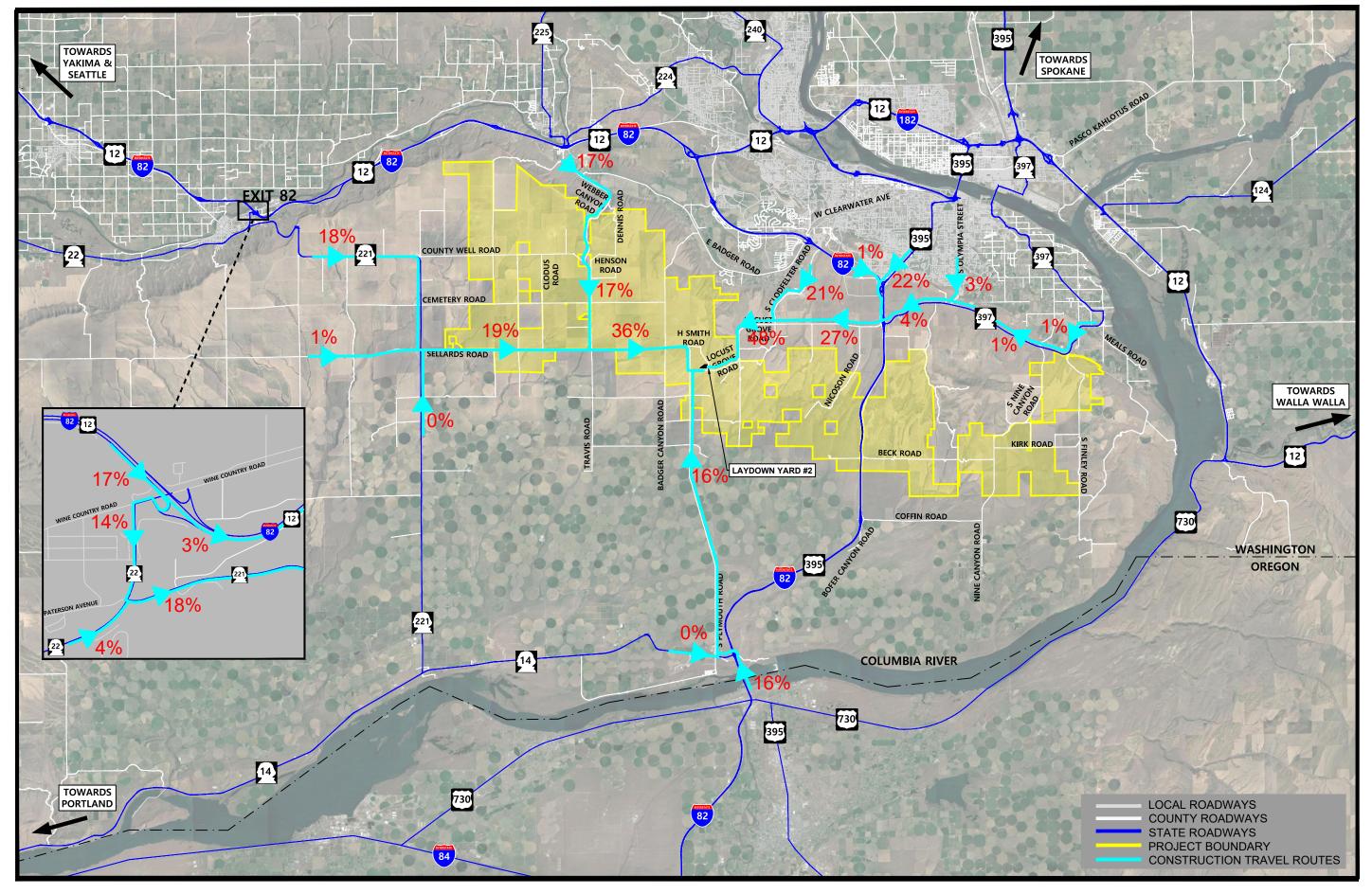




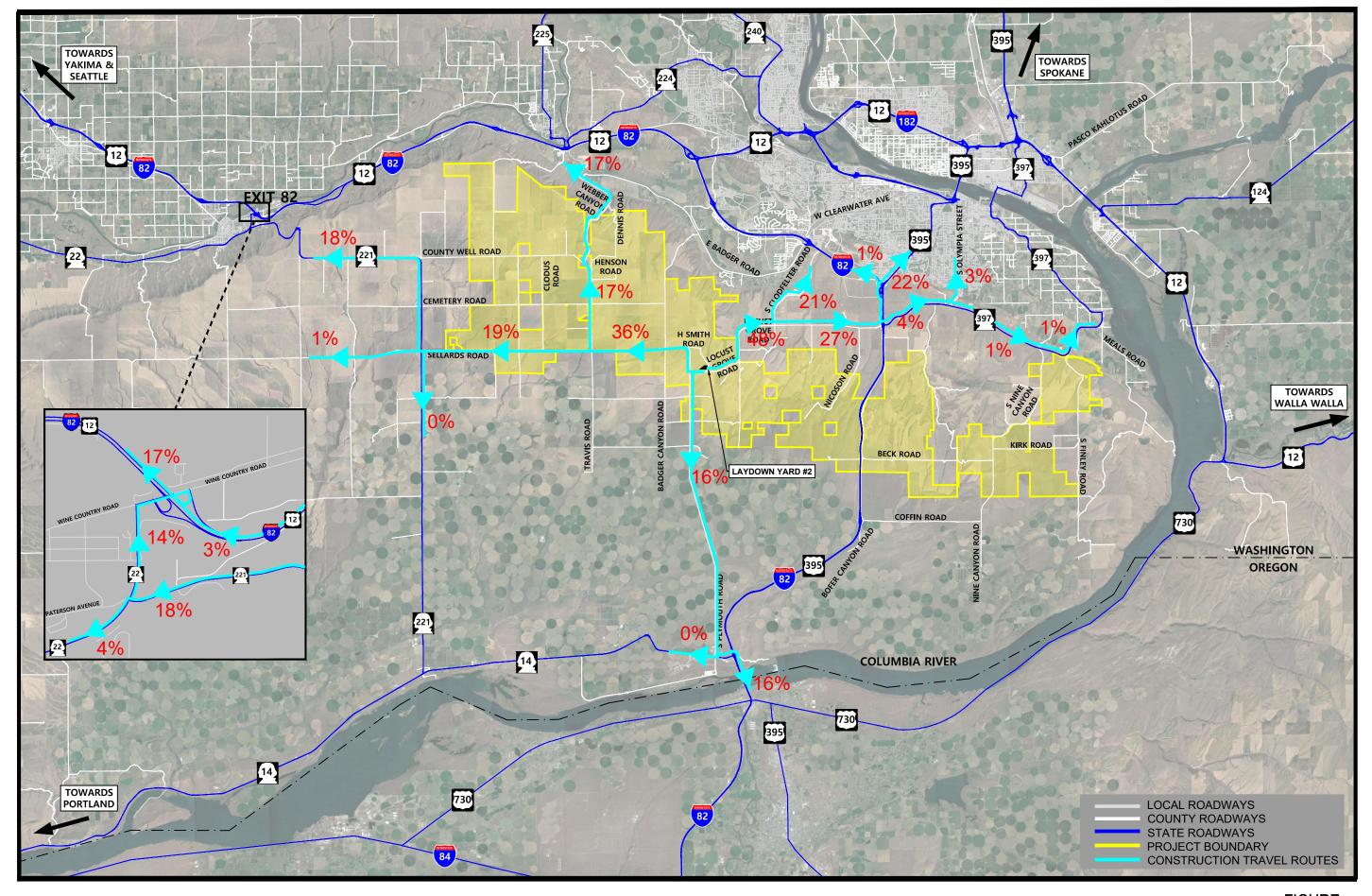




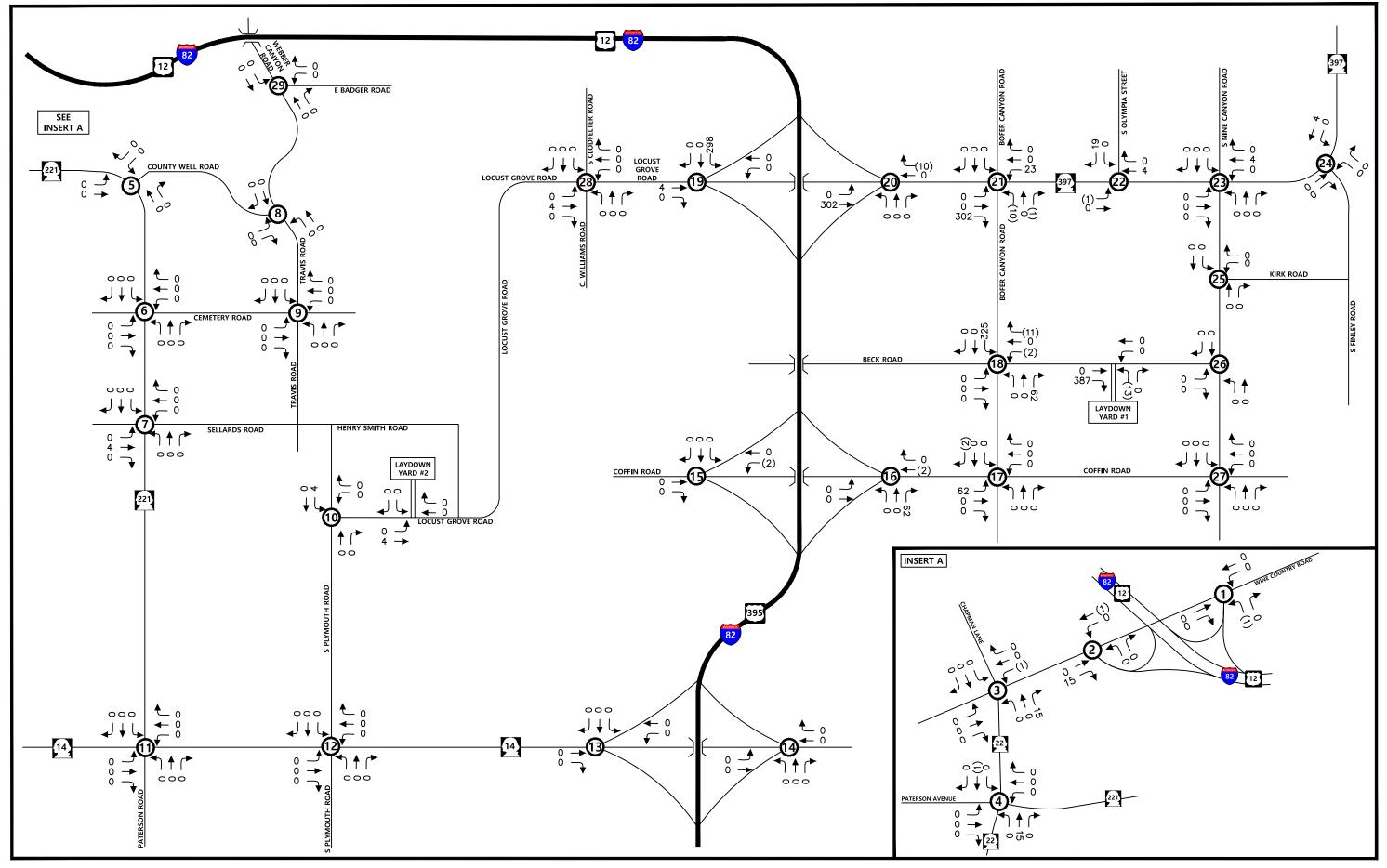






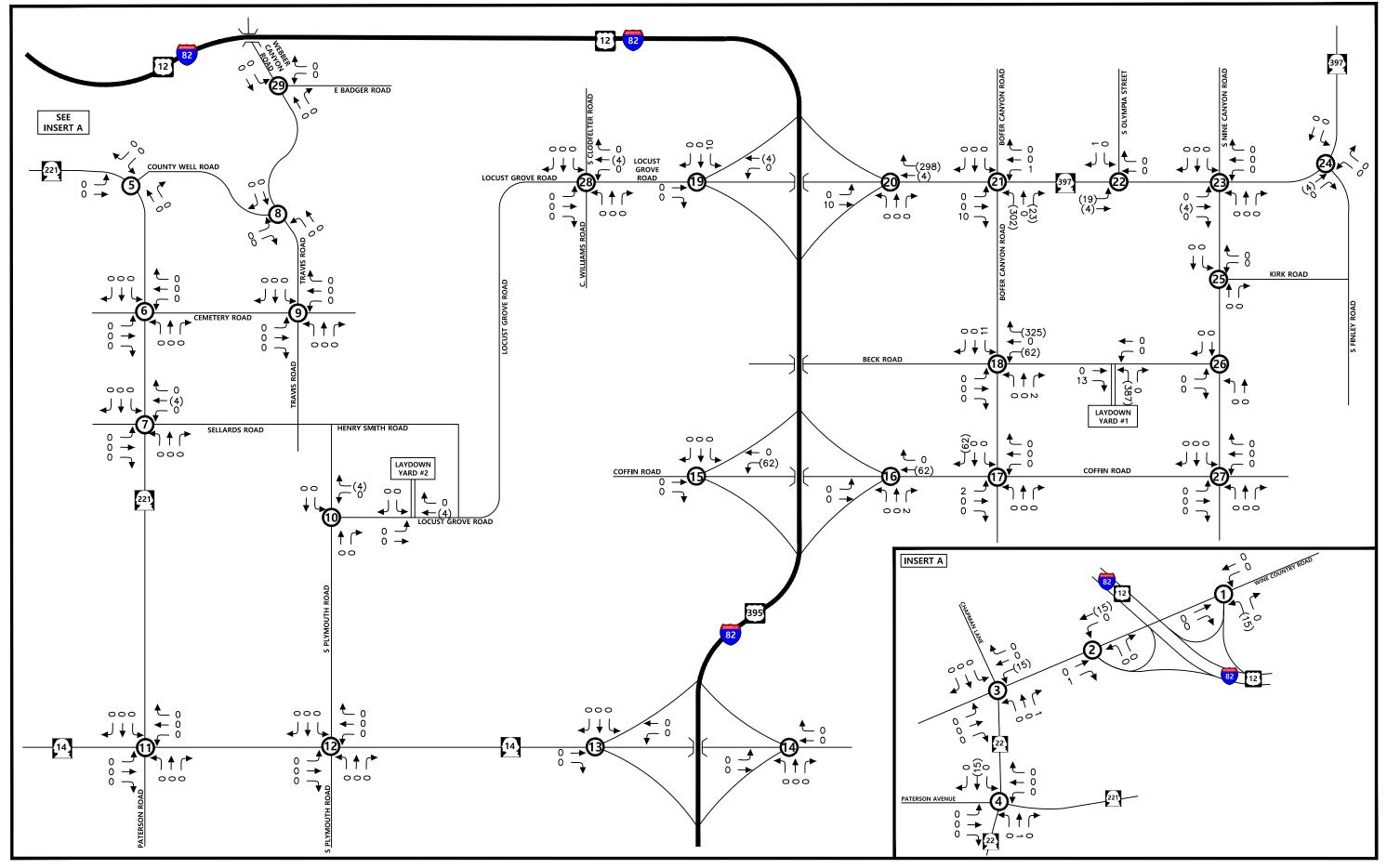






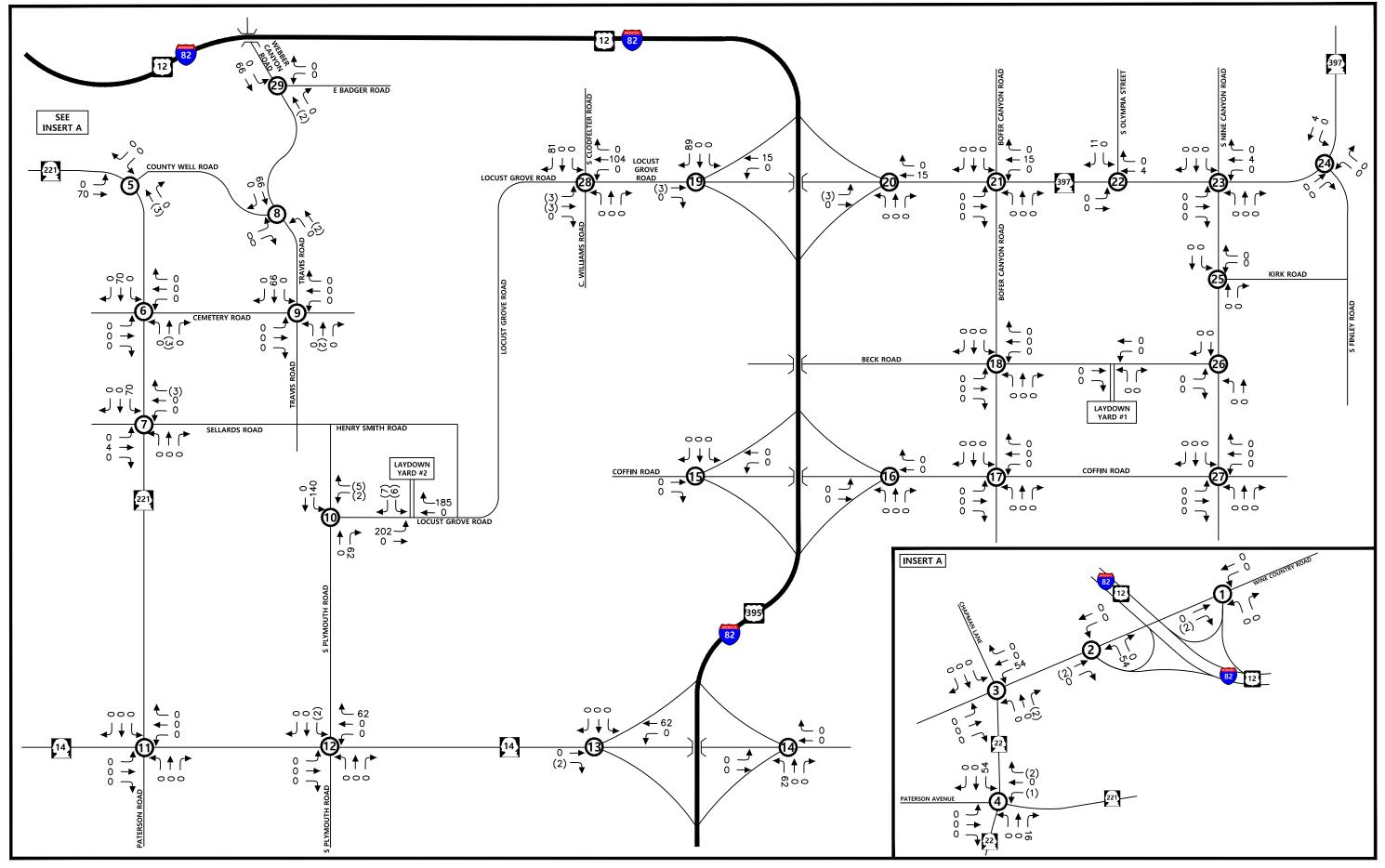




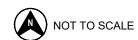


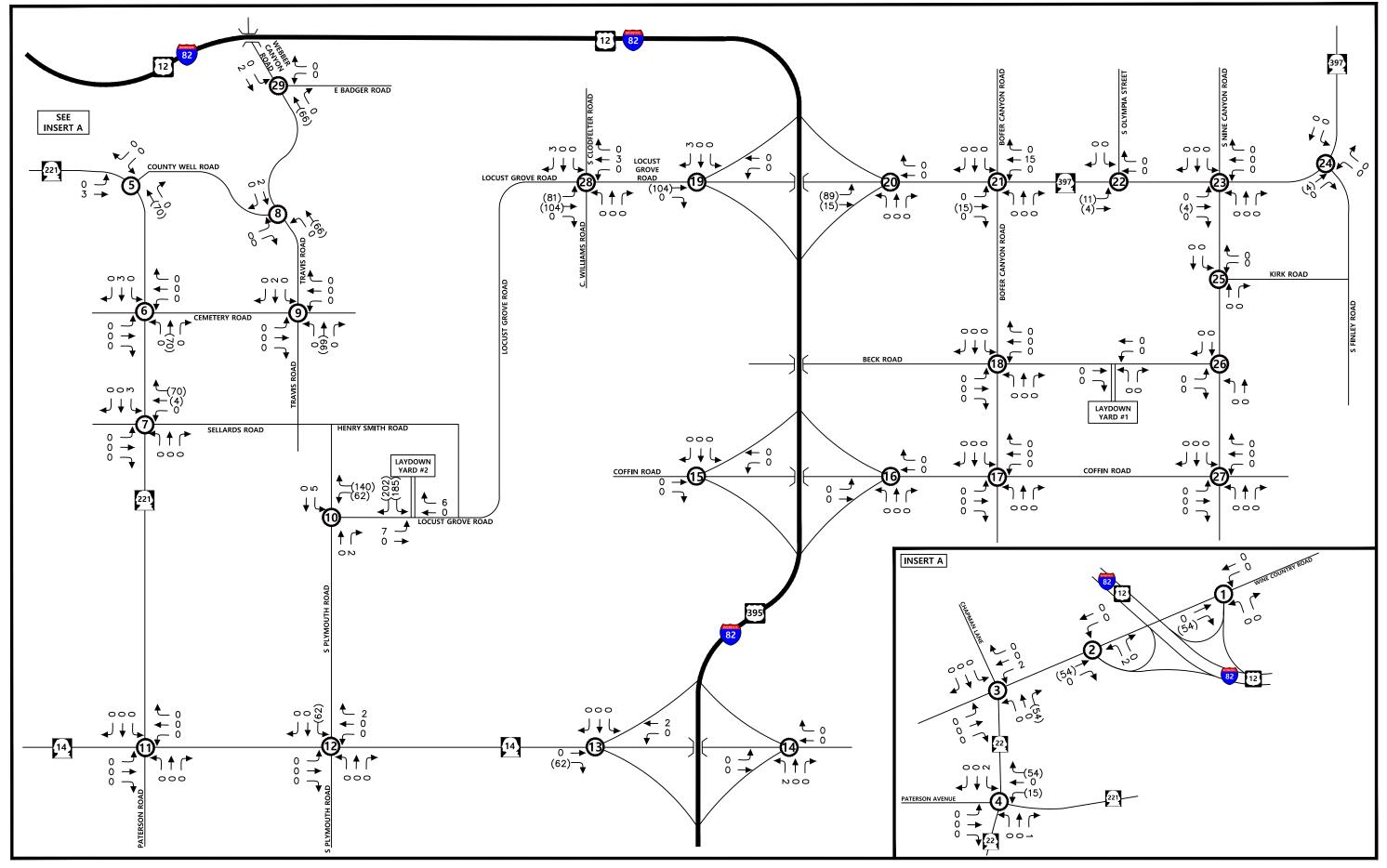






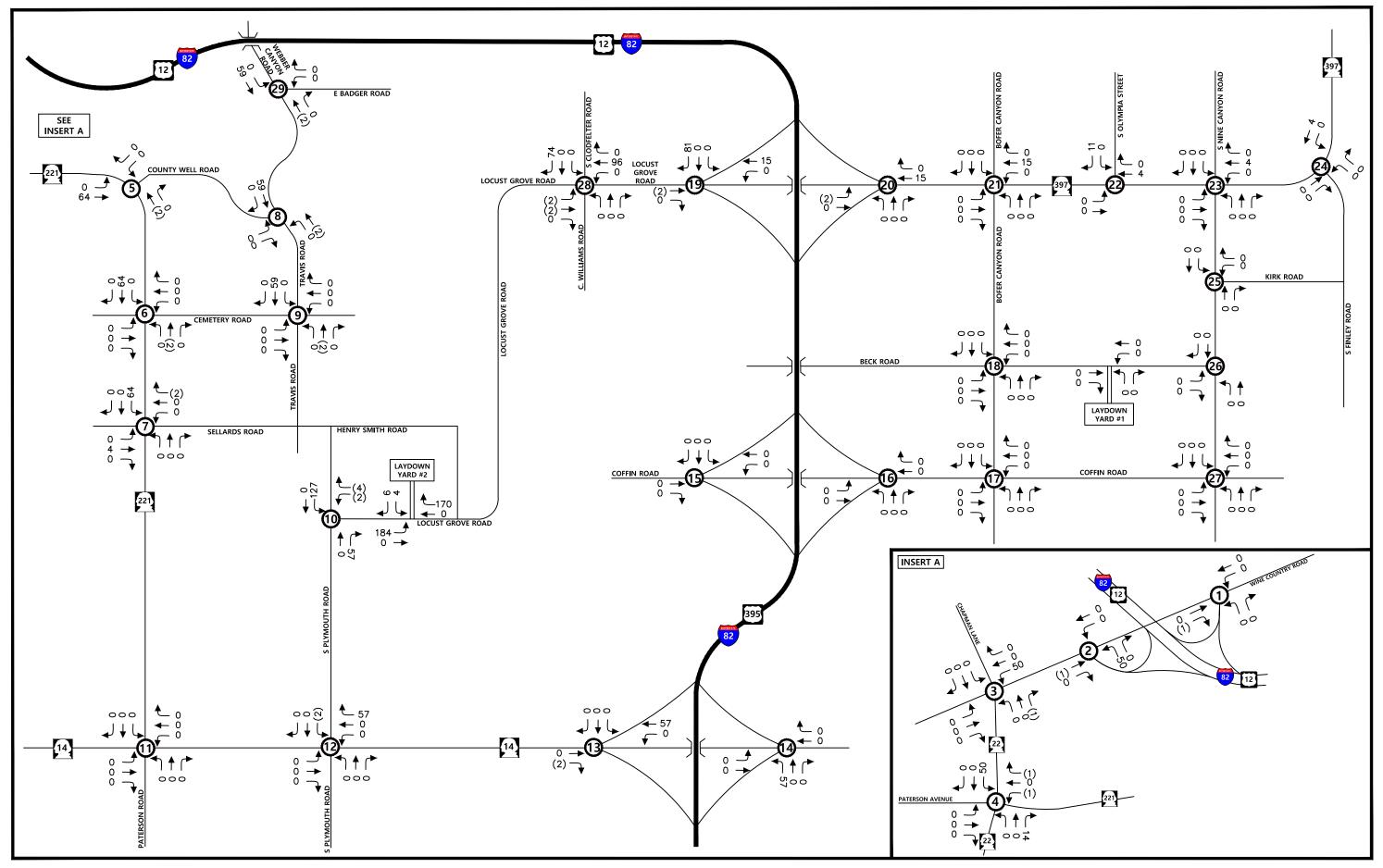




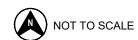


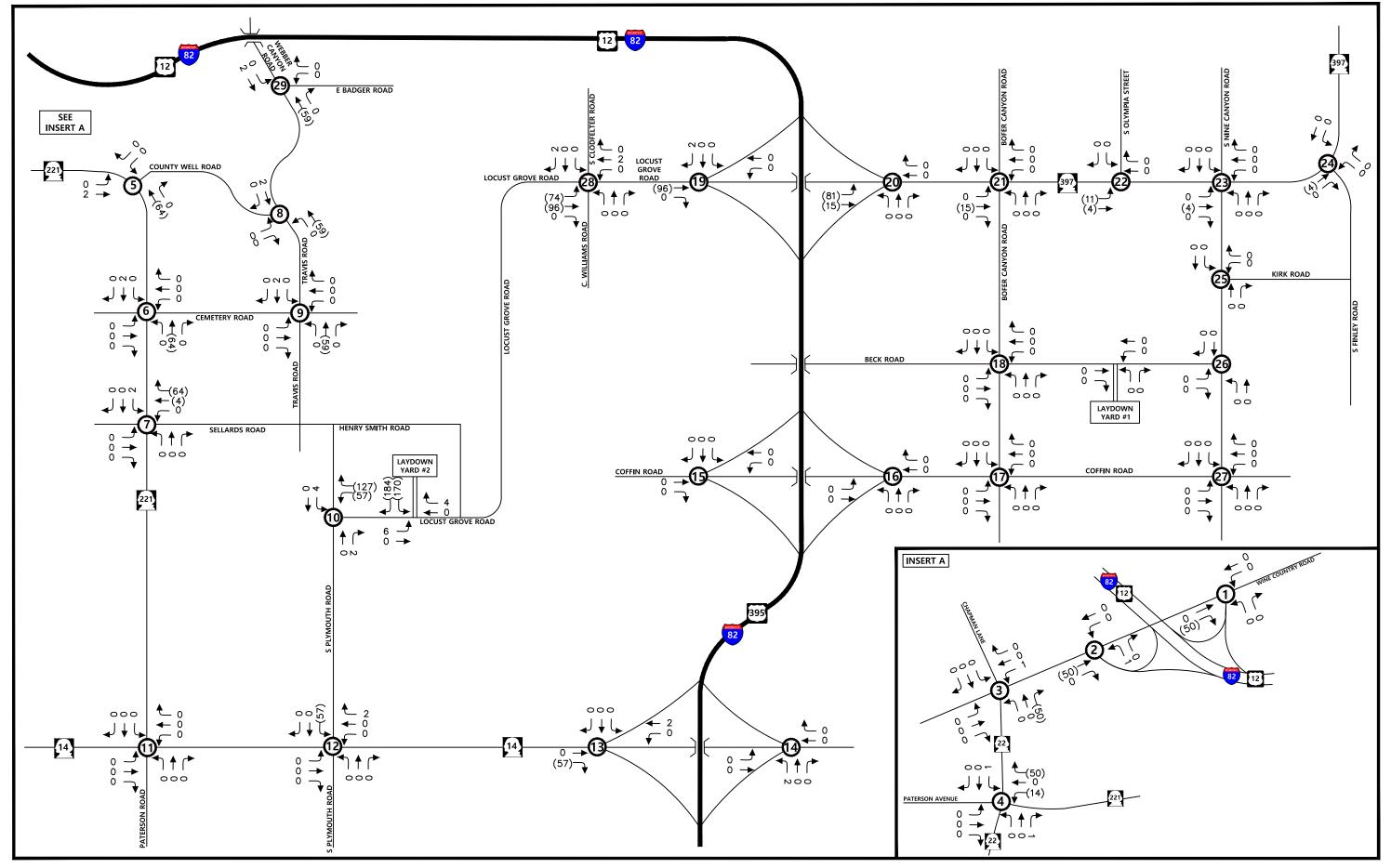




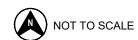


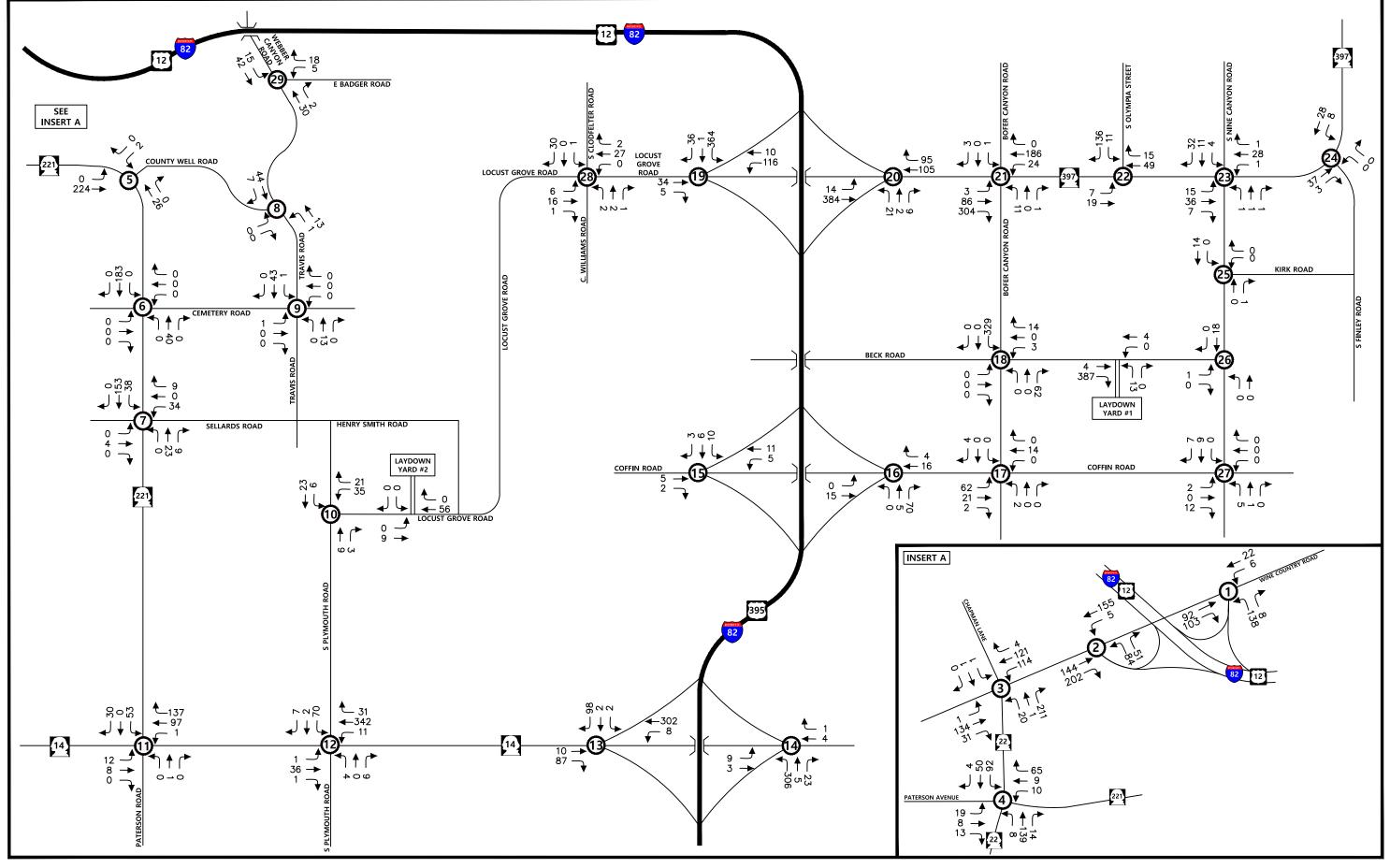




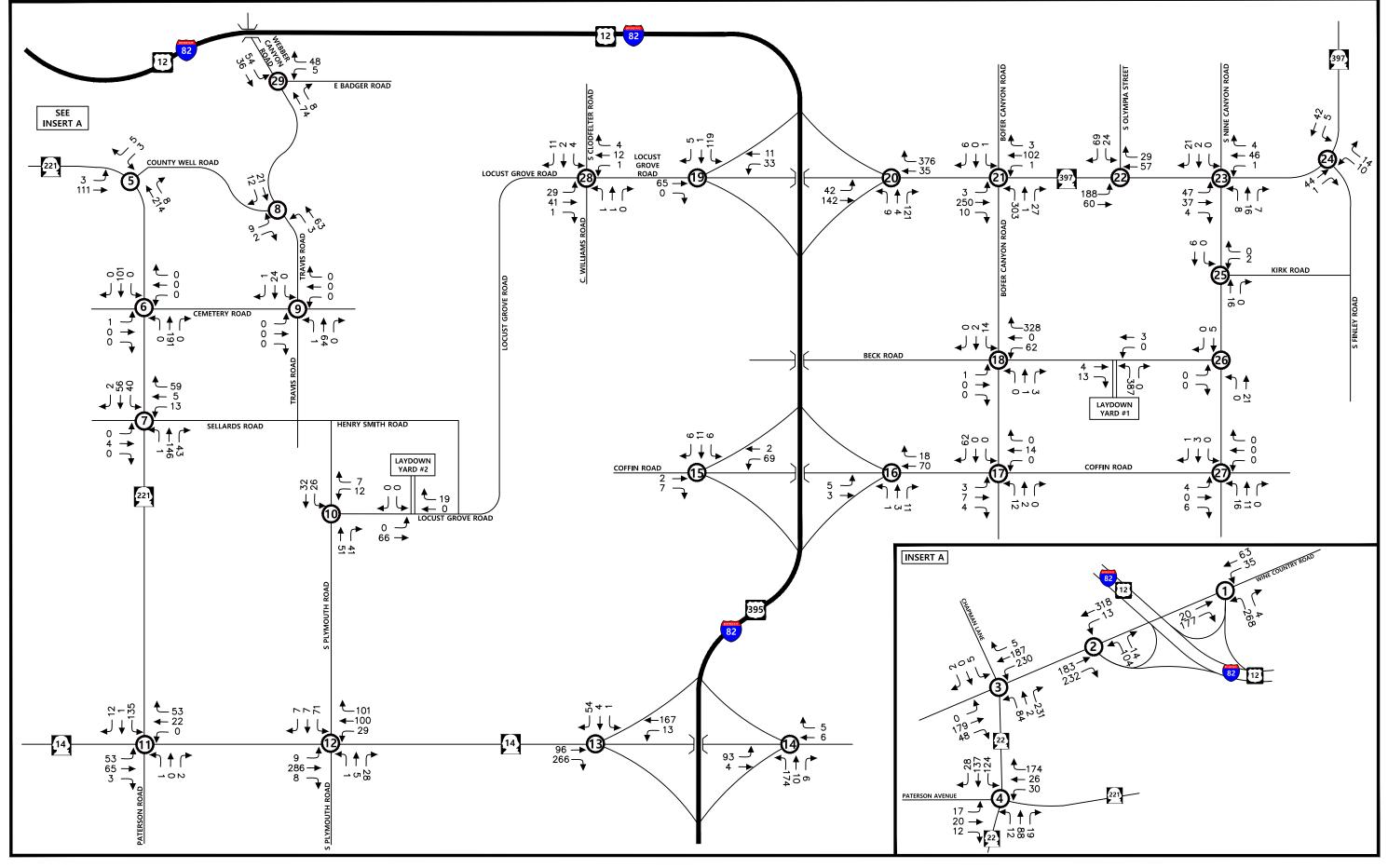




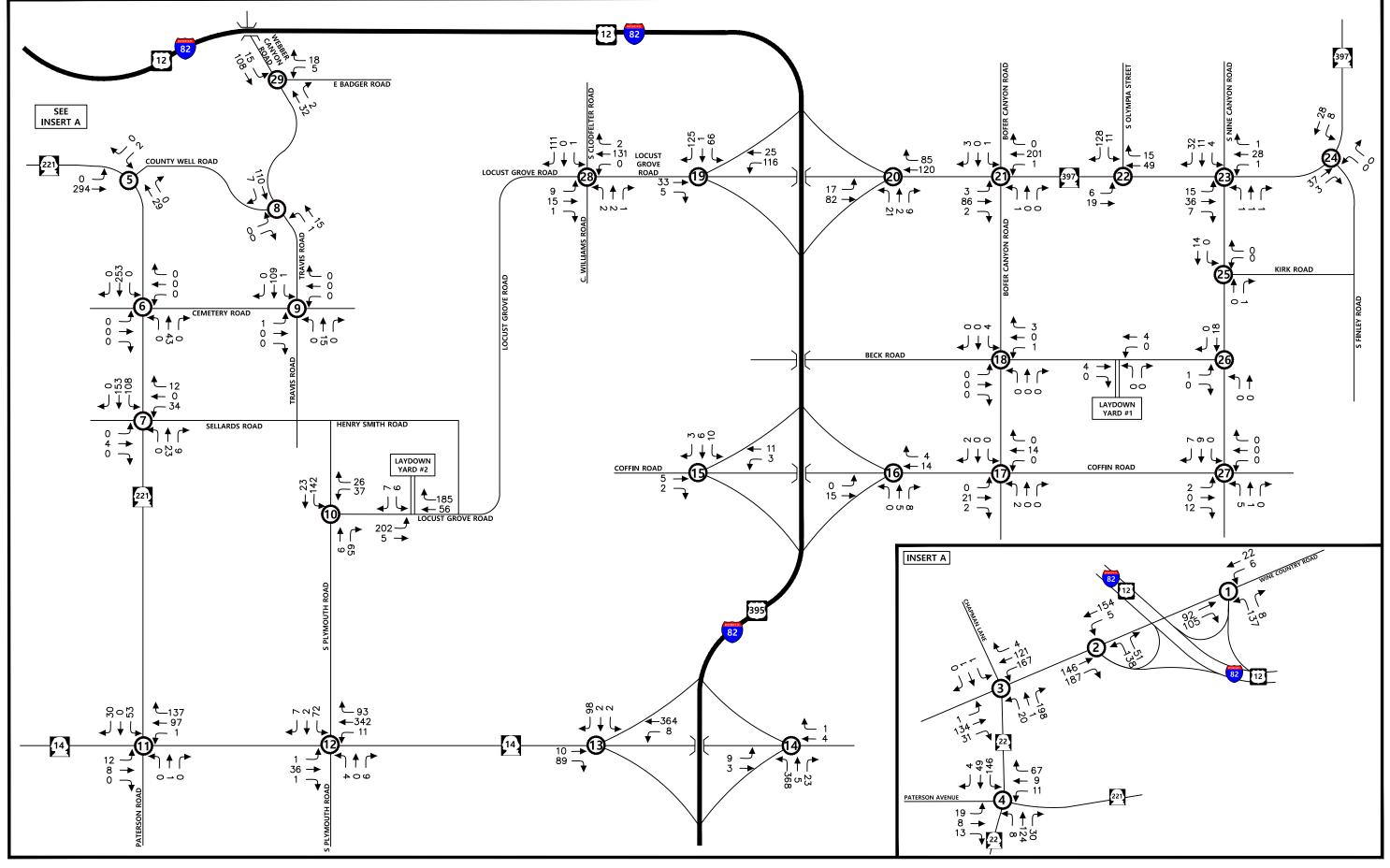




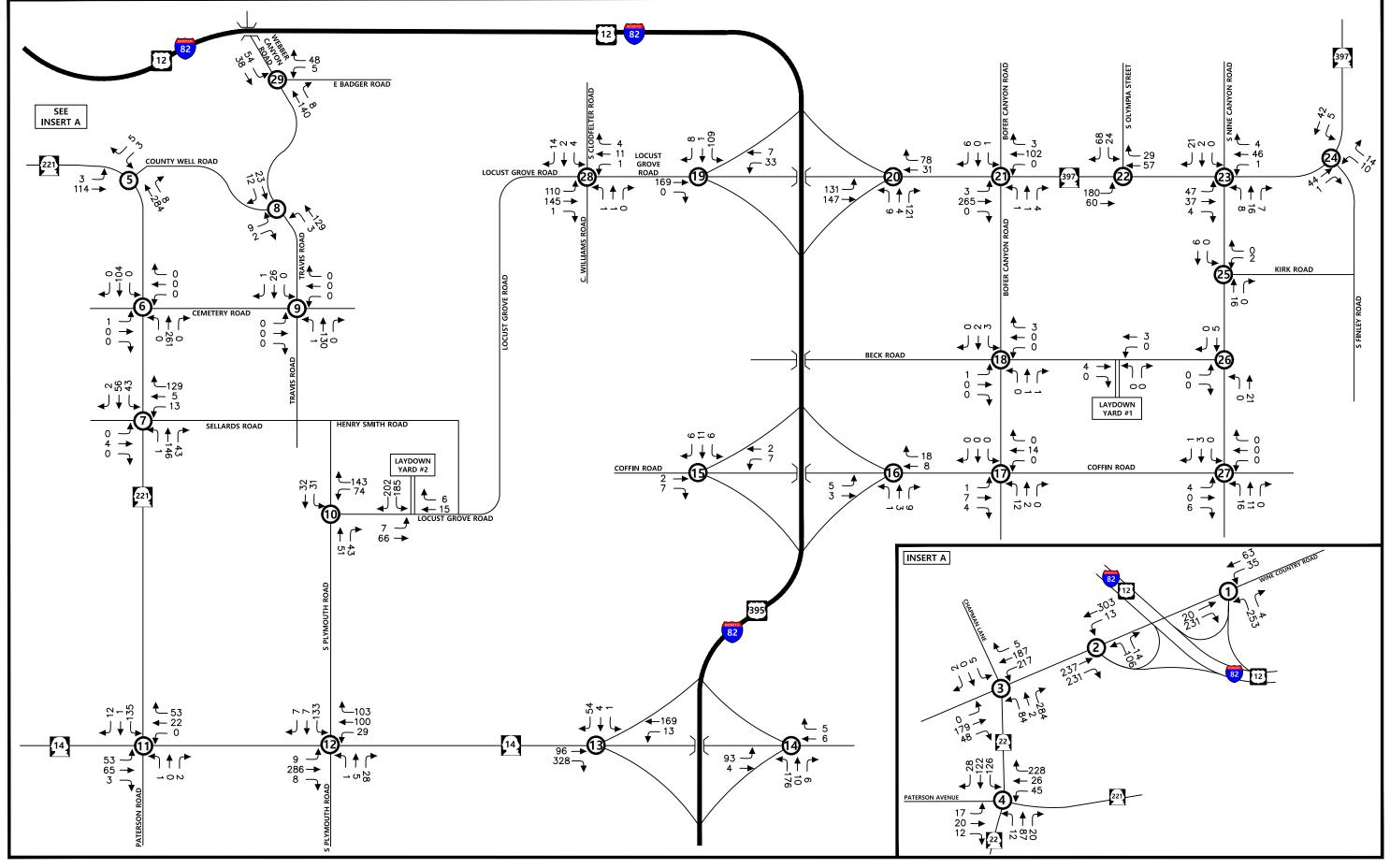


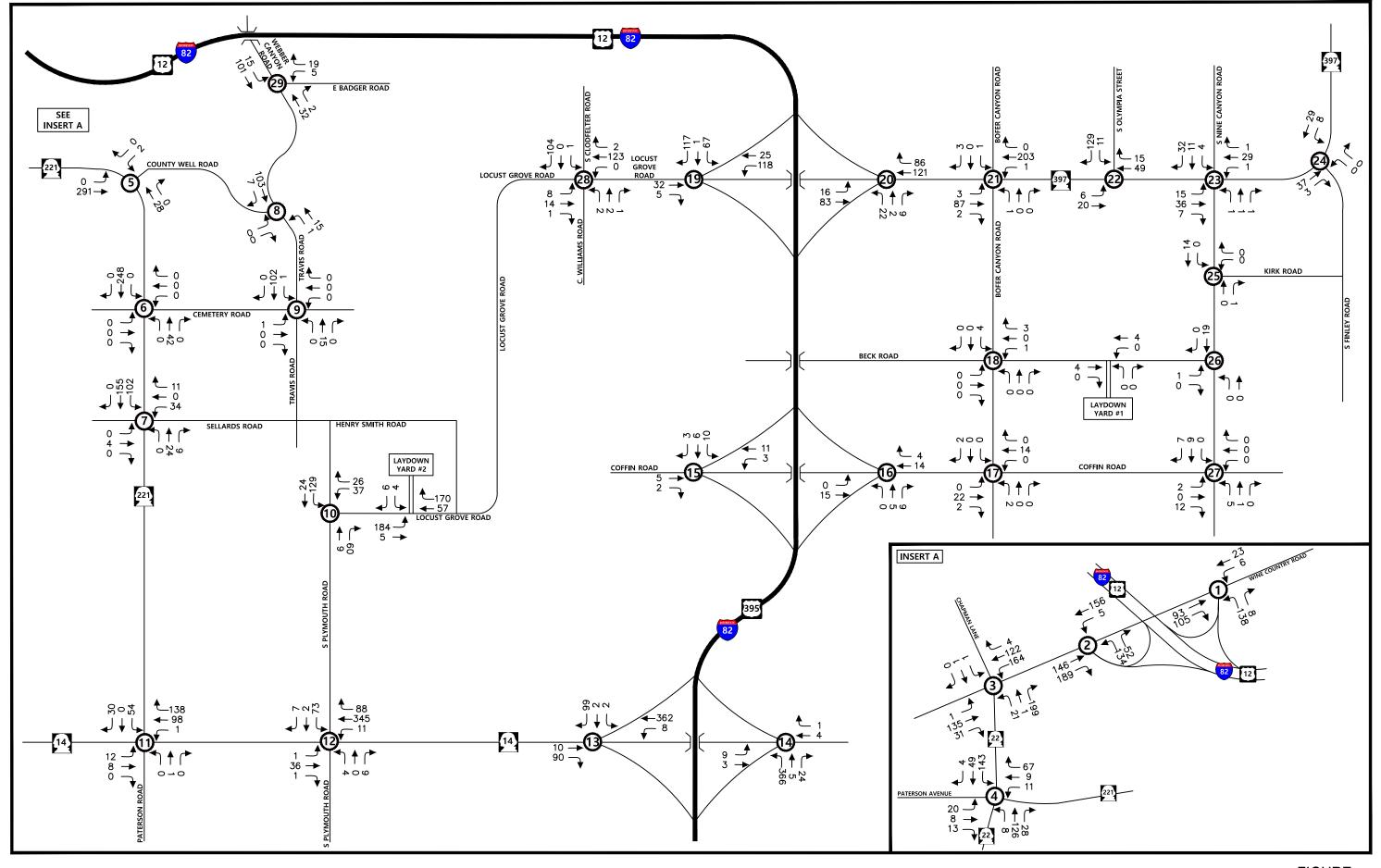




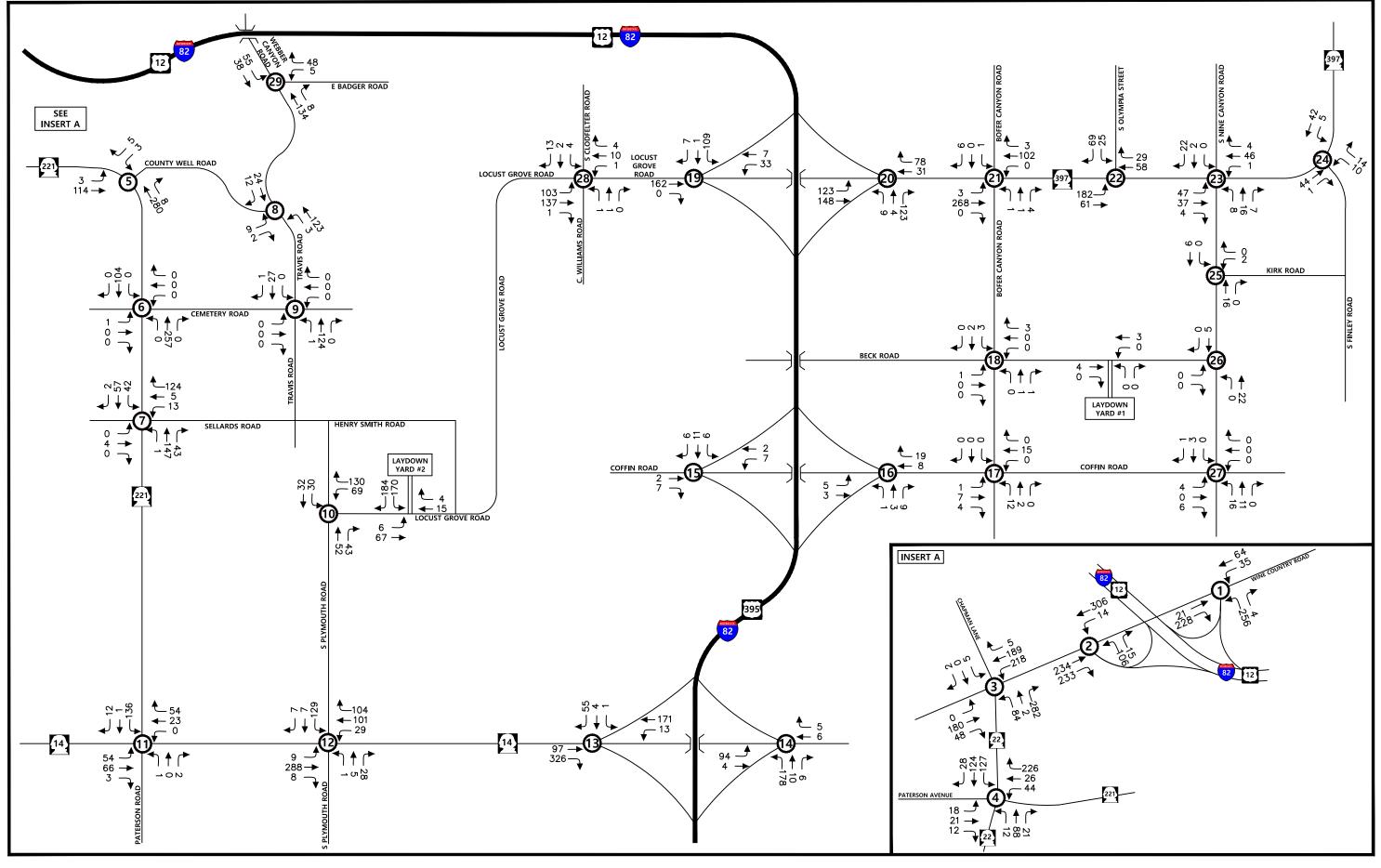




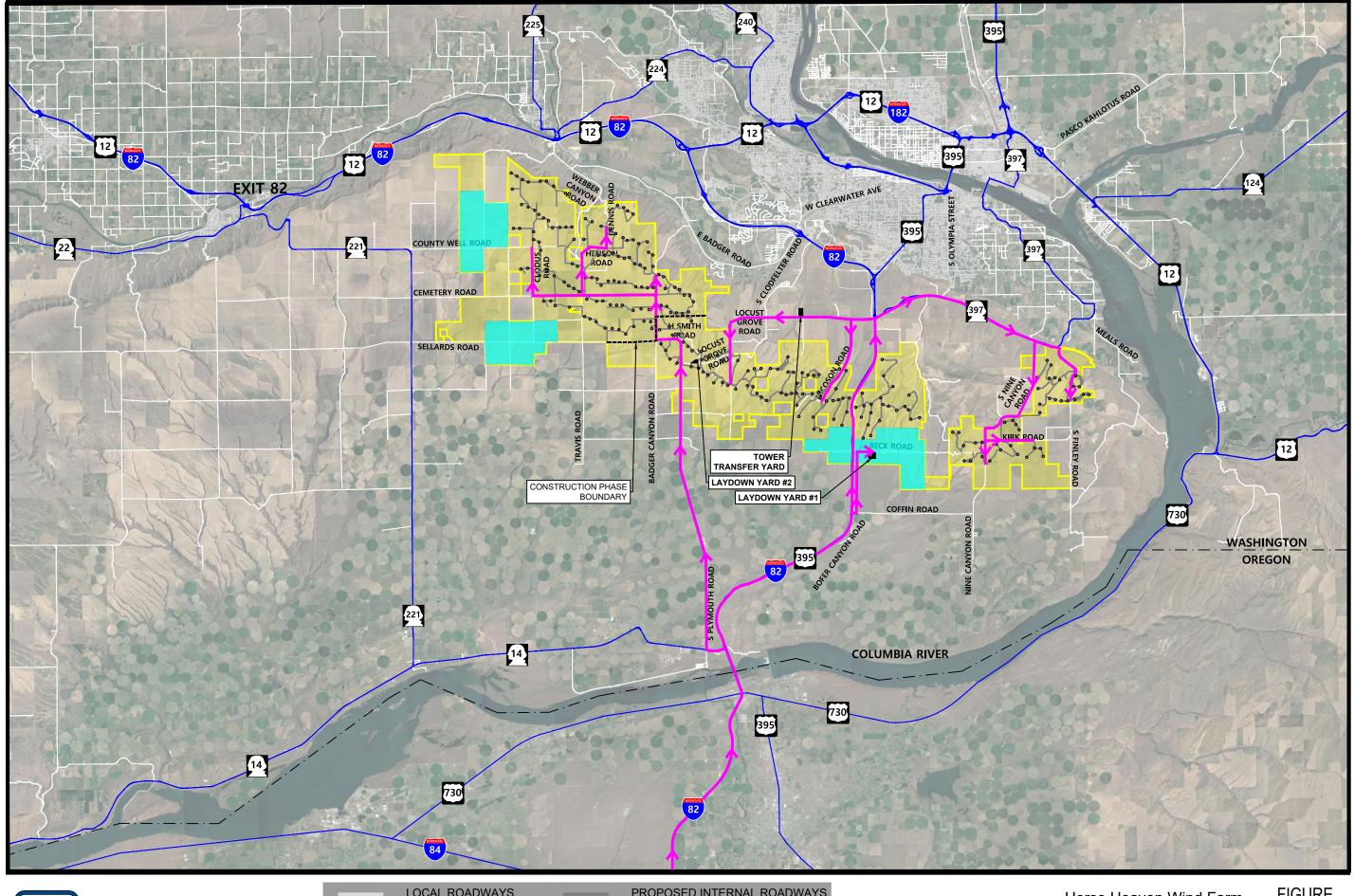








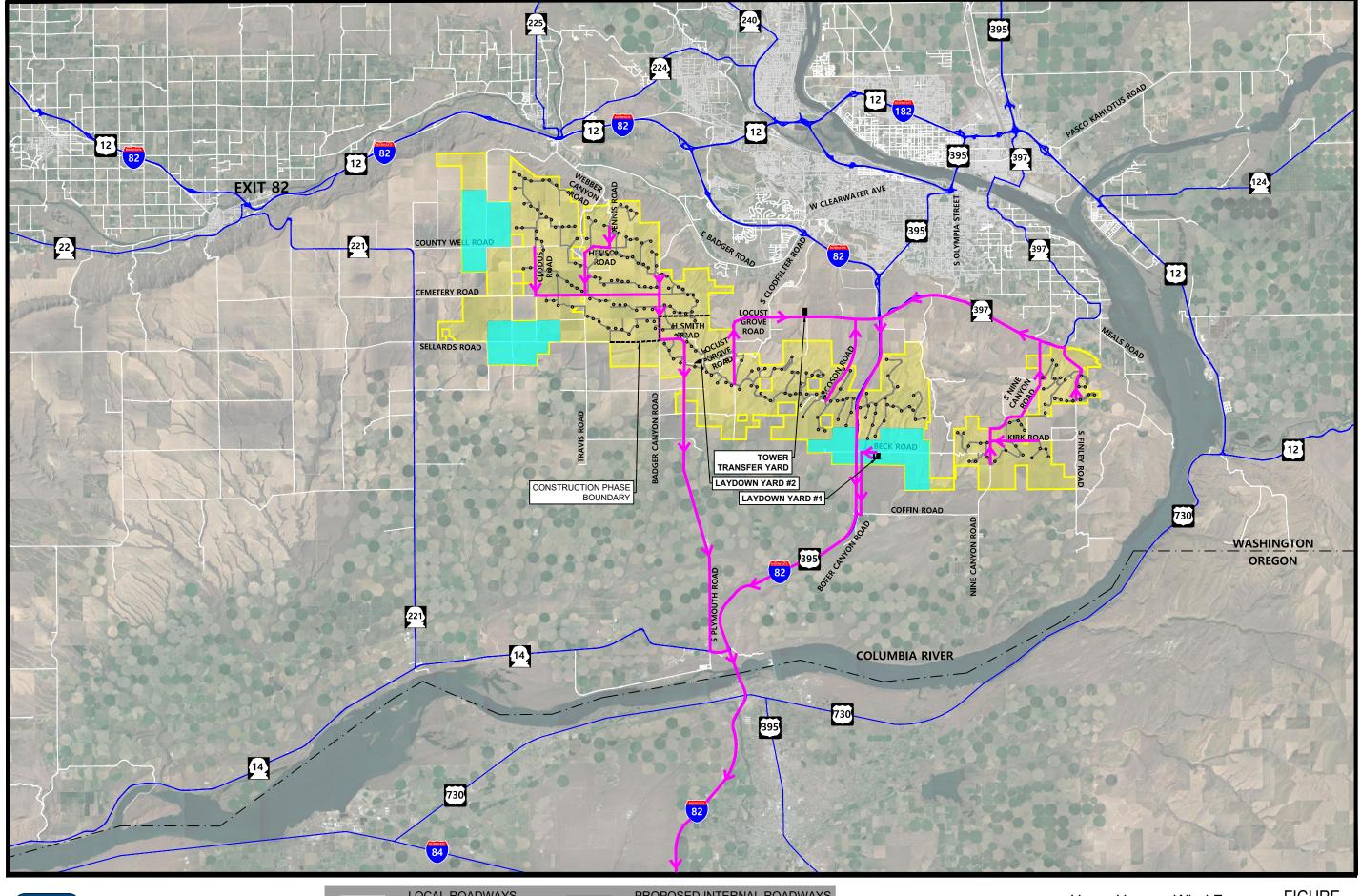






LOCAL ROADWAYS
COUNTY ROADWAYS
STATE ROADWAYS
PROJECT BOUNDARY
TRUCK ROUTE

PROPOSED INTERNAL ROADWAYS SOLAR FIELDS WIND TURBINES

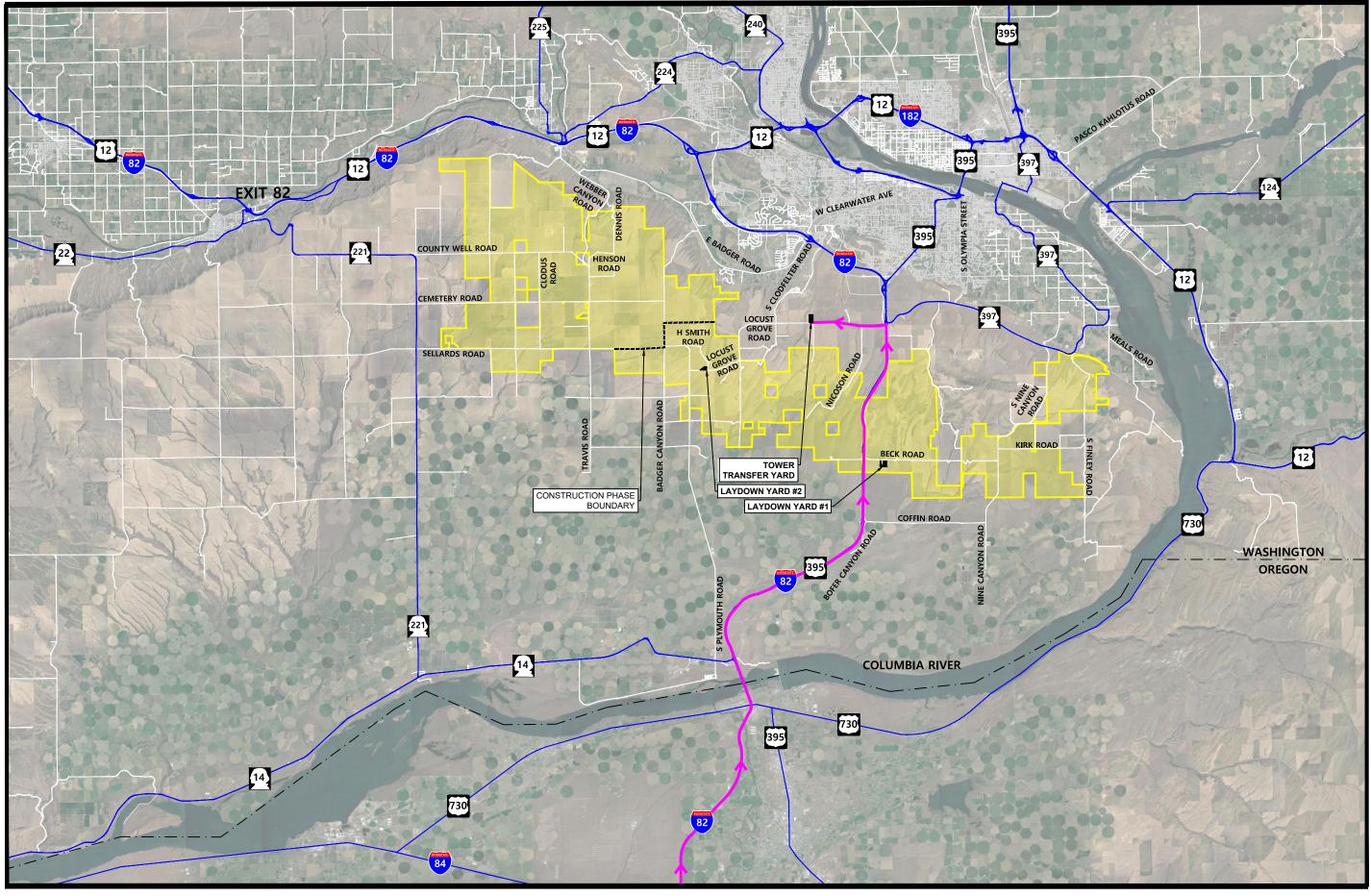




STATE ROADWAYS PROJECT BOUNDARY TRUCK ROUTE

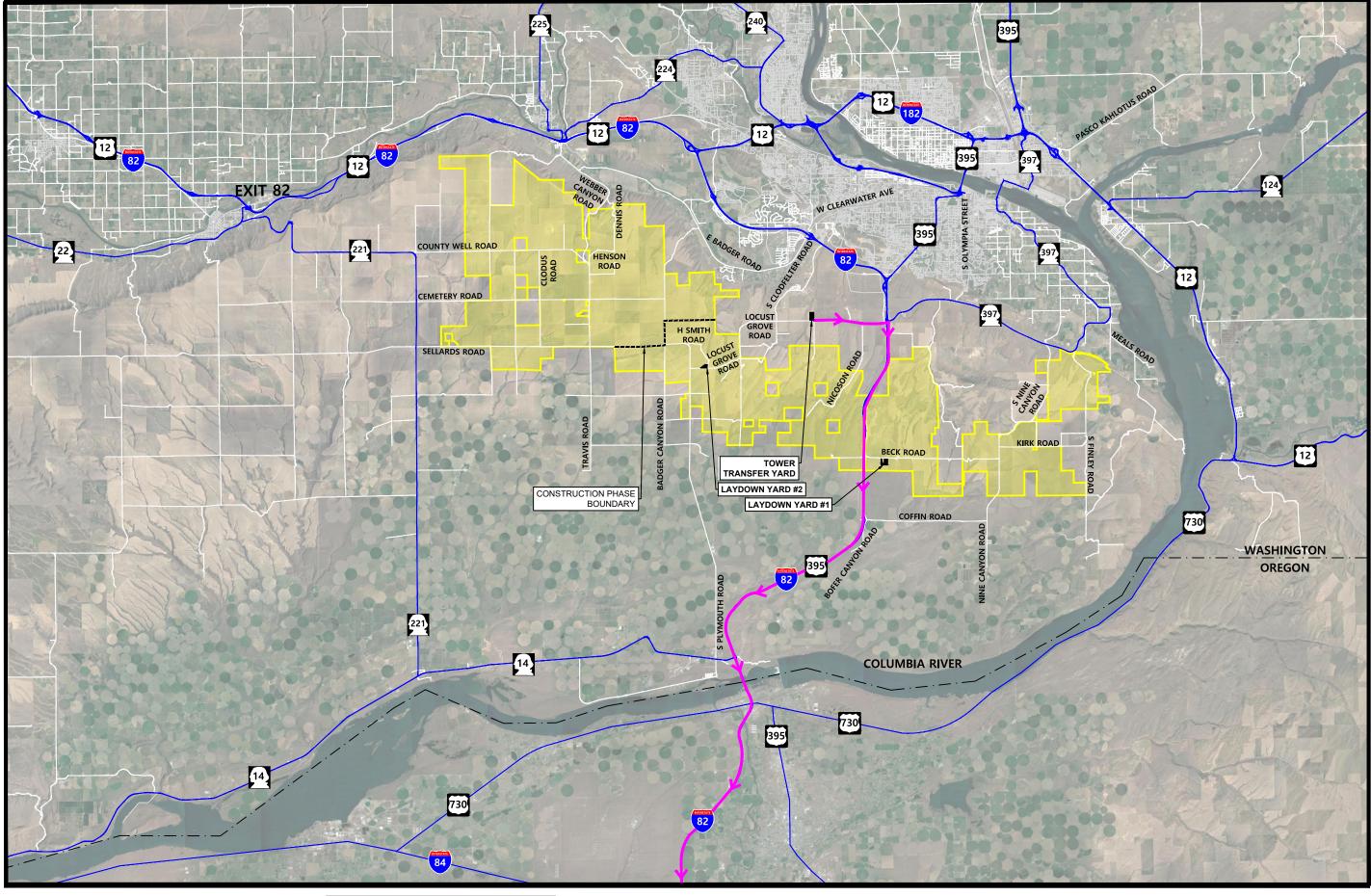
SOLAR FIELDS WIND TURBINES

Horse Heaven Wind Farm Benton County, WA **EXITING TRUCK ROUTES - OVERSIZED LOADS**  **FIGURE** 

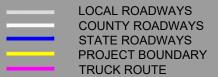


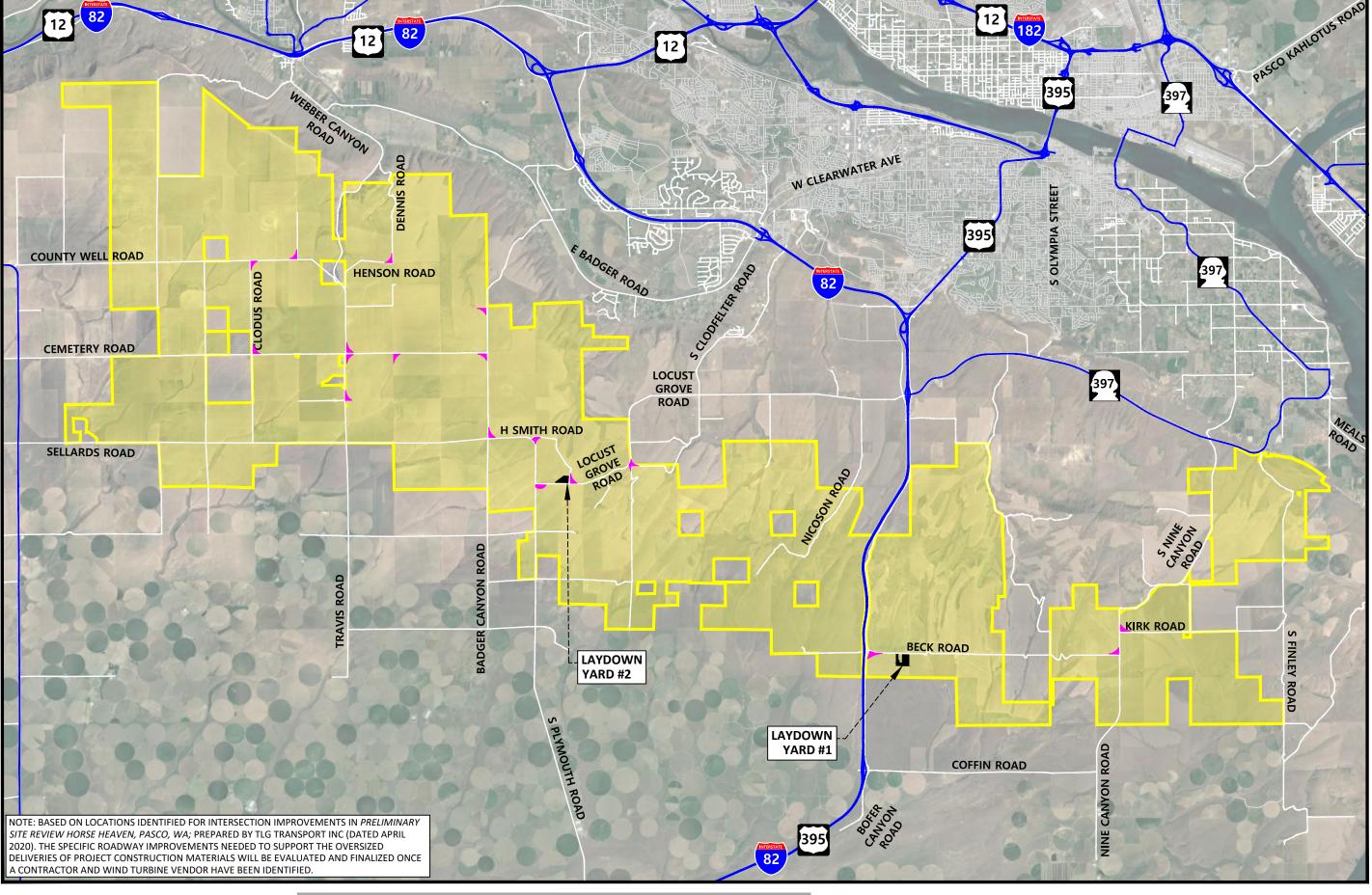






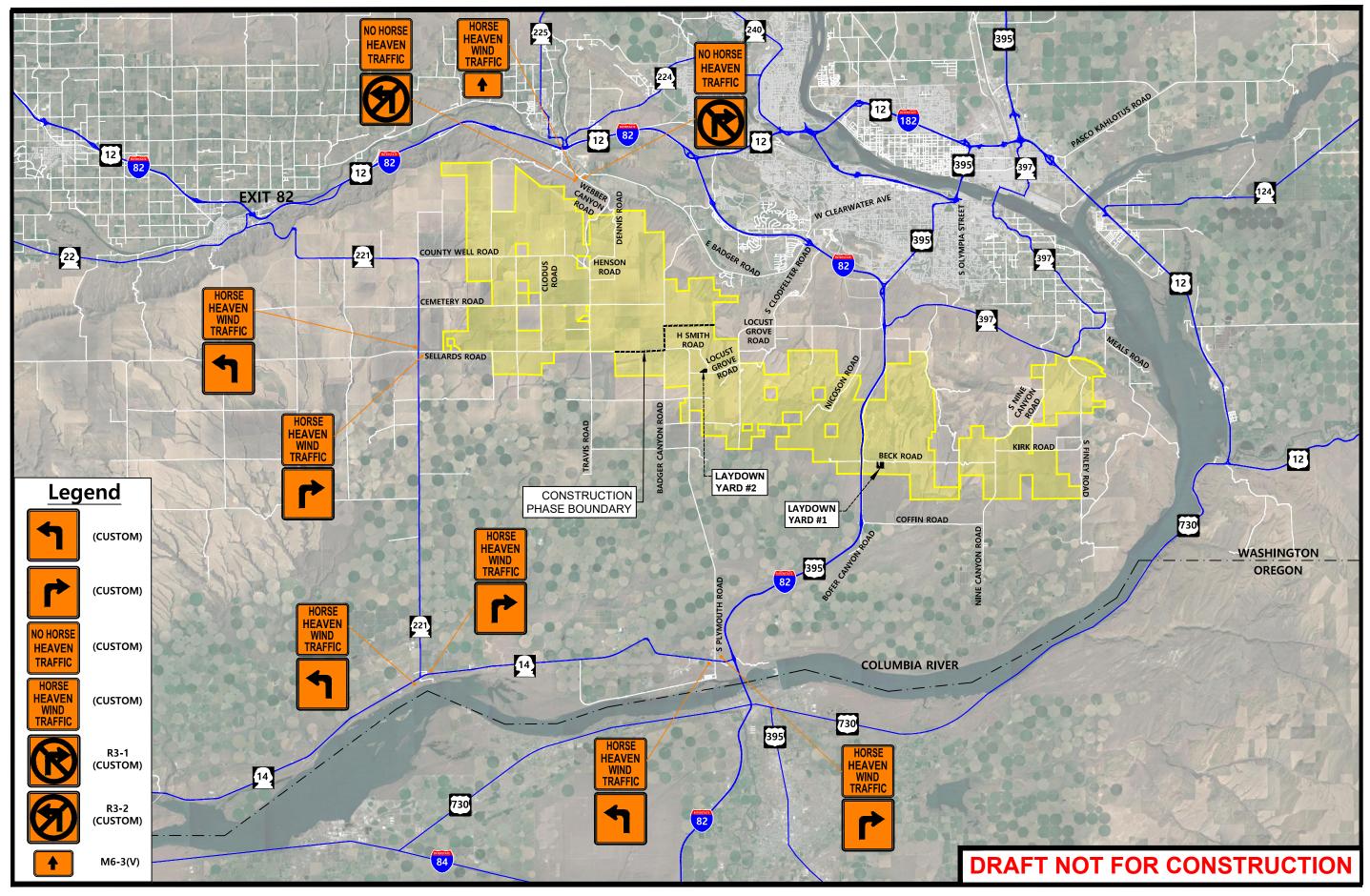
















# **APPENDIX A: TRAFFIC SCOPING LETTER**



May 26, 2023

Ms. Amy Moon
Siting and Compliance Lead
Washington State Energy Facility Site Evaluation Council
P.O. Box 43172
Olympia, WA 98504-3172
Amy.Moon@efsec.wa.gov

RE: Traffic Scoping Letter
Horse Heaven Wind Farm

EFSEC Docket Number: EF-210011

Benton County, WA

Dear Ms. Moon,

Tetra Tech has prepared this Transportation Scoping Letter (TSL) to provide the Washington State Energy Facility Site Evaluation Council (EFSEC) and Washington State Department of Transportation (WSDOT) with the basic scope of analysis, technical assumptions, and key transportation issues to be addressed in the Traffic Impact Analysis (TIA) to be prepared for the Horse Heaven Wind Farm project (the Project). The TIA will be prepared in accordance with WSDOT TIA guidelines. Preliminary transportation assessments were previously included in the December 2022 Draft Environmental Impact Statement (DEIS) and February 2021 (as revised December 2022) Updated Application for Site Certification (ASC) for the Project.

Pending EFSEC approval of the ASC at its earliest opportunity, given recent delays, the Applicant now anticipates beginning construction of the first phase of the Project in late 2024 or early 2025 with commercial operation approximately 11 months following start of construction. A second phase of the Project would begin construction in 2025 and begin operation in 2026. The construction schedule would be revised according to the actual approval of the Site Certification Agreement and implementation of commercial agreements for power purchase, and a copy provided to EFSEC at least sixty (60) days prior to the start of construction.

The TSL provides a brief discussion of key elements of the transportation analysis for the Project including:

- Project Description
- Planned Roadway Improvements
- Transportation Demand Management
- Project Construction Trip Generation
- Project Trip Distribution Patterns
- Study Area and Transportation Network
- Analysis Periods
- Traffic Safety
- Truck Haul Route Evaluation
- Traffic Mitigation

Tetra Tech is seeking EFSEC's and WSDOT's concurrence on the proposed study methodology so that we can proceed with preparation of the TIA.

## **Project Description**

Horse Heaven Wind Farm, LLC (Applicant) is proposing to construct and operate the Project in unincorporated Benton County, Washington, within the Horse Heaven Hills area. The Project would consist of a renewable energy generation facility that would have a maximum grid injection capacity of up to 1,150 megawatts for a combination of wind and solar facilities, battery energy storage systems (BESS), and other Project components, including underground and overhead electrical collection lines, underground communication lines, new Project substations, access roads, operations and maintenance facilities, and meteorological towers.

At its closest point, the Project would be located approximately 4 miles south/southwest of the City of Kennewick and the larger Tri-Cities urban area, along the Columbia River. Figure 1 shows the current Project Lease Boundary and Project vicinity. The Project's Lease Boundary (approximately 72,428 acres) incorporates all of the parcels for which the Applicant has executed a lease to construct the turbines, solar arrays, and associated facilities. The Project's Wind Energy Micrositing Corridor encompasses 11,850 acres within the Lease Boundary and consists of the areas where the turbines and supporting facilities would be sited during the final design. The Applicant seeks authorization for up to 244 turbine locations and a maximum of three solar arrays, with all possible turbine locations and solar arrays cumulatively reviewed to analyze potential resource impacts.

There are two options for the wind turbines (Option 1 and Option 2). The final number and location of turbines within the proposed Wind Energy Micrositing Corridor would reflect the final engineering design, model selection, and any additional avoidance and mitigation identified in the Final EIS. The specific model used would depend on the commercial availability and technology at the time of construction. The number of turbines would not exceed 244, and the maximum turbine height (at blade tip) would not exceed 671 feet. The DEIS assumed that the road disturbance associated with Turbine Option 1 and Turbine Option 2 would be identical.

As currently proposed, the Project would be built in two construction phases (Phase 1 and Phase 2). Phase 1 construction could generate power via wind and solar. Phase 1 could also include a BESS capable of storing energy. Phase 2 construction has two alternatives (Phase 2a and Phase 2b). Phase 2a could consist of the construction of both wind and solar facilities. The Applicant's Phase 2a scenario also includes the construction of a BESS. Phase 2b could increase power generation via the construction of additional wind turbines, but construction would not include a BESS.

During peak construction of each phase, a typical day would include the transportation of workers, materials, and movement of heavy equipment. Numerous on-site roadways are proposed to support the Project as well as one laydown area per construction phase as shown in Figure 1. The laydown areas would facilitate the delivery and assembly of materials and equipment. They would also serve as parking areas for the construction workforce.

#### **Planned Roadway Improvements**

As part of the TIA preparation, Tetra Tech will coordinate with WSDOT and Benton County to identify roadway improvements planned to be implemented by Others prior to the Project construction at study area roadways and intersections. If the construction of any roadway improvement projects planned by Others are anticipated to overlap with the proposed Project construction, Tetra Tech will coordinate closely with WSDOT and Benton County staff to implement additional construction management strategies and traffic mitigation, where appropriate, to ensure that cumulative construction impacts are minimized.

#### **Transportation Demand Management**

There are currently no public transportation services in the vicinity of the proposed laydown areas. However, the Applicant commits to encouraging carpooling among the Project's construction workforce to reduce single

occupancy vehicle trips to and from the site. Additionally, the Applicant will explore the feasibility of other potential transportation demand management (TDM) measures as part of the TIA.

## **Project Construction Trip Generation**

The Project will consist of three stages: construction, O&M, and decommissioning. The highest volume of site-related trips will occur during the peak construction phase of the Project. Therefore, the TIA will be based on the vehicle trip generation associated with the peak construction phase workforce levels.

Preliminary vehicle trip generation estimates were included in the Project's ASC. These estimates were further refined as part of this TSL to inform the study area and be used as the basis for analysis in the TIA. Construction of the proposed energy facility is expected to include grading, panel installation, inspections, and equipment deliveries. It is anticipated that, at peak operations, the site could experience construction workforce levels of up to 467 construction workers at one time. Construction hours of operation are assumed to generally be 7:00 AM to 7:00 PM with the majority of construction workers arriving prior to 7:00 AM and departing after 7:00 PM. Since the peak hours of the adjacent street traffic are expected to occur sometime during the peak commuting periods of 7:00 AM to 9:00 AM and 4:00 PM to 6:00 PM, it is expected that the majority of construction workers would arrive and depart the site outside of the typical weekday morning and weekday evening commuter peak hours of the adjacent streets. However, to present a conservative assessment of potential traffic increases associated with the Project, it is assumed the TIA will assume they arrive to the laydown sites and park, then drive to the specific worksites, then return to the laydown sites at the end of the day and that all of the construction workers would arrive during the weekday morning peak hour and depart during the weekday evening peak hour. Additionally, peak construction activities are currently anticipated to occur for only a portion of each the two successive 11-month periods. The remainder of the construction periods is anticipated to generate fewer vehicle trips. The supporting trip generation calculations and assumptions for the proposed Project's peak construction workforce levels are provided in the Attachments.

There are no public transportation services in the vicinity of the Project site that are anticipated to be used for the Project. Therefore, for the purposes of this assessment, it was assumed that no construction workers would use public transit to access the site. Additionally, it is anticipated that some construction workers would arrive and depart the site together (carpooling). For purposes of this assessment, it was assumed that the average vehicle would have 1.25 occupants to represent carpooling to/from the site.

There are currently two alternatives being considered for Phase 2 of the Project construction. Table 1 presents a summary of the trip generation estimates for the proposed Project's peak construction workforce activities by phase. The more conservative trip generation of the two Phase 2 alternatives (Phase 2a) will be used for the TIA analyses.

Table 1 Construction Trip Generation Summary<sup>1</sup>

	Phase 1			Phase 2a			Phase 2b		
Time Period/ Direction	Workforce <sup>2</sup>	Trucks <sup>3</sup>	Total	Workforce <sup>4</sup>	Trucks⁵	Total	Workforce <sup>6</sup>	Trucks <sup>7</sup>	Total
Weekday Daily									
Enter	748	250	998	688	200	888	660	206	866
Exit	748	250	998	688	200	888	660	206	866
Total	1,496	500	1,996	1,376	400	1,776	1,320	412	1,732
Weekday Morning Peak Hour									
Enter	374	13	387	344	10	354	330	11	341
Exit	0	13	13	0	10	10	0	11	11
Total	374	26	400	344	20	364	330	22	352
Weekday Evening Peak Hour									
Enter	0	13	13	0	10	10	0	11	11
Exit	374	13	387	344	10	354	330	11	341
Total	374	26	400	344	20	364	330	22	352

- 1) Based on the Horse Heaven Wind Farm Updated ASC (December 2022).
- 2) Used ASC estimated maximum Phase 1 peak period worker vehicle trips of 374. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.
- 3) Used ASC estimated maximum Phase 1 250 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hours.
- 4) Used ASC estimated maximum Phase 2a peak period worker vehicle trips of 344. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.
- 5) Used ASC estimated maximum Phase 2a 200 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hours.
- 6) Used ASC estimated maximum Phase 2b peak period worker vehicle trips of 330. Weekday daily workforce traffic volumes account for all workers travelling within the site from the laydown yard to the worksite and back before heading home.
- 7) Used ASC estimated maximum Phase 2b 206 truck trips per day. Used ASC assumption of 5% of daily truck trips occur during the peak hours.

#### **Project Trip Distribution Patterns**

Tetra Tech has developed separate Project trip distribution patterns for the Project's peak construction activities at the two proposed laydown areas. The trip distribution patterns were developed based on consideration of the effective population within the anticipated employment workforce (approximate 2-hour drivetime zone) and the currently proposed location of the laydown area for each phase. The Project construction workforce trip distribution patterns are shown in Figures 2 and 3, respectively.

#### **Study Area Intersections**

A comprehensive group of intersections have been chosen for detailed analysis in the TIA to be prepared for the Project. These study area intersections are shown in Figure 1 and include the 29 existing locations listed below. These are the intersections that have the potential be materially impacted by the Project. The TIA will also include an analysis of the proposed laydown site driveways where they intersect the public roadway system.

- Wine Country Road at I-82 NB Ramps
- Wine Country Road at I-82 SB Ramps
- Wine Country Road at Route 22
- Route 22 at Route 221 and Paterson Avenue
- Route 221 at County Well Road
- Route 221 at Cemetery Road
- Route 221 at Sellards Road
- Webber Canyon road at County Well Road
- · Cemetery Road at Travis Road
- S. Plymouth Road at S. Clodfelter Road
- Route 221 at Route 14
- Route 14 at S. Plymouth Road
- Route 14 at I-82 SB Ramps
- Route 14 at I-82 NB Ramps
- Coffin Road at I-82 SB Ramps
- Coffin Road at I-82 NB Ramps
- Coffin Road at Bofer Canyon Road
- Bofer Canyon Road at Beck Road
- Locust Grove Road at I-82 SB Ramps
- Locust Grove Road at I-82 NB Ramps
- Route 397/Locust Grove Road at Bofer Canyon Road
- Route 397 at S. Owens Road
- Route 397 at S. Nine Canyon Road
- Route 397 at S. Finley Road
- Nine Canyon Road at Kirk Road
- Nine Canyon Road at Beck Road
- Nine Canyon Road at Coffin Road
- Locust Grove Road at S. Clodfelter Road
- Webber Canyon Road at Badger Road

# **Analysis Periods**

The TIA will be prepared based on WSDOT guidelines and provide a detailed analysis of the 2023 Existing, 2025 No Build (Without Project), 2025 Phase 1 Build (With Phase 1 Peak Construction) and 2026 Phase 2 Build (With Phase 2 Peak Construction) traffic operations at the study intersections for the weekday morning and weekday evening peak hours, when the combination of Project construction traffic and existing traffic already traveling on the adjacent area roadways would be greatest. As described under Project Description, Phases 1 and 2 of Project construction are anticipated to occur in successive 11-month periods, with Phase 1 beginning in late 2024/early 2025. As a conservative measure, the future year chosen for analysis in the TIA will be the design year 2025 for Phase 1 and the year 2026 for Phase 2.

The 2023 existing traffic volumes will be developed based on intersection Turning Movement Counts (TMCs) to be collected at the study area intersections Construction hours of operation are anticipated to generally occur from 7:00 AM to 7:00 PM. Therefore, the TMCs will be collected at the study area intersections on a typical weekday (Tuesday, Wednesday, or Thursday) from 6:00 AM to 9:00 AM and 4:00 PM to 8:00 PM to establish the weekday morning and evening commuter peak hour conditions. Any necessary seasonal adjustment factors to be applied to the observed traffic volumes will be identified in consultation with WSDOT.

Analysis of roadway segments will also be included in the TIA for the following three key locations. Permanent count station data from WSDOT will be used for locations 1 and 2. New Automatic Traffic Recorder (ATR) counts will be collected for a 24-hour weekday period at Locations 2 and 4 through 10.

- 1. I- 82 (North of Coffin Road)
- 2. Route 397 (West of Nine Canyon Road)
- 3. Route 221 (South of Sellards Road)
- 4. Bofer Canyon Road (north of Coffin Road)
- 5. Nine Canyon Road (south of Route 397)
- 6. Locust Grove Road (between Nicosin Road and I-82)
- 7. Travis Road (north of Sellards Road)
- 8. Plymouth Road (north of Route 14)
- 9. Sellards Road (between Route 221 and Tyack Road)
- 10. Badger Canyon Road (north of Sellards Road)

The future year (2025) baseline peak hour volumes will be forecasted using a general background growth rate to be identified in consultation with WSDOT staff. The general background growth rate will be applied to the 2023 existing traffic volumes to develop the future 2025 Baseline peak hour traffic volumes. Additionally, Tetra Tech will coordinate with WSDOT and Benton County planning staff to identify any specific background development projects to include in the future year forecasts. Only projects anticipated to be built and occupied by 2025 will be included in the development of future year traffic volumes. The Phase 1 and Phase 2 project trips will then be added to the 2025 No Build (Without Project) peak hour traffic volumes to estimate the 2025 Phase 1 Build (Peak Construction) and 2026 Phase 2 Build (Peak Construction) peak hour traffic volumes.

Capacity analyses of the study area intersections will be conducted using Synchro Version 11 software for the 2023 Existing, 2025 No Build, 2025 Phase 1 Build (Peak Construction) and 2026 Phase 2 Build (Peak Construction) weekday morning and weekday evening peak hours.

## **Traffic Safety Evaluation**

As part of the TIA, Tetra Tech will submit a Request for Crash Data form to WSDOT to obtain crash data for the study area roadways and intersections for the most recent 5-year period available. Highway Safety Manual (HSM) predictive methods will be used to analyze safety at each of the study area intersections in accordance with WSDOT guidelines using Transportation Research Board (TRB) spreadsheets and/or the Federal Highway Administration (FHWA) IHSDM software.

Stopping sight distance (SSD) and intersection sight distance (ISD) analyses will be conducted at the proposed laydown area site driveways where they intersect the public roadway system. The sight distance analyses will be performed in accordance with WSDOT and American Association of State Highway Transportation Officials (AASHTO) guidelines.

A Draft Safety Management Plan will also be prepared for the Project-related construction activity separate from the TIA in accordance with WSDOT standards.

#### **Truck Haul Route Evaluation**

The ASC included a preliminary truck haul assessment prepared by TLG Transport Inc. The preliminary truck haul routes identified in the DEIS and Updated ASC for Phase 1 and Phase 2 of Project construction are shown in Figures 4 through 7. The truck haul routes will continue to be evaluated as part of the TIA and the estimated truck trips summarized in Table 1 will be assigned to the study area roadway network based on the final truck routes.

## **Traffic Mitigation**

Traffic mitigation needed to support peak construction activities associated with the Project will be recommended in the TIA. Mitigation will be identified for any study area intersection or roadway segment determined to exceed LOS C operations for rural facilities and LOS D for urban non-NHS facilities and/or locations identified in the TIA to have safety deficiencies that may be enhanced with geometric, traffic control or signage improvements. Traffic mitigation will also be proposed at locations where existing geometry does not currently support the largest vehicles anticipated to be used during construction of the Project. The 2020 WSDOT State Highway Log will be used to determine urban or rural status for study area roadways.

Traffic mitigation will be designed in accordance with WSDOT and Benton County standards. The Applicant indicates in a signed franchise agreement with Benton County Public Works, dated July 2, 2019, that all work done on existing Benton County roads will be done in accordance with Benton County requirements and with review and approval by the County Engineer. Preliminary findings and traffic mitigation recommendations will be reviewed with WSDOT prior to the finalization of the TIA.

We respectfully request concurrence from EFSEC and WSODT on the proposed study area and methodologies discussed above. Please do not hesitate to contact Robert Woodland with any questions or concerns at (781) 910-7015 or Kristen Daniel at (509) 372-5819.

Sincerely,

Kristen Daniel, PE Principal Civil Engineer Robert Woodland, PE Senior Project Manager

Bleath Ordh

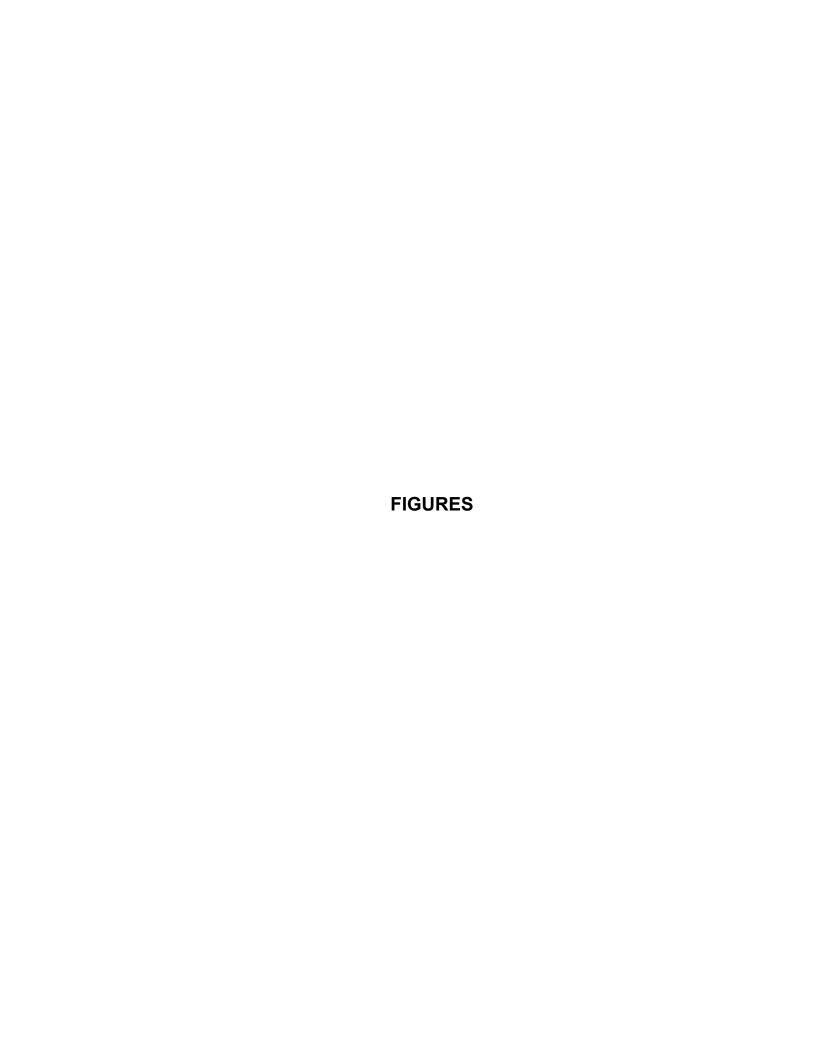
cc (by email): Ami Hafkemeyer, Sean Greene - EFSEC

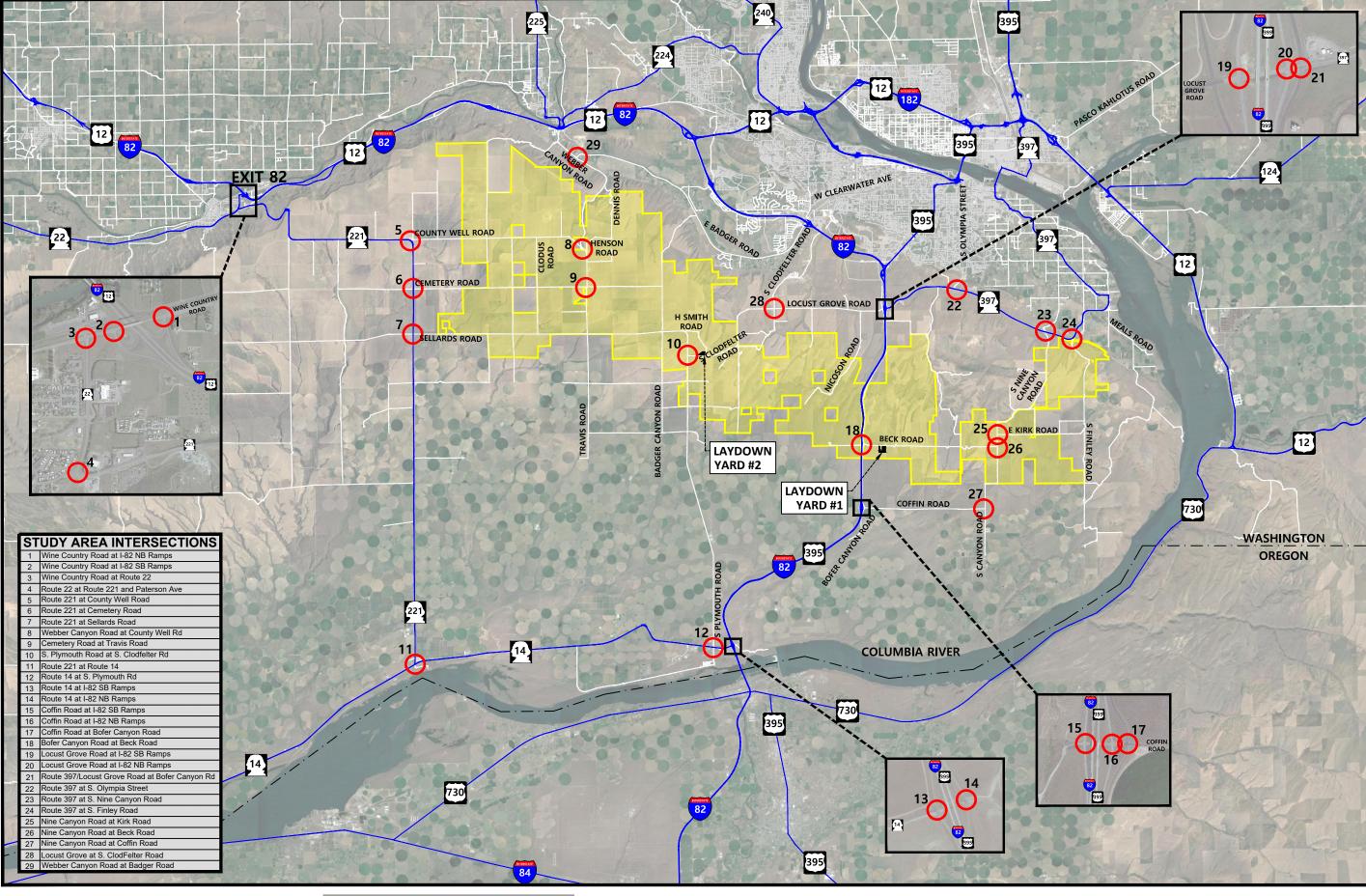
Jacob Palucik, Paul Gonseth, Todd Daley – WSDOT Sierra Harmening, Vamshi Akkinepally – WSP

Dave Kobus - Scout Clean Energy

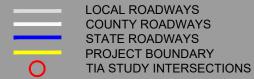
Attachments: Figures 1 – 7, Trip Distribution Calculations

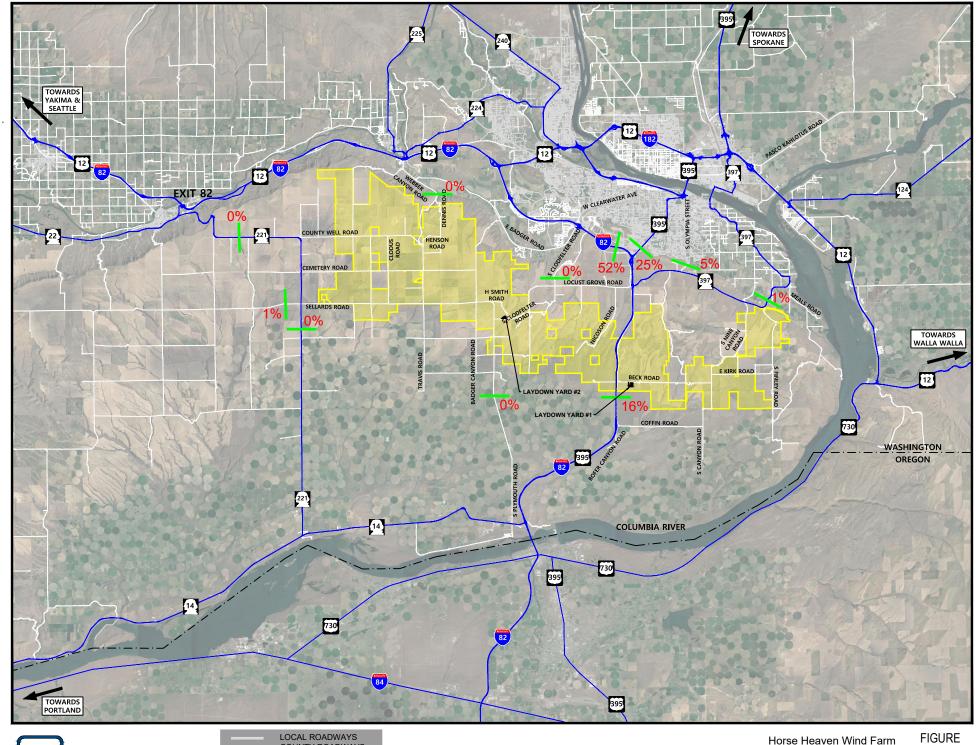
**Attachments** 



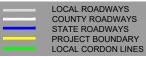


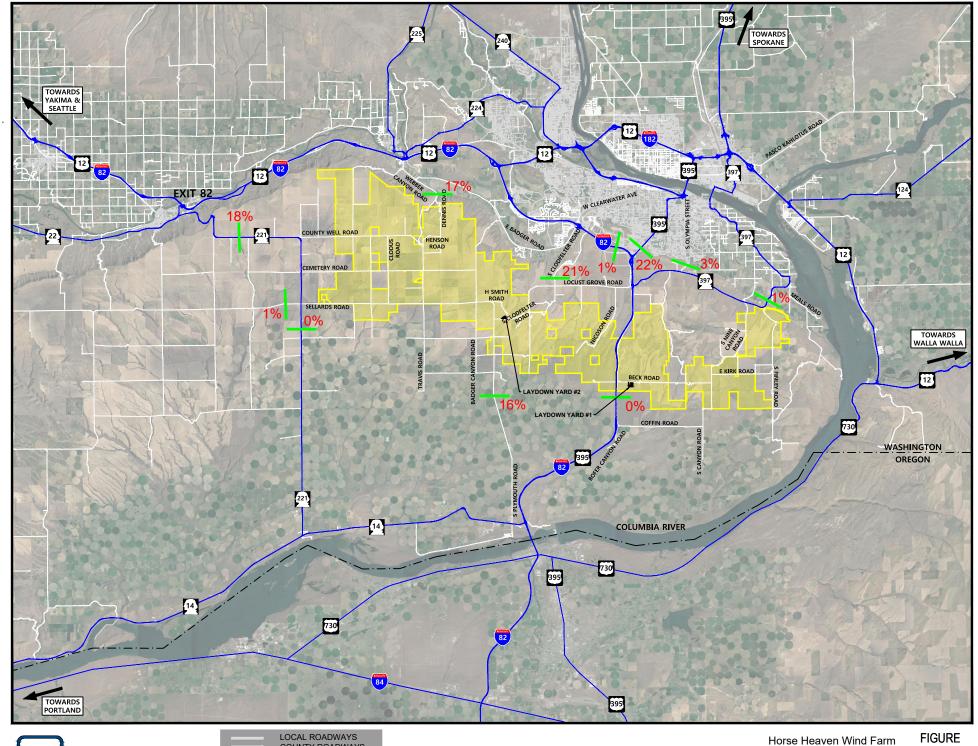




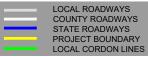




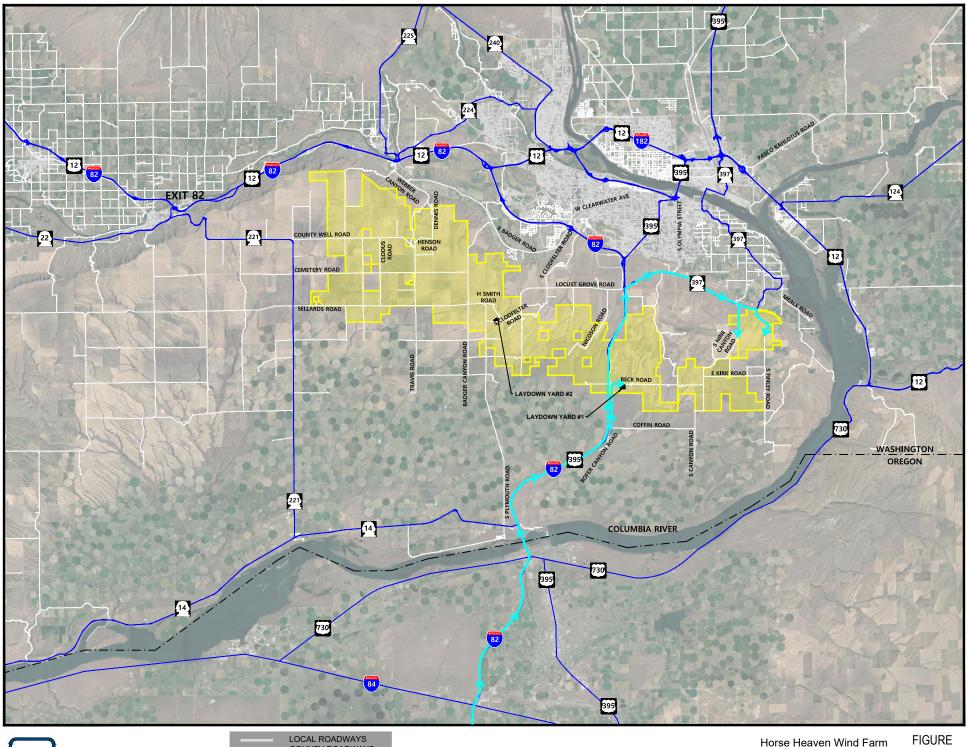






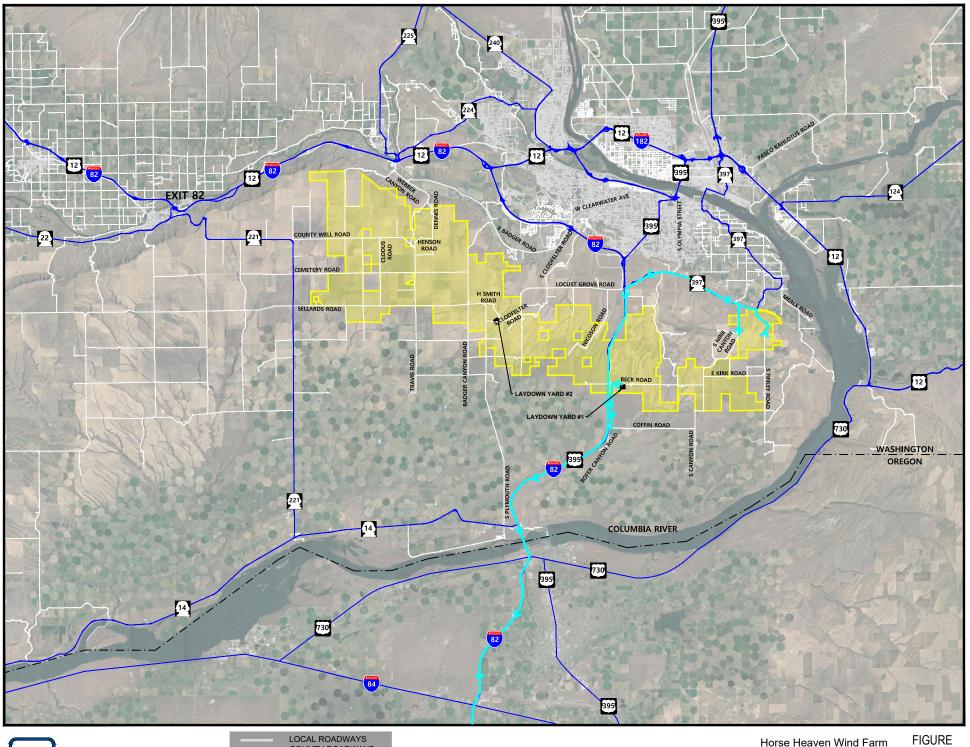


Horse Heaven Wind Farm Benton County, WA



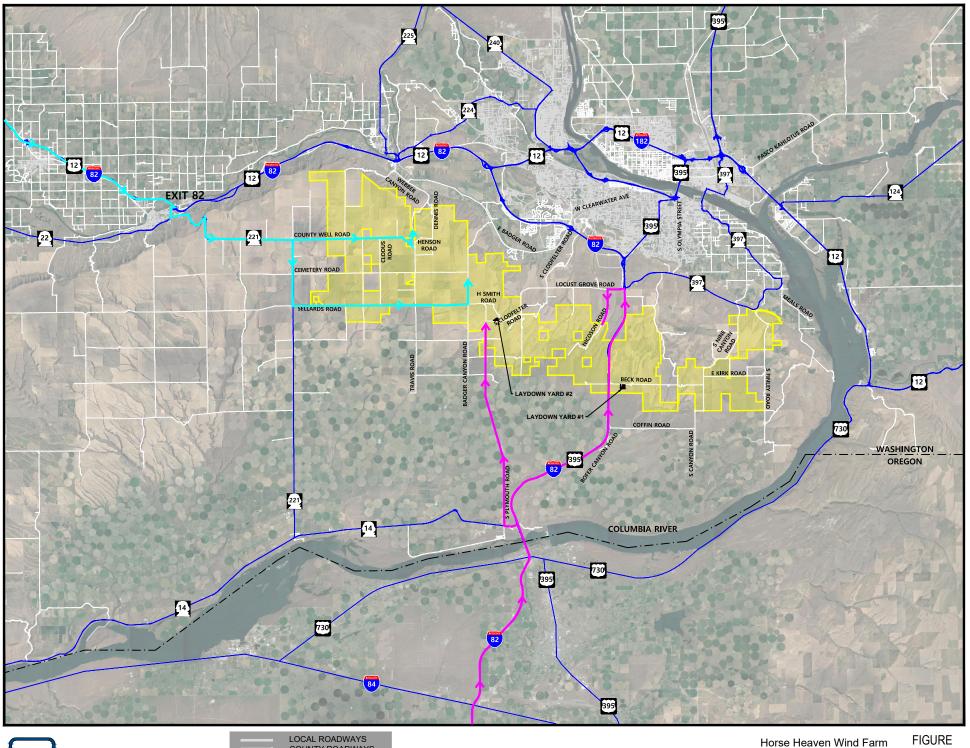






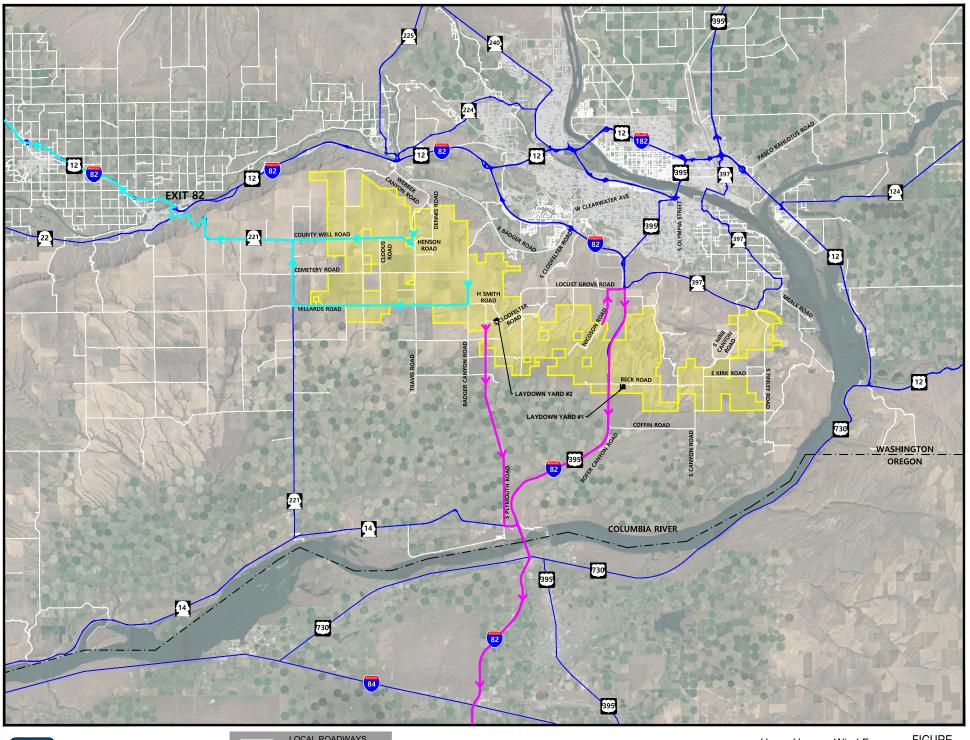




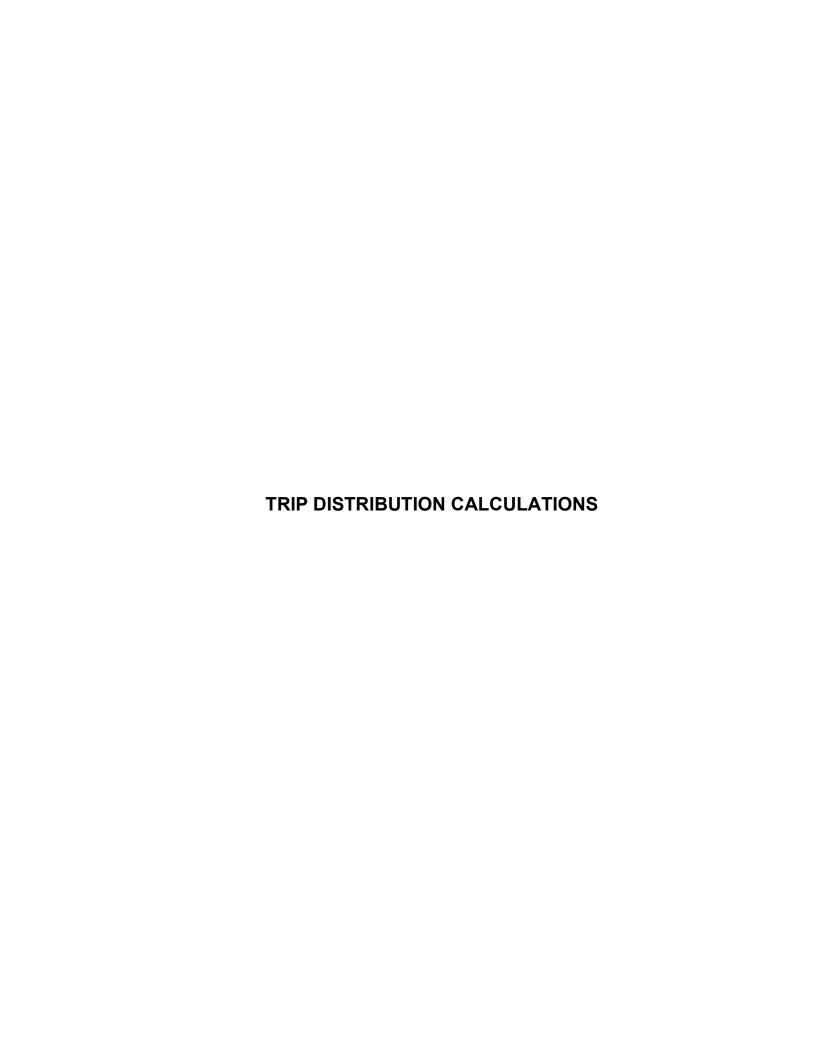












										<u>.</u>	diation Gravity		/N YARD #1 - LC	CAL TRAVEL ROU	TES			1	
								To/From	ı West			To/Fror	n North			To/From South		1	
						Effective													
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
County		State	Population	Time to Site	Zone	Zone	Population			Road								100% CHECK	Total
Benton County	101	WA	4,873	30 mins	100.0%	4873	0.661%					100%						100%	0.7%
Benton County	102.01	WA	5,718	30 mins	100.0%	5718	0.776%					100%						100%	0.8%
Benton County	102.03	WA	4,637	30 mins	100.0%	4637	0.629%					100%						100%	0.6%
Benton County	102.04 103	WA WA	2,830 6,153	30 mins	100.0% 100.0%	2830 6153	0.384% 0.835%					100%						100% 100%	0.4%
Benton County Benton County	103	WA	3,637	30 mins	100.0%	3637	0.833%					100%						100%	0.5%
Benton County	105	WA	3,271	30 mins	100.0%	3271	0.444%					100%						100%	0.4%
Benton County	106	WA	4,930	30 mins	100.0%	4930	0.669%					100%						100%	0.7%
Benton County	107.01	WA	2,021	35 mins	98.3%	1986	0.270%					100%						100%	0.3%
Benton County Benton County	107.03 107.05	WA WA	3,424 5,741	35 mins 35 mins	98.3% 98.3%	3365 5643	0.457% 0.766%					100%						100% 100%	0.5% 0.8%
Benton County	107.07	WA	3,775	30 mins	100.0%	3775	0.512%					100%						100%	0.5%
Benton County	107.08	WA	4,332	30 mins	100.0%	4332	0.588%					100%						100%	0.6%
Benton County	108.07	WA	1,762	25 mins	100.0%	1762	0.239%					100%						100%	0.2%
Benton County Benton County	108.09 108.10	WA WA	6,391 4,861	25 mins 25 mins	100.0% 100.0%	6391 4861	0.867% 0.660%	<b> </b>		<del> </del>	<del>                                     </del>	100%		<del>                                     </del>		<del>                                     </del>		100% 100%	0.9% 0.7%
Benton County	108.10	WA	5,275	20 Mins or less	100.0%	5275	0.860%					100%				+ +		100%	0.7%
Benton County	108.14	WA	5,186	20 Mins or less	100.0%	5186	0.704%			<u> </u>		100%						100%	0.7%
Benton County	108.15	WA	8,567	20 Mins or less	100.0%	8567	1.163%					50%	50%					100%	1.2%
Benton County	108.16	WA	5,589	20 Mins or less	100.0%	5589	0.759%			1		50%	50%			<del>                                     </del>		100%	0.8%
Benton County Benton County	108.17 108.18	WA WA	6,198 3,274	25 mins 25 mins	100.0% 100.0%	6198 3274	0.841% 0.444%					100%						100% 100%	0.8%
Benton County	108.19	WA	3,304	25 mins	100.0%	3304	0.448%					100%						100%	0.4%
Benton County	108.20	WA	3,737	25 mins	100.0%	3737	0.507%					50%	50%					100%	0.5%
Benton County	109.01	WA	6,251	25 mins	100.0%	6251	0.848%					50%	50%					100%	0.8%
Benton County	109.02	WA	5,698	20 Mins or less	100.0%	5698	0.773%						100%	F00/				100%	0.8%
Benton County Benton County	110.01 110.02	WA WA	6,025 4,859	25 mins 20 Mins or less	100.0% 100.0%	6025 4859	0.818% 0.659%						50% 100%	50%				100% 100%	0.8%
Benton County	111	WA	7,879	20 Mins or less	100.0%	7879	1.069%						50%	50%				100%	1.1%
Benton County	112.01	WA	4,267	20 Mins or less	100.0%	4267	0.579%						50%	50%				100%	0.6%
Benton County	112.02	WA	3,323	20 Mins or less	100.0%	3323	0.451%						50%	50%				100%	0.5%
Benton County Benton County	113 114.01	WA WA	5,040 3,580	25 mins 20 Mins or less	100.0% 100.0%	5040 3580	0.684%						50% 50%	50% 50%				100% 100%	0.7% 0.5%
Benton County	114.01	WA	5,415	20 Mins or less	100.0%	5415	0.480%						30%	100%				100%	0.7%
Benton County	115.01	WA	6,443	25 mins	100.0%	6443	0.874%								100%			100%	0.9%
Benton County	115.04	WA	2,866	20 Mins or less	100.0%	2866	0.389%							100%				100%	0.4%
Benton County Benton County	115.05 115.06	WA WA	4,177 7,519	20 Mins or less 20 Mins or less	100.0% 100.0%	4177 7519	0.567% 1.020%						100%	100%				100% 100%	0.6% 1.0%
Benton County	117.01	WA	3,012	40 Mins	96.5%	2906	0.394%					100%		100%				100%	0.4%
Benton County	117.02	WA	5,132	40 Mins	96.5%	4952	0.672%					100%						100%	0.7%
Benton County	118.01	WA	3,655	45 mins	94.6%	3457	0.469%					100%						100%	0.5%
Benton County	118.02	WA	2,665	45 mins	94.6%	2520	0.342%			1		100%				1		100%	0.3%
Benton County Benton County	119 120	WA WA	6,325 0	30 Mins 40 mins	100.0% 96.5%	6325 0	0.858%					100%				+ +		100% 0%	0.9%
Franklin County	201.01	WA	1,828	30 Mins	100.0%	1828	0.248%			Ì			50%	50%				100%	0.2%
Franklin County	201.02	WA	6,609	30 Mins	100.0%	6609	0.897%						100%					100%	0.9%
Franklin County	201.03	WA	3,811	30 Mins	100.0%	3811	0.517%			-			75%	25%				100%	0.5%
Franklin County Franklin County	202.01 202.02	WA WA	2,201 4,142	25 Mins 30 Mins	100.0% 100.0%	2201 4142	0.299% 0.562%			1			75% 75%	25% 25%		<del>                                     </del>		100% 100%	0.3%
Franklin County Franklin County	202.02	WA	6,088	30 Mins	100.0%	6088	0.826%			1			100%	23/0		<del>                                     </del>		100%	0.8%
Franklin County	204.01	WA	1,065	25 Mins	100.0%	1065	0.145%						100%					100%	0.1%
Franklin County	204.02	WA	1,101	25 Mins	100.0%	1101	0.149%		·			· · · · · · · · · · · · · · · · · · ·	100%		·		·	100%	0.1%
Franklin County Franklin County	204.03 204.04	WA	3,611	25 Mins	100.0%	3611	0.490%			1			100% 100%			+ + +		100% 100%	0.5%
Franklin County Franklin County	204.04	WA WA	2,928 5,161	25 Mins 30 Mins	100.0% 100.0%	2928 5161	0.397% 0.700%			1		50%	50%			+		100%	0.4%
Franklin County	205	WA	3,296	30 Mins	100.0%	3296	0.447%					3070	100%					100%	0.4%
Franklin County	205	WA	6,522	30 Mins	100.0%	6522	0.885%						100%					100%	0.9%
Franklin County	206	WA	4,546	35 Mins	98.3%	4468	0.606%			ļ		50%	50%					100%	0.6%
Franklin County Franklin County	206 206	WA WA	9,548 8,729	35 Mins 35 Mins	98.3% 98.3%	9385 8580	1.274% 1.164%			<del> </del>		50% 50%	50% 50%			1		100% 100%	1.3%
Franklin County	206	WA	6,719	35 Mins	98.3%	6604	0.896%			1		50%	50%			<del>                                     </del>		100%	0.9%
Franklin County	206	WA	6,601	35 Mins	98.3%	6488	0.881%			<u> </u>		50%	50%					100%	0.9%
Morrow County	9701.01	OR	5,034	45 Mins	94.6%	4761	0.646%									100%		100%	0.6%
Morrow County	9701.02	OR	3,676	30 Mins	100.0%	3676	0.499%									100%		100%	0.5%

												LAYDOW	VN YARD #1 - LO	CAL TRAVEL ROU	TES			
								To/From	West			To/From	m North			To/From South		
						Effective												
	Census		Total	Estimated Drive	% Population in Drive Time	Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road Roue 221		
County		State	Population	Time to Site	Zone	Zone	Population			Nodu							100% CHECK	Total
Morrow County	9702	OR	3,254	0	88.1%	2867	0.389%									100%	100%	0.4%
Umatilla County	9400	OR	3,072	0	88.1%	2706	0.367%						250/			100%	100%	0.4%
Umatilla County Umatilla County	9501 9502.01	OR OR	4,659 3,712	0	76.9% 76.9%	3581 2853	0.486% 0.387%			+			25% 25%			75% 75%	100% 100%	0.5%
Umatilla County	9502.02	OR	4,043	0	76.9%	3108	0.422%						25%			75%	100%	0.4%
Umatilla County	9503	OR	3,371	0	82.9%	2796	0.379%									100%	100%	0.4%
Umatilla County Umatilla County	9504 9505	OR OR	6,038 4,754	0	88.1% 88.1%	5319 4188	0.722% 0.568%			<del> </del>						100%	100% 100%	0.7% 0.6%
Umatilla County	9506.01	OR	2,600	0	88.1%	2290	0.311%			1						100%	100%	0.3%
Umatilla County	9506.02	OR	3,212	0	88.1%	2830	0.384%									100%	100%	0.4%
Umatilla County	9507	OR	2,513	55 Mins	90.4%	2272	0.308%			<u> </u>						100%	100%	0.3%
Umatilla County Umatilla County	9508 9509	OR OR	7,508 5,133	25 Mins 20 Mins or less	100.0% 100.0%	7508 5133	1.019% 0.697%									100%	100% 100%	1.0% 0.7%
Umatilla County	9510	OR	6,224	25 Mins	100.0%	6224	0.845%									100%	100%	0.8%
Umatilla County	9511	OR	6,018	25 Mins	100.0%	6018	0.817%									100%	100%	0.8%
Umatilla County Umatilla County	9512.01 9512.02	OR OR	6,207 4,040	30 Mins 30 Mins	100.0% 100.0%	6207 4040	0.842% 0.548%			+						100%	100% 100%	0.8% 0.5%
Umatilla County	9513	OR	3,989	30 Mins	100.0%	3989	0.541%			1			1			100%	100%	0.5%
Umatilla County	9514	OR	2,416	1:10	82.9%	2004	0.272%									100%	100%	0.3%
Union County	9701 9702	OR OR	3,238 3,376	2:00 1:55	23.1% 36.7%	749 1238	0.102% 0.168%			1						100%	100% 100%	0.1% 0.2%
Union County Union County	9702	OR	2,427	1:55	36.7%	890	0.168%			+				+		100%	100%	0.2%
Union County	9704	OR	2,732	1:50	46.3%	1264	0.172%									100%	100%	0.2%
Union County	9705	OR	3,196	1:35	65.0%	2076	0.282%									100%	100%	0.3%
Union County Union County	9706 9707	OR OR	3,927 3,416	1:50 1:40	46.3% 59.8%	1817 2043	0.247% 0.277%									100%	100% 100%	0.2%
Union County	9708	OR	3,943	1:40	59.8%	2358	0.320%			1						100%	100%	0.3%
Adams County	9501	WA	2,577	1:30	69.4%	1789	0.243%						100%				100%	0.2%
Adams County	9502	WA	1,794	1:20	76.9%	1379	0.187%			-			100%				100%	0.2%
Adams County Adams County	9503.01 9503.02	WA WA	1,790 2,738	1:20 1:20	76.9% 76.9%	1376 2104	0.187% 0.286%					50%	100% 50%				100% 100%	0.2%
Adams County	9503.03	WA	2,555	1:15	80.0%	2045	0.278%					50%	50%				100%	0.3%
Adams County	9504	WA	3,100	1:15	80.0%	2481	0.337%					50%	50%				100%	0.3%
Adams County Columbia County	9505 9602	WA WA	5,799 3,969	1:15 1:30	80.0% 69.4%	4642 2755	0.630% 0.374%					50%	50% 100%				100% 100%	0.6% 0.4%
Franklin County	207	WA	1,277	1:15	80.0%	1022	0.139%			†			100%				100%	0.1%
Franklin County	208.01	WA	3,401	1:00	88.1%	2996	0.407%						100%				100%	0.4%
Franklin County	208.02	WA	3,129	1:00	88.1%	2756	0.374%			<u> </u>			100%				100%	0.4%
Grant County Grant County	101 102	WA WA	3,610 3,382	greater than 2:00 greater than 2:00	0.0%	0	0.000%						100% 100%				100%	0.0%
Grant County	103	WA	5,425	greater than 2:00	0.0%	0	0.000%						100%				100%	0.0%
Grant County	104.01	WA	3,148	greater than 2:00	0.0%	0	0.000%						100%				100%	0.0%
Grant County Grant County	104.02 105	WA WA	5,495 3,270	greater than 2:00 greater than 2:00	0.0%	0	0.000%			+		50%	100% 50%				100%	0.0%
Grant County	106	WA	7,614	greater than 2:00	0.0%	0	0.000%			1		50%	50%				100%	0.0%
Grant County	107	WA	3,186	1:45	53.7%	1712	0.232%			1		50%	50%				100%	0.2%
Grant County Grant County	108 109.01	WA WA	5,398 1,679	1:30 1:30	69.4% 69.4%	3747 1165	0.509% 0.158%			<del>                                     </del>			100% 100%			<del>                                     </del>	100% 100%	0.5% 0.2%
Grant County  Grant County	109.01	WA	5,281	1:30	69.4%	3666	0.158%						100%			<del>                                     </del>	100%	0.2%
Grant County	109.04	WA	6,136	1:30	69.4%	4259	0.578%						100%				100%	0.6%
Grant County	110.01	WA	5,723	1:30	69.4%	3973	0.539%						100%				100%	0.5%
Grant County Grant County	110.02 111.01	WA WA	6,225 4,657	1:30 1:30	69.4% 69.4%	4321 3233	0.586% 0.439%			+			100% 100%			<del>                                     </del>	100% 100%	0.6%
Grant County	111.02	WA	2,891	1:30	69.4%	2007	0.272%			1			100%				100%	0.3%
Grant County	112	WA	7,100	1:45	53.7%	3814	0.518%					50%	50%				100%	0.5%
Grant County Grant County	113 114.01	WA WA	3,367 2,249	1:20 1:20	76.9% 76.9%	2588 1729	0.351% 0.235%			<del> </del>			100% 100%			<del>                                     </del>	100% 100%	0.4%
Grant County  Grant County	114.01	WA	4,871	1:15	80.0%	3899	0.235%			+		100%	100%	+		<del>                                     </del>	100%	0.2%
Grant County	114.04	WA	963	1:30	69.4%	668	0.091%					100%					100%	0.1%
Grant County	114.05	WA	3,019	1:30	69.4%	2096	0.284%			<u> </u>		50%	50%				100%	0.3%
Grant County Kittitas County	114.06 9751.01	WA WA	3,185 2,363	1:30 greater than 2:00	69.4% 0.0%	2211 0	0.300%			+		50% 100%	50%				100% 100%	0.3%
Kittitas County  Kittitas County	9751.01	WA	1,290	greater than 2:00	0.0%	0	0.000%					100%				<del>                                     </del>	100%	0.0%
Kittitas County	9751.03	WA	1,444	greater than 2:00	0.0%	0	0.000%					100%					100%	0.0%

										<u> </u>	diation Gravity i		VN YARD #1 - LC	CAL TRAVEL ROU	TES			1	
								To/Fron	n West				m North			To/From South			
						Effective													
				5.1	-	Population in	0/ . 5 = 1	Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
County	Census Tract	State I	Total Population	Estimated Drive Time to Site	in Drive Time Zone	Drive Time Zone	% of Total Population			Road				, , ,				100% CHECK	Total
Kittitas County		WA	1,644	greater than 2:00	0.0%	0	0.000%					100%						100%	0.0%
Kittitas County		WA	3,364	greater than 2:00	0.0%	0	0.000%					100%						100%	0.0%
Kittitas County	9752.02	WA	1,395	1:45	53.7%	749	0.102%					100%						100%	0.1%
Kittitas County	9752.03	WA	1,098	1:45	53.7%	590	0.080%					100%						100%	0.1%
Kittitas County	9753	WA	5,316	2:00	23.1%	1230	0.167%					100%						100%	0.2%
Kittitas County Kittitas County	9754.02 9754.03	WA WA	4,713 2,921	1:45 1:45	53.7% 53.7%	2532 1569	0.344% 0.213%					100%						100% 100%	0.3% 0.2%
Kittitas County	9754.04	WA	5,145	1:45	53.7%	2764	0.375%					100%						100%	0.4%
Kittitas County	9755	WA	5,956	1:45	53.7%	3200	0.434%					100%						100%	0.4%
Kittitas County	9756	WA	2,790	1:45	53.7%	1499	0.203%					100%						100%	0.2%
Kittitas County	9757	WA	4,708	2:00	23.1%	1089	0.148%	500/				100%						100%	0.1%
Klickitat County Klickitat County	9501.01 9501.02	WA WA	1,538 3,507	1:00 1:20	88.1% 76.9%	1355 2696	0.184% 0.366%	50%				50% 100%						100% 100%	0.2% 0.4%
Klickitat County	9501.03	WA	4,189	1:45	53.7%	2251	0.305%					100%				100%		100%	0.3%
Klickitat County	9502	WA	4,383	greater than 2:00	0.0%	0	0.000%									100%		100%	0.0%
Klickitat County	9503.01	WA	3,465	greater than 2:00	0.0%	0	0.000%									100%		100%	0.0%
Klickitat County	9503.02	WA	5,396	greater than 2:00	0.0%	0	0.000%						40001			100%		100%	0.0%
Walla Walla County Walla Walla County	9200 9201	WA WA	6,176 5,165	1:00 1:00	88.1% 88.1%	5441 4550	0.738% 0.618%			+	<del>                                     </del>		100% 75%	+		25%		100% 100%	0.7% 0.6%
Walla Walla County	9201	WA	4,715	1:15	88.1%	3774	0.512%			+	+ +		50%	+		50%		100%	0.5%
Walla Walla County	9203.01	WA	3,243	1:15	80.0%	2596	0.352%			1			50%			50%		100%	0.4%
Walla Walla County	9203.02	WA	5,434	1:15	80.0%	4350	0.590%						50%			50%		100%	0.6%
Walla Walla County	9204	WA	2,640	1:15	80.0%	2113	0.287%						50%			50%		100%	0.3%
Walla Walla County Walla Walla County	9205 9206	WA WA	2,959 6,205	1:15 1:15	80.0% 80.0%	2368 4967	0.321% 0.674%						50% 50%			50%		100% 100%	0.3% 0.7%
Walla Walla County		WA	3,545	1:15	80.0%	2838	0.874%						50%			50%		100%	0.7%
Walla Walla County		WA	4,293	1:15	80.0%	3436	0.466%						50%			50%		100%	0.5%
Walla Walla County	9208.01	WA	4,945	1:15	80.0%	3958	0.537%						50%			50%		100%	0.5%
Walla Walla County		WA	3,223	1:15	80.0%	2580	0.350%						50%			50%		100%	0.4%
Walla Walla County	9209.01 9209.02	WA WA	4,134 5,491	1:15 1:15	80.0% 80.0%	3309 4395	0.449% 0.596%						50% 50%			50%		100% 100%	0.4%
Walla Walla County Yakima County	1	WA	3,072	1:15	80.0%	2459	0.334%					100%	30%			50%		100%	0.8%
Yakima County	2	WA	5,595	1:15	80.0%	4478	0.608%					100%						100%	0.6%
Yakima County	3.01	WA	2,473	1:15	80.0%	1979	0.269%					100%						100%	0.3%
Yakima County	3.02	WA	2,283	1:15	80.0%	1827	0.248%					100%						100%	0.2%
Yakima County	4.01	WA	5,958	1:15	80.0%	4769	0.647%					100%						100%	0.6%
Yakima County Yakima County	4.02 5	WA WA	2,407 4,599	1:15 1:15	80.0% 80.0%	1927 3681	0.261% 0.500%					100%						100% 100%	0.3% 0.5%
Yakima County	6	WA	5,696	1:15	80.0%	4559	0.619%					100%						100%	0.6%
Yakima County	7	WA	7,077	1:15	80.0%	5665	0.769%					100%						100%	0.8%
Yakima County	8	WA	4,484	1:15	80.0%	3589	0.487%					100%						100%	0.5%
Yakima County	9.02	WA	4,507	1:20	76.9%	3464	0.470%					100%				<del>                                     </del>		100% 100%	0.5%
Yakima County Yakima County	9.03 9.04	WA WA	4,008 3,332	1:20 1:20	76.9% 76.9%	3081 2561	0.418% 0.348%			+		100%				<del>                                     </del>		100%	0.4%
Yakima County	10	WA	6,499	1:20	76.9%	4995	0.678%			†		100%						100%	0.7%
Yakima County	11	WA	7,361	1:15	80.0%	5892	0.800%					100%						100%	0.8%
Yakima County	12.01	WA	4,723	1:15	80.0%	3780	0.513%					100%						100%	0.5%
Yakima County	12.02	WA	7,051	1:15	80.0%	5644	0.766%					100%				+ + +		100%	0.8%
Yakima County Yakima County	13 14	WA WA	2,653 4,099	1:10 1:10	82.9% 82.9%	2201 3400	0.299% 0.461%			1		100%				+ +		100% 100%	0.3% 0.5%
Yakima County	15.02	WA	2,658	1:10	82.9%	2205	0.461%			1		100%				<del>                                     </del>		100%	0.3%
Yakima County	15.03	WA	4,558	1:10	82.9%	3781	0.513%					100%						100%	0.5%
Yakima County	15.04	WA	2,894	1:10	82.9%	2401	0.326%		<u></u>			100%			<u></u>			100%	0.3%
Yakima County	16.01	WA	2,537	1:15	80.0%	2031	0.276%					100%						100%	0.3%
Yakima County Yakima County	16.02 17.01	WA WA	8,633 3,654	1:15 1:10	80.0% 82.9%	6910 3031	0.938% 0.411%					100%						100% 100%	0.9%
Yakima County Yakima County	17.01	WA	6,565	1:10	82.9% 82.9%	5446	0.411%			1		100%						100%	0.4%
Yakima County	18.01	WA	4,419	40 Mins	96.5%	4264	0.579%			1		100%						100%	0.6%
Yakima County	18.02	WA	2,933	40 Mins	96.5%	2830	0.384%					100%						100%	0.4%
Yakima County	19.01	WA	3,680	40 Mins	96.5%	3551	0.482%					100%			<u></u>			100%	0.5%
Yakima County	19.02	WA	6,678	40 Mins	96.5%	6443	0.874%			1		100%				1		100%	0.9%
Yakima County Yakima County	20.03 20.04	WA WA	5,057 4,734	45 Mins 45 Mins	94.6% 94.6%	4783 4477	0.649% 0.608%					100%				+ +		100% 100%	0.6% 0.6%
Yakima County Yakima County	20.04	WA	2,544	45 Mins	94.6%	2406	0.808%			+		100%				<del>                                     </del>		100%	0.6%
a country	_0.00		-, •	.515	3/0		2.02.70				1		1	ı				200,0	5.576

										odiation Crarit	,							_	,
											LAYDOV	VN YARD #1 - L	OCAL TRAVEL ROU	TES					•
							To/Fro	m West			To/Fro	m North				To/From South		1	,
County	Census Tract	Tot State Popul		_		% of Total Population	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221	100% CHECK	Total
Yakima County	20.06	WA 6,9	34 45 Mins	94.6%	6558	0.890%					100%							100%	0.9%
Yakima County	21.01	WA 2,4	68 45 Mins	94.6%	2334	0.317%					100%							100%	0.3%
Yakima County	21.03	WA 2,7	09 45 Mins	94.6%	2562	0.348%					100%							100%	0.3%
Yakima County	21.04	WA 5,0	99 50 Mins	92.6%	4719	0.640%					100%							100%	0.6%
Yakima County	22.01	WA 5,1	53 55 Mins	90.4%	4658	0.632%					100%							100%	0.6%
Yakima County	22.02	WA 2,0	17 1:00	88.1%	1777	0.241%					100%							100%	0.2%
Yakima County	27.01	WA 3,4	66 40 Mins	96.5%	3344	0.454%	100%											100%	0.5%
Yakima County	28.01	WA 5,6	27 1:20	76.9%	4325	0.587%					100%							100%	0.6%
Yakima County	28.03	WA 5,8	09 1:20	76.9%	4465	0.606%					100%							100%	0.6%
Yakima County	28.04	WA 3,6	07 1:20	76.9%	2772	0.376%					100%							100%	0.4%
Yakima County	29	WA 7,1	31 1:30	69.4%	4950	0.672%					100%							100%	0.7%
Yakima County	30.02	WA 4,0	1:30	69.4%	2836	0.385%					100%							100%	0.4%
Yakima County	30.03	WA 1,7	24 greater than 2:	0.0%	0	0.000%					100%							100%	0.0%
Yakima County	30.04	WA 2,6	1:40	59.8%	1581	0.215%					100%							100%	0.2%
Yakima County	31	WA 5,2	97 1:15	80.0%	4240	0.575%					100%							100%	0.6%
Yakima County	32	WA 7,0	12 1:15	80.0%	5613	0.762%					100%							100%	0.8%
Yakima County	34	WA 5,2		80.0%	4185	0.568%					100%							100%	0.6%
Yakima County	9400.01	WA 6,5	1:10	82.9%	5420	0.736%					100%							100%	0.7%
Yakima County	9400.02	WA 4,7		88.1%	4195	0.569%					100%							100%	0.6%
Yakima County	9400.03	WA 3,2		80.0%	2635	0.358%					100%							100%	0.4%
Yakima County	9400.05	WA 4,7		88.1%	4207	0.571%					100%							100%	0.6%
Yakima County	9400.06	WA 4,7		88.1%	4192	0.569%					100%			·				100%	0.6%
Yakima County	9400.07	WA 3,4		82.9%	2861	0.388%					100%							100%	0.4%
Yakima County	9400.08	WA 2,1	49 1:10	82.9%	1783	0.242%					100%							100%	0.2%
		Total 919,	798		736,840	100.00%	0.55%	0.00%	0.00%	0.00%	52.26%	25.63%	4.66%	0.87%	16.03%	0.00%	0.00%	100%	100.00%
						Use	1%	0%	0%	0%	52%	25%	5%	1%	16%	0%	0%	100%	1

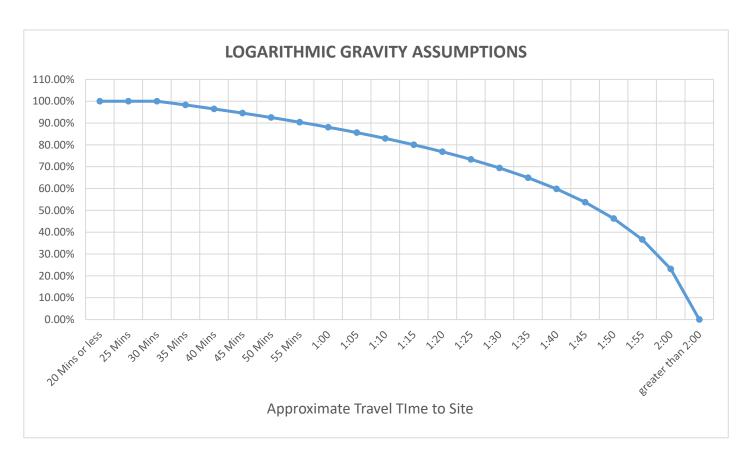
Part											<u> </u>	diation Gravity		/N YARD #2 - LC	CAL TRAVEL ROU	TES				
									To/From	West						-	To/From South			
Property   Property							Effective													
Part							-		Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
Section   Sect			ı						Senarus Roau	Noute 221	Road	3. Clouretter Road	1-02	Noute 333	3. Olympia street	Noute 337	Borer carryon Road 3.1 Tymoden Road	Node 221	4000/ 011501/	
Section   Sect		+	-									1000/								
Section   Sect																				_
September   1964   1965   1966   19																				_
Section   19.2																				_
Marchardy   18   18   19   19   19   19   19   19		103																	100%	0.8%
Marche   19	Benton County	104	WA 3,	637	30 mins	100.0%	3637	0.493%				100%							100%	0.5%
Martin   1977   1978   1978   1979																				
The section											4000/	100%								
Section   1970																				_
March   Marc											100%	100%								
Section   Sect											33%		33%							_
Peter Scored   1980																			100%	_
											33%		33%	<del></del>						
Sente County   1997   10   10   10   10   10   10   10   1											ļ	<del>                                     </del>	2501							_
Section   Sect									<b> </b>		<del> </del>		25%		+		<del>                                     </del>			
											1						+			_
Description   Colory   Color   Color									<del> </del>		1						<del>                                     </del>			
Section   Graph   Gr											<u> </u>	+								
Petrol County   10-21   10-22   10-2	Benton County	108.17	WA 6,	198	20 Mins or less	100.0%	6198	0.840%											100%	0.8%
												+								
Demon Courty   1931   W.   C.   S.   Demon Courty   1931   W.   C.   Demon Courty   1931   W.   Dem																				_
Sector County   150   100																				_
Remon County   15001   1002   100																				_
Decention Country   13,000   No.   4,450   20 Million Units   15,000   15													2570	50%						
Betton County   122.01   WA   4.287   228 Mins or less   10.00 K   4287   0.79 K		110.02			20 Mins or less	100.0%	4859					50%		50%					100%	0.7%
Retroa County   113   WA   5,78   5,700   100	Benton County	111	WA 7,	879	20 Mins or less	100.0%	7879	1.068%						50%	50%				100%	1.1%
Refers County   13   VA																				_
Betton County   1140   VA   3.580   20 Mms or less   10.0%   5.9%   5.0%   5.																				_
Betton County   13   13   13   14   13   13   14   14																				
Betted County   15   WA   5,443   25 mins   100%   0.88%																				
Retent County   115.05   WA   4.177   20 Mins or less   100.06   4177   0.5666   100%   50%   50%   50%   50%   50%   100%   10%   61867   100%   10																100%				_
Betton County   131-06   VA   7-319   20 Mins or less   1000%   7-319   1019%   100%	Benton County	115.04	WA 2,	866	20 Mins or less	100.0%	2866	0.388%							50%				100%	0.4%
Bertin County																				_
Benton County   117 02   WA   5.132   30 Mins   100 0%   5132   0.696%   100%										1000/				50%	50%					
Betton County   118.01   WA   3,555   35 mins   98.3%   3592   0.487%   100%   100%											<del> </del>						1			
Benton County   118 OZ   WA   2.665   40 mins   50.5%   2.571   0.349%   100%   100%   0.5%   100%   100%   0.5%   100%									+		1						<del>                                     </del>			
Benton County   119   WA   6,325   2.5 mins   100.0%   6325   0.857%   0.000%   100%   100%   0.96%   100%   100%   1.00%											İ									
Franklin County   201.01   WA   1.828   30 Mins   100.0%   1828   0.248%     100%			WA 6,	325	25 mins						100%								100%	0.9%
Franklin County   201.02   WA   6,699   30 Mins   100.0%   6699   0.989%     100%	· · · · · · · · · · · · · · · · · · ·										100%									
Franklin County   201.03   WA   3.811   30 Mins   100.0%   3811   0.517%	•										1						1			_
Franklin County   20.2.0.1   WA   2.2.01   30 Mins   100.0%   2201   0.298%     100%   100%   0.3%   100.0%   4.142   30 Mins   100.0%   6.088   0.325%   100.0%   100%											<del> </del>						1			
Franklin County   202.02   WA   4,142   30 Mins   100.0%   4142   0.561%											1				+		<del>                                     </del>			_
Franklin County   203   WA   6,088   30 Mins   100.0%   6088   0.825%	· · · · · · · · · · · · · · · · · · ·										1						<del>                                     </del>			_
Franklin County   204.02   WA   1,101   25 Mins   100.0%   1101   0.149%																				_
Franklin County         204.03         WA         3,611         25 Mins         100.0%         3611         0.489%           Franklin County         204.04         WA         2,928         25 Mins         100.0%         2928         0.397%         100%         0.7%           Franklin County         205.01         WA         5,161         30 Mins         100.0%         5161         0.700%         0.7%           Franklin County         205.03         WA         3,296         30 Mins         100.0%         50%         50%         50%         50%         100%         0.4%           Franklin County         205.03         WA         6,522         30 Mins         100.0%         6522         0.884%         50%         50%         50%         50%         50%         50%         50%         50%         60% </td <td></td>																				
Franklin County         204.04         WA         2,928         25 Mins         100.0%         2928         0.397%           Franklin County         205.01         WA         5,161         30 Mins         100.0%         5161         0.70%           Franklin County         205.03         WA         3,296         30 Mins         100.0%         3296         0.447%         50%         50%         50%         50%         100%         0.4%           Franklin County         205.03         WA         3,296         30 Mins         100.0%         3296         0.447%         50%         50%         50%         50%         50%         50%         60%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.4%         0.5%         0.5%         0.4%											ļ						1			_
Franklin County         205.01         WA         5,161         30 Mins         100.0%         5161         0.70%           Franklin County         205.03         WA         3,296         30 Mins         100.0%         3296         0.447%         50%         50%         50%         50%         100%         0.4%           Franklin County         205.04         WA         6,522         30 Mins         100.0%         6522         0.884%         50%         50%         50%         50%         50%         100%         0.9%           Franklin County         206.03         WA         4,546         30 Mins         100.0%         4546         0.616%         100%         50% <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td><b> </b></td> <td></td> <td><del> </del></td> <td></td> <td></td> <td></td> <td>+</td> <td></td> <td><del>                                     </del></td> <td></td> <td></td> <td></td>									<b> </b>		<del> </del>				+		<del>                                     </del>			
Franklin County         205.03         WA         3,296         30 Mins         100.0%         3296         0,447%           Franklin County         205.04         WA         6,522         30 Mins         100.0%         6522         0.884%         50%         50%         50%         50%         50%         0.9%           Franklin County         206.03         WA         4,546         30 Mins         100.0%         4546         0.616%         100%         0.6%           Franklin County         206.05         WA         9,548         30 Mins         100.0%         4546         0.616%         100%         0.6%           Franklin County         206.05         WA         9,548         30 Mins         100.0%         9548         1.29%         0.6%         50%         50%         50%         50%         100%         1.3%         1.3%         1.3%         100%         1.3%											1	100%		100%			+			
Franklin County         205.04         WA         6,522         30 Mins         100%         6.9%           Franklin County         206.03         WA         4,546         30 Mins         100.0%         4546         0.616%         100%         0.6%           Franklin County         206.05         WA         9,548         30 Mins         100.0%         9548         1.294%         0.6%         50%         50%         50%         50%         1.3%           Franklin County         206.05         WA         8,729         25 mins         100.0%         8729         1.183%         100%         1.2%           Franklin County         206.07         WA         6,719         35 Mins         98.3%         6604         0.895%         50%         50%         50%         50%         100%         0.9%           Franklin County         206.08         WA         6,719         35 Mins         98.3%         6604         0.895%         50%         50%         50%         50%         50%         100%         0.9%           Franklin County         206.08         WA         6,601         40 Mins         96.5%         6369         0.863%         50%         50%         50%         50%									<del> </del>		1			50%	1		<del>                                     </del>			
Franklin County         206.05         WA         9,548         30 Mins         100.0%         9548         1.294%           Franklin County         206.06         WA         8,729         25 mins         100.0%         8729         1.183%           Franklin County         206.07         WA         6,719         35 Mins         98.3%         6604         0.895%         50%         50%         50%         50%         100%         0.9%           Franklin County         206.08         WA         6,601         40 Mins         96.5%         6369         0.863%         50%         50%         50%         50%         50%         100%         0.9%           Morrow County         9701.01         OR         5,034         45 Mins         94.6%         4761         0.645%         60         0.645%         0.66%											<u> </u>						<u>                                       </u>			_
Franklin County         206.06         WA         8,729         25 mins         100%         1.2%           Franklin County         206.07         WA         6,719         35 Mins         98.3%         6604         0.895%         50%         50%         50%         100%         0.9%           Franklin County         206.08         WA         6,601         40 Mins         96.5%         6369         0.863%         50%         50%         50%         50%         100%         0.9%           Morrow County         9701.01         OR         5,034         45 Mins         94.6%         4761         0.645%         0.645%         0.6%         0.6%	Franklin County	206.03	WA 4,	546	30 Mins	100.0%	4546	0.616%											100%	0.6%
Franklin County         206.07         WA         6,719         35 Mins         98.3%         6604         0.895%           Franklin County         206.08         WA         6,601         40 Mins         96.5%         6369         0.863%         50%         50%         50%         50%         100%         0.9%           Morrow County         9701.01         OR         5,034         45 Mins         94.6%         4761         0.645%         0.6%         0.6%														50%						
Franklin County         206.08         WA         6,601         40 Mins         96.5%         6369         0.863%           Morrow County         9701.01         OR         5,034         45 Mins         94.6%         4761         0.645%											1			F00/			1			
Morrow County 9701.01 OR 5,034 45 Mins 94.6% 4761 0.645% 100% 100% 0.6%									+		+				+		+ +			
									+		1	30/0		30/0			100%			_
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												LAYDOV	VN YARD #2 - LC	OCAL TRAVEL ROUTE	S				]	
								To/Fro	m West			To/Fro	m North				To/From South			
					0/ B	Effective														
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221		1
County		State I	Population	Time to Site	Zone	Zone	Population			Noau									100% CHECK	Total
Morrow County	9702	OR	3,254	1:00	88.1%	2867	0.389%										100%		100%	0.4%
Umatilla County	9400	OR	3,072	1:00	88.1%	2706	0.367%										100%		100%	0.4%
Umatilla County	9501	OR	4,659	1:20	76.9%	3581	0.485%						25%				75%		100%	0.5%
Umatilla County Umatilla County	9502.01 9502.02	OR OR	3,712 4,043	1:20 1:20	76.9% 76.9%	2853 3108	0.387% 0.421%						25% 25%				75% 75%		100% 100%	0.4%
Umatilla County	9503	OR	3,371	1:10	82.9%	2796	0.421%						25/6				100%		100%	0.4%
Umatilla County	9504	OR	6,038	1:00	88.1%	5319	0.721%										100%		100%	0.7%
Umatilla County	9505	OR	4,754	1:00	88.1%	4188	0.568%										100%		100%	0.6%
Umatilla County	9506.01	OR	2,600	1:00	88.1%	2290	0.310%										100%		100%	0.3%
Umatilla County Umatilla County	9506.02 9507	OR OR	3,212 2,513	1:00 55 Mins	88.1% 90.4%	2830 2272	0.384%										100% 100%		100% 100%	0.4%
Umatilla County	9508	OR	7,508	25 Mins	100.0%	7508	1.018%										100%		100%	1.0%
Umatilla County	9509	OR	5,133	20 Mins or less	100.0%	5133	0.696%										100%		100%	0.7%
Umatilla County	9510	OR	6,224	25 Mins	100.0%	6224	0.844%										100%		100%	0.8%
Umatilla County	9511	OR	6,018	25 Mins	100.0%	6018	0.816%										100%		100%	0.8%
Umatilla County	9512.01	OR	6,207	30 Mins	100.0%	6207	0.841%									1	100%		100%	0.8%
Umatilla County Umatilla County	9512.02 9513	OR OR	4,040 3,989	30 Mins 30 Mins	100.0% 100.0%	4040 3989	0.548% 0.541%	<del> </del>			+			+		+	100% 100%		100% 100%	0.5% 0.5%
Umatilla County	9514	OR	2,416	1:10	82.9%	2004	0.341%									1	100%		100%	0.3%
Union County	9701	OR	3,238	2:00	23.1%	749	0.102%										100%		100%	0.1%
Union County	9702	OR	3,376	1:55	36.7%	1238	0.168%										100%		100%	0.2%
Union County	9703	OR	2,427	1:55	36.7%	890	0.121%										100%		100%	0.1%
Union County Union County	9704 9705	OR OR	2,732 3,196	1:50 1:35	46.3% 65.0%	1264 2076	0.171% 0.281%										100% 100%		100% 100%	0.2%
Union County	9706	OR	3,927	1:50	46.3%	1817	0.246%										100%		100%	0.3%
Union County	9707	OR	3,416	1:40	59.8%	2043	0.277%										100%		100%	0.3%
Union County	9708	OR	3,943	1:40	59.8%	2358	0.320%										100%		100%	0.3%
Adams County	9501	WA	2,577	1:30	69.4%	1789	0.242%						100%						100%	0.2%
Adams County Adams County	9502 9503.01	WA WA	1,794 1,790	1:20 1:20	76.9% 76.9%	1379 1376	0.187% 0.186%						100% 100%						100% 100%	0.2%
Adams County	9503.01	WA	2,738	1:20	76.9%	2104	0.186%				50%		50%						100%	0.2%
Adams County	9503.03	WA	2,555	1:15	80.0%	2045	0.277%				50%		50%						100%	0.3%
Adams County	9504	WA	3,100	1:15	80.0%	2481	0.336%				50%		50%						100%	0.3%
Adams County	9505	WA	5,799	1:15	80.0%	4642	0.629%				50%		50%						100%	0.6%
Columbia County	9602 207	WA	3,969 1,277	1:30 1:15	69.4% 80.0%	2755 1022	0.373% 0.139%						100% 100%						100% 100%	0.4%
Franklin County Franklin County	208.01	WA	3,401	1:00	88.1%	2996	0.139%						100%						100%	0.1%
Franklin County	208.02	WA	3,129	1:00	88.1%	2756	0.374%						100%						100%	0.4%
Grant County	101	WA	3,610	greater than 2:00	0.0%	0	0.000%						100%						100%	0.0%
Grant County	102	WA	3,382	greater than 2:00	0.0%	0	0.000%						100%						100%	0.0%
Grant County	103	WA	5,425	greater than 2:00	0.0%	0	0.000%						100%						100% 100%	0.0%
Grant County Grant County	104.01 104.02	WA WA	3,148 5,495	greater than 2:00 greater than 2:00	0.0%	0	0.000%						100% 100%						100%	0.0%
Grant County	104.02	WA	3,270	greater than 2:00	0.0%	0	0.000%	1		25%	25%		50%			1	†		100%	0.0%
Grant County	106	WA	7,614	greater than 2:00	0.0%	0	0.000%			25%	25%		50%		_				100%	0.0%
Grant County	107	WA	3,186	1:45	53.7%	1712	0.232%			25%	25%		50%						100%	0.2%
Grant County	108	WA	5,398	1:30	69.4%	3747	0.508%						100%			1			100%	0.5%
Grant County Grant County	109.01 109.03	WA WA	1,679 5,281	1:30 1:30	69.4% 69.4%	1165 3666	0.158% 0.497%	<del> </del>		1	+		100% 100%			+	+		100% 100%	0.2% 0.5%
Grant County	109.04	WA	6,136	1:30	69.4%	4259	0.577%						100%	+		†	†		100%	0.6%
Grant County	110.01	WA	5,723	1:30	69.4%	3973	0.538%						100%						100%	0.5%
Grant County	110.02	WA	6,225	1:30	69.4%	4321	0.586%						100%						100%	0.6%
Grant County	111.01	WA	4,657	1:30	69.4%	3233	0.438%						100%						100%	0.4%
Grant County Grant County	111.02 112	WA WA	2,891 7,100	1:30 1:45	69.4% 53.7%	2007 3814	0.272% 0.517%			25%	25%		100% 50%				-		100% 100%	0.3% 0.5%
Grant County	113	WA	3,367	1:20	76.9%	2588	0.317%			23/0	23/0		100%	+			+		100%	0.3%
Grant County	114.01	WA	2,249	1:20	76.9%	1729	0.234%				<u> </u>		100%				<u> </u>		100%	0.2%
Grant County	114.03	WA	4,871	1:15	80.0%	3899	0.528%			50%	50%								100%	0.5%
Grant County	114.04	WA	963	1:30	69.4%	668	0.091%			50%	50%			<u> </u>					100%	0.1%
Grant County	114.05 114.06	WA	3,019 3,185	1:30 1:30	69.4% 69.4%	2096 2211	0.284%			25% 25%	25% 25%		50% 50%	+		+			100% 100%	0.3%
Grant County Kittitas County	9751.01	WA WA	2,363	greater than 2:00	0.0%	0	0.300%		25%	50%	25%		30%			+			100%	0.3%
Kittitas County  Kittitas County	9751.02	WA	1,290	greater than 2:00	0.0%	0	0.000%	1	25%	50%	25%					1			100%	0.0%
Kittitas County	9751.03	WA	1,444	greater than 2:00	0.0%	0	0.000%		25%	50%	25%								100%	0.0%

												LAYDOV	VN YARD #2 - LC	CAL TRAVEL ROU	TES		7	
								To/Fron	n West			To/Fro	m North			To/From South	1	
						Effective												
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road Roue 221		
County		State I	opulation	Time to Site	Zone	Zone	Population			Noau							100% CHECK	Total
Kittitas County	9751.04	WA	1,644	greater than 2:00	0.0%	0	0.000%		25%	50%	25%						100%	0.0%
Kittitas County	9752.01	WA	3,364	greater than 2:00	0.0%	0	0.000%		25%	50%	25%						100%	0.0%
Kittitas County	9752.02	WA	1,395	1:45	53.7%	749	0.102%		25%	50%	25%						100%	0.1%
Kittitas County Kittitas County	9752.03 9753	WA WA	1,098 5,316	1:45 2:00	53.7% 23.1%	590 1230	0.080% 0.167%	+	25% 25%	50% 50%	25% 25%						100% 100%	0.1%
Kittitas County	9754.02	WA	4,713	1:45	53.7%	2532	0.343%		25%	50%	25%					<del> </del>	100%	0.2%
Kittitas County	9754.03	WA	2,921	1:45	53.7%	1569	0.213%		25%	50%	25%						100%	0.2%
Kittitas County	9754.04	WA	5,145	1:45	53.7%	2764	0.375%		25%	50%	25%						100%	0.4%
Kittitas County	9755 9756	WA WA	5,956 2,790	1:45 1:45	53.7% 53.7%	3200	0.434%		25% 25%	50% 50%	25% 25%						100% 100%	0.4%
Kittitas County Kittitas County	9757	WA	4,708	2:00	23.1%	1499 1089	0.203% 0.148%		25%	50%	25%						100%	0.2%
Klickitat County	9501.01	WA	1,538	1:00	88.1%	1355	0.184%	50%	50%	1	1000						100%	0.2%
Klickitat County	9501.02	WA	3,507	1:20	76.9%	2696	0.365%		75%	25%							100%	0.4%
Klickitat County	9501.03	WA	4,189	1:45	53.7%	2251	0.305%			+			-			100%	100%	0.3%
Klickitat County Klickitat County	9502 9503.01	WA WA	4,383 3,465	greater than 2:00 greater than 2:00	0.0%	0	0.000%									100%	100% 100%	0.0%
Klickitat County	9503.02	WA	5,396	greater than 2:00	0.0%	0	0.000%			1				+		100%	100%	0.0%
Walla Walla County	9200	WA	6,176	1:00	88.1%	5441	0.737%						100%				100%	0.7%
Walla Walla County	9201	WA	5,165	1:00	88.1%	4550	0.617%						75%			25%	100%	0.6%
Walla Walla County	9202	WA	4,715	1:15	80.0%	3774	0.512%			1			50%			50%	100%	0.5%
Walla Walla County Walla Walla County	9203.01 9203.02	WA WA	3,243 5,434	1:15 1:15	80.0% 80.0%	2596 4350	0.352% 0.590%			1			50% 50%			50%	100% 100%	0.4%
Walla Walla County	9204	WA	2,640	1:15	80.0%	2113	0.286%						50%			50%	100%	0.3%
Walla Walla County	9205	WA	2,959	1:15	80.0%	2368	0.321%						50%			50%	100%	0.3%
Walla Walla County	9206	WA	6,205	1:15	80.0%	4967	0.673%						50%			50%	100%	0.7%
Walla Walla County Walla Walla County	9207.01 9207.02	WA WA	3,545 4,293	1:15 1:15	80.0% 80.0%	2838 3436	0.385% 0.466%						50% 50%			50%	100% 100%	0.4% 0.5%
Walla Walla County	9208.01	WA	4,293	1:15	80.0%	3958	0.400%						50%			50%	100%	0.5%
Walla Walla County	9208.02	WA	3,223	1:15	80.0%	2580	0.350%						50%			50%	100%	0.3%
Walla Walla County	9209.01	WA	4,134	1:15	80.0%	3309	0.449%						50%			50%	100%	0.4%
Walla Walla County	9209.02	WA WA	5,491	1:15	80.0% 80.0%	4395	0.596%		F00/	F.00/			50%			50%	100% 100%	0.6%
Yakima County Yakima County	2	WA	3,072 5,595	1:15 1:15	80.0%	2459 4478	0.333% 0.607%		50% 50%	50% 50%							100%	0.3%
Yakima County	3.01	WA	2,473	1:15	80.0%	1979	0.268%		50%	50%							100%	0.3%
Yakima County	3.02	WA	2,283	1:15	80.0%	1827	0.248%		50%	50%							100%	0.2%
Yakima County	4.01	WA	5,958	1:15	80.0%	4769	0.646%		50%	50%							100%	0.6%
Yakima County Yakima County	4.02 5	WA WA	2,407 4,599	1:15 1:15	80.0% 80.0%	1927 3681	0.261% 0.499%		50% 50%	50% 50%							100% 100%	0.3% 0.5%
Yakima County	6	WA	5,696	1:15	80.0%	4559	0.499%		50%	50%						<del> </del>	100%	0.5%
Yakima County	7	WA	7,077	1:15	80.0%	5665	0.768%		50%	50%							100%	0.8%
Yakima County	8	WA	4,484	1:15	80.0%	3589	0.486%		50%	50%							100%	0.5%
Yakima County	9.02	WA	4,507	1:20	76.9%	3464	0.470%	<del>                                     </del>	50%	50%			<del> </del>	+		<del>                                     </del>	100%	0.5%
Yakima County Yakima County	9.03 9.04	WA WA	4,008 3,332	1:20 1:20	76.9% 76.9%	3081 2561	0.418% 0.347%		50% 50%	50% 50%				+		<del>                                     </del>	100% 100%	0.4%
Yakima County	10	WA	6,499	1:20	76.9%	4995	0.677%		50%	50%				<u>                                     </u>			100%	0.7%
Yakima County	11	WA	7,361	1:15	80.0%	5892	0.799%		50%	50%		·					100%	0.8%
Yakima County	12.01	WA	4,723	1:15	80.0%	3780	0.512%		50%	50%			-				100%	0.5%
Yakima County Yakima County	12.02 13	WA WA	7,051 2,653	1:15 1:10	80.0% 82.9%	5644 2201	0.765% 0.298%	+	50% 50%	50% 50%						<del>                                     </del>	100% 100%	0.8%
Yakima County	14	WA	4,099	1:10	82.9%	3400	0.461%	1	50%	50%			1				100%	0.5%
Yakima County	15.02	WA	2,658	1:10	82.9%	2205	0.299%		50%	50%							100%	0.3%
Yakima County	15.03	WA	4,558	1:10	82.9%	3781	0.512%		50%	50%							100%	0.5%
Yakima County Yakima County	15.04 16.01	WA WA	2,894 2,537	1:10 1:15	82.9% 80.0%	2401 2031	0.325% 0.275%		50% 50%	50% 50%							100% 100%	0.3%
Yakima County	16.01	WA	8,633	1:15	80.0%	6910	0.273%		50%	50%	+			+		<del>                                     </del>	100%	0.5%
Yakima County	17.01	WA	3,654	1:10	82.9%	3031	0.411%		25%	50%	25%			<u> </u>			100%	0.4%
Yakima County	17.02	WA	6,565	1:10	82.9%	5446	0.738%		50%	50%							100%	0.7%
Yakima County	18.01	WA	4,419	40 Mins	96.5%	4264	0.578%		50%	50%						<del>                                     </del>	100%	0.6%
Yakima County Yakima County	18.02 19.01	WA WA	2,933 3,680	40 Mins 40 Mins	96.5% 96.5%	2830 3551	0.384% 0.481%	+	75% 50%	25% 50%						<del>                                     </del>	100% 100%	0.4%
Yakima County	19.02	WA	6,678	40 Mins	96.5%	6443	0.873%	1	50%	50%							100%	0.9%
Yakima County	20.03	WA	5,057	45 Mins	94.6%	4783	0.648%		50%	50%							100%	0.6%
Yakima County	20.04	WA	4,734	45 Mins	94.6%	4477	0.607%		50%	50%							100%	0.6%
Yakima County	20.05	WA	2,544	45 Mins	94.6%	2406	0.326%		50%	50%							100%	0.3%

												LAYDOV	VN YARD #2 - LO	OCAL TRAVEL ROUT	TES			·		
								To/Fro	m West			To/Fro	m North				To/From South			
County	Census Tract	State P	Total opulation	Estimated Drive Time to Site	% Population in Drive Time Zone	Effective Population in Drive Time Zone	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221	100% CHECK	Total
Yakima County	20.06	WA	6,934	45 Mins	94.6%	6558	0.889%		50%	50%									100%	0.9%
Yakima County	21.01		2,468	45 Mins	94.6%	2334	0.316%		50%	50%									100%	0.3%
Yakima County	21.03	WA	2,709	45 Mins	94.6%	2562	0.347%		50%	50%									100%	0.3%
Yakima County	21.04	WA	5,099	50 Mins	92.6%	4719	0.640%		50%	50%									100%	0.6%
Yakima County	22.01	WA	5,153	55 Mins	90.4%	4658	0.631%		50%	50%									100%	0.6%
Yakima County	22.02	WA	2,017	1:00	88.1%	1777	0.241%		50%	50%									100%	0.2%
Yakima County	27.01	WA	3,466	40 Mins	96.5%	3344	0.453%	100%											100%	0.5%
Yakima County	28.01	WA	5,627	1:20	76.9%	4325	0.586%		50%	50%									100%	0.6%
Yakima County	28.03	WA	5,809	1:20	76.9%	4465	0.605%		50%	50%									100%	0.6%
Yakima County	28.04	WA	3,607	1:20	76.9%	2772	0.376%		50%	50%									100%	0.4%
Yakima County	29	WA	7,131	1:30	69.4%	4950	0.671%		50%	50%									100%	0.7%
Yakima County	30.02	WA	4,085	1:30	69.4%	2836	0.384%		50%	50%									100%	0.4%
Yakima County	30.03	WA	1,724	greater than 2:00	0.0%	0	0.000%		50%	50%									100%	0.0%
Yakima County	30.04	WA	2,644	1:40	59.8%	1581	0.214%		50%	50%									100%	0.2%
Yakima County	31	WA	5,297	1:15	80.0%	4240	0.575%		50%	50%									100%	0.6%
Yakima County	32	WA	7,012	1:15	80.0%	5613	0.761%		50%	50%									100%	0.8%
Yakima County	34		5,228	1:15	80.0%	4185	0.567%		50%	50%									100%	0.6%
Yakima County	9400.01	WA	6,534	1:10	82.9%	5420	0.735%		50%	50%									100%	0.7%
Yakima County	9400.02	WA	4,762	1:00	88.1%	4195	0.569%		75%	25%									100%	0.6%
Yakima County	9400.03	WA	3,292	1:15	80.0%	2635	0.357%		75%	25%									100%	0.4%
Yakima County	9400.05	WA	4,776	1:00	88.1%	4207	0.570%		75%	25%			-						100%	0.6%
Yakima County	9400.06	WA	4,758	1:00	88.1%	4192	0.568%		75%	25%									100%	0.6%
Yakima County	9400.07	WA	3,449	1:10	82.9%	2861	0.388%		75%	25%									100%	0.4%
Yakima County	9400.08		2,149	1:10	82.9%	1783	0.242%	0.55%	75%	25%	24.460/	4.570/	22.500/	2.700/	0.070/	0.000/	16.010/	0.000/	100%	0.2%
		Total	919,798			737,776	100.00%	0.55%	17.54%	17.09%	21.16%	1.57%	22.50%	2.70%	0.87%	0.00%	16.01%	0.00%	100%	100.00%
							Use	1%	18%	17%	21%	1%	22%	3%	1%	0%	16%	0%	100%	I

LOGARITHMIC GRA	VITY ASSUMPTIONS
Travel time to Site (approx.)	Adjustment Factor (0-100%)
20 Mins or less	100.00%
25 Mins	100.00%
30 Mins	100.00%
35 Mins	98.29%
40 Mins	96.48%
45 Mins	94.57%
50 Mins	92.55%
55 Mins	90.40%
1:00	88.09%
1:05	85.62%
1:10	82.95%
1:15	80.04%
1:20	76.86%
1:25	73.35%
1:30	69.41%
1:35	64.96%
1:40	59.81%
1:45	53.72%
1:50	46.28%
1:55	36.67%
2:00	23.14%
greater than 2:00	0.00%



# APPENDIX B: TRAFFIC COUNT DATA

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 46-55 12:15 AM 12:30 AM 41-50 12:45 AM 56-65 01:00 AM 51-60 01:15 AM 46-55 01:30 AM 51-60 01:45 AM 41-50 02:00 AM O 51-60 02:15 AM 1-10 02:30 AM 51-60 02:45 AM 51-60 03:00 AM 1-10 03:15 AM 1-10 03:30 AM 36-45 03:45 AM 1-10 04:00 AM 41-50 04:15 AM 46-55 04:30 AM O 51-60 04:45 AM 51-60 05:00 AM 46-55 05:15 AM 46-55 05:30 AM 46-55 05:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 41-50 06:15 AM 06:30 AM 46-55 06:45 AM 46-55 07:00 AM 46-55 07:15 AM 51-60 46-55 07:30 AM 07:45 AM 46-55 08:00 AM O 51-60 08:15 AM 46-55 08:30 AM 51-60 08:45 AM 51-60 09:00 AM 46-55 09:15 AM 46-55 09:30 AM 46-55 09:45 AM 51-60 10:00 AM 46-55 10:15 AM 46-55 10:30 AM O 51-60 10:45 AM 46-55 11:00 AM 51-60 11:15 AM 46-55 11:30 AM 51-60 11:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 46-55 12:15 PM 51-60 12:30 PM 51-60 12:45 PM 51-60 01:00 PM 46-55 01:15 PM 46-55 01:30 PM 51-60 01:45 PM 46-55 02:00 PM O 51-60 02:15 PM 46-55 02:30 PM 51-60 02:45 PM 51-60 03:00 PM 46-55 03:15 PM 51-60 03:30 PM 46-55 03:45 PM 46-55 04:00 PM 46-55 04:15 PM 50-59 04:30 PM O 46-55 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 51-60 05:30 PM 46-55 05:45 PM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd

QC JOB #: 16236659

SPECIFIC LOCATION:

DIRECTION: EB

CITY/STATE: Benton, WA

	Benton,	WA														DATE: Jun	13 202
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
start rille	15	20	25	30	35	40	45	50	55	60	65	70	75	999	IOLAI	Pace Speed	in Pac
06:00 PM	0	0	0	0	0	0	1	1	3	3	1	0	0	0	9	51-60	6
06:15 PM	0	0	0	0	0	0	1	0	3	2	0	0	0	1	7	51-60	5
06:30 PM	0	0	0	0	0	0	0	0	1	4	0	0	0	0	5	51-60	5
06:45 PM	0	0	0	0	0	0	0	3	3	2	2	0	0	0	10	46-55	6
07:00 PM	0	0	0	0	0	0	0	1	0	2	1	0	1	0	5	56-65	3
07:15 PM	0	0	0	0	0	0	1	1	3	2	0	0	0	0	7	51-60	5
07:30 PM	0	0	0	0	0	0	0	2	0	3	3	0	0	0	8	56-65	6
07:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:00 PM	0	0	0	0	0	0	0	1	1	0	1	0	0	0	3	46-55	2
08:15 PM	0	0	0	0	0	0	0	3	2	3	0	0	0	0	8	46-55	5
08:30 PM	0	0	0	0	0	0	0	1	3	0	1	0	0	0	5	46-55	4
08:45 PM	0	0	0	0	0	0	0	1	3	0	0	0	0	0	4	46-55	4
09:00 PM	0	0	0	0	0	0	0	0	1	1	0	1	0	0	3	51-60	2
09:15 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
10:00 PM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	2	46-55	1
10:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
10:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
10:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:00 PM	0	0	0	0	0	0	0	1	<b>1</b>	0	0	0	0	0	2	46-55	2
11:15 PM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
11:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	0	0	0	0	4	25	136	209	129	46	11	5	1	566	46.55	245
Percent	0%	0%	0%	0%	0%	0.7%	4.4%	24%	36.9%	22.8%	8.1%	1.9%	0.9%	0.2%	566	46-55	345
AM Peak					12:00 AM	5:45 AM	6:15 AM	6:15 AM	8:45 AM	7:15 AM		11:45 AM			5:45 AM		
15-min Vol	0	0	0	0	0	1	3	6	5	5	2	2	1	0	12		
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	2:30 PM 1	12:00 PM 1	3:30 PM 7	3:15 PM 9	5:45 PM 6	5:00 PM 4	3:00 PM 2	1:45 PM 1	6:15 PM 1	3:15 PM 19		

,, ,																	
OCATION: Sel	llards Rd	btwn Rte	221 and	Tyack Rd												QC JOB	#: 162366!
PECIFIC LOCA	TION:															DI	RECTION:
ITY/STATE: Be	enton, W	/A														DATE:	Jun 13 20
	1	16	21	26	31	36	41	46	51	56	61	66	71	76			Number
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	Pace
Grand Total	0	0	0	0	0	4	25	136	209	129	46	11	5	1	566	46.55	245
Percent	0%	0%	0%	0%	0%	0.7%	4.4%	24%	36.9%	22.8%	8.1%	1.9%	0.9%	0.2%	500	46-55	345
Cumulative Percent	0%	0%	0%	0%	0%	0.7%	5.1%	29.2%	66.1%	88.9%	97%	98.9%	99.8%	100%			
																85th Percer	
ADT															Me	an Speed(Avera	i <b>ge):</b> 52 MI
566																Med	lian: 52 MI
																М	ode: 53 MI

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: EB

**DATE**: Jun 13 2023

CITY/STATE: Be	nton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
01:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
01:15 AM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2
01:30 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
01:45 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
02:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:15 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
04:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:45 AM	0	2	0	1	0	0	0	1	0	0	0	0	0	0	4
05:00 AM	0	6	1	1	0	0	0	0	0	0	0	0	0	0	8
05:15 AM	0	3	2	0	1	0	0	1	0	0	0	0	0	0	7
05:30 AM	0	1	0	0	1	0	0	2	0	0	0	0	0	0	4
05:45 AM	0	7	3	0	0	0	0	2	0	0	0	0	0	0	12
Day Total															
Percent				DATA	IHA	ALD.	RIVE	500	MM	UNIT	11-5				
ADT															
566															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
omments:															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236659 DIRECTION: EB
DATE: Jun 13 2023

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 AM	0	4	1	0	0	0	0	1	0	0	0	0	0	0	6
06:15 AM	0	7	0	1	1	0	0	1	0	0	0	0	0	0	10
06:30 AM	0	4	0	0	1	0	0	1	0	0	0	0	0	0	6
06:45 AM	0	3	2	0	0	0	0	0	1	0	0	0	0	0	6
07:00 AM	0	4	3	0	2	0	0	1	0	0	0	0	0	0	10
07:15 AM	0	3	1	1	0	0	0	3	0	0	0	0	0	0	8
07:30 AM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
07:45 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
08:00 AM	0	2	1	1	0	0	0	2	0	0	0	0	0	0	6
08:15 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
08:30 AM	0	1	1	0	1	0	0	1	0	0	0	0	0	0	4
08:45 AM	0	4	2	1	0	0	0	2	0	0	0	0	0	0	9
09:00 AM	0	4	2	2	0	0	0	2	0	0	0	0	0	0	10
09:15 AM	0	3	1	1	0	0	0	3	0	0	1	0	0	0	9
09:30 AM	0	2	2	0	2	0	0	0	0	0	0	0	0	0	6
09:45 AM	0	4	0	1	0	0	0	4	0	0	0	0	0	0	9
10:00 AM	0	3	3	1	1	0	0	1	0	0	0	0	0	0	9
10:15 AM	0	3	1	1	1	0	0	1	0	0	0	0	0	0	7
10:30 AM	0	4	0	0	0	0	0	3	0	0	0	0	0	0	7
10:45 AM	0	1	1	1	0	0	0	1	0	0	0	0	0	0	4
11:00 AM	0	1	3	0	1	0	0	1	0	0	0	0	0	0	6
11:15 AM	0	2	1	1	0	0	0	0	0	0	0	0	0	0	4
11:30 AM	0	3	2	1	0	0	0	0	0	0	0	0	0	0	6
11:45 AM	0	6	2	0	1	0	0	3	0	0	0	0	0	0	12
Day Total Percent															
refeelt				JAIL	4 11 12		NIVE	3.00	/IVIIVI	UIVIII	1155				
ADT 566															
AM Peak 15-min Vol															
PM Peak 15-min Vol															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236659 DIRECTION: EB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA	6. 0	241		24	24 '	4.6.1	.e ^ 1	<b>.</b>		.c. 1	C A .			un 13 202
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 PM	0	4	4	1	0	0	0	2	0	0	0	0	0	0	11
12:15 PM	0	3	0	1	1	0	0	1	0	0	0	0	0	0	6
12:30 PM	0	4	2	0	0	0	0	1	0	0	0	0	0	0	7
12:45 PM	0	3	2	1	0	0	0	1	1	0	0	0	0	0	8
01:00 PM	0	5	2	0	1	0	0	2	0	0	0	0	0	0	10
01:15 PM	0	2	1	1	0	0	0	2	0	0	0	0	0	0	6
01:30 PM	0	1	2	0	1	0	0	3	0	0	0	0	0	0	7
01:45 PM	0	3	5	0	0	0	0	0	0	0	0	0	0	0	8
02:00 PM	0	3	2	0	1	0	0	1	0	0	0	0	0	0	7
02:15 PM	0	6	1	1	2	0	0	2	0	0	0	0	0	0	12
02:30 PM	0	6	5	0	0	0	0	1	0	0	0	0	0	0	12
02:45 PM	0	5	2	1	0	0	0	3	1	0	0	0	0	0	12
03:00 PM	0	1	2	0	0	0	0	4	0	0	0	0	0	0	7
03:15 PM	0	6	8	0	1	0	0	4	0	0	0	0	0	0	19
03:30 PM	0	4	3	2	1	0	0	2	0	0	0	0	0	0	12
03:45 PM	0	5	2	0	0	0	0	3	0	0	0	0	0	0	10
04:00 PM	0	7	4	2	0	0	0	1	0	0	0	0	0	0	14
04:15 PM	0	6	2	1	0	0	0	2	0	0	0	0	0	0	11
04:30 PM	0	10	1	0	0	0	0	4	0	0	0	0	0	0	15
04:45 PM	0	9	2	0	1	0	0	0	0	0	0	0	0	0	12
05:00 PM	0	8	1	0	1	0	0	5	0	0	1_1_	0	0	0	16
05:15 PM	0	5	4	0	1	0	0	2	0	0	0	0	0	0	12
05:30 PM	0	6	1	0	1	0	0	4	0	0	0	0	0	0	12
05:45 PM	0	4	9	0	0	0	0	2	0	0	0	0	0	0	15
Day Total Percent															
							XIVI.		/1V11V1						
ADT 566															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	C/20/2/	222 7 50 484									COLIDEE O	1:4	/	//	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: EB

**DATE:** Jun 13 2023

	enton, WA	C 0	2 4-4-		2 4-4- 6	2 A.d.	4 4	4E A!	E A.da	> C A!	4C A!	C A.de	> C A!		un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	0	3	0	0	0	0	6	0	0	0	0	0	0	9
06:15 PM	0	3	1	1	0	0	0	2	0	0	0	0	0	0	7
06:30 PM	0	1	0	0	1	0	0	3	0	0	0	0	0	0	5
06:45 PM	0	5	0	0	2	0	0	3	0	0	0	0	0	0	10
07:00 PM	0	1	4	0	0	0	0	0	0	0	0	0	0	0	5
07:15 PM	0	3	2	0	0	0	0	2	0	0	0	0	0	0	7
07:30 PM	0	4	0	1	1	0	0	2	0	0	0	0	0	0	8
07:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
08:00 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
08:15 PM	0	5	1	0	2	0	0	0	0	0	0	0	0	0	8
08:30 PM	0	2	1	0	0	0	0	2	0	0	0	0	0	0	5
08:45 PM	0	1	0	0	0	0	0	3	0	0	0	0	0	0	4
09:00 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
09:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
10:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
10:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
11:00 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
11:15 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
11:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	249	121	29	32	0	0	127	4	0	4	0	0	0	566
Percent	0%	44%	21.4%	5.1%	5.7%	0%	0%	22.4%	0.7%	0%	0.7%	0%	0%	0%	300
ADT 566															
AM Peak	12:00 AM	5:45 AM	5:45 AM	9:00 AM	7:00 AM	12:00 AM	12:00 AM	9:45 AM	1:15 AM	12:00 AM	12:45 AM	12:00 AM	12:00 AM	12:00 AM	5:45 AM
15-min Vol	0	7	3	2	2	0	0	4	1	0	1	0	0	0	12
PM Peak	12:00 PM	4:30 PM	5:45 PM	3:30 PM	2:15 PM	12:00 PM	12:00 PM	6:00 PM	12:45 PM	12:00 PM	5:00 PM	12:00 PM	12:00 PM	12:00 PM	3:15 PM
15-min Vol	0	10	9	2	2	0	0	6	1	0	1	0	0	0	19
Comments:															

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd

SPECIFIC LOCATION:

DIRECTION: EB

CITY/STATE: Benton, WA

OC JOB #: 16236659

DIRECTION: EB

DATE: Jun 13 2023

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Grand Total Percent	0 0%	Trailers 249 44%	121 21.4%	29 5.1%	32 5.7%	Single 0 0%	Single 0 0%	127 22.4%	4 0.7%	O 0 0%	<b>Multi</b> 4 0.7%	Multi 0 0%	Multi 0 0%	Classed 0 0%	566
ADT 566															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
Start Time		13 Jun 23				15-min Traffic			15-min Traffic	Average week Frome
12:00 AM		0				0			0	
12:15 AM		1				1			1	
12:30 AM		1				1			1	
12:45 AM		1				1			1	
01:00 AM		1				1			1	
01:15 AM		2				2			2	
01:30 AM		2				2			2	
01:45 AM		2				2			2	
02:00 AM		1				1			1	
02:15 AM		0				0			0	
02:30 AM		1				1			1	
02:45 AM		1				1			1	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		0				0			0	
04:00 AM		1				1			1	
04:15 AM		2				2			2	
04:30 AM		1				1			1	
04:45 AM		4			261	4			4	
05:00 AM		8				8			8	
05:15 AM		7				7 4 5	01/11/		7	
05:30 AM		4			HALL	4	DIVIN		4	
05:45 AM		12				12			12	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:	<u>-</u>	<del></del>	<u>-</u>	<u></u>						

QC JOB #: 16236659 DIRECTION: EB

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DIRECTION: EB

DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236659

Start Time	Mon	Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		6				6			6	
06:15 AM		10				10			10	
06:30 AM		6				6			6	
06:45 AM		6				6			6	
07:00 AM		10				10			10	
07:15 AM		8				8			8	
07:30 AM		4				4			4	
07:45 AM		2				2			2	
08:00 AM		6				6			6	
08:15 AM		3				3			3	
08:30 AM		4				4			4	
08:45 AM		9				9			9	
09:00 AM		10				10			10	
09:15 AM		9				9			9	
09:30 AM		6				6			6	
09:45 AM		9				9			9	
10:00 AM		9				9			9	
10:15 AM		7				7			7	
10:30 AM		7			211	7			7	
10:45 AM		4			all	4			4	
11:00 AM		6				6			6	
11:15 AM		4			TIATI	4	00.00.0		4	
11:30 AM		6			HALL	6	DIVIIVI		6	
11:45 AM		12				12			12	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236659

DIRECTION: EB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		11				11			11	
12:15 PM		6				6			6	
12:30 PM		7				7			7	
12:45 PM		8				8			8	
01:00 PM		10				10			10	
01:15 PM		6				6			6	
01:30 PM		7				7			7	
01:45 PM		8				8			8	
02:00 PM		7				7			7	
02:15 PM		12				12			12	
02:30 PM		12				12			12	
02:45 PM		12				12			12	
03:00 PM		7				7			7	
03:15 PM		19				19			19	
03:30 PM		12				12			12	
03:45 PM		10				10			10	
04:00 PM		14				14			14	
04:15 PM		11				11			11	
04:30 PM		15				15			15	
04:45 PM		12				12			12	
05:00 PM		16				16			16	
05:15 PM		12				12	00.00.0		12	
05:30 PM		12				12	JIVIIVI		12	
05:45 PM		15				15			15	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

**DIRECTION:** EB **DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236659

Start Time	13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	9				9			9	
06:15 PM	7				7			7	
06:30 PM	5				5			5	
06:45 PM	10				10			10	
07:00 PM	5				5			5	
07:15 PM	7				7			7	
07:30 PM	8				8			8	
07:45 PM	1				1			1	
08:00 PM	3				3			3	
08:15 PM	8				8			8	
08:30 PM	5				5			5	
08:45 PM	4				4			4	
09:00 PM	3				3			3	
09:15 PM	2				2			2	
09:30 PM	0				0			0	
09:45 PM	2				2			2	
10:00 PM	2				2			2	
10:15 PM	1				1			1	
10:30 PM	1				1			1	
10:45 PM	1				1			1	
11:00 PM	2				2			2	
11:15 PM	2				$\frac{2}{1}$ C	00 00 0		2	
11:30 PM	1				JKIVES CO	JIVIIVI		1 1	
11:45 PM	0				0			0	
Day Total	566				566			566	
% Weekday Average	100%								
% Week Average	100%				100%				
AM Peak	5:45 AM				5:45 AM			5:45 AM	
15-min Vol	12				12			12	
PM Peak	3:15 PM				3:15 PM			3:15 PM	
15-min Vol	19				19			19	

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total **Pace Speed** in Pace 12:00 AM 1-10 46-55 12:15 AM 12:30 AM 41-50 12:45 AM 46-55 01:00 AM 51-60 01:15 AM 46-55 48-57 01:30 AM 01:45 AM 41-50 02:00 AM O 51-60 02:15 AM 46-55 02:30 AM 51-60 02:45 AM 51-60 03:00 AM 41-50 03:15 AM 1-10 03:30 AM 41-50 03:45 AM 56-65 41-50 04:00 AM 04:15 AM 53-62 04:30 AM O 41-50 04:45 AM 46-55 05:00 AM 46-55 05:15 AM 46-55 05:30 AM 46-55 05:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 06:15 AM 46-55 06:30 AM 46-55 06:45 AM 41-50 07:00 AM 46-55 07:15 AM 51-60 07:30 AM 46-55 07:45 AM 48-57 08:00 AM O 46-55 08:15 AM 51-60 08:30 AM 50-59 08:45 AM 47-56 09:00 AM 46-55 09:15 AM 46-55 09:30 AM 46-55 09:45 AM 51-60 10:00 AM 46-55 10:15 AM 46-55 10:30 AM O 51-60 10:45 AM 46-55 11:00 AM 51-60 41-50 11:15 AM 11:30 AM 46-55 11:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd SPECIFIC LOCATION: **DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 46-55 12:15 PM 51-60 12:30 PM 51-60 12:45 PM 51-60 01:00 PM 46-55 01:15 PM 46-55 01:30 PM 51-60 01:45 PM 50-59 02:00 PM O 51-60 02:15 PM 46-55 02:30 PM 46-55 02:45 PM 51-60 03:00 PM 46-55 03:15 PM 51-60 03:30 PM 41-50 03:45 PM 46-55 04:00 PM 46-55 04:15 PM 46-55 04:30 PM O 46-55 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 51-60 05:30 PM 46-55 05:45 PM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236659 DIRECTION: EB, WB

Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
otare rillic	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	r dee Speed	in Pac
06:00 PM	0	0	0	0	0	0	1	3	3	3	1	1	0	0	12	46-55	6
06:15 PM	0	0	0	0	0	0	2	3	6	2	1	0	0	1	15	46-55	9
06:30 PM	0	0	0	0	0	0	0	2	4	4	0	1	0	0	11	51-60	8
06:45 PM	0	0	0	0	0	0	2	4	5	3	2	1	0	0	17	46-55	9
07:00 PM	0	0	0	0	0	0	0	2	1	2	2	0	1	0	8	56-65	4
07:15 PM	0	0	0	0	0	1	2	2	3	3	0	0	0	0	11	51-60	6
07:30 PM	0	0	0	0	0	0	1	3	1	3	3	0	0	0	11	56-65	6
07:45 PM	0	0	0	0	0	0	0	1	2	0	0	0	0	0	3	46-55	3
08:00 PM	0	0	0	0	0	0	0	2	1	2	1	0	0	0	6	46-55	3
08:15 PM	0	0	0	0	0	0	0	3	3	3	0	0	0	0	9	46-55	6
08:30 PM	0	0	0	0	0	0	0	1	3	1	1	0	0	0	6	48-57	4
08:45 PM	0	0	0	0	0	0	0	3	4	0	0	0	0	0	7	46-55	7
09:00 PM	0	0	0	0	1	0	0	0	1	1	0	1	0	0	4	51-60	2
09:15 PM	0	0	0	0	0	0	0	3	2	0	0	0	0	0	5	46-55	5
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	3	1	0	0	0	0	0	4	46-55	4
10:00 PM	0	0	0	0	0	0	0	0	2	0	0	1	0	0	3	46-55	2
10:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
10:30 PM	0	0	0	0	0	0	0	1	3	0	0	0	0	0	4	46-55	4
10:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
11:00 PM	0	0	0	0	0	0	0	1	<b>1</b>	0	0	0	0	0	2	46-55	2
11:15 PM	0	0	0	0	0	0	0	2	1	0	1	0	0	0	4	46-55	3
11:30 PM	0	0	0	0	0	0	0	2	0	2	0	0	0	0	4	41-50	2
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	0	0	0	1	7	67	285	391	209	63	18	6	1	1048	46-55	676
Percent	0%	0%	0%	0%	0.1%	0.7%	6.4%	27.2%	37.3%	19.9%	6%	1.7%	0.6%	0.1%	1046	40-33	070
AM Peak		12:00 AM						11:45 AM	7:30 AM	7:15 AM	9:30 AM	5:30 AM		12:00 AM	11:45 AM		
15-min Vol	0	0	0	0	0	1	4	11	9	8	3	2	1	0	23		
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	9:00 PM 1	2:30 PM 1	3:30 PM 5	3:30 PM 10	3:15 PM 15	4:45 PM 9	5:00 PM 4	3:00 PM 2	1:45 PM 1	6:15 PM 1	3:15 PM 27		

Type of report: Tube Count - Speed Data

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Sel		btwn Rte	221 and	Tyack Rd													#: 16236659
SPECIFIC LOCA <sup>.</sup> CITY/STATE: Be		/A															<b>ION</b> : EB, WE Jun 13 2023
Speed Range	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
Grand Total Percent	0 0%	0 0%	0 0%	0 0%	1 0.1%	7 0.7%	67 6.4%	285 27.2%	391 37.3%	209 19.9%	63 6%	18 1.7%	6 0.6%	1 0.1%	1048	46-55	676
Cumulative Percent	0%	0%	0%	0%	0.1%	0.8%	7.2%	34.4%	71.7%	91.6%	97.6%	99.3%	99.9%	100%			
ADT 1048															Me	an Speed(Avera	ntile: 58 MPI age): 52 MPI dian: 52 MPI ode: 53 MPI
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236659 DIRECTION: EB, WB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
12:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:45 AM	0	1	0	0	0	0	0	0	0	0	1	0	0	0	2
01:00 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
01:15 AM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2
01:30 AM	0	3	0	1	0	0	0	1	0	0	0	0	0	0	5
01:45 AM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
02:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
02:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
02:30 AM	0	1	0	0	0	0	0	1	0	0	0	1	0	0	3
02:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
03:45 AM	0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
04:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:15 AM	0	2	1	0	0	0	0	3	0	0	0	0	0	0	6
04:30 AM	0	2	1	0	0	0	0	0	0	1	0	0	0	0	4
04:45 AM	0	3	3	1	1	0	0	3	0	0	0	0	0	0	11
05:00 AM	0	9	2	1	0	0	00	0	0	0	0	0	0	0	12
05:15 AM	0	5	4	0	1	0	0	1	0	0	0	0	0	0	11
05:30 AM	0	5	3	1	1	0	0	2	0	0	0	0	0	0	12
05:45 AM	0	8	3	0	0	0	0	2	0	0	0	0	0	0	13
Day Total				DAT	A TOTAL I	A T .	ESD ZE		11/1/1	LIMIT	-1				
Percent				DAIA	A I H	AL D	RIVE	500	IIVIIVI	UNII	IES				
ADT 1048															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	L C /20 /20	222 7 50 484			•					•	COLUDER O	l'ann Carran	110/11	. / /	4

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: EB, WB

CITY/STATE: Be	IIIOII, WA	6. 0	241		2416	2.4.1	4.6.1	.e	5 A 1		.c. 4. 1	C A .			un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
06:00 AM	0	5	4	0	1	0	0	3	0	0	0	0	0	0	13
06:15 AM	0	9	3	1	1	0	0	2	0	0	0	0	0	0	16
06:30 AM	0	4	2	0	1	0	0	3	0	0	0	0	0	0	10
06:45 AM	0	5	4	3	1	0	0	2	1	0	0	0	0	0	16
07:00 AM	0	7	5	1	3	0	0	3	0	0	0	0	0	0	19
07:15 AM	0	7	6	2	0	0	0	4	0	0	0	0	0	0	19
07:30 AM	0	6	4	0	0	0	0	4	0	0	0	0	0	0	14
07:45 AM	0	3	2	0	2	0	0	0	0	0	0	0	0	0	7
08:00 AM	0	3	4	1	0	0	0	2	0	0	0	0	0	0	10
08:15 AM	0	6	1	0	0	0	0	5	0	0	0	0	0	0	12
08:30 AM	0	2	3	0	2	0	0	1	0	0	0	0	0	0	8
08:45 AM	0	5	2	1	1	0	0	3	0	0	0	0	0	0	12
09:00 AM	0	8	4	3	0	0	0	2	1	0	0	0	0	0	18
09:15 AM	0	4	2	2	0	0	0	5	0	0	1	0	0	0	14
09:30 AM	0	5	7	0	2	0	0	3	0	0	0	0	0	0	17
09:45 AM	0	6	1	3	1	0	0	7	1	0	0	0	0	0	19
10:00 AM	0	7	4	2	1	0	0	4	0	0	0	0	0	0	18
10:15 AM	0	7	2	2	1	0	0	4	0	0	0	0	0	0	16
10:30 AM	0	9	1	1	0	0	0	5	0	0	0	0	0	0	16
10:45 AM	0	3	3	2	1	0	0	4	0	0	0	0	0	0	13
11:00 AM	0	3	4	0	1 1	0	0	4	0	0	0	0	0	0	12
11:15 AM	0	3	5	1		0	0	1	0	0	0	0	0	0	11
11:30 AM	0	8	4	1	0	0	0	2	0	0	0	0	0	0	15
11:45 AM	0	12	4	0	1	0	0	4	2	0	0	0	0	0	23
Day Total															
Percent				$)\Delta I \lambda$	AIHA		RIVE	5 ((	IMM	UNIT	11-5				
ADT															
1048															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
omments.															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659

DIRECTION: EB, WB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	0	8	5	1	0	0	0	4	0	0	0	0	0	0	18
12:15 PM	0	9	3	3	1	0	0	1	0	0	0	0	0	0	17
12:30 PM	0	5	4	0	0	0	0	2	0	0	0	0	0	0	11
12:45 PM	0	6	2	2	0	0	0	2	1	0	0	0	0	0	13
01:00 PM	0	6	3	1	1	0	0	5	0	0	0	0	0	0	16
01:15 PM	0	8	2	2	1	0	0	2	0	0	0	0	0	0	15
01:30 PM	0	3	2	0	1	0	0	6	0	0	0	0	0	0	12
01:45 PM	0	8	6	0	0	0	0	2	0	0	0	0	0	0	16
02:00 PM	0	7	2	1	1	0	0	5	1	0	0	0	0	0	17
02:15 PM	0	7	2	1	2	0	0	3	1	0	0	0	0	0	16
02:30 PM	0	10	5	1	2	0	0	3	1	0	0	0	0	0	22
02:45 PM	0	7	4	1	0	0	0	3	1	0	0	0	0	0	16
03:00 PM	0	5	2	0	0	0	0	5	0	0	0	0	0	0	12
03:15 PM	0	8	11	0	2	0	0	6	0	0	0	0	0	0	27
03:30 PM	0	8	4	5	1	0	0	3	1	0	0	0	0	0	22
03:45 PM	0	14	3	0	1	0	0	3	0	0	0	0	0	0	21
04:00 PM	0	12	5	2	2	0	0	2	0	0	0	0	0	0	23
04:15 PM	0	9	3	3	0	0	0	2	0	0	0	0	0	0	17
04:30 PM	0	16	2	0	0	0	0	4	0	0	0	0	0	0	22
04:45 PM	0	16	4	0	1	0	0	2	0	0	0	0	0	0	23
05:00 PM	0	12	2	1	1	0	0	8	0	0	1_1_	0	0	0	25
05:15 PM	0	17	4	1	1	0	0	3	0	0	0	0	0	0	26
05:30 PM	0	10	2	0	1	0	0	6	0	0	0	0	0	0	19
05:45 PM	0	11	10	0	1	0	0	3	0	0	0	0	0	0	25
Day Total															
Percent				DATA	AIHA		RIVF	500	MM	UNIT	1-5				
4.0.7															
ADT 1048															
1048															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
concrt ganaratad	I an 6/20/20	123 7·50 VV									SOLIBCE: Or	uality Count	c IIC (http:	الحييم بممممال	tycounts not

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: EB, WB

-	nton, WA	C 0	2 4 1		2 4.1. 6	2 4 1	4 4 1 -	4F A I			-C A I	CAL			un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	1	4	0	0	0	0	7	0	0	0	0	0	0	12
06:15 PM	0	7	2	1	0	0	0	5	0	0	0	0	0	0	15
06:30 PM	0	4	2	0	1	0	0	4	0	0	0	0	0	0	11
06:45 PM	0	9	2	0	3	0	0	3	0	0	0	0	0	0	17
07:00 PM	0	4	4	0	0	0	0	0	0	0	0	0	0	0	8
07:15 PM	0	7	2	0	0	0	0	2	0	0	0	0	0	0	11
07:30 PM	0	5	1	1	1	0	0	3	0	0	0	0	0	0	11
07:45 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
08:00 PM	0	4	0	0	0	0	0	2	0	0	0	0	0	0	6
08:15 PM	0	6	1	0	2	0	0	0	0	0	0	0	0	0	9
08:30 PM	0	2	1	0	0	0	0	3	0	0	0	0	0	0	6
08:45 PM	0	2	1	0	1	0	0	3	0	0	0	0	0	0	7
09:00 PM	0	3	0	0	0	0	0	1	0	0	0	0	0	0	4
09:15 PM	0	4	0	0	0	0	0	1	0	0	0	0	0	0	5
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	1	0	0	0	0	0	3	0	0	0	0	0	0	4
10:00 PM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
10:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	1	2	0	0	0	0	0	0	0	1	0	0	0	4
10:45 PM	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2
11:00 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
11:15 PM	0	1	0	0	0	0	0	3	0	0	0	0	0	0	4
11:30 PM	0	1	1	0	0	0	0	2	0	0	0	0	0	0	4
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	475	216	60	52	0	0	226	12	1	5	1	0	0	1048
Percent	0%	45.3%	20.6%	5.7%	5%	0%	0%	21.6%	1.1%	0.1%	0.5%	0.1%	0%	0%	1046
ADT 1048															
AM Peak	12:00 AM	11:45 AM	9:30 AM	6:45 AM	7:00 AM	12:00 AM	12:00 AM	9:45 AM	11:45 AM	4:30 AM	12:45 AM	2:30 AM	12:00 AM	12:00 AM	11:45 AM
15-min Vol	0	12	7	3	3	0	0	7	2	1	1	1	0	0	23
PM Peak	12:00 PM	5:15 PM	3:15 PM	3:30 PM	6:45 PM	12:00 PM	12:00 PM	5:00 PM	12:45 PM	12:00 PM	5:00 PM	12:00 PM	12:00 PM	12:00 PM	3:15 PM
15-min Vol	0	17	11	5	3	0	0	8	1	0	1	0	0	0	27
Comments:															

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd

SPECIFIC LOCATION:
CITY/STATE: Benton, WA

QC JOB #: 16236659
DIRECTION: EB, WB
DATE: Jun 13 2023

CITY/STATE: Bet	itori, WA													DATE:	Jun 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	0	475	216	60	52	0	0	226	12	1	5	1	0	0	1048
Percent	0%	45.3%	20.6%	5.7%	5%	0%	0%	21.6%	1.1%	0.1%	0.5%	0.1%	0%	0%	1046
ADT 1048															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



CITY/STATE: Benton, WA

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd QC JOB #: 16236659 **DIRECTION:** EB, WB SPECIFIC LOCATION: **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		2				2			2	
12:30 AM		1				1			1	
12:45 AM		2				2			2	
01:00 AM		2				2			2	
01:15 AM		2				2			2	
01:30 AM		5				5			5	
01:45 AM		3				3			3	
02:00 AM		1				1			1	
02:15 AM		2				2			2	
02:30 AM		3				3			3	
02:45 AM		1				1			1	
03:00 AM		1				1			1	
03:15 AM		0				0			0	
03:30 AM		2				2			2	
03:45 AM		3				3			3	
04:00 AM		1				1			1	
04:15 AM		6				6			6	
04:30 AM		4				4			4	
04:45 AM		11				11			11	
05:00 AM		12				12			12	
05:15 AM		11				11 12	00 40 4		11	
05:30 AM		12					JIVIIVI		12	
05:45 AM		13				13			13	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd QC JOB #: 16236659 **DIRECTION:** EB, WB **SPECIFIC LOCATION:** 

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023 Thu Fri Average Weekday Mon Tue Wed Sat Sun Average Week **Average Week Profile Start Time** 13 Jun 23 15-min Traffic 15-min Traffic 06:00 AM 13 13 13 06:15 AM 16 16 16 06:30 AM 10 10 10 16 06:45 AM 16 16 07:00 AM 19 19 19 07:15 AM 19 19 19 07:30 AM 14 14 14 7 7 07:45 AM 7 08:00 AM 10 10 10 08:15 AM 12 12 12 08:30 AM 8 8 8 08:45 AM 12 12 12 09:00 AM 18 18 18 09:15 AM 14 14 14 17 09:30 AM 17 17 09:45 AM 19 19 19 10:00 AM 18 18 18 10:15 AM 16 16 16 16 10:30 AM 16 16 13 10:45 AM 13 13 11:00 AM 12 12 12 11:15 AM 11 11 11 15 15 15 11:30 AM 11:45 AM 23 23 23 Day Total % Weekday Average % Week Average AM Peak 15-min Vol PM Peak 15-min Vol

Comments:

SPECIFIC LOCATION:

DIRECTION: EB, WB **DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236659

CITY/STATE: Benton, WA

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		18				18			18	
12:15 PM		17				17			17	
12:30 PM		11				11			11	
12:45 PM		13				13			13	
01:00 PM		16				16			16	
01:15 PM		15				15			15	
01:30 PM		12				12			12	
01:45 PM		16				16			16	
02:00 PM		17				17			17	
02:15 PM		16				16			16	
02:30 PM		22				22			22	
02:45 PM		16				16			16	
03:00 PM		12				12			12	
03:15 PM		27				27			27	
03:30 PM		22				22			22	
03:45 PM		21				21			21	
04:00 PM		23				23			23	
04:15 PM		17				17			17	
04:30 PM		22				22			22	
04:45 PM		23				23			23	
05:00 PM		25				25			25	
05:15 PM		26				26	O A A A A		26	
05:30 PM		19				19	JIVIIVI		19	
05:45 PM		25				25			25	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

	Mon Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	
Start Time	13 Jun		ma		15-min Traffic	Jui	Juli	15-min Traffic	Average Week Profile
06:00 PM	12				12			12	
06:15 PM	15				15			15	
06:30 PM	11				11			11	
06:45 PM	17				17			17	
07:00 PM	8				8			8	
07:15 PM	11				11			11	
07:30 PM	11				11			11	
07:45 PM	3				3			3	
08:00 PM	6				6			6	
08:15 PM	9				9			9	
08:30 PM	6				6			6	
08:45 PM	7				7			7	
09:00 PM	4				4			4	
09:15 PM	5				5			5	
09:30 PM	0				0			0	
09:45 PM	4				4			4	
10:00 PM	3				3			3	
10:15 PM	1			_ I ° .	1	-		1	
10:30 PM	4				4			4	
10:45 PM	2				2			2	
11:00 PM	2				2			2	
11:15 PM	4			TIATI	4	30 AB A		4	
11:30 PM	4			MALL	4	JIVIIVI		4	
11:45 PM	0				0			0	
Day Total	1048	3			1048			1048	
% Weekday	100%	<b>6</b>							
Average	1007	·							
% Week	100%	6			100%				
Average									
AM Peak	11:45 /	MA			11:45 AM			11:45 AM	
15-min Vol	23				23			23	
PM Peak	3:15 P	М			3:15 PM			3:15 PM	
15-min Vol	27				27			27	

QC JOB #: 16236659

**DIRECTION:** EB, WB

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd SPECIFIC LOCATION: **DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 46-55 12:15 AM 12:30 AM 1-10 12:45 AM 46-55 01:00 AM 46-55 01:15 AM 1-10 46-55 01:30 AM 01:45 AM 41-50 02:00 AM O 1-10 02:15 AM 46-55 02:30 AM 51-60 02:45 AM 1-10 03:00 AM 41-50 03:15 AM 1-10 03:30 AM 41-50 03:45 AM 56-65 04:00 AM 1-10 04:15 AM 56-65 04:30 AM O 41-50 04:45 AM 46-55 05:00 AM 51-60 41-50 05:15 AM 05:30 AM 46-55 05:45 AM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd SPECIFIC LOCATION: **DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 06:15 AM 51-60 06:30 AM 41-50 06:45 AM 41-50 07:00 AM 46-55 07:15 AM 46-55 46-55 07:30 AM 07:45 AM 46-55 08:00 AM O 46-55 08:15 AM 51-60 08:30 AM 46-55 08:45 AM 46-55 09:00 AM 46-55 09:15 AM 51-60 09:30 AM 46-55 09:45 AM 51-60 10:00 AM 46-55 10:15 AM 46-55 10:30 AM O 46-55 10:45 AM 46-55 11:00 AM 46-55 11:15 AM 41-50 11:30 AM 46-55 11:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236659 LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd **SPECIFIC LOCATION: DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 46-55 12:15 PM 51-60 12:30 PM 51-60 12:45 PM 46-55 01:00 PM 46-55 01:15 PM 46-55 01:30 PM 51-60 01:45 PM 51-60 02:00 PM O 46-55 02:15 PM 46-55 02:30 PM 46-55 02:45 PM 41-50 03:00 PM 46-55 03:15 PM O 46-55 03:30 PM 41-50 03:45 PM 46-55 04:00 PM 46-55 04:15 PM 46-55 04:30 PM O 46-55 04:45 PM 51-60 05:00 PM 50-59 05:15 PM 51-60 05:30 PM 46-55 05:45 PM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd QC JOB #: 16236659 SPECIFIC LOCATION: **DIRECTION: WB** 

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pac
06:00 PM	0	0	0	0	0	0	0	2	0	0	0	1	0	0	3	41-50	2
06:15 PM	0	0	0	0	0	0	1	3	3	0	1	0	0	0	8	46-55	6
06:30 PM	0	0	0	0	0	0	0	2	3	0	0	1	0	0	6	46-55	5
06:45 PM	0	0	0	0	0	0	2	1	2	1	0	1	0	0	7	41-50	3
07:00 PM	0	0	0	0	0	0	0	1	1	0	1	0	0	0	3	46-55	2
07:15 PM	0	0	0	0	0	1	1	1	0	1	0	0	0	0	4	36-45	2
07:30 PM	0	0	0	0	0	0	1	1	1	0	0	0	0	0	3	41-50	2
07:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
08:00 PM	0	0	0	0	0	0	0	1	0	2	0	0	0	0	3	51-60	2
08:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
08:45 PM	0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	46-55	3
09:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
09:15 PM	0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	46-55	3
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2	41-50	2
10:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:30 PM	0	0	0	0	0	0	0	1	2	0	0	0	0	0	3	46-55	3
10:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
11:30 PM	0	0	0	0	0	0	0	1	0	2	0	0	0	0	3	51-60	2
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	0	0	0	1	3	42	149	182	80	17	7	_11	0	402	46.55	331
Percent	0%	0%	0%	0%	0.2%	0.6%	8.7%	30.9%	37.8%	16.6%	3.5%	1.5%	0.2%	0%	482	46-55	331
AM Peak	12:00 AM 0	12:00 AM 0	12:00 AM 0	12:00 AM 0	12:00 AM 0	7:00 AM	6:45 AM 4	11:45 AM 7	7:30 AM 6	9:45 AM 4	3:45 AM 2	5:30 AM 2	7:00 AM 1	12:00 AM 0	7:15 AM 11		
15-min Vol						1											
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	9:00 PM 1	3:30 PM 1	3:30 PM 5	4:00 PM 6	12:15 PM 10	4:45 PM 5	3:30 PM 1	6:00 PM 1	12:00 PM 0	12:00 PM 0	5:15 PM 14		

## **SUMMARY - Tube Count - Speed Data**

ellaras Ka	btwn Rte	221 and	Tyack Rd												QC JOB	#: 16236659
ATION:															DIR	ECTION: WB
Benton, W	VA														DATE:	Jun 13 2023
1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Dage Speed	Number in
15	20	25	30	35	40	45	50	55	60	65	70	75	999	TOLAI	race speed	Pace
0	0	0	0	1	3	42	149	182	80	17	7	1	0	402	46 55	331
0%	0%	0%	0%	0.2%	0.6%	8.7%	30.9%	37.8%	16.6%	3.5%	1.5%	0.2%	0%	402	40-33	331
0%	0%	0%	0%	0.2%	0.8%	9.5%	40.5%	78.2%	94.8%	98.3%	99.8%	100%	100%			
														Me	an Speed(Avera Med	ntile: 57 MPH nge): 51 MPH lian: 51 MPH ode: 53 MPH
,	ATION: enton, V 1 15 0 0%	ATION: enton, WA  1 16 15 20 0 0 0% 0%	ATION: enton, WA  1 16 21 15 20 25 0 0 0 0% 0% 0%	ATION: enton, WA  1 16 21 26 15 20 25 30 0 0 0 0 0 0% 0% 0% 0%	ATION: enton, WA  1 16 21 26 31 15 20 25 30 35 0 0 0 0 1 0% 0% 0% 0% 0% 0.2%	ATION: enton, WA  1 16 21 26 31 36 15 20 25 30 35 40 0 0 0 0 1 3 0% 0% 0% 0% 0.2% 0.6%	ATION: enton, WA  1 16 21 26 31 36 41 15 20 25 30 35 40 45 0 0 0 0 1 3 42 0% 0% 0% 0% 0.2% 0.6% 8.7%	ATION: enton, WA  1 16 21 26 31 36 41 46 15 20 25 30 35 40 45 50 0 0 0 0 1 3 42 149 0% 0% 0% 0% 0.2% 0.6% 8.7% 30.9%	ATION: enton, WA  1 16 21 26 31 36 41 46 51 15 20 25 30 35 40 45 50 55 0 0 0 0 0 1 3 42 149 182 0% 0% 0% 0% 0.2% 0.6% 8.7% 30.9% 37.8%	ATION: enton, WA  1 16 21 26 31 36 41 46 51 56 15 20 25 30 35 40 45 50 55 60  0 0 0 0 1 3 42 149 182 80 0% 0% 0% 0% 0.2% 0.6% 8.7% 30.9% 37.8% 16.6%	ATION: enton, WA  1 16 21 26 31 36 41 46 51 56 61 15 20 25 30 35 40 45 50 55 60 65 0 0 0 0 1 3 42 149 182 80 17 0% 0% 0% 0% 0.2% 0.6% 8.7% 30.9% 37.8% 16.6% 3.5%	ATION: enton, WA  1	ATION: enton, WA  1	ATION: enton, WA  1	ATION: enton, WA  1	DIR DATE:  1 16 21 26 31 36 41 46 51 56 61 66 71 76 101 Pace Speed  15 20 25 30 35 40 45 50 55 60 65 70 75 999 Total  0 0 0 0 0 1 3 42 149 182 80 17 7 1 0 482 46-55  0% 0% 0% 0% 0% 0.2% 0.6% 8.7% 30.9% 37.8% 16.6% 3.5% 1.5% 0.2% 0% 482 46-55  0% 0% 0% 0% 0% 0.2% 0.8% 9.5% 40.5% 78.2% 94.8% 98.3% 99.8% 100% 100%  85th Percentage of the company of the

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: WB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Hille	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
01:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
02:30 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
03:45 AM	0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	1	1	0	0	0	0	2	0	0	0	0	0	0	4
04:30 AM	0	1	1	0	0	0	0	0	0	1	0	0	0	0	3
04:45 AM	0	1	3	0	1	0	0	2	0	0	0	0	0	0	7
05:00 AM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
05:15 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
05:30 AM	0	4	3	1	0	0	0	0	0	0	0	0	0	0	8
05:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Day Total															
Percent				DATA	ALHA	$\Delta I D$	RIVF	566	IMIM	UNIT	1-5				
ADT															
482															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: WB

October   Start   March   Start   March   Start   St	CITY/STATE: Be	IIIOII, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 2023
Geola AM	Start Time	Bikes			Buses											Total
06:15 AM	06:00 414	0			0											7
06:30 AM																
0645 AM 0 2 2 2 3 1 0 0 0 2 0 0 0 0 0 0 0 0 0 0 10 0 700 AM 0 3 2 1 1 1 0 0 0 2 0 0 0 0 0 0 0 0 0 0 9 9 07:15 AM 0 4 5 1 0 0 0 0 0 4 0 0 0 0 0 0 0 0 11 0 0 0 0															_	
07:00 AM										-				-	_	
07:15 AM										•				ŭ	_	
07:30 AM					-				1	·	_			-	_	
07:45 AM			3	•	-	_	_		4	0	•	_	-	·	•	
08:00 AM					-				-	•	_			-	_	
08.15 AM 0 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				_						_	_			-	_	
08:30 AM				_	•	_				_	_			•	-	-
08:45 AM 0 1 0 0 0 1 0 0 0 1 0 0 0 0 0 0 0 0 0				_	-	_					_			-	•	_
09:00 AM					0					0	0			0	0	3
09:15 AM										1				-	_	
09:30 AM			1	1	1	0	0			0	0	0	0	0	0	
10:00 AM		0	3	5	0	0	0	0		0	0	0	0	0	0	
10:15 AM	09:45 AM	0	2	1	2	1	0	0	3	1	0	0	0	0	0	10
10:30 AM	10:00 AM	0	4	1	1	0	0	0	3	0	0	0	0	0	0	9
10:45 AM	10:15 AM	0	4	1	1	0	0	0	3	0	0	0	0	0	0	9
11:00 AM	10:30 AM	0	5	1	1	0	0	0	2	0	0	0	0	0	0	9
11:15 AM	10:45 AM	0	2	2	1	1		0	3	0	0	0	0	0	0	9
11:30 AM	11:00 AM	0	2	1	0	0	0	0	3	0		0	0	0	0	6
11:45 AM	11:15 AM	0		4	0			0		0			0	0	0	7
ADT 482  AM Peak 15-min Vol  PM Peak 15-min Vol  AM Peak 15-min Vol  AM Peak 15-min Vol	11:30 AM	0	5	2	0	0	0	0	2	0	0	0	0	0	0	9
Percent  ADT 482  AM Peak 15-min Vol  PM Peak 15-min Vol  15-min Vol  PM Peak 15-min Vol  PM Peak 15-min Vol  PM Peak	11:45 AM	0	6	2	0	0	0	0	1	2	0	0	0	0	0	11
ADT 482  AM Peak 15-min Vol PM Peak 15-min Vol	Day Total															
482 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol	Percent															
482 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol																
AM Peak 15-min Vol  PM Peak 15-min Vol  15-min Vol	ADT															
15-min Vol PM Peak 15-min Vol	482															
15-min Vol PM Peak 15-min Vol																
PM Peak 15-min Vol																
15-min Vol																
	comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: WB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	0	4	1	0	0	0	0	2	0	0	0	0	0	0	7
12:15 PM	0	6	3	2	0	0	0	0	0	0	0	0	0	0	11
12:30 PM	0	1	2	0	0	0	0	1	0	0	0	0	0	0	4
12:45 PM	0	3	0	1	0	0	0	1	0	0	0	0	0	0	5
01:00 PM	0	1	1	1	0	0	0	3	0	0	0	0	0	0	6
01:15 PM	0	6	1	1	1	0	0	0	0	0	0	0	0	0	9
01:30 PM	0	2	0	0	0	0	0	3	0	0	0	0	0	0	5
01:45 PM	0	5	1	0	0	0	0	2	0	0	0	0	0	0	8
02:00 PM	0	4	0	1	0	0	0	4	1	0	0	0	0	0	10
02:15 PM	0	1	1	0	0	0	0	1	1	0	0	0	0	0	4
02:30 PM	0	4	0	1	2	0	0	2	1	0	0	0	0	0	10
02:45 PM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
03:00 PM	0	4	0	0	0	0	0	1	0	0	0	0	0	0	5
03:15 PM	0	2	3	0	1	0	0	2	0	0	0	0	0	0	8
03:30 PM	0	4	1	3	0	0	0	1	1	0	0	0	0	0	10
03:45 PM	0	9	1	0	1	0	0	0	0	0	0	0	0	0	11
04:00 PM	0	5	1	0	2	0	0	1	0	0	0	0	0	0	9
04:15 PM	0	3	1	2	0	0	0	0	0	0	0	0	0	0	6
04:30 PM	0	6	1	0	0	0	0	0	0	0	0	0	0	0	7
04:45 PM	0	7	2	0	0	0	0	2	0	0	0	0	0	0	11
05:00 PM	0	4	1	1	0	0	0	3	0	0	0	0	0	0	9
05:15 PM	0	12	0	1	0	0	0	1	0	0	0	0	0	0	14
05:30 PM	0	4	1	0	0	0	0	2	0	0	0	0	0	0	7
05:45 PM	0	7	1	0	1	0	0	1	0	0	0	0	0	0	10
Day Total															
Percent				DATA	ATH		RIVF	SCC	MM	UNIT	11-5				
ADT															
482															
.02															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236659 DIRECTION: WB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIKES	Trailers	Long	Duses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
06:00 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
06:15 PM	0	4	1	0	0	0	0	3	0	0	0	0	0	0	8
06:30 PM	0	3	2	0	0	0	0	1	0	0	0	0	0	0	6
06:45 PM	0	4	2	0	1	0	0	0	0	0	0	0	0	0	7
07:00 PM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
07:15 PM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
07:30 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
07:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
08:00 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
08:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
08:45 PM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	3
09:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:15 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
10:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	1	1	0	0	0	0	0	0	0	1	0	0	0	3
10:45 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
11:30 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	226	95	31	20	0	0	99	8	1	1	1	0	0	482
Percent	0%	46.9%	19.7%	6.4%	4.1%	0%	0%	20.5%	1.7%	0.2%	0.2%	0.2%	0%	0%	402
ADT															
482															
702															
AM Peak	12:00 AM	11:45 AM	7:15 AM	6:45 AM	4:45 AM	12:00 AM	12:00 AM	8:15 AM	11:45 AM	4:30 AM	12:00 AM	2:30 AM	12:00 AM	12:00 AM	7:15 AM
15-min Vol	0	6	5	3	1	0	0	5	2	1	0	1	0	0	11
PM Peak	12:00 PM	5:15 PM	12:15 PM	3:30 PM	2:30 PM	12:00 PM	12:00 PM	2:00 PM	2:00 PM	12:00 PM	10:30 PM	12:00 PM		12:00 PM	5:15 PM
15-min Vol	0	12	3	3	2	0	0	4	1	0	1	0	0	0	14
Comments:	, in the second				_			•			-				
minicino.															

LOCATION: Sellards Rd btwn Rte 221 and Tyack Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236659

DIRECTION: WB

DATE: Jun 13 2023

CITY/STATE: Bei	iton, WA													DATE:	Jun 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	0	226	95	31	20	0	0	99	8	1	1	1	0	0	482
Percent	0%	46.9%	19.7%	6.4%	4.1%	0%	0%	20.5%	1.7%	0.2%	0.2%	0.2%	0%	0%	462
ADT 482															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

DIRECTION: WB DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236659

CITY/STATE: Benton, WA DATE:

Start Time	Mon	Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		1				1			1	
12:30 AM		0				0			0	
12:45 AM		1				1			1	
01:00 AM		1				1			1	
01:15 AM		0				0			0	
01:30 AM		3				3			3	
01:45 AM		1				1			1	
02:00 AM		0				0			0	
02:15 AM		2				2			2	
02:30 AM		2				2			2	
02:45 AM		0				0			0	
03:00 AM		1				1			1	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		3				3			3	
04:00 AM		0				0			0	
04:15 AM		4				4	-		4	
04:30 AM		3				3			3	
04:45 AM		7			161	7			7	
05:00 AM		4				4			4	
05:15 AM		4			LIATI	4	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \		4	
05:30 AM		8			MALL	8	JIVIIVI		8	
05:45 AM		1				1			1	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236659 DIRECTION: WB DATE: Jun 13 2023 - Jun 13 2023

Start Time	Tue Jun 23	Wed	Thu	Fri	Average Weekda 15-min Traffic	y Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM	7				7	ì		7	
06:15 AM	6				6			6	
06:30 AM	4				4			4	
06:45 AM	10				10			10	
07:00 AM	9				9			9	
07:15 AM	11				11			11	
07:30 AM	10				10			10	
07:45 AM	5				5			5	
08:00 AM	4				4			4	
08:15 AM	9				9			9	
08:30 AM	4				4			4	
08:45 AM	3				3			3	
09:00 AM	8				8	7		8	
09:15 AM	5				5			5	
09:30 AM	11				11			11	
09:45 AM	10				10			10	
10:00 AM	9				9			9	
10:15 AM	9			~ I .	9			9	
10:30 AM	9				9			9	
10:45 AM	9			761	9			9	
11:00 AM	6				6			6	
11:15 AM	7			LIATI	7 9	COMM		7 7 7 P	
11:30 AM	9			$\square A \sqcap \Gamma$		COIVIIVI		9	
11:45 AM	11				11			11	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak 15-min Vol									
PM Peak									
15-min Vol									
Comments:									

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236659 DIRECTION: WB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		7				7			7	
12:00 PM 12:15 PM		11				11			11	
12:15 PM 12:30 PM		11 4				4			4	
12:45 PM		5				5			5	
01:00 PM		6				6			6	
01:00 PM 01:15 PM		9				9			9	
01:13 PM 01:30 PM		5				5			5	
01:45 PM		8				8			8	
01:43 PM 02:00 PM		10				10			10	
02:00 FM 02:15 PM		4				4			4	
02:30 PM		10				10			10	
02:45 PM		4				4			4	
03:00 PM		5				5			5	
03:15 PM		8				8			8	
03:30 PM		10				10			10	
03:45 PM		11				11			11	
04:00 PM		9				9			9	
04:15 PM		6				6			6	
04:30 PM		7				7			7	
04:45 PM		11				11	-		11	
05:00 PM		9				9			9	
05:15 PM		14				14 7	00.00.0		14	
05:30 PM		7					DIVIIVI		IES 7	
05:45 PM		10				10			10	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DAT

QC JOB #: 16236659 DIRECTION: WB DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	3				3			3	
06:15 PM	8				8			8	
06:30 PM	6				6			6	
06:45 PM	7				7			7	
07:00 PM	3				3			3	
07:15 PM	4				4			4	
07:30 PM	3				3			3	
07:45 PM	2				2			2	
08:00 PM	3				3			3	
08:15 PM	1				1			1	
08:30 PM	1				1			1	
08:45 PM	3				3			3	
09:00 PM	1				1			1	
09:15 PM	3				3			3	
09:30 PM	0				0			0	
09:45 PM	2				2			2	
10:00 PM	1				1			1	
10:15 PM	0				0			0	
10:30 PM	3				3		In.	3	
10:45 PM	1				1		411	1	
11:00 PM	0				0			0	
11:15 PM	2				2	0000		2	
11:30 PM	3				DR/\\2 3 S C(	DIVIN	UNII	3	
11:45 PM	0				0			0	
Day Total	482				482			482	
% Weekday	100%								
Average	100%								
% Week	100%				100%				
Average	100%				100%				
AM Peak	7:15 AM				7:15 AM			7:15 AM	
15-min Vol	11				11			11	
PM Peak	5:15 PM				5:15 PM			5:15 PM	
15-min Vol	14				14			14	

LOCATION: T SPECIFIC LOC CITY/STATE:	ATION:		Sellards	Rd												QC JOB #: 1 DIREC DATE: Jun	TION: NB
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
12:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	1	0	1	0	0	1	0	0	0	0	0	0	0	3	11-20	1
12:45 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
04:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
04:30 AM	0	0	0	0	0	0	1	1	1	0	0	0	0	0	3	41-50	2
04:45 AM	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2	36-45	1
05:00 AM	0	0	0	0	1	0	0	0	2	0	1	0	0	0	4	46-55	2
05:15 AM	0	0	0	0	2	1	2	1	0	0	0	0	0	0	6	31-40	3
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 AM	0	0	0	0	1	0	1	1	0	0	0	0	0	0	3	41-50	2
Day Total			-			<del>-</del> 11	A === 1	300	/=/		0 00	71 18				41 30	
Percent				DA	1A	LH	AII	DRIN	/E5	CC	MN	1UN		-5			
AM Peak 15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

CITY/STATE:			24	20	21	20	44	4.0	Г1		<u>C1</u>		71	7.0		DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
06:00 AM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
06:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
06:30 AM	0	0	0	0	0	1	2	0	1	0	0	0	0	0	4	36-45	3
06:45 AM	0	0	0	0	0	0	2	3	2	0	0	0	0	0	7	43-52	5
07:00 AM	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2	26-35	1
07:15 AM	0	0	0	0	2	0	0	2	1	0	0	0	0	0	5	46-55	3
07:30 AM	0	0	0	0	0	1	0	1	1	0	0	0	0	0	3	46-55	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:00 AM	0	0	0	0	3	0	2	1	0	0	0	0	0	0	6	41-50	3
08:15 AM	0	0	0	0	1	0	0	1	0	1	0	0	0	0	3	26-35	1
08:30 AM	0	0	0	0	3	1	0	2	1	0	0	0	0	0	7	31-40	4
08:45 AM	0	0	0	0	1	1	1	0	2	0	0	0	0	0	5	46-55	2
09:00 AM	0	0	0	0	0	1	1	1	0	1	0	0	0	0	4	36-45	2
09:15 AM	0	0	0	1	0	0	1	0	0	0	1	0	0	0	3	21-30	1
09:30 AM	0	0	0	0	1	1	0	1	0	0	0	0	0	0	3	31-40	2
09:45 AM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	2	26-35	1
10:00 AM	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2	26-35	1
10:00 AM	0	0	0	0	1	2	1	0	0	0	0	0	0	0	4	36-45	3
	0	0	0	1	3	1	2		1	1	0	0	0		10		
10:30 AM		-				_		1			-	-		0		28-37	4
10:45 AM	0	0	0	2	0	0	0	3	0	1	0	0	0	0	6	41-50	3
11:00 AM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	26-35	2
11:15 AM	0	0	0	0	5	0	0	0	0	0	0	0	0	0	5	26-35	5
11:30 AM	0	0	0	0	1	5	2	0	0	0	0	0	0	0	8	36-45	7
11:45 AM	0	0	0	0	0	2	2	1	3	0	0	0	0	0	8	46-55	4
Day Total Percent																	
AM Peak 15-min Vol																	
PM Peak																	
5-min Vol																	

CITY/STATE:	Benton,															DATE: Jun	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12:00 PM	0	0	0	0	2	3	2	0	1	0	0	0	0	0	8	33-42	5
12:15 PM	0	0	0	0	1	4	0	0	1	0	1	0	0	0	7	31-40	5
12:30 PM	0	0	0	0	0	1	1	2	1	0	0	0	0	0	5	46-55	3
12:45 PM	0	0	1	0	0	0	0	0	1	0	0	0	0	0	2	16-25	1
01:00 PM	0	0	0	0	2	2	1	0	0	1	0	0	0	0	6	31-40	4
01:15 PM	0	0	0	0	1	1	1	2	0	0	0	0	0	0	5	41-50	3
01:30 PM	0	0	0	0	1	3	2	0	1	0	0	0	0	0	7	36-45	5
01:45 PM	0	0	0	0	0	1	0	0	0	1	0	1	0	0	3	31-40	1
02:00 PM	0	0	0	0	0	0	1	3	3	0	0	0	0	0	7	46-55	6
02:15 PM	0	0	0	0	0	2	4	1	2	1	0	0	0	0	10	36-45	6
02:30 PM	0	0	0	0	0	1	2	5	0	0	0	0	0	0	8	41-50	7
02:45 PM	0	0	0	0	1	2	0	1	1	0	1	0	0	0	6	31-40	3
03:00 PM	0	0	0	1	1	0	1	2	2	0	1	0	0	0	8	46-55	4
03:15 PM	0	0	0	0	1	0	4	2	2	0	1	0	0	0	10	41-50	6
03:30 PM	0	0	0	0	4	0	0	5	2	0	0	0	0	0	11	46-55	7
03:45 PM	0	0	0	0	6	1	2	4	2	0	0	0	0	0	15	31-40	7
03.43 FW	0	0	0	0	1	0	4	1	3	1	0	0	0	0	10	41-50	5
04:00 PM	0	0	0	0	2	0	1	3	3	0	0	0	0	0	9	46-55	6
04:30 PM	0	0	0	0	2	1	3	6	5	0	1	0	0	0	18		11
	0	0	0	0	1	0			5 4		_	0	0	0	14	46-55	
04:45 PM							1	6		2	0					46-55	10
05:00 PM	0	0	0	0	0	2	5	2	5	0	1	0	0	0	15	40-49	7
05:15 PM	0	0	0	0	0	0	2	2	3	0	1	0	0	0	8	46-55	5
05:30 PM	0	0	0	0	0	2	6	3	2	1	0	0	0	0	14	41-50	9
05:45 PM Day Total	0	0	0	0	0	1	1	2	1	0	0	0	0	0	5	46-55	3
Percent														-5			
AM Peak 15-min Vol																	
PM Peak 15-min Vol																	

LOCATION: Travis Rd north of Sellards Rd

SPECIFIC LOCATION:

DIRECTION: NB

CITY/STATE: Benton, WA DATE: Jun 13 202

CITI/STATE.	Benton,	WA														DATE: Jun	13 2023
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	in Pace
06:00 PM	0	0	0	2	1	2	1	0	0	0	0	0	0	0	6	26-35	3
06:15 PM	0	0	0	0	1	0	2	1	1	1	0	1	0	0	7	41-50	3
06:30 PM	0	0	0	0	3	5	0	0	1	0	0	0	0	0	9	31-40	8
06:45 PM	0	0	0	1	0	0	3	2	4	0	0	0	0	0	10	46-55	6
07:00 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
07:15 PM	0	0	0	0	0	2	1	0	0	0	1	0	0	0	4	36-45	3
07:30 PM	0	0	0	0	2	2	0	0	1	1	1	0	0	0	7	31-40	4
07:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
08:15 PM	0	0	0	0	0	2	1	0	0	0	0	0	0	0	3	36-45	3
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 PM	0	0	0	0	2	0	0	1	0	0	0	0	0	0	3	26-35	2
09:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
09:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	2	31-40	1
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	3	0	1	0	0	0	0	0	0	0	4	26-35	3
10:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
11:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	1	1	9	73	60	76	83	72	13	12	2	0	0	100	<u> </u>	450
Percent	0%	0.2%	0.2%	2.2%	18.2%	14.9%	18.9%	20.6%	17.9%	3.2%	3%	0.5%	0%	0%	402	41-50	159
AM Peak					11:15 AM		5:15 AM		11:45 AM			12:00 AM			10:30 AM		
15-min Vol	0	1	0	2	5	5	2	3	3	1	1	0	0	0	10		
PM Peak		12:00 PM		6:00 PM	3:45 PM	6:30 PM	5:30 PM	4:30 PM	4:30 PM	4:45 PM	12:15 PM		12:00 PM		4:30 PM		
15-min Vol	0	0	1	2	6	5	6	6	5	2	1	1	0	0	18		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Tra	avis Rd n	orth of Se	ellards Rd													QC JOB	#: 16236660
SPECIFIC LOCA	TION:															DIF	RECTION: NB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Name	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 ace Speed	Pace
Grand Total	0	1	1	9	73	60	76	83	72	13	12	2	0	0	402	41-50	159
Percent	0%	0.2%	0.2%	2.2%	18.2%	14.9%	18.9%	20.6%	17.9%	3.2%	3%	0.5%	0%	0%	402	41-30	133
Cumulative Percent	0%	0.2%	0.5%	2.7%	20.9%	35.8%	54.7%	75.4%	93.3%	96.5%	99.5%	100%	100%	100%			
ADT 402															Me		
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB

CITY/STATE: Be	nton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Hime	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
12:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:30 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
04:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:00 AM	0	3	0	0	0	0	0	0	1	0	0	0	0	0	4
05:15 AM	0	2	1	2	0	0	0	0	1	0	0	0	0	0	6
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 AM	0	2	0	0	0	1	0	0	0	0	0	0	0	0	3
Day Total															
Percent				DATA	IHA	AT D	RIVF	566	MMM	UNIT	11-5				
ADT															
402															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660

DIRECTION: NB
DATE: Jun 13 2023

Trailers  2 1 2 5 0 2 2 0 1 1 0 1 0 0 0 0	Dong  0 0 1 1 1 1 0 0 1 2 3 1 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0 0 0 0 0 0 0 0 0 0 0 0	Tire  0 0 1 1 0 1 0 2 0 2 1 1 0 0 0 0 0 0 0	Single  0 0 0 0 0 0 0 0 0 0 2 0 0 0 0 0	Single  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 1 0 0 0 0 0	0 0 0 0 0 1 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	Multi 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Multi 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Multi 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0	2 1 4 7 2 5 3 0 6 3 7
1 2 5 0 2 2 0 1 1 0 1 2 1 1 0	0 1 1 1 1 0 0 0 1 2 3 1 1 0 0	0 0 0 0 0 0 0 0 0 0 0 0	0 1 1 0 1 1 0 2 0 2 2 1 1 1	0 0 0 0 0 0 0 0 0 0 2 0	0 0 0 0 0 0 0 0	0 0 0 0 1 0 0 0 0 0	0 0 0 1 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	1 4 7 2 5 3 0 6 3 7
2 5 0 2 2 0 1 1 0 1 2 1 1 0	1 1 1 0 0 1 2 3 1 1 0 0	0 0 0 0 0 0 0 2 0 0 0 0	1 1 0 1 1 0 2 0 2 2 2 1 1	0 0 0 0 0 0 0 0 0 0 2 0	0 0 0 0 0 0 0 0	0 0 0 1 0 0 0 0 0	0 0 1 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	4 7 2 5 3 0 6 3 7
5 0 2 2 0 1 1 0 1 2 1 1 0	1 1 0 0 1 2 3 1 1 0 0	0 0 0 0 0 2 0 0 0 0	1 0 1 1 0 2 0 2 2 2 1 1	0 0 0 0 0 0 0 0 2 0	0 0 0 0 0 0 0 0	0 0 1 0 0 0 0 0 0	0 1 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	7 2 5 3 0 6 3 7
0 2 2 0 1 1 0 1 2 1 1 0 0	1 1 0 0 1 2 3 1 1 0 0	0 0 0 0 2 0 0 0 0 0	0 1 1 0 2 0 2 2 2 1 1	0 0 0 0 0 0 0 2 0 0	0 0 0 0 0 0 0	0 1 0 0 0 0 0 0	1 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	2 5 3 0 6 3 7
2 2 0 1 1 0 1 2 1 1 0	1 0 0 1 2 3 1 1 0 0	0 0 2 0 0 0 0 0	1 1 0 2 0 2 2 2 1 1	0 0 0 0 0 0 2 0 0	0 0 0 0 0 0	1 0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	5 3 0 6 3 7
2 0 1 1 0 1 2 1 1 0 0	1 2 3 1 1 0 0	0 2 0 0 0 0 0 0	0 2 0 2 2 1 1	0 0 0 0 2 0 0	0 0 0 0 0 0	0 0 0 0 1	0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	3 0 6 3 7
0 1 1 0 1 2 1 1 0 0	1 2 3 1 1 0 0	2 0 0 0 0 0 0	0 2 0 2 2 1 1	0 0 0 2 0 0	0 0 0 0 0	0 0 0 0 1	0 0 0 0	0 0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	6 3 7
1 1 0 1 2 1 1 0	2 3 1 1 0 0	0 0 0 0 0 0	2 0 2 2 1 1 0	0 0 2 0 0	0 0 0 0 0	0 0 0 1 0	0 0 0	0 0 0	0 0 0	0 0 0	0	0	3 7
1 0 1 2 1 1 0	3 1 1 0 0	0 0 0 0 0 0	0 2 2 1 1 0	0 2 0 0	0 0 0	0 0 1 0	0	0	0	0	0	0	7
1 2 1 1 0	1 1 0 0 0	0 0 0 0 0	2 1 1 0	2 0 0 0	0 0 0	1 0	·	0	0		•	·	•
2 1 1 0	1 0 0 0	0 0 0 1	1 1 0	0 0 0	0 0	1 0	·	_		0	0		_
1 1 0 0	0 0 0	0 0 1	1 0	0	0		0	0	_		U	0	5
1 0 0	0	0 1	0			0		U	0	0	0	0	4
0	0	1	ū	0		U	0	0	0	0	1	0	3
0		-	0		0	0	2	0	0	0	0	0	3
	1	0		1	0	0	0	0	0	0	0	0	2
Λ		•	0	0	0	0	0	0	0	0	1	0	2
U	0	0	1	0	0	2	1	0	0	0	0	0	4
3	1	0	2	1	0	1	2	0	0	0	0	0	10
1	2	0	1	0	0	1	0	0	0	0	1	0	6
0	0	0	0	0	0	1	0	11	0	0	0	0	2
0	0	2	0	0	0	2	1	0	0	0	0	0	5
0	0	2	3	0	0	1	1	0	0	0	0	0	8
4	1	2	1	0	0	0	0	0	0	0	0	0	8
		DAT	A TLI	ATE	DA /E	crr	NANA	1 TK 117	TIEC				
		DAIA	1 1 1 1 6	411/	KIVE	3.00	/IVIIVI	UNII	IES				
						222.7.50.444	222.7.FO ANA	222.7.50.404	222.7.50.004	223.7.FO AM	222.7.50 AM	SOURCE, Quality Counts, LLC / https://doi.org/10.1001/j.j.com/	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB

CITY/STATE: Be	enton, WA													DATE: J	un 13 2023
Chart Time -	Dilene	Cars &	2 Axle	Duna	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	1	2	0	0	3	2	0	0	0	0	0	0	0	0	8
12:15 PM	0	1	1	1	2	0	0	1	1	0	0	0	0	0	7
12:30 PM	1	3	0	0	1	0	0	0	0	0	0	0	0	0	5
12:45 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2
01:00 PM	1	1	0	0	3	0	0	0	0	1	0	0	0	0	6
01:15 PM	0	0	2	0	1	0	0	1	1	0	0	0	0	0	5
01:30 PM	0	1	0	0	2	1	0	3	0	0	0	0	0	0	7
01:45 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
02:00 PM	0	1	3	0	3	0	0	0	0	0	0	0	0	0	7
02:15 PM	0	1	0	1	5	0	0	3	0	0	0	0	0	0	10
02:30 PM	0	3	0	1	2	0	0	2	0	0	0	0	0	0	8
02:45 PM	0	1	0	0	2	0	0	1	1	1	0	0	0	0	6
03:00 PM	0	3	0	2	3	0	0	0	0	0	0	0	0	0	8
03:15 PM	0	4	1	1	2	0	0	1	0	1	0	0	0	0	10
03:30 PM	0	3	4	0	1	0	0	1	2	0	0	0	0	0	11
03:45 PM	1	2	4	2	3	1	0	1	0	0	0	0	1	0	15
04:00 PM	0	3	5	0	1	0	0	0	1	0	0	0	0	0	10
04:15 PM	0	6	2	1	0	0	0	0	0	0	0	0	0	0	9
04:30 PM	0	8	6	1	0	0	0	1	1	0	0	0	1	0	18
04:45 PM	0	10	1	0	3	0	0	0	0	0	0	0	0	0	14
05:00 PM	0	11	1	0	1	0	0	0	1	0	1_1_	0	0	0	15
05:15 PM	0	5	2	1	0	0	0	0	0	0	0	0	0	0	8
05:30 PM	0	11	1	1	1	0	0	0	0	0	0	0	0	0	14
05:45 PM	0	3	1	0	0	0	0	1	0	0	0	0	0	0	5
Day Total															
Percent				DATA	THA	$\Delta I D$	RIVF	566	IIVIIVI	UNIT	1-5				
ADT															
402															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	1	1	0	2	1	0	0	0	1	0	0	0	0	0	6
06:15 PM	0	2	2	0	1	0	0	1	1	0	0	0	0	0	7
06:30 PM	1	1	1	2	1	1	0	1	1	0	0	0	0	0	9
06:45 PM	0	7	2	0	0	0	0	0	1	0	0	0	0	0	10
07:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
07:15 PM	0	1	1	0	0	0	0	2	0	0	0	0	0	0	4
07:30 PM	1	4	0	1	0	0	0	0	1	0	0	0	0	0	7
07:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
08:00 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
08:15 PM	0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	1	0	1	0	0	0	0	0	0	0	0	1	0	3
09:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
09:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	1	0	1	0	0	1	0	0	1	0	0	0	0	0	4
10:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
11:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	9	153	62	32	64	12	0	32	27	4	1	0	6	0	402
Percent	2.2%	38.1%	15.4%	8%	15.9%	3%	0%	8%	6.7%	1%	0.2%	0%	1.5%	0%	402
ADT 402															
AM Peak	11:30 AM	6:45 AM	8:30 AM	5:15 AM	11:30 AM	8:30 AM	12:00 AM	10:15 AM	9:30 AM	11:00 AM	12:00 AM	12:00 AM	9:15 AM	12:00 AM	10:30 A
15-min Vol	1	5	3	2	3	2	0	2	2	1	0	0	1	0	10
PM Peak	12:00 PM	5:00 PM	4:30 PM	3:00 PM	2:15 PM	12:00 PM		1:30 PM	3:30 PM	1:00 PM	5:00 PM	12:00 PM	3:45 PM	12:00 PM	4:30 PN
15-min Vol	1	11	6	2	5	2	0	3	2	1	1	0	1	0	18
omments:															

LOCATION: Travis Rd north of Sellards Rd QC JOB #: 16236660 SPECIFIC LOCATION: **DIRECTION: NB** CITY/STATE: Benton, WA **DATE:** Jun 13 2023 Cars & 2 Axle 2 Axle 6 4 Axle <5 Axl 5 Axle >6 Axl 3 Axle <6 Axl 6 Axle >6 Axl Not Start Time **Bikes Buses** Total **Trailers** Long Tire Single Single Double Double Double Multi Multi Multi Classed **Grand Total** 9 153 62 32 64 12 0 32 27 4 1 0 6 0 402 2.2% 8% 8% 1.5% 0% Percent 38.1% 15.4% 15.9% 3% 0% 6.7% 1% 0.2% 0% ADT 402

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

**DIRECTION: NB** 

QC JOB #: 16236660

	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	
Start Time		13 Jun 23				15-min Traffic		<b>5</b>	15-min Traffic	Average Week Profile
12:00 AM		1				1			1	
12:15 AM		0				0			0	
12:30 AM		3				3			3	
12:45 AM		1				1			1	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		1				1			1	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		0				0			0	
04:00 AM		1				1			1	
04:15 AM		1				1			1	
04:30 AM		3				3		In.	3	
04:45 AM		2				2		411	2	
05:00 AM		4				4			4	
05:15 AM		6				6 0	00.00.0		6	
05:30 AM		0				DRIVOSCO	DIVIIV	UNII	<b>1 E 0</b>	
05:45 AM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660 DIRECTION: NB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		2				2			2	
06:15 AM		1				1			1	
06:30 AM		4				4			4	
06:45 AM		7				7			7	
07:00 AM		2				2			2	
07:15 AM		5				5			5	
07:30 AM		3				3			3	
07:45 AM		0				0			0	
08:00 AM		6				6			6	
08:15 AM		3				3			3	
08:30 AM		7				7			7	
08:45 AM		5				5			5	
09:00 AM		4				4			4	
09:15 AM		3				3			3	
09:30 AM		3				3			3	
09:45 AM		2				2			2	
10:00 AM		2				2			2	
10:15 AM		4			_ 0	4			4	
10:30 AM		10				10		In'	10	
10:45 AM		6			<b>a</b> L.	6	-	411	6	
11:00 AM		2				2			2	
11:15 AM		5				5 8	01/11/	C 18 119	5	
11:30 AM		8			HALL	8	DIVIN	UNH	8	
11:45 AM		8				8			8	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660 DIRECTION: NB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		8				8			8	
12:15 PM		7				7			7	
12:30 PM		5				5			5	
12:45 PM		2				2			2	
01:00 PM		6				6			6	
01:15 PM		5				5			5	
01:30 PM		7				7			7	
01:45 PM		3				3			3	
02:00 PM		7				7			7	
02:15 PM		10				10			10	
02:30 PM		8				8			8	
02:45 PM		6				6			6	
03:00 PM		8				8			8	
03:15 PM		10				10			10	
03:30 PM		11				11			11	
03:45 PM		15				15			15	
04:00 PM		10				10			10	
04:15 PM		9			_   0	9			9	
04:30 PM		18			21 1	18			18	
04:45 PM		14				14			14	
05:00 PM		15				15			15	
05:15 PM		8			TIATI	8 14	00 00 0		8	
05:30 PM		14			HALL		DIVIIVI		14	
05:45 PM		5				5			5	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		6				6			6	
06:15 PM		7				7			7	
06:30 PM		9				9			9	
06:45 PM		10				10			10	
07:00 PM		1				1			1	
07:15 PM		4				4			4	
07:30 PM		7				7			7	
07:45 PM		1				1			1	
08:00 PM		1				1			1	
08:15 PM		3				3			3	
08:30 PM		0				0			0	
08:45 PM		3				3			3	
09:00 PM		1				1			1	
09:15 PM		2				2			2	
09:30 PM		0				0			0	
09:45 PM		0				0			0	
10:00 PM		4				4			4	
10:15 PM		1				1			1	
10:30 PM		0				0			0	
10:45 PM		0				0	$\cdot$		0	
11:00 PM		1				1			1	
11:15 PM		1				11			1	
11:30 PM		0				DRIVOES CO	DMM		/ <del>-</del> 5 0	
11:45 PM		0				0			0	
Day Total		402				402			402	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		10:30 AM				10:30 AM			10:30 AM	
15-min Vol		10				10			10	
PM Peak		4:30 PM				4:30 PM			4:30 PM	
15-min Vol		18				18			18	
omments:										

LOCATION: T SPECIFIC LOC		north of	Sellards	Rd												QC JOB #: 1 DIRECTION	
CITY/STATE:	Benton,	WA														DATE: Jun	13 2023
CL . I T	1	16	21	26	31	36	41	46	51	56	61	66	71	76	T l	D C 1	Number
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pace
12:00 AM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
12:15 AM	0	0	0	1	1	1	0	0	0	0	0	0	0	0	3	26-35	2
12:30 AM	0	1	0	2	0	0	1	0	0	0	0	0	0	0	4	21-30	2
12:45 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
03:45 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
04:00 AM	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2	21-30	1
04:15 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2	41-50	2
04:30 AM	0	0	0	0	0	1	1	1	1	0	0	0	0	0	4	36-45	2
04:45 AM	0	0	0	0	1	2	2	_ 1	1	0	0	0	0	0	7	36-45	4
05:00 AM	0	0	0	0	1	1	5	3	4	0	1	0	0	0	15	41-50	8
05:15 AM	0	0	0	1	3	5	8	4	0	0	0	0	0	0	21	36-45	13
05:30 AM	0	0	0	0	0	2	3	0	1	0	0	0	0	0	6	36-45	5
05:45 AM	0	0	0	0	1	0	2	3	0	0	0	0	0	0	6	41-50	5
Day Total				DA	TA	THE	ATE	NOA	/EC	00	A AA	AL IN	HTH				
Percent				UF	MA	$\Box\Box$	9/1	JKIN	/ED	$\cup\cup$	IVIIV	1UN				1	
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	
Comments:																	
		0/2023 7:5											60	- O !!:		tp://www.guality	

QC JOB #: 16236660 LOCATION: Travis Rd north of Sellards Rd **SPECIFIC LOCATION: DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 41-50 38-47 06:15 AM 06:30 AM 36-45 06:45 AM 36-45 07:00 AM 41-50 07:15 AM 41-50 07:30 AM 36-45 36-45 07:45 AM 08:00 AM O 31-40 08:15 AM 29-38 08:30 AM 30-39 08:45 AM 36-45 09:00 AM 46-55 09:15 AM 36-45 09:30 AM 31-40 09:45 AM 31-40 10:00 AM 31-40 10:15 AM 36-45 10:30 AM 31-40 10:45 AM 41-50 11:00 AM 31-40 11:15 AM 26-35 11:30 AM 36-45 11:45 AM 36-45 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236660 LOCATION: Travis Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 31-40 12:15 PM 36-45 46-55 12:30 PM 12:45 PM 26-35 01:00 PM 31-40 01:15 PM 31-40 01:30 PM 35-44 01:45 PM 26-35 02:00 PM 46-55 02:15 PM 39-48 02:30 PM 41-50 02:45 PM 31-40 03:00 PM 46-55 03:15 PM 41-50 03:30 PM 46-55 03:45 PM 41-50 04:00 PM 41-50 04:15 PM 46-55 04:30 PM 46-55 04:45 PM 46-55 05:00 PM 36-45 05:15 PM 41-50 05:30 PM 36-45 05:45 PM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

DIRECTION: NB, SB DATE: Jun 13 2023

QC JOB #: 16236660

CITY/STATE: Benton, WA Number **Start Time** Total Pace Speed in Pace 06:00 PM 23-32 41-50 06:15 PM 06:30 PM 31-40 06:45 PM 46-55 07:00 PM 31-40 07:15 PM 36-45 07:30 PM 31-40 07:45 PM 41-50 08:00 PM O 26-35 08:15 PM 36-45 08:30 PM 36-45 08:45 PM 31-40 09:00 PM 26-35 09:15 PM O 31-40 09:30 PM n 1-10 09:45 PM 1-10 10:00 PM 26-35 10:15 PM 31-40 10:30 PM O 31-40 10:45 PM 1-10 11:00 PM 26-35 11:15 PM 26-35 11:30 PM 21-30 11:45 PM 1-10 O **Day Total** 36-45 0% 0.3% 0.8% 6.6% 20.4% 18.3% 2.4% 1.8% 0.4% 0.1% Percent 17.1% 19.1% 12.1% 0.5% AM Peak 12:00 AM 12:30 AM 12:00 AM 9:15 AM 8:00 AM 6:45 AM 5:15 AM 10:45 AM 5:00 AM 8:15 AM 5:00 AM 10:00 AM 11:00 AM 9:15 AM 5:15 AM 15-min Vol PM Peak 12:00 PM 12:00 PM 2:15 PM 12:00 PM 3:30 PM 6:30 PM 4:00 PM 4:30 PM 4:30 PM 4:45 PM 12:15 PM 12:00 PM 1:00 PM 12:00 PM 2:15 PM 15-min Vol

Comments:

LOCATION: Tra	avis Rd n	orth of Se	ellards Rd														#: 16236660
SPECIFIC LOCA																DIREC	ΓΙΟΝ: NB, SB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	Pace
Grand Total	0	2	6	49	126	141	150	135	89	18	13	4	3	1	737	36-45	291
Percent	0%	0.3%	0.8%	6.6%	17.1%	19.1%	20.4%	18.3%	12.1%	2.4%	1.8%	0.5%	0.4%	0.1%	/5/	30-43	291
Cumulative Percent	0%	0.3%	1.1%	7.7%	24.8%	44%	64.3%	82.6%	94.7%	97.2%	98.9%	99.5%	99.9%	100%			
ADT 737															Me		
Comments:															•		

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB, SB

**DATE**: Jun 13 2023

CITY/STATE: Be	IIIOII, WA	Cars &	2 Ande		2 Avla C	3 Axle	4 Avls	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 AxI		un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle	Buses	2 Axle 6		4 Axle		5 Axie Double	>6 AXI Double				Not	Total
			Long		Tire	Single	Single	Double			Multi	Multi	Multi	Classed	<u> </u>
12:00 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
12:15 AM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
12:30 AM	0	2	0	0	1	0	0	1	0	0	0	0	0	0	4
12:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
04:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
04:30 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
04:45 AM	0	2	2	2	0	0	0	1	0	0	0	0	0	0	7
05:00 AM	0	9	5	0	0	0	0	0	1	0	0	0	0	0	15
05:15 AM	0	12	5	2	0	0	0	1	1	0	0	0	0	0	21
05:30 AM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
05:45 AM	0	5	0	0	0	1	0	0	0	0	0	0	0	0	6
Day Total															I
Percent				DAIL	AIHA	ALDI	RIVE	366	IVIIVI	$\cup$ IVIII	15				<b></b>
															I
															I
ADT															I
737															I
															I
															I
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
comments:															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236660 DIRECTION: NB, SB DATE: Jun 13 2023

15-min Vol PM Peak 15-min Vol	Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 2023 <b>Total</b>
06:15 AM																
06:30 AM																5
06:45 AM		_		•	-	-	•			U	_	•	_	-	•	7
07:00 AM										•				-	_	8
07:15 AM				•	_					•				•	-	19
07:30 AM				•	•					_	_			-		9
07:45 AM		_		•	-	_				Ū	•	_		•		11
08:00 AM				_	_					•	•	_	_	•	ū	4
08:15 AM														-	_	5
08:30 AM				7	•						-			•	-	13
08:45 AM		_		•	-					0				-	•	9
09:00 AM					-					0	_	_		•	ū	16
09:15 AM					_					Ū				•	-	6
09:30 AM										·	_			-		7
09:45 AM				_	_					·	_			_	-	11
10:00 AM		_	_	_	-					_	•	_	-	-	•	5
10:15 AM											_	_			ū	9
10:30 AM				_	_				_		_			_	_	13
10:45 AM			•	-	•						_	_	_	•	_	9
11:00 AM			-		_	_					•			_	-	15
11:15 AM		_								ū	-			_	_	11
11:30 AM						0								-		13
11:45 AM														•		8
Day Total Percent  ADT 737  AM Peak 15-min Vol  PM Peak 15-min Vol  AM Peak 15-min Vol  AM Peak 15-min Vol  AM Peak 15-min Vol  AM Peak 15-min Vol																12
ADT 737  AM Peak 15-min Vol  PM Peak 15-min Vol		1	5	1	3	2	0	0	0	1	0	0	0	0	0	13
ADT 737  AM Peak 15-min Vol PPAR 15-min Vol	•															
AM Peak 15-min Vol  PM Peak 15-min Vol							41 10	NIVI.		7101101	CHUIL.					
15-min Vol PM Peak 15-min Vol																
15-min Vol	AM Peak 15-min Vol															
	PM Peak 15-min Vol															
mments;	mments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB, SB DATE: Jun 13 2023

Cars & 2 Axle 2 Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Bikes Start Time Buses Total **Trailers** Tire Single Single Double Double **Double** Multi Multi Multi Classed Long 12:00 PM 12:15 PM 12:30 PM 12:45 PM 01:00 PM 01:15 PM 01:30 PM 01:45 PM 02:00 PM 02:15 PM 02:30 PM 02:45 PM 03:00 PM 03:15 PM 03:30 PM 03:45 PM 04:00 PM 04:15 PM 04:30 PM 04:45 PM 05:00 PM 05:15 PM 05:30 PM 05:45 PM Day Total Percent ADT AM Peak 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB, SB

**DATE:** Jun 13 2023

	enton, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
Start Time	Bikes	Trailers	Long	Buses	Z Axie 6 Tire	Single	Single	Nouble	Double	Double	Nulti	Multi	>6 Axi Multi	Classed	Total
06:00 PM	2	2	1	2	1	0	0	0	1	0	0	0	0	0	9
06:15 PM	0	4	3	0	1	0	0	1	1	0	0	0	0	0	10
06:30 PM	1	3	1	3	1	1	0	1	1	0	0	0	0	0	12
06:45 PM	0	8	3	0	0	0	0	0	1	0	0	0	0	0	12
07:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
07:15 PM	0	3	1	0	0	0	0	3	0	0	0	0	0	0	7
07:30 PM	2	5	0	1	0	0	0	0	1	0	0	0	0	0	9
07:45 PM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
08:00 PM	0	0	2	0	0	1	0	0	0	0	0	0	0	0	3
08:15 PM	0	2	1	1	0	0	0	0	0	0	0	0	0	0	4
08:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:45 PM	0	2	1	1	0	0	0	0	0	0	0	0	1	0	5
09:00 PM	0	0	0	0	2	0	0	1	1	0	0	0	0	0	4
09:15 PM	1	2	0	1	0	0	0	0	0	0	0	0	0	0	4
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	2	0	1	0	0	1	0	0	1	0	0	0	0	0	5
10:15 PM	0	2	1	0	0	0	0	1	0	0	0	0	0	0	4
10:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
11:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
11:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	19	269	168	57	88	14	0	75	32	7	1	1	6	0	737
Percent	2.6%	36.5%	22.8%	7.7%	11.9%	1.9%	0%	10.2%	4.3%	0.9%	0.1%	0.1%	0.8%	0%	/3/
ADT															
737															
AM Peak	10:00 AM	5:15 AM	8:30 AM	8:00 AM	6:30 AM	8:30 AM	12:00 AM	10:15 AM	9:30 AM	8:15 AM	12:00 AM	8:30 AM	9:15 AM	12:00 AM	5:15 A
15-min Vol	1	12	8	4	3	3	0	4	2	1	0	1	1	0	21
PM Peak	6:00 PM	5:00 PM	3:45 PM	2:15 PM	2:15 PM	12:00 PM		1:30 PM	3:30 PM	1:00 PM	5:00 PM	12:00 PM	3:45 PM	12:00 PM	2:15 PI
15-min Vol	2	14	5.45 FIVI 6	3	7 7	2	0	1.30 FW	3.30 FW	1.00 FW	1	0	3.43 FW	0	2.13 F1
omments:															

LOCATION: Travis Rd north of Sellards Rd QC JOB #: 16236660 SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Cars & 2 Axle 2 Axle 6 4 Axle <5 Axl 5 Axle >6 Axl 3 Axle <6 Axl 6 Axle >6 Axl Not Start Time **Bikes Buses** Total **Trailers** Long Tire Single Single Double Double Double Multi Multi Multi Classed **Grand Total** 19 269 168 57 88 14 0 75 32 7 1 1 6 0 737 10.2% 0.8% 0% Percent 2.6% 36.5% 22.8% 7.7% 11.9% 1.9% 0% 4.3% 0.9% 0.1% 0.1% ADT 737 Comments:

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: NB, SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		2				2			2	
12:15 AM		3				3			3	
12:30 AM		4				4			4	
12:45 AM		1				1			1	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		1				1			1	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		1				1			1	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		1				1			1	
04:00 AM		2				2			2	
04:15 AM		2				2			2	
04:30 AM		4				4		IIO'	4	
04:45 AM		7				7		411	7	
05:00 AM		15				15			15	
05:15 AM		21				21	00.000		21	
05:30 AM		6				JK / 6 5 C	DIVIIVI	UNII	6	
05:45 AM		6				6			6	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660 DIRECTION: NB, SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		5				5			5	
06:15 AM		7				7			7	
06:30 AM		8				8			8	
06:45 AM		19				19			19	
07:00 AM		9				9			9	
07:15 AM		11				11			11	
07:30 AM		4				4			4	
07:45 AM		5				5			5	
08:00 AM		13				13			13	
08:15 AM		9				9			9	
08:30 AM		16				16			16	
08:45 AM		6				6			6	
09:00 AM		7				7			7	
09:15 AM		11				11			11	
09:30 AM		5				5			5	
09:45 AM		9				9			9	
10:00 AM		13				13			13	
10:15 AM		9				9			9	
10:30 AM		15				9 15			15	
10:45 AM		11				11	-		11	
11:00 AM		13				13			13	
11:15 AM		8				8			8	
11:30 AM		12				8 12	DIVIN		12	
11:45 AM		13				13			13	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660 DIRECTION: NB, SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		16				16			16	
12:15 PM		13				13			13	
12:30 PM		11				11			11	
12:45 PM		7				7			7	
01:00 PM		11				11			11	
01:15 PM		11				11			11	
01:30 PM		11				11			11	
01:45 PM		8				8			8	
02:00 PM		9				9			9	
02:15 PM		24				24			24	
02:30 PM		9				9			9	
02:45 PM		8				8			8	
03:00 PM		12				12			12	
03:15 PM		13				13			13	
03:30 PM		17				17			17	
03:45 PM		22				22			22	
04:00 PM		15				15			15	
04:15 PM		10				10			10	
04:30 PM		23				23			23	
04:45 PM		18				18			18	
05:00 PM		21				21			21	
05:15 PM		13				13 17	00 40 4		13	
05:30 PM		17					JIVIIVI		17	
05:45 PM		5				5			5	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION: CITY/STATE: Benton, WA DIRECTION: NB, SB

**DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236660

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		9				9			9	
06:15 PM		10				10			10	
06:30 PM		12				12			12	
06:45 PM		12				12			12	
07:00 PM		1				1			1	
07:15 PM		7				7			7	_
07:30 PM		9				9			9	
07:45 PM		4				4			4	
08:00 PM		3				3			3	
08:15 PM		4				4			4	
08:30 PM		1				1			1	
08:45 PM		5				5			5	
09:00 PM		4				4			4	
09:15 PM		4				4			4	
09:30 PM		0				0			0	
09:45 PM		0				0			0	
10:00 PM		5				5			5	
10:15 PM		4				4			4	
10:30 PM		1			211	1		IIO.	1	
10:45 PM		0			all	0		<b>411</b>	0	
11:00 PM		1				1			1	
11:15 PM		1			7 1 4 77 1	DRIVES CO	00.00.		1	
11:30 PM		1			HALL	DRIVES CO	DIVIN	UNII	/ES 1	
11:45 PM		0				0			0	
Day Total		737				737			737	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		5:15 AM				5:15 AM			5:15 AM	
15-min Vol		21				21			21	
PM Peak		2:15 PM				2:15 PM			2:15 PM	
15-min Vol		24				24			24	
Comments:										

Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12.00 414															1	41.50	
12:00 AM 12:15 AM	0 0	0 0	0 0	0 1	0 1	0 1	0 0	1 0	0 0	0 0	0 0	0 0	0 0	0 0	1 3	41-50 26-35	1 2
12:30 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	21-30	1
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:05 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
04:00 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	21-30	1
04:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
04:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
04:45 AM	0	0	0	0	1	2	1	1	0	0	0	0	0	0	5	36-45	3
05:00 AM	0	0	0	0	0	1	5	3	2	0	0	0	0	0	11	41-50	8
05:15 AM	0	0	0	1	1	4	6	3	0	0	0	0	0	0	15	36-45	10
05:30 AM	0	0	0	0	0	2	3	0	1	0	0	0	0	0	6	36-45	5
05:45 AM	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3	41-50	3
Day Total				-	-				·								
Percent																	
AM Peak															•		
5-min Vol																	
PM Peak																	

LOCATION: T SPECIFIC LOC CITY/STATE:	CATION:															QC JOB #: 1 DIREC DATE: Jun	TION: SE
	<b>1</b>	16	21	26	31	36	41	46	51	56	61	66	71	76			Numbe
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pace
06:00 AM	0	0	0	1	0	0	1	1	0	0	0	0	0	0	3	41-50	2
06:15 AM	0	0	0	0	0	1	3	0	2	0	0	0	0	0	6	36-45	4
06:30 AM	0	0	0	0	1	1	1	1	0	0	0	0	0	0	4	31-40	2
06:45 AM	0	0	0	0	1	8	2	1	0	0	0	0	0	0	12	36-45	10
07:00 AM	0	0	0	1	0	1	3	2	0	0	0	0	0	0	7	41-50	5
07:15 AM	0	0	0	0	0	0	6	0	0	0	0	0	0	0	6	36-45	6
07:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
07:45 AM	0	1	0	0	1	1	2	0	0	0	0	0	0	0	5	36-45	3
08:00 AM	0	0	0	0	3	2	0	2	0	0	0	0	0	0	7	31-40	5
08:15 AM	0	0	0	1	4	1	0	0	0	0	0	0	0	0	6	30-39	5
08:30 AM	0	0	0	2	2	1	1	1	2	0	0	0	0	0	9	26-35	4
08:45 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
09:00 AM	0	0	0	1	0	0	0	1	1	0	0	0	0	0	3	46-55	2
09:15 AM	0	0	0	2	0	4	0	1	0	0	0	0	0	1	8	31-40	4
09:30 AM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	26-35	1
09:45 AM	0	0	0	1	2	3	1	0	0	0	0	0	0	0	7	31-40	5
10:00 AM	0	0	0	1	2	2	3	1	1	0	0	1	0	0	11	36-45	5
10:15 AM	0	0	0	0	0	2	2	1	0	0	0	0	0	0	5	36-45	4
10:30 AM	0	0	0	1	1	2	0	1	0	0	0	0	0	0	5	31-40	3
10:45 AM	0	0	0	0	1	0	1	2	0	0	1	0	0	0	5	41-50	3
11:00 AM	0	0	0	1	3	4	0	1	0	1	0	0	1	0	11	31-40	7
11:15 AM	0	0	0	0	0	0	0	0	2	0	0	0	1	0	3	46-55	2
11:30 AM	0	0	0	0	1	0	1	2	0	0	0	0	0	0	4	41-50	3
11:45 AM	0	0	0	2	1	1	1	0	0	0	0	0	0	0	5	26-35	3
Day Total				DA	TA	TH	ATI	NOA	/EC	0	N // N	AL IN	IITII				
Percent				UF	MA	$\Box$	ALL	JKI	/ED	$\Box$	IVIIV	TUIN					
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

QC JOB #: 16236660 LOCATION: Travis Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 26-35 41-50 12:15 PM 46-55 12:30 PM 12:45 PM 26-35 01:00 PM 31-40 01:15 PM 31-40 01:30 PM 31-40 01:45 PM 26-35 02:00 PM 26-35 02:15 PM 41-50 02:30 PM 31-40 02:45 PM 26-35 03:00 PM 31-40 03:15 PM 31-40 03:30 PM 26-35 03:45 PM 41-50 04:00 PM 41-50 04:15 PM 41-50 04:30 PM O 36-45 04:45 PM 31-40 05:00 PM 26-35 05:15 PM 41-50 05:30 PM 31-40 05:45 PM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: SB

ATE: Jun 13 2023

CITY/STATE:	Benton,	WA														DATE: Jur	า 13 202
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numb
	15	20	25	30	35	40	45	50	55	60	65	70	75	999		. ass speed	in Pa
06:00 PM	0	0	1	1	0	1	0	0	0	0	0	0	0	0	3	21-30	2
06:15 PM	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3	41-50	3
06:30 PM	0	0	0	0	0	2	0	0	1	0	0	0	0	0	3	31-40	2
06:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:15 PM	0	0	0	0	0	0	2	1	0	0	0	0	0	0	3	41-50	3
07:30 PM	0	0	0	0	0	1	1	0	0	0	0	0	0	0	2	36-45	2
07:45 PM	0	0	0	0	1	0	1	1	0	0	0	0	0	0	3	41-50	2
08:00 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2	26-35	1
08:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
08:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
08:45 PM	0	0	0	0	0	1	1	0	0	0	0	0	0	0	2	36-45	2
09:00 PM	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3	21-30	3
09:15 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	2	31-40	2
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
10:15 PM	0	0	0	0	1	1	1	0	0	0	0	0	0	0	3	31-40	2
10:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	21-30	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	1	5	40	53	81	74	52	17	5	1	2	3	1	335	36-45	155
Percent	0%	0.3%	1.5%	11.9%	15.8%	24.2%	22.1%	15.5%	5.1%	1.5%	0.3%	0.6%	0.9%	0.3%	333	30-43	133
AM Peak	12:00 AM		12:00 AM	8:30 AM	8:15 AM	6:45 AM	5:15 AM	5:00 AM	5:00 AM			10:00 AM			5:15 AM		
15-min Vol	0	1	0	2	4	8	6	3	2	1	1	1	1	1	15		
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	2:15 PM 2	12:00 PM 3	3:30 PM 4	12:00 PM 3	4:00 PM 4	2:15 PM 3	12:30 PM 2	1:00 PM 1	12:00 PM 0	12:00 PM 1	1:00 PM 1	12:00 PM 0	2:15 PM 14		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Tra	vis Rd n	orth of Se	ellards Rd													QC JOB	#: 16236660
SPECIFIC LOCA	TION:															DI	RECTION: SB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	l lotai	Tucc Specu	Pace
Grand Total	0	1	5	40	53	81	74	52	17	5	1	2	3	1	335	36-45	155
Percent	0%	0.3%	1.5%	11.9%	15.8%	24.2%	22.1%	15.5%	5.1%	1.5%	0.3%	0.6%	0.9%	0.3%	333	30-43	133
Cumulative Percent	0%	0.3%	1.8%	13.7%	29.6%	53.7%	75.8%	91.3%	96.4%	97.9%	98.2%	98.8%	99.7%	100%			
ADT 335															Me	an Speed(Avera Med	ntile: 47 MPH age): 39 MPH dian: 39 MPH ode: 38 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236660 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 AM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
12:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:45 AM	0	0	2	2	0	0	0	1	0	0	0	0	0	0	5
05:00 AM	0	6	5	0	0	0	0	0	0	0	0	0	0	0	11
05:15 AM	0	10	4	0	0	0	0	1	0	0	0	0	0	0	15
05:30 AM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
05:45 AM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
Day Total				DAT	A 77 I	A T D	DA /E	000	11/1/1	LIMIT	-11-0				
Percent				DAIA	A 1171/	ALD	RIVE	366	IIVIIVI	UIVILI	IES				
ADT 335															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	L C /20 /20	222 7 50 414									COLLBGE O			. / /	4 4

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: SB

**DATE:** Jun 13 2023

Start Time	Start line	CITY/STATE: Be	enton, WA														un 13 2023
Trailers   Long   Tire   Single   Single   Double   Double   Double   Double   Multi   Multi   Multi   Classed	Trailers   Long   Tire   Single   Single   Double   Double   Double   Double   Multi   Multi   Multi   Classed	Start Time	Rikes			Ruses											Total
06:15 AM 0 2 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 6 6 6 6 30 AM 0 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:39 AM 0 0 2 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
06:35 AM 0 0 0 1 0 0 2 0 0 0 1 0 0 0 0 0 0 0 0 1 1 0 0 0 0	06:39 AM		0	0	2	0	0	0	0	1	0	0	0	0	0	0	3
06.45 AM 0 6 6 3 1 0 0 0 0 2 0 0 0 0 0 0 0 0 0 12 0 0 0 0	06:45 AM 0 6 6 3 1 0 0 0 0 2 0 0 0 0 0 0 0 0 0 0 7 7 07:15 AM 0 1 3 3 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0		0	2	4	0	0	0	0	0	0	0	0	0	0	0	6
07:0AM	07:00 AM		0	0	1	0	2	0	0	1	0	0	0	0	0	0	4
07:15 AM	07:15 AM		0	6	3	1	0	0	0	2	0	0	0	0	0	0	
07-30 AM	07:45 AM		0	3	3	0	1	0	0	0	0	0	0	0	0	0	7
07.55 AM	02:45 AM		0	1	3	1	0	0	0	1	0	0	0	0	0	0	6
08:05 AM	08:00 AM		0	0	1	0	0	0	0		0	0	0	0	0	0	1
08:15 AM 0 1 1 1 1 1 1 0 0 0 1 0 1 0 0 0 0 0 0	08:15 AM		0	0	3	0	0	1	0	0	1	0	0	0	0	0	5
08:30 AM	08:30 AM		0	1	3	2	0	0	0	1	0	0	0	0	0	0	7
08:45 AM	08:45 AM			1		1					0	1		0	0	0	
09:00 AM	09:00 AM		0	1	5	0	0			0	0	1	0	1	0	0	9
09:15 AM	09:15 AM		0	0	1	0	0			0	0	0			0	0	
09:30 AM	09:30 AM		0	1	1	0	1	0		0	0	0	0	0	0	0	
09:45 AM	09:45 AM		0	1	5	1	0	0	0	1	0	0	0	0	0	0	8
10:00 AM	10:00 AM		0	0	2	0	0				0	0	0	0	0	0	
10:15 AM	10:15 AM		0	1	0	1	2	0	0	3	0	0	0	0	0	0	=
10:30 AM	10:30 AM	10:00 AM	1	5	2	1	1	0	0		0	0	0	0	0	0	11
10:45 AM	10:45 AM	10:15 AM	0	0	3	0	0	0	0	2	0	0	0	0	0	0	5
11:00 AM	11:00 AM		1	1	0	1	1	0	0	1	0	0	0	0	0	0	5
11:15 AM	11:15 AM	10:45 AM	1	2	0	1	0		0	1	0	0	0	0	0	0	5
11:30 AM	11:30 AM	11:00 AM	0	4	4	2			00	1	0			0	0	0	11
11:45 AM	11:45 AM		0	1	1				0	0	0			0	0	0	3
Day Total Percent  ADT 335  AM Peak 15-min Vol PM P	Day Total Percent  ADT 335  AM Peak 15-min Vol  PM Peak 15-min Vol	11:30 AM	0	1	3		0	0	0	0	0	0	0	0	0	0	4
ADT 335  AM Peak 15-min Vol PM Peak 15-min Vol	ADT 335  AM Peak 15-min Vol  PM Peak 15-min Vol	11:45 AM	1	1	0	1	1	0	0	0	1	0	0	0	0	0	5
ADT 335  AM Peak 15-min Vol  PM Peak 15-min Vol	ADT 335  AM Peak 15-min Vol  PM Peak 15-min Vol	Day Total															
335 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol	335  AM Peak 15-min Vol  PM Peak 15-min Vol	Percent				DATA	ATH	ATD	RIVE	SCC	MM	UNIT	11-5				
335 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol	335  AM Peak 15-min Vol  PM Peak 15-min Vol																
335 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol	335  AM Peak 15-min Vol  PM Peak 15-min Vol																
335 AM Peak 15-min Vol PM Peak 15-min Vol  15-min Vol	335  AM Peak 15-min Vol  PM Peak 15-min Vol	ADT															
AM Peak 15-min Vol  PM Peak 15-min Vol	AM Peak 15-min Vol  PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	15-min Vol PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	15-min Vol PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	15-min Vol PM Peak 15-min Vol	ANA Dook															
PM Peak 15-min Vol	PM Peak 15-min Vol																
15-min Vol	15-min Vol																
<u> </u>																	
	Comments:																

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236660 DIRECTION: SB
DATE: Jun 13 2023

1		Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
Start Time	Bikes	Trailers	Long	Buses	Z Axie 6 Tire	Single	4 Axie Single	<5 Axi Double	5 Axie Double	>6 AXI Double	< 6 Axi Multi	6 Axie Multi	>6 AXI Multi	Classed	Total
12:00 PM	0	3	2	2	0	0	0	1	0	0	0	0	0	0	8
12:15 PM	0	2	2	1	1	0	0	0	0	0	0	0	0	0	6
12:30 PM	0	1	3	1	1	0	0	0	0	0	0	0	0	0	6
12:45 PM	0	0	2	0	1	0	0	2	0	0	0	0	0	0	5
01:00 PM	0	3	1	0	0	0	0	1	0	0	0	0	0	0	5
01:15 PM	0	1	1	0	1	0	0	1	1	1	0	0	0	0	6
01:30 PM	0	0	2	0	0	0	0	2	0	0	0	0	0	0	4
01:45 PM	0	1	3	0	0	0	0	1	0	0	0	0	0	0	5
02:00 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
02:15 PM	1	3	4	2	2	0	0	2	0	0	0	0	0	0	14
02:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
02:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
03:00 PM	0	2	1	0	0	0	0	1	0	0	0	0	0	0	4
03:15 PM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
03:30 PM	0	2	0	1	1	0	0	1	1	0	0	0	0	0	6
03:45 PM	0	3	2	0	0	0	0	1	1	0	0	0	0	0	7
04:00 PM	0	3	1	0	0	0	0	1	0	0	0	0	0	0	5
04:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:30 PM	0	5	0	0	0	0	0	0	0	0	0	0	0	0	5
04:45 PM	0	1	3	0	0	0	0	0	0	0	0	0	0	0	4
05:00 PM	0	3	1	0	1	0	0	1	0	0	0	0	0	0	6
05:15 PM	1	3	1	0	0	0	0	0	0	0	0	0	0	0	5
05:30 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total Percent															
ADT 335			-												
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:		22.7.50.484		-		-	-	-				-		//	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236660 DIRECTION: SB

**DATE:** Jun 13 2023

Tart Lime    Bikes   Trailers   Long   Buses   Tire   Single   Single   Double   Double   Double   Multi   Multi   Multi   Classed   Into	LITY/STATE: B	<del></del>	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
196.15 PM 0 2 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	Bikes			Buses											Total
16:30 PM	06:00 PM	1	1	1	0	0	0	0	0	0	0	0	0	0	0	3
15645 PM 0 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:15 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
077.0 PM	06:30 PM	0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
177:15 PM	06:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
17:30 PM	07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
177-45 PM	07:15 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
18:00 PM	07:30 PM	1	1	0	0	0	0	0	0	0	0	0	0	0	0	2
18:15 PM	07:45 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
08:30 PM	08:00 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
18:45 PM 0 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	08:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
19:00 PM	08:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
99:15 PM	08:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
09:30 PM	09:00 PM	0	0	0	0	2	0	0	1	0	0	0	0	0	0	3
09:45 PM	09:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	2
10:00 PM	09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	10:00 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
10:45 PM	10:15 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
11:00 PM	10:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
AMP Peak   10:00 AM   5:15 AM   5:00 AM   4:45 AM   6:30 AM   7:45 AM   12:00 PM   12:45 PM   12:00	10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 335  ADT 335  ADT BY PERCENT 10:00 AM 5:15 AM 5:00 AM 4:45 AM 6:30 AM 7:45 AM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 12:00 PM 12:0	11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total Percent         10         116         106         25         24         2         0         43         5         3         0         1         0         0         0         0         33           ADT 335         ADT 335         10:00 AM 5:15 AM 5:00 AM 4:45 AM 6:30 AM 7:45 AM 12:00 AM 7:45 AM 12:00 AM 9:45 AM 7:45 AM 8:15 AM 12:00 AM 8:30 AM 12:00 AM 8:30 AM 12:00 AM 12:00 AM 5:15 AM 12:00	11:30 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
ADT 335  AM Peak 5-min Vol 1 50 AM 2:15 PM 4:30 PM 2:15 PM 12:00 PM 2:15 PM 12:00 PM 12:00 PM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 1:15 PM 12:00 PM 12	11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 335  AM Peak 10:00 AM 5:15 AM 5:00 AM 4:45 AM 6:30 AM 7:45 AM 12:00 AM 9:45 AM 7:45 AM 8:15 AM 12:00 AM 8:30 AM 12:00 AM 12:00 AM 5:15 AM 10 5 2 2 1 1 0 3 1 1 0 1 0 0 1 1 0 0 1 1 0 0 1 1 0 0 1 1 0 0 0 1 1 0 0 0 1 1 0 0 0 0 1 1 0	Day Total														-	335
335 AM Peak 10:00 AM 5:15 AM 5:00 AM 4:45 AM 6:30 AM 7:45 AM 12:00 AM 9:45 AM 7:45 AM 8:15 AM 12:00 AM 8:30 AM 12:00 AM 12:00 AM 5:15 AM 5:min Vol 1 10 5 2 2 1 1 0 3 1 1 0 1 0 0 1 0 0 1 12:00 PM 12:00 PM Peak 2:15 PM 4:30 PM 2:15 PM 12:00 PM 2:15 PM 12:00 PM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 12:00 P	Percent	3%	34.6%	31.6%	7.5%	7.2%	0.6%	0%	12.8%	1.5%	0.9%	0%	0.3%	0%	0%	
AM Peak 5-min Vol 1 10 5 2 2 1 10 00 PM 12:00 PM 2:15 PM 12:00 PM 2:15 PM 12:00 PM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 12:00 PM 12:																
5-min Vol 1 10 5 2 2 1 0 3 1 1 0 1 0 0 0 15  PM Peak 2:15 PM 4:30 PM 2:15 PM 12:00 PM 2:15 PM 12:00 PM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 12:00 PM 1	335															
PM Peak 2:15 PM 4:30 PM 2:15 PM 12:00 PM 2:15 PM 12:00 PM 12:00 PM 12:00 PM 12:45 PM 1:15 PM 1:15 PM 12:00 PM 12:00 PM 12:00 PM 12:00 PM 2:15 PM 12:00 PM 12	AM Peak	10:00 AM	5:15 AM	5:00 AM	4:45 AM	6:30 AM	7:45 AM	12:00 AM	9:45 AM	7:45 AM	8:15 AM	12:00 AM	8:30 AM	12:00 AM	12:00 AM	5:15 A
5-min Vol 1 5 4 2 2 0 0 2 1 1 0 0 0 14	15-min Vol	1	10	5	2	2	1	0	3	1	1	0	1	0	0	15
5-min Vol 1 5 4 2 2 0 0 2 1 1 0 0 0 14	PM Peak	2:15 PM	4:30 PM	2:15 PM	12:00 PM	2:15 PM	12:00 PM	12:00 PM	12:45 PM	1:15 PM	1:15 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	2:15 PI
nments:	15-min Vol	1		4		2	0				1	0	0	0	0	14
	mments:															

LOCATION: Travis Rd north of Sellards Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

Start Time

Bikes

Cars & 2 Axle

Buses

Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl 6 Axle >6 Axl 6 Axle >6 Axl Not

Total

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	10 3%	116 34.6%	106 31.6%	25 7.5%	24 7.2%	2 0.6%	0 0%	43 12.8%	5 1.5%	3 0.9%	0 0%	1 0.3%	0 0%	0 0%	335
ADT 335															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA

**DIRECTION: SB DATE**: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236660

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		1				1			1	
12:15 AM		3				3			3	
12:30 AM		1				1			1	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		1				1			1	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		1				1			1	
04:00 AM		1				1			1	
04:15 AM		1			. I °	1			1	
04:30 AM		1				1		ın.	1	
04:45 AM		5				5		<i>.</i> 411	5	
05:00 AM		11				11			11	
05:15 AM		15			TIATI	15 6	O A A A A	11 TK 117	15	
05:30 AM		6			HALL		JIVIIVI	UNII	6	
05:45 AM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: E								DA	TE: Jun 13 2023 - Jun 13 2023
Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM	3				3			3	
06:15 AM	6				6			6	
06:30 AM	4				4			4	
06:45 AM	12				12			12	
07:00 AM	7				7			7	
07:15 AM	6				6			6	
07:30 AM	1				1			1	
07:45 AM	5				5			5	
08:00 AM	7				7			7	
08:15 AM	6				6			6	
08:30 AM	9				9			9	
08:45 AM	1				1			1	
09:00 AM	3				3			3	
09:15 AM	8				8			8	
09:30 AM	2				2			2	
09:45 AM	7				7			7	
10:00 AM	11				11			11	
10:15 AM	5				5			5	
10:30 AM	5				5			5	
10:45 AM	5				5	-		5	
11:00 AM	11				11			11	
11:15 AM	3				3			3	
11:30 AM	4				3	DIVIN		4	
11:45 AM	5				5			5	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak									
15-min Vol									
PM Peak									
15-min Vol									

Comments:

QC JOB #: 16236660 **DIRECTION: SB** 

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236660 DIRECTION: SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		8				8			8	
12:15 PM		6				6			6	
12:30 PM		6				6			6	
12:45 PM		5				5			5	
01:00 PM		5				5			5	
01:15 PM		6				6			6	
01:30 PM		4				4			4	
01:45 PM		5				5			5	
02:00 PM		2				2			2	
02:15 PM		14				14			14	
02:30 PM		1				1			1	
02:45 PM		2				2			2	
03:00 PM		4				4			4	
03:15 PM		3				3			3	
03:30 PM		6				6			6	
03:45 PM		7				7			7	
04:00 PM		5				5			5	
04:15 PM		1			_ I º .	1	-		1	
04:30 PM		5				5		In.	5	
04:45 PM		4			161	4		411	4	
05:00 PM		6				6			6	
05:15 PM		5			LIATI	DRIV3ES CO	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	LIKIT	5	
05:30 PM		3			MALL		JIVIIVI	UIVII	3	
05:45 PM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

**DATE**: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236660

**DIRECTION: SB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		3				3			3	
06:15 PM		3				3			3	
06:30 PM		3				3			3	
06:45 PM		2				2			2	
07:00 PM		0				0			0	
07:15 PM		3				3			3	
07:30 PM		2				2			2	
07:45 PM		3				3			3	
08:00 PM		2				2			2	
08:15 PM		1				1			1	
08:30 PM		1				1			1	
08:45 PM		2				2			2	
09:00 PM		3				3			3	
09:15 PM		2				2			2	
09:30 PM		0				0			0	
09:45 PM		0				0			0	
10:00 PM		1				1			1	
10:15 PM		3				3			3	
10:30 PM		1				1		In	1	
10:45 PM		0				0	$\sim$	411	0	
11:00 PM		0				0			0	
11:15 PM		0				0	0.000		0	
11:30 PM		1				DRIVIES CO	DIVIN	UNII	1 1	
11:45 PM		0				0			0	
Day Total		335				335			335	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		5:15 AM				5:15 AM			5:15 AM	
15-min Vol		15				15			15	
PM Peak		2:15 PM				2:15 PM			2:15 PM	
15-min Vol		14				14			14	

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 12:15 AM 1-10 12:30 AM 1-10 12:45 AM 1-10 01:00 AM 1-10 01:15 AM 1-10 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O O 1-10 02:15 AM 1-10 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 1-10 03:15 AM 1-10 03:30 AM 1-10 03:45 AM 1-10 04:00 AM 1-10 04:15 AM 1-10 04:30 AM O 1-10 04:45 AM 26-35 05:00 AM 1-10 05:15 AM 1-10 05:30 AM 1-10 05:45 AM 26-35 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 1-10 06:15 AM 36-45 06:30 AM 1-10 06:45 AM 46-55 07:00 AM 1-10 07:15 AM 1-10 07:30 AM 1-10 07:45 AM 1-10 08:00 AM O O 1-10 08:15 AM 1-10 08:30 AM 1-10 08:45 AM 36-45 09:00 AM 1-10 09:15 AM 1-10 09:30 AM 11-20 09:45 AM 41-50 10:00 AM 1-10 10:15 AM 1-10 10:30 AM O 1-10 10:45 AM 1-10 11:00 AM 26-35 11:15 AM 36-45 11:30 AM 1-10 11:45 AM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 1-10 12:15 PM 1-10 12:30 PM 26-35 12:45 PM 31-40 01:00 PM 1-10 01:15 PM 1-10 26-35 01:30 PM 01:45 PM 41-50 02:00 PM O 1-10 02:15 PM 1-10 02:30 PM 26-35 02:45 PM 1-10 03:00 PM 31-40 03:15 PM 21-30 03:30 PM 1-10 03:45 PM 16-25 36-45 04:00 PM 04:15 PM 31-40 04:30 PM O 1-10 04:45 PM 31-40 05:00 PM 36-45 05:15 PM 46-55 05:30 PM 31-40 05:45 PM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Badger Canyon Rd north of Sellards Rd

SPECIFIC LOCATION:

DIRECTION: NB

DIRECTION: NB

CITY/STATE: Benton, WA DATE: Jun 13 2023

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	l	1 ace speed	in Pace
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:00 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	26-35	2
07:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	26-35	2
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1-10	1
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	1	1	2	3	10	11	9	4	2	0	0	0	0	0	43	31-40	21
Percent	2.3%	2.3%	4.7%	7%	23.3%	25.6%	20.9%	9.3%	4.7%	0%	0%	0%	0%	0%	43	31-40	21
AM Peak				11:00 AM		12:00 AM	6:15 AM	9:45 AM	6:45 AM						11:00 AM		
15-min Vol	0	1	0	2	1	0	1	1	1	0	0	0	0	0	3		
PM Peak 15-min Vol	8:15 PM 1	12:00 PM 0	3:45 PM 1	3:15 PM 1	7:00 PM 2	5:30 PM 3	5:00 PM 2	1:45 PM 1	5:15 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	3:15 PM 3		
TO-UIIII VOI	1	U	1	1				1	1	U	U	U	U	0	3		

LOCATION: Ba	dger Can	yon Rd n	orth of Se	llards Rd												QC JOB	#: 16236661
SPECIFIC LOCA	TION:															DIF	RECTION: NB
CITY/STATE: B	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Kange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	TOTAL	Pace Speed	Pace
<b>Grand Total</b>	1	1	2	3	10	11	9	4	2	0	0	0	0	0	43	21 40	21
Percent	2.3%	2.3%	4.7%	7%	23.3%	25.6%	20.9%	9.3%	4.7%	0%	0%	0%	0%	0%	43	31-40	21
Cumulative Percent	2.3%	4.7%	9.3%	16.3%	39.5%	65.1%	86%	95.3%	100%	100%	100%	100%	100%	100%			
ADT 43										7					Me	an Speed(Avera Med	ntile: 44 MPH age): 37 MPH dian: 37 MPH ode: 38 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236661 DIRECTION: NB
DATE: Jun 13 2023

Start Ime    Bikes   Trailers   Long   Buses   Trire   Single   Single   Double   Double   Double   Multi   Multi   Multi   Classed   II	CITY/STATE: Be	inton, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
12:00 AM	Start Time	Bikes			Buses											Total
12:30 AM	12:00 AM	0	0		0	0			0	0	0	0	0	0	0	0
12:45 AM	12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:35 AM	01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	03:30 AM	0	0	0	0	0		0	0	0	0	0	0	0	0	0
04:15 AM	03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 AM	04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	04:45 AM	0	0	1	0	0		0	0	0	0	0	0	0	0	1
05:30 AM	05:00 AM	0	0	0	0			0	0	0		0	0	0	0	0
O5:45 AM	05:15 AM	0	0	0	0	0	0	0	0	0		0	0	0	0	0
ADT 43  AM Peak 15-min Vol  PM Peak 15-min Vol  AM Peak 15-min Vol  PM Peak 15-min Vol	05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 43  AM Peak 15-min Vol  PM Peak 15-min Vol	05:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
ADT 43  AM Peak 15-min Vol PM Peak 15-min Vol																
AM Peak 15-min Vol PM Peak 15-min Vol  The state of the s	refeelit				LALA		41 1/1	KIVE	3.00	ZIVIIVI	ОІМП	163				
15-min Vol PM Peak 15-min Vol																
15-min Vol	AM Peak 15-min Vol															
mments:	PM Peak 15-min Vol															
COURCE Out to AM	mments:															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236661 DIRECTION: NB
DATE: Jun 13 2023

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
09:45 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	2	0	0	0	1	0	0	0	0	0	0	0	0	0	3
11:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total Percent															
ADT 43															
AM Peak 15-min Vol															
PM Peak 15-min Vol															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB

12:00 PM	CITY/STATE: Be	enton, WA														un 13 2023
12:00 PM	Start Time	Bikes			Buses											Total
12:15 PM			Trailers	Long			Single		Double	Double		Multi	Multi		Classed	
12:30 PM																0
12:45 PM 0 0 1:10 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					-					0	_				-	0
01:15 PM				0						0				0	•	1
01:15 PM										0	-			_	-	1
01:30 PM			-	-	-				_	0	_		_	•	-	0
01:45 PM			-	-	•				_	·	_			•	-	0
02:00 PM		-	•	_	-					•	•	-		-	ū	2
02:15 PM		-	•	_	-	_					•		_	•	ū	1
02:30 PM														_	-	0
02:45 PM			-	•	•					· ·	_			•	-	0
03:00 PM			-			-				·	_				-	1
03:15 PM			-	•	-					·	_			•	ū	0
03:30 PM					_	_				0	_			•	-	1
03:45 PM				<del>-</del>						0				_	-	3
04:00 PM				-	•	_				Ū	•	-		•	-	0
04:15 PM		-		_	·	ū					•	-	_	-	ū	2
04:30 PM				0	0					_	0			0	-	1
04:45 PM					0					0				0	-	1
05:00 PM			-	0	0	•	-		-	0	•	-	_	0	0	0
05:15 PM										0				0	0	1
05:30 PM		_												0	ū	2
05:45 PM			1											0	-	3
Day Total Percent  ADT 43  AM Peak																3
ADT 43  AM Peak		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 43  AM Peak																
AM Peak  AM Peak	Percent				DAL	ALHA	ALD	RIVE	5 CC	MM	UNLI	11-5				
AM Peak  AM Peak																
AM Peak	ADT															
	AM Doole															
PM Peak																
15-min Vol																
Comments:																

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB

	nton, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
07:15 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	3	10	17	0	9	1	0	3	0	0	0	0	0	0	43
Percent	7%	23.3%	39.5%	0%	20.9%	2.3%	0%	7%	0%	0%	0%	0%	0%	0%	43
ADT 43															
AM Peak	11:00 AM	6:15 AM	4:45 AM	12:00 AM	8:45 AM	12:00 AM		9:30 AM	12:00 AM		12:00 AM	12:00 AM 0		12:00 AM 0	11:00 A
15-min Vol	2 20 014	12.20.004	1	0	1 20 004	0 15 004	0	2.45.004	0	0	0		0	-	
PM Peak 15-min Vol	2:30 PM 1	12:30 PM 1	5:15 PM 2	12:00 PM 0	1:30 PM 1	8:15 PM 1	12:00 PM 0	3:45 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	3:15 P 3
mments:	<del>-</del>	-			-	-								-	

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION: NB** 

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
Grand Total	3	10	17	0	9	1	0	3	0	0	0	0	0	0	43
Percent	7%	23.3%	39.5%	0%	20.9%	2.3%	0%	7%	0%	0%	0%	0%	0%	0%	45
ADT 43															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

DIRECTION: NB DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236661

CITY/STATE: Benton, WA

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		0				0			0	
04:15 AM		0			_ I ° .	0			0	
04:30 AM		0				0			0	
04:45 AM		1				1			1	
05:00 AM		0				0			0	
05:15 AM		0			LIATI		O A A A A		0	
05:30 AM		0			MAI L		JIVIIVI		0	
05:45 AM		1				1			1	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236661

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		0				0			0	
06:15 AM		1				1			1	
06:30 AM		0				0			0	
06:45 AM		1				1			1	
07:00 AM		0				0			0	
07:15 AM		0				0			0	
07:30 AM		0				0			0	
07:45 AM		0				0			0	
08:00 AM		0				0			0	
08:15 AM		0				0			0	
08:30 AM		0				0			0	
08:45 AM		1				1			1	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		1				1			1	
09:45 AM		2				2			2	
10:00 AM		0				0			0	
10:15 AM		0				0			0	
10:30 AM		0				0			0	
10:45 AM		0				0	$\cdot \cup \iota$		0	
11:00 AM		3				3			3	
11:15 AM		1				1 0			1	
11:30 AM		0				DRIVES CO	DIVIN		<b>1 5</b> 0	
11:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236661

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		0				0			0	
12:15 PM		0				0			0	
12:30 PM		1				1			1	
12:45 PM		1				1			1	
01:00 PM		0				0			0	
01:15 PM		0				0			0	
01:30 PM		2				2			2	
01:45 PM		1				1			1	
02:00 PM		0				0			0	
02:15 PM		0				0			0	
02:30 PM		1				1			1	
02:45 PM		0				0			0	
03:00 PM		1				1			1	
03:15 PM		3				3			3	
03:30 PM		0				0			0	
03:45 PM		2				2			2	
04:00 PM		1				1			1	
04:15 PM		1				1			1	
04:30 PM		0				0		In.	0	
04:45 PM		1				L V 1	$\cdot \cup \iota$	411	1	
05:00 PM		2				2			2	
05:15 PM		3				3			3	
05:30 PM		3				3 3	OMIM	UNII	3	
05:45 PM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236661 DIRECTION: NB

13 Jun 23  66:00 PM  06:15 PM  0 0  06:15 PM  0 0  06:30 PM  1 1  106:45 PM  0 0  07:00 PM  2 2  2 2  07:15 PM  0 0  07:30 PM  0 0  07:30 PM  0 0  07:30 PM  0 0  08:00 PM  0 0  08:00 PM  0 0  08:15 PM  1 1  08:30 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  09:15 PM  0 0  00:15 PM  10:16 PM  0 0  11:16 PM  0 0  11:15 PM  11:1	Sat Sun Average Week 15-min Traffic Average Week Profile
06:15 PM     0       06:30 PM     1       06:35 PM     0       07:00 PM     2       07:00 PM     2       07:15 PM     2       207:30 PM     0       07:45 PM     0       08:00 PM     0       08:05 PM     0       08:30 PM     0       08:33 PM     0       09:00 PM     0       09:00 PM     0       09:35 PM     0       09:35 PM     0       09:45 PM     0       00 PM     1       10:00 PM     1       10:15 PM     0       00 PM     1       10:30 PM     1       10:30 PM     1       11:00 PM     0       11:15 PM     0       0     0       11:30 PM     0       0 Pay Total     43       43     43       Weekday Average     100%       AW Peak     11:00 AM       15-min Vol     3	0
06:30 PM       1       1       1       06:45 PM       0        0       0       0       0       0       0       0       0       0       0       0       0       0       0       0        0	0
06:45 PM       0<	1
07:00 PM     2       07:15 PM     2       07:30 PM     0       07:45 PM     0       08:00 PM     0       08:00 PM     0       08:00 PM     0       08:15 PM     1       08:30 PM     0       09:00 PM     0       09:00 PM     0       09:15 PM     0       09:30 PM     0       09:30 PM     0       09:30 PM     0       09:30 PM     0       10:00 PM     1       10:15 PM     0       10:30 PM     1       10:30 PM     0       11:00 PM     0       0     0       11:30 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       0     0       0     0       0     0 </td <td>0</td>	0
07:15 PM       2         07:30 PM       0         07:45 PM       0         08:00 PM       0         08:15 PM       1         08:30 PM       0         08:30 PM       0         08:45 PM       0         09:00 PM       0         09:015 PM       0         09:30 PM       0         09:30 PM       0         09:30 PM       0         09:45 PM       0         10:00 PM       1         10:15 PM       0         10:30 PM       1         10:45 PM       0         0       0         11:00 PM       0         11:30 PM       0         0       0         11:30 PM       0         0       0         11:45 PM       0         0       0         11:45 PM       0         0       0         11:45 PM       0         0       0         0       0         11:45 PM       0         0       0         0       0         0       0	2
07:30 PM       0       0         07:45 PM       0       0         08:00 PM       0       0         08:15 PM       1       1         08:30 PM       0       0         08:45 PM       0       0         09:00 PM       0       0         09:30 PM       0       0         09:30 PM       0       0         09:34 PM       0       0         09:35 PM       0       0         09:35 PM       0       0         10:00 PM       1       1         10:30 PM       1       1         10:30 PM       1       1         11:00 PM       0       0         11:30 PM       0       0         11:45 PM       0       0         11:45 PM       0       0         0ay Total       43       43         % Weekday Average       100%       100%         AM Peak       11:00 AM       11:00 AM         15-min Vol       3       3	2
07:45 PM       0       0         08:00 PM       0       0         08:15 PM       1       1         08:30 PM       0       0         08:45 PM       0       0         09:00 PM       0       0         09:15 PM       0       0         09:30 PM       0       0         09:30 PM       0       0         09:34 PM       0       0         09:39 PM       0       0         09:30 PM       0       0         10:00 PM       1       1         10:30 PM       1       1         10:30 PM       0       0         11:00 PM       0       0         11:35 PM       0       0         11:35 PM       0       0         11:45 PM       0       0         0 Day Total       43       43         % Weekday Average       100%       100%         AW Peak       11:00 AM       11:00 AM         15-min Vol       3       3	0
08:00 PM     0       08:15 PM     1       08:30 PM     0       08:45 PM     0       09:00 PM     0       09:00 PM     0       09:15 PM     0       09:30 PM     0       09:34 PM     0       09:45 PM     0       10:00 PM     1       10:15 PM     0       10:30 PM     1       10:45 PM     0       11:00 PM     0       11:30 PM     0       11:30 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       100%     43       % Weekday Average     100%       % Week     100%       AM Peak Average     11:00 AM       15-min Vol     3	0
08:30 PM       0<	0
08:45 PM       0<	1
09:00 PM       0       1<	0
09:15 PM     0       09:30 PM     0       09:45 PM     0       10:00 PM     1       10:15 PM     0       10:30 PM     1       10:45 PM     0       0     0       11:00 PM     0       0     0       11:15 PM     0       0     0       11:30 PM     0       0     0       11:45 PM     0       0     0       11:45 PM     0       0     0       100%     43       Weekday Average     43       Weekday Average     100%       AM Peak Average     11:00 AM       15-min Vol     3	0
09:30 PM       0       0       0         09:45 PM       0       0       0         10:00 PM       1       1       1         10:15 PM       0       0       1         10:30 PM       1       1       1         10:45 PM       0       0       0         11:15 PM       0       0       0         11:30 PM       0       0       0         11:45 PM       0       0       0         Day Total       43       43       43         % Weekday Average       100%       100%       100%         AM Peak Average       11:00 AM       11:00 AM       11:00 AM         15-min Vol       3       3       3	0
09:45 PM       0       0       1         10:00 PM       1       1       1         10:15 PM       0       0       1         10:30 PM       1       1       1         10:45 PM       0       0       0         11:00 PM       0       0       0         11:30 PM       0       0       0         11:45 PM       0       0       0         12:45 PM       0       0       0         1007 Total       43       43       43         Weekday Average       100%       100%       100%         AM Peak Average       11:00 AM       11:00 AM       11:00 AM         15-min Vol       3       3       3	0
10:00 PM       1       1       1         10:15 PM       0       0       0         10:30 PM       1       1       1         10:45 PM       0       0       0         11:00 PM       0       0       0         11:15 PM       0       0       0         11:30 PM       0       0       0         11:45 PM       0       0       0         Day Total       43       43       43         % Weekday Average       100%       100%       100%         % Week Average       100%       11:00 AM       11:00 AM         15-min Vol       3       3       3	0
10:15 PM       0       0         10:30 PM       1       1         10:45 PM       0       0         11:00 PM       0       0         11:15 PM       0       0         11:30 PM       0       0         11:45 PM       0       0         Day Total       43       43         % Weekday Average       100%       100%         % Week Average       100%       100%         AM Peak Average       11:00 AM       11:00 AM         15-min Vol       3       3	0
10:30 PM 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1
10:45 PM       0       0       0         11:00 PM       0       0       0         11:15 PM       0       0       0         11:30 PM       0       0       0         11:45 PM       0       0       0         Day Total       43       43       43         % Weekday Average       100%       100%       100%         AW Peak Average       11:00 AM       11:00 AM       15-min Vol       3	0
11:00 PM       0       0       0       11:15 PM       0       0       0       0       11:30 PM       0 <td>1</td>	1
11:15 PM       0       0       0       11:30 PM       0       0       0       0       11:45 PM       0 <td>0</td>	0
11:30 PM     0     0       11:45 PM     0     0       Day Total     43     43       % Weekday Average     100%     100%       % Week Average     100%     100%       AM Peak 11:00 AM 15-min Vol     3     3	0
11:45 PM       0       0         Day Total       43       43         % Weekday Average       100%       100%         % Week Average       100%       100%         AM Peak 11:00 AM 15-min Vol       3       3	0
Day Total         43         43           % Weekday Average         100%         100%           % Week Average         100%         100%           AM Peak 15-min Vol         11:00 AM         11:00 AM           15-min Vol         3         3	0
% Weekday Average     100%       % Week Average     100%       AM Peak 11:00 AM 15-min Vol     3       15-min Vol     3	0
Average         100%           % Week Average         100%           AM Peak 15-min Vol         11:00 AM 3           15-min Vol         3	43
AVerage     100%       AM Peak     11:00 AM       15-min Vol     3       15-min Vol     3	
15-min Vol 3 3	
	11:00 AM
PM Peak 3:15 PM 3:15 PM	3
	3:15 PM
15-min Vol 3 3	3

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total **Pace Speed** in Pace 12:00 AM 1-10 12:15 AM 1-10 12:30 AM 1-10 12:45 AM 1-10 01:00 AM 1-10 01:15 AM 1-10 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O O 1-10 02:15 AM 1-10 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 1-10 03:15 AM 1-10 03:30 AM 1-10 03:45 AM 1-10 31-40 04:00 AM 04:15 AM 41-50 04:30 AM O 36-45 04:45 AM 26-35 05:00 AM 1-10 05:15 AM 1-10 05:30 AM 21-30 05:45 AM 26-35 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total **Pace Speed** in Pace 06:00 AM 1-10 06:15 AM 36-45 06:30 AM 36-45 06:45 AM 46-55 07:00 AM 1-10 07:15 AM 1-10 07:30 AM 1-10 41-50 07:45 AM 08:00 AM O O 1-10 08:15 AM 41-50 08:30 AM 1-10 08:45 AM 36-45 09:00 AM 1-10 09:15 AM 1-10 09:30 AM 11-20 09:45 AM 41-50 10:00 AM 1-10 10:15 AM 1-10 10:30 AM O 1-10 10:45 AM 1-10 11:00 AM 26-35 11:15 AM 36-45 11:30 AM 1-10 11:45 AM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total **Pace Speed** in Pace 12:00 PM 36-45 12:15 PM 1-10 12:30 PM 26-35 12:45 PM 31-40 01:00 PM 1-10 01:15 PM 1-10 01:30 PM 26-35 01:45 PM 41-50 02:00 PM O O 1-10 02:15 PM 1-10 02:30 PM 26-35 02:45 PM 1-10 03:00 PM 31-40 03:15 PM 21-30 03:30 PM 1-10 03:45 PM 16-25 04:00 PM 11-20 04:15 PM 31-40 04:30 PM O 1-10 04:45 PM 31-40 05:00 PM 36-45 05:15 PM 46-55 05:30 PM 31-40 05:45 PM 26-35 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION: NB, SB** 

CITY/STATE:	Benton,	WA														DATE: Jun	13 2023
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:00 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	26-35	2
07:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2	26-35	2
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1-10	1
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
09:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	2	26-35	1
10:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	1	3	2	4	13	14	13	11	_5_	2	0	0	0	0	68	33-42	27
Percent	1.5%	4.4%	2.9%	5.9%	19.1%	20.6%	19.1%	16.2%	7.4%	2.9%	0%	0%	0%	0%			
AM Peak	12:00 AM		12:00 AM		4:45 AM	4:00 AM	4:30 AM	7:45 AM	6:45 AM	12:00 AM					11:00 AM		
15-min Vol	0	1	0	2	1	1	1	2	1	0	0	0	0	0	3		
PM Peak	8:15 PM 1	4:00 PM 2	3:45 PM 1	3:15 PM 1	7:00 PM 2	5:30 PM 3	5:00 PM 2	1:45 PM 1	12:30 PM 1	12:45 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	3:15 PM 4		
15-min Vol  Comments:	1	2	1	1	2	3	2	1	1	1	0	0	0	0	4		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Ba	dger Can	ıyon Rd n	orth of Se	llards Rd												QC JOB	#: 16236661
SPECIFIC LOCA	ATION:															DIREC	TION: NB, SB
CITY/STATE: B	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Nullge	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 dec speed	Pace
Grand Total	1	3	2	4	13	14	13	11	5	2	0	0	0	0	68	33-42	27
Percent	1.5%	4.4%	2.9%	5.9%	19.1%	20.6%	19.1%	16.2%	7.4%	2.9%	0%	0%	0%	0%	08	33-42	27
Cumulative Percent	1.5%	5.9%	8.8%	14.7%	33.8%	54.4%	73.5%	89.7%	97.1%	100%	100%	100%	100%	100%			
ADT 68															Me	an Speed(Avera Med	ntile: 48 MPH age): 39 MPH dian: 39 MPH ode: 38 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	IIIOII, WA	C 0	241		2.4.10	2.4.1.	4.4.1	-F A 1			.c.a.l	CAL			un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
	_	Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:45 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
05:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Day Total															
Percent				DAIL	ALHA		RIVE	500	MM	UNIT	11-5				
ADT															
68															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
Comments:								<u> </u>			<u> </u>				

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	IIIOII, WA	C 0	2.4.1.		2 4.1. 6	2 4 1:	4 A 1:	-F A I	ГАТ		-C A I	C A 1:	.C.A.I		un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
	_	Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
06:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
06:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
09:45 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0 0	0 0
10:45 AM	0	0	0	0	0	0	0	0	·	0	0	0	Ū	•	-
11:00 AM	2	0	0	0	1 0	0	0	0	0	0	0	0	0	0	3
11:15 AM	0	1 0	0	0			0	0	0	0	0	0	0	0	1
11:30 AM 11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0 0	0 0	0 0	0 0
	U	U	U	U	U	U	U	U	U	U	U	U	U	U	U
Day Total															
Percent				DAIL	$A = \Box A$	ALLI	TIVE	366	IIVIIVI	UMI					
ADT															
68															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	IIIOII, WA		2.4.1		2412	2.4.1	4 4 1					<b>CA</b> 1			un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
12:45 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
01:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
03:15 PM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	1	0	0	0	0	1	0	0	0	0	0	0	2
04:00 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
04:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:00 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
05:15 PM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
05:30 PM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
05:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Day Total															
Percent				DATA	ATH		RIVE	SCC	IMIM	UNU	1-5				
ADT															
68															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
Conninents:		22275075									COLIDOR	lii o	110/11:	·//www.guali	<del></del>
anort gonorator	1006/20/20	112 11511 1111									STREET CO.	Indiana / Count			

SPECIFIC LOCATION:

QC JOB #: 16236661 **DIRECTION:** NB, SB

CITY/STATE: Benton, WA **DATE:** Jun 13 2023

Start Time   Bikes   Trailers   Long   Buses   Tire   Single   Single   Double   Double   Double   Double   Multi   Multi   Multi   Classed   Total	6	5.1	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	
G6:15 PM	Start Time	BIKES	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	ıotaı
O6:35 PM	06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
O6-15 PM	06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
O7:10 PM	06:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
O7:15 PM	06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
O7:45 PM	07:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
07:45 PM	07:15 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
08:15 PM	07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	07:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
08:30 PM	08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	08:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1
09:00 PM	08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	08:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
O9:30 PM	09:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:45 PM	09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	10:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
10:45 PM   0	10:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:00 PM		0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:15 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM		0	1	0	0			0	0	0	THE RESIDENCE	0	0	0	0	1
11:45 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	0	0	0			0	0	0	0	0	0	0	0	0
Day Total Percent         3		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 68	11:45 PM	0			0		0	0	0	0	0	0	0	0	0	0
ADT 68  ADT 68  AM Peak 11:00 AM 4:00 AM 7:45 AM 12:00 AM 12:00 AM 8:45 AM 12:00 AM	Day Total	3												0		68
68	Percent	4.4%	23.5%	48.5%	0%	16.2%	1.5%	0%	5.9%	0%	0%	0%	0%	0%	0%	08
15-min Vol         2         1         2         0         1         0         0         1         0         0         0         0         0         0         0         0         0         0         3           PM Peak 15-min Vol         1:00 PM         12:00 PM																
15-min Vol         2         1         2         0         1         0         0         1         0         0         0         0         0         0         0         0         0         0         3           PM Peak 15-min Vol         1:00 PM         12:00 PM	AM Peak	11:00 AM	4:00 AM	7:45 AM	12:00 AM	8:45 AM	12:00 AM	12:00 AM	9:30 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	11:00 AM
15-min Vol 1 1 2 0 1 1 0 1 0 0 0 0 0 4																
15-min Vol 1 1 2 0 1 1 0 1 0 0 0 0 0 4	PM Peak	2:30 PM	12:00 PM	12:45 PM	12:00 PM	12:00 PM	8:15 PM	12:00 PM	3:45 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	3:15 PM
JUHHICHUS.	Comments:															

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE:** Jun 13 2023 Cars & 2 Axle 2 Axle 6 4 Axle <5 Axl 5 Axle >6 Axl 3 Axle <6 Axl 6 Axle >6 Axl Not Start Time **Bikes Buses** Total **Trailers** Long Tire Single Single Double Double Double Multi Multi Multi Classed **Grand Total** 3 16 33 0 11 1 0 4 0 0 0 0 0 0 68 4.4% 0% 0% 0% 0% 0% 0% 0% Percent 23.5% 48.5% 16.2% 1.5% 5.9% 0% ADT 68

Comments:

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: NB, SB

12:00 AM	Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:15 AM	12:00 AM										
12:30 AM	12:15 AM		0								
01:00 AM	12:30 AM		0				0			0	
01:15 AM	12:45 AM		0				0			0	
01:30 AM	01:00 AM		0				0			0	
01:45 AM	01:15 AM		0				0			0	
02:00 AM	01:30 AM		0				0			0	
02:15 AM	01:45 AM		0				0			0	
02:30 AM	02:00 AM		0				0			0	
02:45 AM	02:15 AM		0				0			0	
03:00 AM	02:30 AM		0				0			0	
03:15 AM	02:45 AM		0				0			0	
03:30 AM	03:00 AM		0							0	
03:45 AM			0							0	
04:00 AM			0				0			0	
04:15 AM			0				0			0	
04:30 AM 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			1							1	
04:45 AM			1			_ °.	1	-		1	
05:00 AM			1					$\cap$	IIO.	1	
05:15 AM       0<			2			161			$\mathcal{A}$		
05:30 AM       1<			0							-	
Day Total  Note that the second of the secon			0			LIATI	0	OA AA	11 18 117		
Day Total  Weekday Average  Week Average  AM Peak 15-min Vol  PM Peak 15-min Vol  Week 15-m						MALL		JIVIIV	UNII		
Weekday Average  Week Average  AM Peak 15-min Vol  PM Peak 15-min Vol  Weekday Average  AN Week Average  AN Peak 15-min Vol  Weekday Average  AN Peak 15-min Vol  Weekday Average  AN Peak 15-min Vol  Weekday Average  Weekday Average  Weekday Average  Weekday Week			1				1			1	
Average	-										
Week Average  AM Peak 15-min Vol  PM Peak 15-min Vol  Tomin Vol  T	% Weekday										
Average  AM Peak 15-min Vol  PM Peak 15-min Vol  Tomin											
AM Peak 15-min Vol PM Peak 15-min Vol											
15-min Vol Peak 15-min Vol Service Ser											
PM Peak 15-min Vol											
15-min Vol											

CITY/STATE: Benton, WA

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION:** NB, SB

06:00 AM	Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:30 AM	06:00 AM		0				0			0	
06:45 AM	06:15 AM		1				1			1	
07:00 AM	06:30 AM		1				1			1	
07:15 AM	06:45 AM		1				1			1	
07:30 AM	07:00 AM		0				0			0	
07:45 AM	07:15 AM		0				0			0	
08:00 AM	07:30 AM		0				0			0	
08:15 AM	07:45 AM		2				2			2	
08:30 AM	08:00 AM		0				0			0	
08:45 AM	08:15 AM		1				1			1	
09:00 AM	08:30 AM		0				0			0	
09:15 AM	08:45 AM		1				1			1	
09:30 AM	09:00 AM		0				0			0	
09:45 AM       2       2       2         10:00 AM       0       0       0         10:15 AM       0       0       0         10:30 AM       0       0       0         10:45 AM       0       0       0         11:00 AM       3       3       3         11:15 AM       1       1       1         11:30 AM       0       0       0         11:45 AM       0       0       0         9w Weekday Average       0       0       0         % Weekday Average       0       0       0         % Week Average       0       0       0         PM Peak       0       0       0         PM Peak       0       0       0	09:15 AM		0				0			0	
10:00 AM	09:30 AM		1				1			1	
10:15 AM	09:45 AM		2				2			2	
10:30 AM	10:00 AM		0				0			0	
10:45 AM	10:15 AM		0				0			0	
11:00 AM	10:30 AM		0				0		IIO.	0	
11:00 AM	10:45 AM		0				0	$\sim$		0	
11:45 AM         0         0         0           Day Total         % Weekday Average         1         2         1         1         2         1         2         2         2         2         2         2         2         2         2         2         2         <	11:00 AM		3							3	
11:45 AM         0         0         0           Day Total         % Weekday Average         1         2         1         1         2         1         2         2         2         2         2         2         2         2         2         2         2         <	11:15 AM		1				1			1	
11:45 AM         0         0         0           Day Total         % Weekday Average         1         2         1         1         2         1         2         2         2         2         2         2         2         2         2         2         2         <			0				DRIVOS CI	DIVIV	IUNIT		
Day TotalMeekday AverageMeekday Average	11:45 AM		0							0	
Average         Meek Average	Day Total										
Week Average  AM Peak 15-min Vol  PM Peak											
Average	Average										
AM Peak         15-min Vol         PM Peak											
15-min Vol	Average										
PM Peak											
	15-min Vol										
15-min Vol											
	15-min Vol										

CITY/STATE: Benton, WA

QC JOB #: 16236661 **DIRECTION: NB, SB SPECIFIC LOCATION: DATE:** Jun 13 2023 - Jun 13 2023

Tue Thu Fri Average Weekday Average Week Mon Wed Sat Sun Average Week Profile **Start Time** 15-min Traffic 15-min Traffic 13 Jun 23 12:00 PM 2 2 2 12:15 PM 0 0 0 12:30 PM 2 2 2 12:45 PM 2 2 2 01:00 PM 0 0 0 01:15 PM 0 0 01:30 PM 2 2 01:45 PM 1 1 02:00 PM 0 0 02:15 PM 0 0 0 02:30 PM 1 1 02:45 PM 0 0 0 03:00 PM 2 2 03:15 PM 4 4 03:30 PM 0 0 2 03:45 PM 2 2 04:00 PM 3 3 3 04:15 PM 1 04:30 PM 0 04:45 PM 1 05:00 PM 2 05:15 PM 3 3 05:30 PM 3 05:45 PM 1 Day Total % Weekday Average % Week Average AM Peak 15-min Vol PM Peak 15-min Vol

Comments:

CITY/STATE: Benton, WA

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION:** NB, SB **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
0C-00 DN4										
06:00 PM		0				0			0	
06:15 PM		0				0			0	
06:30 PM		1				1			1	
06:45 PM		0				0			0	
07:00 PM		2				2			2	
07:15 PM		2				2			2	
07:30 PM		0				0			0	
07:45 PM		2				2			2	
08:00 PM		0				0			0	
08:15 PM		1				1			1	
08:30 PM		0				0			0	
08:45 PM		1				1			1	
09:00 PM		1				1			1	
09:15 PM		0				0			0	
09:30 PM		0				0			0	
09:45 PM		ŭ				_			0	
10:00 PM		2				2			2	
10:15 PM		1				1 1				
10:30 PM 10:45 PM		1 0				0			0	
10:45 PM 11:00 PM		1							1	
11:00 PM 11:15 PM		0				1			0	
11:30 PM		0				$\frac{0}{0}$	$\Delta \Lambda \Lambda \Lambda \Lambda \Gamma$		7ES 0	
11:45 PM		0				0	DIVITAL		0	
Day Total		68				68			68	
% Weekday		08				06			08	
Average		100%								
% Week										
Average		100%				100%				
AM Peak		11:00 AM				11:00 AM			11:00 AM	
15-min Vol		3				3			3	
PM Peak		3:15 PM				3:15 PM			3:15 PM	
15-min Vol		4				4			4	

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 12:15 AM 1-10 12:30 AM 1-10 12:45 AM 1-10 01:00 AM 1-10 01:15 AM 1-10 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O O 1-10 02:15 AM 1-10 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 1-10 03:15 AM 1-10 03:30 AM 1-10 03:45 AM 1-10 31-40 04:00 AM 04:15 AM 41-50 04:30 AM O 36-45 04:45 AM 41-50 05:00 AM 1-10 05:15 AM 1-10 05:30 AM 21-30 05:45 AM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 1-10 06:15 AM 1-10 06:30 AM 36-45 06:45 AM 1-10 07:00 AM 1-10 07:15 AM 1-10 07:30 AM 1-10 41-50 07:45 AM 08:00 AM O O 1-10 08:15 AM 41-50 08:30 AM 1-10 08:45 AM 1-10 09:00 AM 1-10 09:15 AM 1-10 09:30 AM 1-10 09:45 AM 1-10 10:00 AM 1-10 10:15 AM 1-10 10:30 AM O 1-10 10:45 AM 1-10 11:00 AM 1-10 11:15 AM 1-10 11:30 AM 1-10 11:45 AM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236661 LOCATION: Badger Canyon Rd north of Sellards Rd SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 36-45 12:15 PM 1-10 12:30 PM 46-55 12:45 PM 51-60 01:00 PM 1-10 01:15 PM 1-10 01:30 PM 1-10 01:45 PM 1-10 02:00 PM O 1-10 02:15 PM 1-10 02:30 PM 1-10 02:45 PM 1-10 03:00 PM 26-35 03:15 PM 51-60 03:30 PM 1-10 03:45 PM 1-10 04:00 PM 11-20 04:15 PM 1-10 04:30 PM O 1-10 04:45 PM 1-10 05:00 PM 1-10 05:15 PM 1-10 05:30 PM 1-10 05:45 PM 26-35 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 SPECIFIC LOCATION: **DIRECTION: SB** DATE: Jun 13 2023

CITY/STATE: Renton WA

06:00 PM 06:15 PM 06:30 PM 06:30 PM 06:45 PM 07:00 PM 07:15 PM	1 15 0 0 0	16 20 0 0	21 25 0 0	<b>26</b> <b>30</b> 0	31 35	36 40	41	46	51	56	61	66	71	76			Numbe
06:00 PM 06:15 PM 06:30 PM 06:45 PM 07:00 PM	0 0 0	0 0	0			40	4-								Total	Pace Speed	
06:15 PM 06:30 PM 06:45 PM 07:00 PM	0 0 0	0		0			45	50	55	60	65	70	75	999	Total	r dec speed	in Pace
06:30 PM 06:45 PM 07:00 PM	0		Λ		0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 PM 07:00 PM	0	^	U	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:00 PM		U	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
	^	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07.1E DN4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:13 PIVI	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
09:00 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26-35	1
10:15 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	2	0	1	3	3	4	7	3	2	0	0	0	0	25	41-50	11
Percent	0%	8%	0%	4%	12%	12%	16%	28%	12%	8%	0%	0%	0%	0%	23	41-30	- 11
-																	
		12:00 AM			12:00 AM	4:00 AM	4:30 AM			12:00 AM					7:45 AM		
15-min Vol	0	0	0	1	0	1	1	2	0	0	0	0	0	0	2		
	12:00 PM	4:00 PM	12:00 PM			12:00 PM				12:45 PM					12:00 PM		
15-min Vol	0	2	0	0	1	1	1	1	1	1	0	0	0	0	2		
Comments:																	

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Ba	dger Can	yon Rd n	orth of Se	llards Rd												QC JOB	#: 16236661
SPECIFIC LOCA	TION:															DI	RECTION: SB
CITY/STATE: B	enton, W	'A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 ace speed	Pace
Grand Total	0	2	0	1	3	3	4	7	3	2	0	0	0	0	25	41-50	11
Percent	0%	8%	0%	4%	12%	12%	16%	28%	12%	8%	0%	0%	0%	0%	25	41-30	"
Cumulative Percent	0%	8%	8%	12%	24%	36%	52%	80%	92%	100%	100%	100%	100%	100%			
ADT 25															Me	an Speed(Avera Med	ntile: 52 MPH age): 44 MPH dian: 44 MPH ode: 48 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: SB

Trailers   Long   Tire   Single   Single   Double   Double   Double   Double   Double   Multi   Multi   Multi   Classed	CITY/STATE: Be	enton, WA														un 13 2023
Trailers   Long   Tire   Single   Single   Double   Double   Double   Double   Double   Multi   Multi   Multi   Classed	Start Time	Bikes			Buses											Total
12:15 AM			Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:39 AM	12:00 AM		0	0	0		0	0		0	0		0	0		
12:45 AM			0	0	0				0	0	0			0	0	0
01:00 AM		0	0	0	0	0			0	0	0	0	0	0	0	0
01:15 AM										0	-			0	-	
01:30 AM			-	0	0				0	0	0		_	0	_	_
01:45 AM			-	0	0				_	0	0			0	_	
02:00 AM		-	•	_	0	_				0	0	-		0	ū	_
02:15 AM		-	•	0	0	_				0	0		_	0	ū	0
02:30 AM														_	_	
02:45 AM			-	•	_						_			•	_	_
03:00 AM			-	0	0	-				0	0			0	0	
03:15 AM			-	•	-					0	_			0	ū	_
03:30 AM				0	_					0	_			0	_	
03:45 AM				_						0	-			_	-	
04:00 AM				•	·	_				Ū	•	-		•	_	
04:15 AM		-		_	·	ū					•	-	_	-	ū	
04:30 AM					-					_		-		-	_	
04:45 AM   0				_	_	_				0				0	_	
05:00 AM			-	_	•	•	_		-	0	•	-	_	·	ū	
05:15 AM										•				-	_	
05:30 AM														•	ū	
05:45 AM														•	_	
ADT 25  AM Peak 15-min Vol PM Peak 15-min Vol																
ADT 25  AM Peak 15-min Vol Peak 15-min Vol		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 25  AM Peak 15-min Vol  PM Peak 15-min Vol																
AM Peak 15-min Vol  PM Peak 15-min Vol	Percent				DAL	ALHA	ALD	RIVE	566	IVIVI	UNL	IES.				
AM Peak 15-min Vol  PM Peak 15-min Vol																
AM Peak 15-min Vol  PM Peak 15-min Vol	ADT															
AM Peak 15-min Vol PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	25															
15-min Vol PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	ANA Doole															
PM Peak 15-min Vol																
15-min Vol																
mments:																
	Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: SB

Start Time         Bikes           06:00 AM         0           06:15 AM         0           06:30 AM         0           06:45 AM         0           07:00 AM         0           07:15 AM         0           07:30 AM         0           07:45 AM         0           08:00 AM         0           08:15 AM         0           08:30 AM         0           09:00 AM         0           09:15 AM         0           09:30 AM         0           09:45 AM         0           10:00 AM         0           10:30 AM         0           11:50 AM         0           11:45 AM         0           Day Total         Percent	Cars & Trailers  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 Axle Long 0 0 1 0 0 0 0 2 0 1 0 0 0 0 0 0 0 0 0 0	8uses 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 Axle 6 Tire  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 Axle Single 0 0 0 0 0 0 0 0 0	4 Axle Single 0 0 0 0 0 0 0 0 0	<5 Axl Double  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	5 Axle Double  0 0 0 0 0 0 0 0 0 0 0 0 0 0	>6 Axl Double  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	<6 Axl Multi 0 0 0 0 0 0 0 0 0 0 0 0 0 0	6 Axle Multi 0 0 0 0 0 0 0 0 0 0 0	>6 Axl Multi 0 0 0 0 0 0 0	Not Classed  0 0 0 0 0 0 0 0 0 0 0 0	0 0 1 0 0 0
06:15 AM	0 0 0 0 0 0 0 0 0 0	0 0 1 0 0 0 0 0 2 0 1 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 1 0 0 0
06:15 AM	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 1 0 0 0 0 2 0 1 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 1 0 0 0
06:30 AM	0 0 0 0 0 0 0 0 0	1 0 0 0 0 2 0 1 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	1 0 0 0
06:45 AM 0 07:00 AM 0 07:15 AM 0 07:30 AM 0 07:45 AM 0 08:00 AM 0 08:30 AM 0 08:45 AM 0 09:00 AM 0 09:15 AM 0 09:30 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0 0 0 0 0 0 0 0 0	0 0 0 0 2 0 1 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0
07:00 AM	0 0 0 0 0 0 0 0	0 0 0 2 0 1 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0	0 0 0 0	0 0 0	0 0 0	0 0 0
07:15 AM	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 2 0 1 0 0 0	0 0 0 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0	0	0
07:30 AM	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 2 0 1 0 0 0	0 0 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0 0	0 0 0	0	0	0	0	0	0
07:45 AM 0 08:00 AM 0 08:15 AM 0 08:31 AM 0 08:45 AM 0 09:00 AM 0 09:15 AM 0 09:45 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 11:45 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0 0 0 0 0 0 0	2 0 1 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0 0	0 0 0	0	0	0	0	-	ū	
08:00 AM	0 0 0 0 0 0	0 1 0 0 0 0	0 0 0 0 0	0 0 0 0	0 0 0	0 0	0 0	0	•	_	_	U		
08:15 AM 0 08:30 AM 0 08:45 AM 0 09:00 AM 0 09:15 AM 0 09:30 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0	0 0 0 0 0 0	1 0 0 0 0	0 0 0 0	0 0 0	0 0	0	0		U		(1)	0	0	0
08:30 AM	0 0 0 0 0	0 0 0 0	0 0 0 0	0 0	0				0	0 0	0 0	0	0	1
08:45 AM 0 09:00 AM 0 09:15 AM 0 09:30 AM 0 09:45 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0 0 0 0	0 0 0 0	0 0	0		U	0	0	0	0	0	0	0	0
09:00 AM	0 0 0 0	0 0 0	0			0	0	0	0	0	0	0	0	0
09:15 AM 0 09:30 AM 0 09:45 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:15 AM 0 11:45 AM 0 Day Total	0 0 0	0	0		0	0	0	0	0	0	0	0	0	0
09:30 AM 0 09:45 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:45 AM 0 Day Total	0	0		0	0	0	0	0	0	0	0	0	0	0
09:45 AM 0 10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0	•		0	0	0	0	0	0	0	0	0	0	0
10:00 AM 0 10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0	•		0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM 0 10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	U	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM 0 10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 AM 0 11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM 0 11:15 AM 0 11:30 AM 0 11:45 AM 0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM 0 11:30 AM 0 11:45 AM 0 Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM 0 11:45 AM 0 Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM 0 Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0
			0	<u> </u>	•	U		•			-	-	Ü	
i ci cciic														
ADT 25														
AM Peak 15-min Vol														
PM Peak 15-min Vol Comments:														

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
12:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 PM	0	0	1	0	0	0	0	1	0	0	0	0	0	0	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Day Total															
Percent				DATA	ATHA		RIVF	SCC	MM	UNII	11-5				
ADT															
ADT 25															
25															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
omments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236661 DIRECTION: SB

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 2023 Total
Start Time	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	6	16	0	2	0	0	1	0	0	0	0	0	0	25
Percent	0%	24%	64%	0%	8%	0%	0%	4%	0%	0%	0%	0%	0%	0%	
ADT 25															
AM Peak	12:00 AM	4:00 AM	7:45 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	7:45 AM
15-min Vol	0	1	2	0	0	0	0	0	0	0	0	0	0	0	2
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 1	12:30 PM 1	12:00 PM 0	12:00 PM 1	12:00 PM 0	12:00 PM 0	4:00 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PN 2
	U	1	1	U	1	U	U	1	U	U	U	U	U	U	2
omments:															

LOCATION: Badger Canyon Rd north of Sellards Rd QC JOB #: 16236661 **DIRECTION: SB** SPECIFIC LOCATION:

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	6 24%	16 64%	0 0%	2 8%	0 0%	0 0%	1 4%	0 0%	0 0%	0 0%	0 0%	0 0%	0 0%	25
ADT 25															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236661 DIRECTION: SB

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		1				1			1	
04:15 AM		1			_ I ° .	1			1	
04:30 AM		1				1			1	
04:45 AM		1			161	1			1	
05:00 AM		0				0			0	
05:15 AM		0			TIATI	DRIVIES CO	O A A A A	O DE DE	0	
05:30 AM		1			HALL		JIVIIV	UNII	<b>1</b> 1	
05:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		0				0			0	
06:15 AM		0				0			0	
06:30 AM		1				1			1	
06:45 AM		0				0			0	
07:00 AM		0				0			0	
07:15 AM		0				0			0	
07:30 AM		0				0			0	
07:45 AM		2				2			2	
08:00 AM		0				0			0	
08:15 AM		1				1			1	
08:30 AM		0				0			0	
08:45 AM		0				0			0	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		0				0			0	
09:45 AM		0				0			0	
10:00 AM		0				0			0	
10:15 AM		0			al °.	0			0	
10:30 AM		0			21 1	0			0	
10:45 AM		0			161	0			0	
11:00 AM		0				0			0	
11:15 AM		0			LIATI	DRIVOES CO	71/1/		0	
11:30 AM		0			MALL		DIVIIV		/E5 0	
11:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak 15-min Vol										
Comments:										

QC JOB #: 16236661

**DIRECTION: SB** 

LOCATION: Badger Canyon Rd north of Sellards Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DIRECTION: SB

DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236661

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		2				2			2	
12:15 PM		0				0			0	
12:30 PM		1				1			1	
12:45 PM		1				1			1	
01:00 PM		0				0			0	
01:15 PM		0				0			0	
01:30 PM		0				0			0	
01:45 PM		0				0			0	
02:00 PM		0				0			0	
02:15 PM		0				0			0	
02:30 PM		0				0			0	
02:45 PM		0				0			0	
03:00 PM		1				1			1	
03:15 PM		1				1			1	
03:30 PM		0				0			0	
03:45 PM		0				0			0	
04:00 PM		2				2			2	
04:15 PM		0				0			0	
04:30 PM		0				0			0	
04:45 PM		0				0	$\cdot \cup \iota$		0	
05:00 PM		0				0			0	
05:15 PM		0				0	0		0	
05:30 PM		0				0	DMM		0	
05:45 PM		1				1			1	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
mments:		-								

LOCATION: Badger Canyon Rd north of Sellards Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236661

DIRECTION: SB

DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		0				0			0	
06:15 PM		0				0			0	
06:30 PM		0				0			0	
06:45 PM		0				0			0	
07:00 PM		0				0			0	
07:15 PM		0				0			0	
07:30 PM		0				0			0	
07:45 PM		2				2			2	
08:00 PM		0				0			0	
08:15 PM		0				0			0	
08:30 PM		0				0			0	
08:45 PM		1				1			1	
09:00 PM		1				1			1	
09:15 PM		0				0			0	
09:30 PM		0				0			0	
09:45 PM		0				0			0	
10:00 PM		1				1			1	
10:15 PM		1				1			1	
10:30 PM		0				0			0	
10:45 PM		0				0	-		0	
11:00 PM		1				1			1	
11:15 PM		0				0			0	
11:30 PM		0				DRIVOS CO	DMM		<b>1 S</b> 0	
11:45 PM		0				0			0	
Day Total		25				25			25	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		7:45 AM				7:45 AM			7:45 AM	
15-min Vol		2				2			2	
PM Peak		12:00 PM				12:00 PM			12:00 PM	
15-min Vol		2				2			2	

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 56-65 51-60 12:15 AM 12:30 AM 51-60 12:45 AM 1-10 01:00 AM 31-40 01:15 AM 51-60 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O 41-50 02:15 AM 1-10 02:30 AM 46-55 02:45 AM 1-10 03:00 AM 1-10 03:15 AM 36-45 03:30 AM 41-50 03:45 AM 46-55 04:00 AM 41-50 04:15 AM 41-50 04:30 AM O 41-50 04:45 AM 51-60 05:00 AM 36-45 05:15 AM 36-45 05:30 AM 51-60 05:45 AM 48-57 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

	•		າ of Rte 1	L- <del>-</del>												QC JOB #: 1	
PECIFIC LOCA																	TION: N
CITY/STATE: E	Benton,	WA														DATE: Jur	13 202
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numb
tart rime	15	20	25	30	35	40	45	50	55	60	65	70	75	999	TOTAL	Pace Speed	in Pac
06:00 AM	0	0	0	0	2	0	1	0	1	2	0	0	0	0	6	51-60	3
06:15 AM	0	0	0	0	0	2	2	0	1	0	0	0	0	0	5	36-45	4
06:30 AM	0	0	0	0	1	1	1	5	2	1	0	0	0	0	11	46-55	7
06:45 AM	0	0	0	0	0	0	0	1	1	5	5	0	0	0	12	56-65	10
07:00 AM	0	0	0	0	0	0	1	2	2	1	1	0	0	0	7	46-55	4
07:15 AM	0	0	0	0	2	1	0	1	3	2	0	0	0	0	9	51-60	5
07:30 AM	0	0	0	0	2	0	0	0	0	1	0	0	0	0	3	26-35	2
07:45 AM	0	0	0	0	1	2	0	2	1	1	1	0	1	0	9	46-55	3
MA 00:80	0	0	0	0	0	1	3	0	2	0	0	0	0	0	6	36-45	4
08:15 AM	0	0	0	0	0	0	2	1	2	0	2	0	0	0	7	41-50	3
08:30 AM	0	0	0	0	1	0	0	2	1	4	0	0	0	0	8	51-60	5
08:45 AM	0	0	0	0	0	0	1	2	2	2	1	0	0	0	8	46-55	4
09:00 AM	0	0	0	0	1	0	0	2	2	1	0	0	0	0	6	46-55	4
09:15 AM	0	0	0	2	3	4	3	0	3	2	0	0	0	0	17	31-40	7
09:30 AM	0	0	0	0	0	1	1	2	1	2	0	0	0	0	7	46-55	3
09:45 AM	0	0	0	0	0	0	0	3	1	1	2	0	0	0	7	46-55	4
10:00 AM	0	0	0	0	0	1	1	2	5	0	0	0	0	0	9	46-55	7
10:15 AM	0	0	0	0	0	1	1	3	3	1	0	0	0	0	9	46-55	6
10:30 AM	0	0	0	0	1	0	1	3	3	1	0	0	0	0	9	46-55	6
10:45 AM	0	0	0	0	0	0	0	4	2	0	1	1	0	0	8	46-55	6
11:00 AM	0	0	0	0	0	2	3	1	5	3	1	0	0	0	15	51-60	8
11:15 AM	0	0	0	0	0	0	1	7	2	1	1	0	0	0	12	46-55	9
11:30 AM	0	0	0	0	1	2	0	5	1	4	0	0	0	0	13	46-55	6
11:45 AM	0	0	0	0	0	1	2	3	3	4	0	0	0	0	13	51-60	7
Day Total					TA	711	ATI	DRIN	/FC	00	A // A	AL IN	11711				
Percent				UF	MA	IH	4/1	JKIN	/ES	CC	IVIIV	1UN					
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 51-60 12:15 PM 51-60 36-45 12:30 PM 12:45 PM 51-60 01:00 PM 48-57 01:15 PM 51-60 01:30 PM 31-40 01:45 PM 46-55 02:00 PM O 46-55 02:15 PM 46-55 02:30 PM 46-55 02:45 PM 46-55 03:00 PM 51-60 03:15 PM 36-45 03:30 PM 51-60 03:45 PM 46-55 04:00 PM 51-60 04:15 PM 50-59 04:30 PM O 46-55 04:45 PM 31-40 05:00 PM 51-60 05:15 PM 51-60 05:30 PM 51-60 05:45 PM 36-45 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

QC JOB #: 16236662 **DIRECTION: NB** 

CITY/STATE:			24	26	24	26		4.0		FC			74	7.0		DATE: Jun	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pac
06:00 PM	0	0	0	0	1	1	2	5	1	2	1	0	1	0	14	41-50	7
06:15 PM	0	0	0	0	0	1	0	3	3	2	0	0	0	0	9	46-55	6
06:30 PM	0	0	0	0	0	0	0	2	0	2	0	0	0	0	4	41-50	2
06:45 PM	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	41-50	3
07:00 PM	0	0	0	0	0	0	3	4	1	1	0	0	0	0	9	41-50	7
07:15 PM	0	0	0	0	0	0	1	0	2	2	0	0	0	0	5	51-60	4
07:30 PM	0	0	0	0	1	0	0	3	2	0	0	0	0	0	6	46-55	5
07:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
08:00 PM	0	0	0	0	0	2	0	0	0	1	1	0	0	0	4	31-40	2
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:30 PM	0	0	0	0	0	0	0	2	0	2	0	0	0	0	4	41-50	2
08:45 PM	0	0	0	0	0	1	0	0	2	0	0	0	0	0	3	46-55	2
09:00 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	2	41-50	2
09:15 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
09:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
09:45 PM	0	0	0	0	0	1	1	0	0	1	0	0	0	0	3	36-45	2
10:00 PM	0	0	0	0	0	0	0	1	3	0	0	1	0	0	5	46-55	4
10:15 PM	0	0	0	2	0	0	0	0	1	0	0	0	0	0	3	21-30	2
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:00 PM	0	0	0	0	0	1	1	0	0	0	1	0	0	0	3	36-45	2
11:15 PM	0	0	0	0	0	0	0	2	0	0	2	0	0	0	4	41-50	2
11:30 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2	46-55	2
11:45 PM	0	0	0	0	1	0	0	1	1	0	0	0	0	0	3	46-55	2
Day Total	0	0	1	8	36	58	78	151	172	118	42	6	3	2	675	46-55	323
Percent	0%	0%	0.1%	1.2%	5.3%	8.6%	11.6%	22.4%	25.5%	17.5%	6.2%	0.9%	0.4%	0.3%	0/3	40-55	323
AM Peak	12:00 AM	12:00 AM	12:00 AM	9:15 AM	9:15 AM	9:15 AM	5:00 AM	11:15 AM	10:00 AM	6:45 AM	6:45 AM	4:30 AM	7:45 AM	12:00 AM	9:15 AM		
15-min Vol	0	0	0	2	3	4	4	7	5	5	5	1	1	0	17		
PM Peak	12:00 PM	12:00 PM	1:15 PM	1:15 PM	4:15 PM	3:00 PM	3:15 PM	1:45 PM	3:30 PM	12:00 PM	1:45 PM	12:30 PM	2:00 PM	2:30 PM	3:00 PM		
15-min Vol	0	0	1	3	4	4	6	6	10	4	2	1	1	1	22		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Ply	mouth R	d north	of Rte 14													QC JOB	#: 16236662
SPECIFIC LOCA	TION:															DIF	RECTION: NB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	Pace
Grand Total	0	0	1	8	36	58	78	151	172	118	42	6	3	2	675	46-55	323
Percent	0%	0%	0.1%	1.2%	5.3%	8.6%	11.6%	22.4%	25.5%	17.5%	6.2%	0.9%	0.4%	0.3%	0/3	40-33	323
Cumulative Percent	0%	0%	0.1%	1.3%	6.7%	15.3%	26.8%	49.2%	74.7%	92.1%	98.4%	99.3%	99.7%	100%			
ADT 675				_											Me	an Speed(Avera Med	ntile: 57 MPH age): 50 MPH dian: 50 MPH ode: 53 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236662 DIRECTION: NB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 AM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
01:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
03:30 AM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
03:45 AM	0	0	0	2	0	0	0	1	0	0	0	0	0	0	3
04:00 AM	0	1	0	0	0	0	0	4	0	0	0	0	0	0	5
04:15 AM	0	2	1	3	1	0	0	1	0	0	0	0	0	0	8
04:30 AM	0	2	3	1	2	0	0	1	0	0	0	0	0	0	9
04:45 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
05:00 AM	0	2	0	1	2	0	0	1	0	0	0	0	0	0	6
05:15 AM	0	0	1	0	1	0	0	1	0	0	0	0	0	0	3
05:30 AM	0	3	2	0	0	0	0	1	0	0	0	0	0	0	6
05:45 AM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
Day Total															
Percent				DATA	A THA	ATD	RIVE	SCC	MM	UNIT	TES				
ADT 675															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	1 6/20/20	222 7 50 484									COLIDEE O	and the contract	/	//::	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB

CITY/STATE: Be	IIIOII, WA	C 0	2 4-1-		2 4-1- 6	2 4-1-	A Ande	4F A!	r A.d.	. C A!	4C A.J	C AI-	> C A!		un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
06:00 AM	0	1	3	0	1	0	0	1	0	0	0	0	0	0	6
06:15 AM	0	2	0	1	1	0	0	1	0	0	0	0	0	0	5
06:30 AM	0	4	1	2	1	0	0	2	1	0	0	0	0	0	11
06:45 AM	0	4	5	0	3	0	0	0	0	0	0	0	0	0	12
07:00 AM	0	3	1	0	2	0	0	1	0	0	0	0	0	0	7
07:15 AM	0	1	2	3	2	0	0	1	0	0	0	0	0	0	9
07:30 AM	0	0	0	0	1	0	0	2	0	0	0	0	0	0	3
07:45 AM	0	0	5	0	0	0	0	4	0	0	0	0	0	0	9
08:00 AM	0	3	0	0	1	0	0	2	0	0	0	0	0	0	6
08:15 AM	0	3	2	0	1	0	0	1	0	0	0	0	0	0	7
08:30 AM	0	1	2	2	1	0	0	2	0	0	0	0	0	0	8
08:45 AM	0	3	2	0	0	0	0	3	0	0	0	0	0	0	8
09:00 AM	0	1	1	0	2	0	0	2	0	0	0	0	0	0	6
09:15 AM	0	3	6	0	1	0	0	5	1	1	0	0	0	0	17
09:30 AM	0	0	1	0	2	0	0	4	0	0	0	0	0	0	7
09:45 AM	0	1	6	0	0	0	0	0	0	0	0	0	0	0	7
10:00 AM	0	0	4	0	3	0	0	2	0	0	0	0	0	0	9
10:15 AM	0	1	4	0	1	0	0	2	0	0	1	0	0	0	9
10:30 AM	0	3	1	0	3	0	0	2	0	0	0	0	0	0	9
10:45 AM	0	3	2	0	1	0	0	2	0	0	0	0	0	0	8
11:00 AM	0	3	5	1	1	0	0	4	1	0	0	0	0	0	15
11:15 AM	0	1	5	0		0	0	5	0	0	0	0	0	0	12
11:30 AM	0	7	1	2	0	0	0	2	1	0	0	0	0	0	13
11:45 AM	0	8	4	0	1	0	0	0	0	0	0	0	0	0	13
Day Total															
Percent				DAIA	AIHA	ALLI	RIVE	366	IVIIVI	ШИШ	IES				
ADT															
675															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB

CITY/STATE: Be	inton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 PM	0	4	6	0	4	0	0	1	0	0	0	0	0	0	15
12:15 PM	0	6	4	2	3	0	0	1	0	0	0	0	0	0	16
12:30 PM	0	1	2	1	2	0	0	1	0	0	0	0	0	0	7
12:45 PM	0	1	1	0	1	0	0	3	0	0	0	0	0	0	6
01:00 PM	0	4	1	1	1	0	0	2	0	0	0	0	0	0	9
01:15 PM	0	2	6	0	3	0	0	1	0	0	0	0	0	0	12
01:30 PM	0	2	2	0	1	0	0	3	0	0	0	0	0	0	8
01:45 PM	0	6	3	0	4	0	0	5	0	0	0	0	0	0	18
02:00 PM	0	2	2	2	0	0	0	1	0	0	0	0	0	0	7
02:15 PM	0	5	3	1	2	0	0	1	0	0	0	0	0	0	12
02:30 PM	0	1	3	0	0	0	0	1	0	0	0	0	0	0	5
02:45 PM	0	3	4	1	5	0	0	2	0	0	0	0	0	0	15
03:00 PM	0	3	9	1	4	0	0	4	0	1	0	0	0	0	22
03:15 PM	0	7	2	2	1	0	0	4	0	0	0	0	0	0	16
03:30 PM	0	5	10	1	0	0	0	4	0	0	0	0	0	0	20
03:45 PM	0	3	7	3	2	0	0	1	0	0	1	0	0	0	17
04:00 PM	0	0	3	0	1	0	0	1	0	0	0	0	0	0	5
04:15 PM	0	6	6	2	1	0	0	4	0	0	0	0	0	0	19
04:30 PM	0	6	4	0	1	0	0	0	0	0	0	0	0	0	11
04:45 PM	0	5	2	1	2	0	0	5	0	0	0	0	0	0	15
05:00 PM	0	8	1	0	2	0	0	2	0	0	0	0	0	0	13
05:15 PM	0	4	2	0	0	0	0	3	0	0	0	0	0	0	9
05:30 PM	0	2	9	1	0	0	0	5	1	0	0	0	0	0	18
05:45 PM	0	1	1	1	2	0	0	3	0	0	0	0	0	0	8
Day Total															
Percent				$)\Delta I Z$	ALHA		RIVF	500	MM	UNIT	1-5				
ADT															
675															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB

		Cars &	2 Axle	_	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
06:00 PM	0	3	3	1	2	0	0	5	0	0	0	0	0	0	14
06:15 PM	0	5	3	0	0	0	0	1	0	0	0	0	0	0	9
06:30 PM	0	1	2	0	0	0	0	1	0	0	0	0	0	0	4
06:45 PM	0	0	1	0	0	0	0	2	0	0	0	0	0	0	3
07:00 PM	0	3	1	0	2	0	0	3	0	0	0	0	0	0	9
07:15 PM	0	1	0	2	1	0	0	1	0	0	0	0	0	0	5
07:30 PM	0	2	0	0	2	0	0	2	0	0	0	0	0	0	6
07:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
08:00 PM	0	0	2	0	2	0	0	0	0	0	0	0	0	0	4
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	0	1	0	1	2	0	0	0	0	0	0	0	0	0	4
08:45 PM	0	0	0	1	1	0	0	1	0	0	0	0	0	0	3
09:00 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2
09:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
09:45 PM	0	1	0	1	0	0	0	1	0	0	0	0	0	0	3
10:00 PM	0	1	2	1	1	0	0	0	0	0	0	0	0	0	5
10:15 PM	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
11:00 PM	0	1	0	0	0	0	0	2	0	0	0	0	0	0	3
11:15 PM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
11:30 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2
11:45 PM	0	1	0	0	1	0	0	1	0	0	0	0	0	0	3
Day Total	0	190	184	49	100	0	0	142	6	2	2	0	0	0	675
Percent	0%	28.1%	27.3%	7.3%	14.8%	0%	0%	21%	0.9%	0.3%	0.3%	0%	0%	0%	0/5
ADT 675															
AM Peak		11:45 AM	9:15 AM	4:15 AM	6:45 AM	12:00 AM		9:15 AM	2:00 AM	9:15 AM			12:00 AM		9:15 A
15-min Vol	0	8	6	3	3	0	0	5	1 5 22 514	1	1	0	0	0	17
PM Peak 15-min Vol	12:00 PM 0	5:00 PM 8	3:30 PM 10	3:45 PM 3	2:45 PM 5	12:00 PM 0	12:00 PM 0	1:45 PM 5	5:30 PM 1	3:00 PM 1	3:45 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	3:00 P 22
mments:															

LOCATION: Plymouth Rd north of Rte 14

SPECIFIC LOCATION:
CITY/STATE: Benton, WA

QC JOB #: 16236662
DIRECTION: NB
DATE: Jun 13 2023

	Bikes	Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	190 28.1%	184 27.3%	49 7.3%	100 14.8%	0 0%	0 0%	142 21%	6 0.9%	2 0.3%	2 0.3%	0 0%	0 0%	0 0%	675
ADT 675															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

DIRECTION: NB DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236662

CITY/STATE: Benton, WA

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
Start Time		13 Jun 23				15-min Traffic			15-min Traffic	Average Week Frome
12:00 AM		1				1			1	
12:15 AM		1				1			1	
12:30 AM		3				3			3	
12:45 AM		0				0			0	
01:00 AM		3				3			3	
01:15 AM		1				1			1	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		1				1			1	
02:15 AM		0				0			0	
02:30 AM		1				1			1	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		2				2			2	
03:30 AM		3				3			3	
03:45 AM		3				3			3	
04:00 AM		5				5			5	
04:15 AM		8				8			8	
04:30 AM		9				9		II M	9	
04:45 AM		4				4	-	41 I	4	
05:00 AM		6				6			6	
05:15 AM		3				3 6	00.000	OF THE TIPE	3	
05:30 AM		6				6	DIVIIVI	UNII	6	
05:45 AM		6				6			6	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662

DIRECTION: NB DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		6				6			6	
06:15 AM		5				5			5	
06:30 AM		11				11			11	
06:45 AM		12				12			12	
07:00 AM		7				7			7	
07:15 AM		9				9			9	
07:30 AM		3				3			3	
07:45 AM		9				9			9	
08:00 AM		6				6			6	
08:15 AM		7				7			7	
08:30 AM		8				8			8	
08:45 AM		8				8			8	
09:00 AM		6				6			6	
09:15 AM		17				17			17	
09:30 AM		7				7			7	
09:45 AM		7				7			7	
10:00 AM		9				9			9	
10:15 AM		9				9			9	
10:30 AM		9				9			9	
10:45 AM		8				8	$\cdot \cup \iota$		8	
11:00 AM		15				15			15	
11:15 AM		12				12	0000		12	
11:30 AM		13				12 13	DIVIN		13	
11:45 AM		13				13			13	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662 DIRECTION: NB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		15				15			15	
12:15 PM		16				16			16	
12:30 PM		7				7			7	
12:45 PM		6				6			6	
01:00 PM		9				9			9	
01:15 PM		12				12			12	
01:30 PM		8				8			8	
01:45 PM		18				18			18	
02:00 PM		7				7			7	
02:15 PM		12				12			12	
02:30 PM		5				5			5	
02:45 PM		15				15			15	
03:00 PM		22				22			22	
03:15 PM		16				16			16	
03:30 PM		20				20			20	
03:45 PM		17				17			17	
04:00 PM		5				5			5	
04:15 PM		19				19			19	
04:30 PM		11				11		In'	11	
04:45 PM		15				15		411	15	
05:00 PM		13				13			13	
05:15 PM		9				9	01/11/		9	
05:30 PM		18				9 18	DIVIN	UNH	18	
05:45 PM		8				8			8	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662 DIRECTION: NB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue 13 Jun 23	Wed Thu	Fri	Average Weekday 15-min Traffic	Sat Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	14			14		14	
06:15 PM	9			9		9	
06:30 PM	4			4		4	
06:45 PM	3			3		3	
07:00 PM	9			9		9	
07:15 PM	5			5		5	
07:30 PM	6			6		6	
07:45 PM	1			1		1	
08:00 PM	4			4		4	
08:15 PM	0			0		0	
08:30 PM	4			4		4	
08:45 PM	3			3		3	
09:00 PM	2			2		2	
09:15 PM	1			1		1	
09:30 PM	1			1		1	
09:45 PM	3			3		3	
10:00 PM	5			5		5	
10:15 PM	3			3		3	
10:30 PM	0			0	Allr	0	
10:45 PM	1		all	1	JUUI	1	
11:00 PM	3			3		3	
11:15 PM	4		TILATI	4	O 8 8 8 8 1 1 1 1 1 1	4	
11:30 PM	2		IHALL	2	DIVIIVIUN	2	
11:45 PM	3			3		3	
Day Total	675			675		675	
% Weekday Average	100%						
% Week Average	100%			100%			
AM Peak	9:15 AM			9:15 AM		9:15 AM	
15-min Vol	17			17		17	
PM Peak	3:00 PM			3:00 PM		3:00 PM	
15-min Vol	22			22		22	
Comments:							

LOCATION: F		n Rd nort	h of Rte	14												QC JOB #: 1	
SPECIFIC LOC																DIRECTIO	•
CITY/STATE:	Benton,															DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
12:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
12:30 AM	0	0	0	0	0	0	0	0	1	3	0	0	0	0	4	51-60	4
12:45 AM	0	0	0	0	0	0	0	1	1	1	0	0	0	0	3	46-55	2
01:00 AM	0	0	0	0	0	2	0	0	1	0	0	1	0	0	4	31-40	2
01:15 AM	0	0	0	0	0	0	0	1	1	1	0	0	0	0	3	46-55	2
01:30 AM	0	0	0	0	0	0	0	0	1	2	2	0	0	0	5	56-65	4
01:45 AM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
02:00 AM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
02:15 AM	0	0	0	0	0	0	0	1	0	2	0	1	0	0	4	51-60	2
02:30 AM	0	0 0	0	0	0	0 0	0	0	1	0 1	0	0	0	0	1	46-55	1
02:45 AM 03:00 AM	0 0	0	0 0	0 0	0	0	0	0 0	0 0	0	0 0	0 0	0 0	0 0	1 0	51-60 1-10	1 0
03:00 AM	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2	36-45	1
03:30 AM	0	0	0	0	0	0	1	1	1	1	0	0	0	0	4	41-50	2
03:45 AM	0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	46-55	3
04:00 AM	0	0	0	0	0	1	1	3	0	1	0	0	1	0	7	41-50	4
04:15 AM	0	0	0	0	0	0	3	3	0	3	0	0	0	0	9	41-50	6
04:30 AM	0	0	0	0	0	1	1	5	1	3	2	1	0	0	14	44-53	6
04:45 AM	0	0	0	0	0	0	1	0	1	2	1	0	0	0	5	56-65	3
05:00 AM	0	0	0	0	0	0	4	0	2	5	6	1	0	0	18	56-65	11
05:15 AM	0	0	0	0	0	1	3	3	1	5	5	1	1	0	20	56-65	10
05:30 AM	0	0	0	0	1	0	0	1	3	5	4	1	0	2	17	56-65	9
05:45 AM	0	0	0	0	0	0	0	3	7	4	3	1	0	0	18	51-60	11
Day Total					TA	THE	ATI	DA	/EC	00	A // A	AL IN	IITII				
Percent				UF	MA	$I \square I$	AII	JKI	/ED	$\Box$	IVIIV	TUN	11.111				
ANA Dools																	
AM Peak 15-min Vol																	
PM Peak																	
15-min Vol																	
Comments:																	
		0/2023 7:5	4 4 4 4										COLIDO	. O !!!		tn://www.auality	

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 06:15 AM 51-60 06:30 AM 45-54 06:45 AM 56-65 07:00 AM 51-60 07:15 AM 51-60 56-65 07:30 AM 61-70 07:45 AM 08:00 AM O 51-60 08:15 AM 56-65 08:30 AM 51-60 08:45 AM 46-55 09:00 AM 46-55 09:15 AM 51-60 09:30 AM 56-65 09:45 AM 46-55 10:00 AM 51-60 10:15 AM 46-55 10:30 AM 51-60 10:45 AM 46-55 11:00 AM 51-60 11:15 AM 50-59 11:30 AM 51-60 11:45 AM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 51-60 12:15 PM 51-60 12:30 PM 41-50 12:45 PM 56-65 01:00 PM 51-60 01:15 PM 51-60 01:30 PM 51-60 01:45 PM 51-60 02:00 PM O 51-60 02:15 PM 51-60 02:30 PM 46-55 02:45 PM 56-65 03:00 PM 46-55 03:15 PM 51-60 03:30 PM 51-60 03:45 PM 46-55 04:00 PM 51-60 04:15 PM 42-51 04:30 PM O 51-60 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 51-60 05:30 PM 51-60 05:45 PM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

QC JOB #: 16236662 DIRECTION: NB, SB

CITY/STATE:	Benton,	WA														DATE: Jur	13 2023
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numbe in Pace
06:00 PM	0	0	0	0	1	1	3	6	4	4	1	0	1	0	21	46-55	10
06:15 PM	0	0	0	0	0	1	1	7	5	3	0	0	0	0	17	46-55	12
06:30 PM	0	0	0	0	0	0	0	3	2	5	1	1	0	0	12	51-60	7
06:45 PM	0	0	0	0	0	0	0	3	1	1	0	2	0	0	7	46-55	4
07:00 PM	0	0	0	0	0	0	3	4	2	2	3	0	0	0	14	41-50	7
07:15 PM	0	0	0	0	0	0	1	0	3	4	1	0	0	0	9	51-60	7
07:30 PM	0	0	0	0	1	0	0	3	4	2	2	0	0	0	12	46-55	7
07:45 PM	0	1	0	0	0	0	2	0	2	2	0	0	0	0	7	51-60	4
08:00 PM	0	0	0	0	0	2	0	1	1	4	3	0	0	0	11	56-65	7
08:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:30 PM	0	0	0	0	0	0	0	3	4	4	0	1	0	0	12	51-60	8
08:45 PM	0	0	0	0	0	3	0	1	3	3	1	1	0	0	12	51-60	6
09:00 PM	0	0	0	0	0	0	1	1	3	0	0	0	0	0	5	46-55	4
09:15 PM	0	0	0	0	0	0	0	1	0	0	2	0	0	0	3	56-65	2
09:30 PM	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2	36-45	1
09:45 PM	0	0	0	1	0	1	1	2	0	1	0	0	0	0	6	41-50	3
10:00 PM	0	0	0	0	0	2	0	1	3	0	0	1	0	0	7	46-55	4
10:15 PM	0	0	0	2	0	0	1	1	1	0	0	0	0	0	5	21-30	2
10:30 PM	0	0	0	0	0	0	2	0	1	0	0	0	0	0	3	36-45	2
10:45 PM	0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	46-55	3
11:00 PM	0	0	0	0	0	1	1	1	0	0	1	0	0	0	4	36-45	2
11:15 PM	0	0	0	0	0	0	0	2	0	1	2	0	0	0	5	56-65	3
11:30 PM	0	0	0	1	2	0	0	0	3	0	0	0	0	0	6	46-55	3
11:45 PM	0	0	0	0	1	0	0	3	2	1	0	0	0	0	7	46-55	5
Day Total	1	1	1	11	40	75	121	229	331	334	150	38	13	7	1352	51-60	665
Percent	0.1%	0.1%	0.1%	0.8%	3%	5.5%	8.9%	16.9%	24.5%	24.7%	11.1%	2.8%	1%	0.5%		32 33	
AM Peak		12:00 AM		9:15 AM	9:15 AM	7:45 AM	11:00 AM		9:00 AM	6:45 AM	6:45 AM	7:45 AM	6:00 AM	5:30 AM	9:15 AM		
15-min Vol	0	0	0	2	3	4	5	9	9	8	7	2	2	2	32		
PM Peak 15-min Vol	2:15 PM 1	7:45 PM 1	1:15 PM 1	1:15 PM 3	4:15 PM 4	3:00 PM 4	3:15 PM 7	3:00 PM 10	3:30 PM 14	3:15 PM 11	2:45 PM 5	5:30 PM 2	12:45 PM 1	2:30 PM 1	3:00 PM 39		
Comments:																	

Type of report: Tube Count - Speed Data

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Ply	mouth R	d north c	of Rte 14													QC JOB	#: 16236662
SPECIFIC LOCA																DIREC	FION: NB, SB
CITY/STATE: B	enton, W	'A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
opeca nange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	- Total	r dec opecu	Pace
Grand Total	1	1	1	11	40	75	121	229	331	334	150	38	13	7	1352	51-60	665
Percent	0.1%	0.1%	0.1%	0.8%	3%	5.5%	8.9%	16.9%	24.5%	24.7%	11.1%	2.8%	1%	0.5%	1552	31-00	003
Cumulative Percent	0.1%	0.1%	0.2%	1%	4%	9.5%	18.5%	35.4%	59.9%	84.6%	95.7%	98.5%	99.5%	100%			
ADT 1352				_											Me	an Speed(Avera Med	ntile: 60 MPH nge): 52 MPH dian: 52 MPH ode: 58 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236662 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	1	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 AM	0	2	0	0	1	0	0	1	0	0	0	0	0	0	4
12:45 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
01:00 AM	0	2	0	0	0	0	0	1	1	0	0	0	0	0	4
01:15 AM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
01:30 AM	0	2	0	0	1	0	0	1	1	0	0	0	0	0	5
01:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
02:00 AM	0	0	0	0	1	0	0	0	1	0	0	0	0	0	2
02:15 AM	0	2	1	0	0	0	0	0	1	0	0	0	0	0	4
02:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
03:30 AM	0	3	0	0	1	0	0	0	0	0	0	0	0	0	4
03:45 AM	0	0	0	2	0	0	0	1	0	0	0	0	0	0	3
04:00 AM	0	2	1	0	0	0	0	4	0	0	0	0	0	0	7
04:15 AM	0	2	2	3	1	0	0	1	0	0	0	0	0	0	9
04:30 AM	0	4	4	1	3	0	0	2	0	0	0	0	0	0	14
04:45 AM	0	3	2	0	0	0	0	0	0	0	0	0	0	0	5
05:00 AM	0	10	2	1	3	0	0	2	0	0	0	0	0	0	18
05:15 AM	0	4	8	2	4	0	0	1	1	0	0	0	0	0	20
05:30 AM	0	7	6	0	3	0	0	1	0	0	0	0	0	0	17
05:45 AM	0	6	7	0	4	0	0	1	0	0	0	0	0	0	18
Day Total															
Percent				DAIA	A 1 1 1 1 A	AL LU	RIVE	3.00	/IVIIVI	UIVIII	IES				
ADT 1352															
1352															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
		122 7.FO ANA										عمدين مناام			

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236662 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202 <b>Total</b>
Start rime	Dikes	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	TOLAI
06:00 AM	0	3	8	0	5	0	0	1	1	0	0	0	0	0	18
06:15 AM	0	2	3	1	5	0	0	2	0	0	0	0	0	0	13
06:30 AM	0	5	4	2	1	0	0	2	2	0	0	0	0	0	16
06:45 AM	0	9	5	0	7	0	0	0	0	0	0	0	0	0	21
07:00 AM	0	7	3	1	2	0	0	2	0	0	0	0	0	0	15
07:15 AM	0	3	5	4	3	0	0	1	0	0	0	0	0	0	16
07:30 AM	0	2	2	0	4	0	0	3	0	0	0	0	0	0	11
07:45 AM	0	4	5	0	2	0	0	4	1	0	0	0	0	0	16
MA 00:80	0	4	4	1	2	0	0	2	1	0	0	0	0	0	14
08:15 AM	0	5	5	0	2	0	0	4	0	0	0	0	0	0	16
08:30 AM	0	3	5	5	1	0	0	2	0	0	0	0	0	0	16
08:45 AM	0	6	5	0	0	0	0	3	0	0	0	0	0	0	14
09:00 AM	0	6	5	0	3	0	0	3	1	0	0	0	0	0	18
09:15 AM	0	7	11	1	5	0	0	6	1	1	0	0	0	0	32
09:30 AM	0	3	2	0	2	0	0	5	0	0	0	0	0	0	12
09:45 AM	0	4	10	1	2	0	0	0	0	0	0	0	0	0	17
10:00 AM	0	4	9	0	4	0	0	3	0	0	0	0	0	0	20
10:15 AM	0	3	4	0	2	0	0	5	0	0	1	0	0	0	15
10:30 AM	0	7	6	0	5	0	0	2	1	0	0	0	0	0	21
10:45 AM	0	7	3	0	1	0	0	4	0	0	0	0	0	0	15
11:00 AM	0	3	7	1	1	0	0	4	2	0	0	0	0	0	18
11:15 AM	0	7	6	0	1	0	0	5	0	0	0	0	0	0	19
11:30 AM	0	9	3	2	1	0	0	2	1	0	0	0	0	0	18
11:45 AM	0	12	7	0	1	0	0	0	0	0	0	0	0	0	20
Day Total															
Percent				DAIA	1.1.7	ALDI	RIVE	3.66	NVIIVI	UIVIII	IES				
ADT 1352															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	C /20 /2/	222750414	•	•		•			•	•	COLUBATE O	1:. 6	110/1	//	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB, SB DATE: Jun 13 2023

Cars & 2 Axle 2 Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Bikes Start Time Buses Total **Trailers** Tire Single Single Double Double **Double** Multi Multi Multi Classed Long 12:00 PM 12:15 PM 12:30 PM 12:45 PM 01:00 PM 01:15 PM 01:30 PM 01:45 PM 02:00 PM 02:15 PM 02:30 PM 02:45 PM 03:00 PM 03:15 PM 03:30 PM 03:45 PM 04:00 PM 04:15 PM 04:30 PM 04:45 PM 05:00 PM 05:15 PM 05:30 PM 05:45 PM Day Total Percent ADT 

AM Peak 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB, SB

O6:05 PM	Total	Not Classed	>6 Axl Multi	6 Axle Multi	<6 Axl Multi	>6 Axl Double	5 Axle Double	<5 Axl Double	4 Axle Single	3 Axle Single	2 Axle 6 Tire	Buses	2 Axle Long	Cars & Trailers	Bikes	Start Time
06:35 PM	21	0	0	0	0	0	0	6	0	0	2	1		7	0	06:00 PM
G6-SF PM	17	0	0	0	0	0	0	3	0	0	1	1	5	7	0	06:15 PM
07:00 PM	12	0	0	0	0	0	0	3	0	0	1	0	3	5	0	06:30 PM
07:15 PM	7	0	0	0	0	0	0	2	0	0	0	0	1	4	0	06:45 PM
07:30 PM	14	0	0	0	0	0	0	5	0	0	3	0	1	5	0	07:00 PM
07:45 PM	9	0	0	0	0	0	0	1	0	0	2	2	2	2	0	07:15 PM
08:00 PM	12	0	0	0	0	0	0	4	0	0	3	0	2	3	0	07:30 PM
08:15 PM	7	0	0	0	0	0	0	3	0	0	0	0	2	2	0	07:45 PM
08:30 PM	11	0	0	0	0	0	1	1	0	0	3	0	5	1	0	08:00 PM
08:45 PM	1	0	0	0	0	0	0	0	0	0	0	0	1	0	0	08:15 PM
09:00 PM	12	0	0	0	0	0	0	0	0	0	3	3	0	6	0	08:30 PM
09:15 PM	12	0	0	0	0	0	0	2	0	0	3	1	3	3	0	08:45 PM
09:30 PM	5	0	0	0	0	0	0	2	0	0	1	0	0	2	0	09:00 PM
09:45 PM	3	0	0	0	0	0	0	0	0	0	1	0	2	0	0	09:15 PM
10:00 PM	2	0	0	0	0	0	0	0	0	0	1	0	0	1	0	09:30 PM
10:15 PM	6	0	1	0	0	0	0	1	0	0	0	1	0	3	0	09:45 PM
10:30 PM	7	0	0	0	0	0	0	0	0	0	2	1	3	1	0	10:00 PM
10:45 PM	5	0	0	0	0	0	1	0	0	0	0	3	0	1	0	10:15 PM
11:00 PM	3	0	0	0	0	0	0	1	0	0	0	0	0	2	0	10:30 PM
11:15 PM 0 3 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3	0	0	0	0	0	0	0	0	0	1	0	2	0	0	10:45 PM
11:30 PM 0 1 1 1 1 2 0 0 0 0 0 1 0 0 0 0 0 0 1 1 1 1	4	0	0	0	0	0	0	2	0	0	0	1	0	1	0	11:00 PM
11:45 PM         0         2         1         0         2         0         0         2         0<	5	0	0	0	0	0	0	0	0	0	0	0	2	3	0	11:15 PM
Day Total Percent         1 465 389 68 193 0 0 0 204 26 2 3 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	6	0	0	0	0	0	1	0	0	0	2	1	1	1	0	11:30 PM
Percent         0.1%         34.4%         28.8%         5%         14.3%         0%         0%         15.1%         1.9%         0.1%         0.2%         0%         0.1%         0%           ADT 1352         1	7	0	0	0	0	0	0	2	0	0	2	0	1	2	0	11:45 PM
ADT 1352	1352	0	1	0	3	2	26	204	0	0	193	68	389	465	1	Day Total
1352	1552	0%	0.1%	0%	0.2%	0.1%	1.9%	15.1%	0%	0%	14.3%	5%	28.8%	34.4%	0.1%	Percent
	9:15 AN	12:00 AM	12:00 AM	12:00 AM	10:15 AM	9:15 AM	6:30 AM	9:15 AM	12:00 AM	12:00 AM	6:45 AM	8:30 AM			12:00 AM	
15-min Vol 0 12 11 5 7 0 0 6 2 1 1 0 0 0	32	0	0	0	1	1	2	6	0	0	7	5	11	12	0	15-min Vol
PM Peak 2:15 PM 5:00 PM 3:00 PM 3:45 PM 1:45 PM 12:00 PM 12:00 PM 1:45 PM 2:00 PM 3:00 PM 3:00 PM 12:00 PM 9:45 PM 12:00 PM	3:00 PM															
15-min Vol 1 18 18 3 6 0 0 9 3 1 1 0 1 0	39	0	1	0	1	1	3	9	0	0	6	3	18	18	1	15-min Vol
omments:					<u> </u>	<u> </u>	<u> </u>					<u> </u>				mments:

LOCATION: Plymouth Rd north of Rte 14

SPECIFIC LOCATION:
CITY/STATE: Benton, WA

Start Time

Bikes

Cars & 2 Axle
Trailers

Long

COLOB #: 16236662

DIRECTION: NB, SB

DATE: Jun 13 2023

Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Total

Total

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	1 0.1%	465 34.4%	389 28.8%	68 5%	193 14.3%	0 0%	0 0%	204 15.1%	26 1.9%	2 0.1%	3 0.2%	0 0%	1 0.1%	0 0%	1352
ADT 1352															
Comments:															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB, SB

**DATE:** Jun 13 2023 - Jun 13 2023

CITI/STATE.	Benton, WA									TE: Jun 13 2023 - Jun 13 2023
Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	/ Sat	Sun	Average Week	Average Week Profile
Start Time		13 Jun 23				15-min Traffic			15-min Traffic	Average week Frome
12:00 AM		1				1			1	
12:15 AM		1				1			1	
12:30 AM		4				4			4	
12:45 AM		3				3			3	
01:00 AM		4				4			4	
01:15 AM		3				3			3	
01:30 AM		5				5			5	
01:45 AM		2				2			2	
02:00 AM		2				2			2	
02:15 AM		4				4			4	
02:30 AM		1				1			1	
02:45 AM		1				1			1	
03:00 AM		0				0	7		0	
03:15 AM		2				2			2	
03:30 AM		4				4			4	
03:45 AM		3				3			3	
04:00 AM		7				7			7	
04:15 AM		9				9			9	
04:30 AM		14				14		In.	14	
04:45 AM		5				5		$A \mid I$	5	
05:00 AM		18				18			18	
05:15 AM		20				20	C 0 1 1 1 1	LIKIT	20	
05:30 AM		17				17	JUIVIU	UNII	17	
05:45 AM		18				18			18	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION:

**DIRECTION:** NB, SB CITY/STATE: Benton, WA **DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		18				18			18	
06:15 AM		13				13			13	
06:30 AM		16				16			16	
06:45 AM		21				21			21	
07:00 AM		15				15			15	
07:15 AM		16				16			16	
07:30 AM		11				11			11	
07:45 AM		16				16			16	
08:00 AM		14				14			14	
08:15 AM		16				16	\ .		16	
08:30 AM		16				16			16	
08:45 AM		14				14			14	
09:00 AM		18				18			18	
09:15 AM		32				32			32	
09:30 AM		12				12			12	
09:45 AM		17				17			17	
10:00 AM		20				20			20	
10:15 AM		15				15			15	
10:30 AM		21				21			21	
10:45 AM		15				15			15	
11:00 AM		18				18			18	
11:15 AM		19				19	00000		19	
11:30 AM		18				18	OIVIIVI		18	
11:45 AM		20				20			20	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak 15-min Vol										
PM Peak 15-min Vol										
Comments:										

QC JOB #: 16236662

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB, SB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		27				27			27	
12:15 PM		24				24			24	
12:30 PM		17				17			17	
12:45 PM		14				14			14	
01:00 PM		20				20			20	
01:15 PM		30				30			30	
01:30 PM		21				21			21	
01:45 PM		30				30			30	
02:00 PM		24				24			24	
02:15 PM		20				20			20	
02:30 PM		15				15			15	
02:45 PM		25				25			25	
03:00 PM		39				39			39	
03:15 PM		32				32			32	
03:30 PM		34				34			34	
03:45 PM		25				25			25	
04:00 PM		22				22			22	
04:15 PM		27				27			27	
04:30 PM		25				25		In.	25	
04:45 PM		32				32		411	32	
05:00 PM		29				29			29	
05:15 PM		27				27	0 0 00 0		27	
05:30 PM		27				27 27	DIVIN	UNII	27	
05:45 PM		16				16			16	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: NB, SB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		21				21			21	
06:15 PM		17				17			17	
06:30 PM		12				12			12	
06:45 PM		7				7			7	
07:00 PM		14				14			14	
07:15 PM		9				9			9	
07:30 PM		12				12			12	
07:45 PM		7				7			7	
08:00 PM		11				11			11	
08:15 PM		1				1			1	
08:30 PM		12				12			12	
08:45 PM		12				12			12	
09:00 PM		5				5			5	
09:15 PM		3				3			3	
09:30 PM		2				2			2	
09:45 PM		6				6			6	
10:00 PM		7				7			7	
10:15 PM		5			_   °	5			5	
10:30 PM		3				3		In.	3	
10:45 PM		3				3	-	$\mathcal{A}$	3	
11:00 PM		4				4			4	
11:15 PM		5			7 I A 77 I	5 6	00 40 4	17 18 117	5	
11:30 PM		6			HALL	6	DIVIIVI	UNII	6	
11:45 PM		7				7			7	
Day Total		1352				1352			1352	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		9:15 AM				9:15 AM			9:15 AM	
15-min Vol		32				32			32	
PM Peak		3:00 PM				3:00 PM			3:00 PM	
15-min Vol		39				39			39	
Comments:										

CITY/STATE:																DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
12:45 AM	0	0	0	0	0	0	0	1	1	1	0	0	0	0	3	46-55	2
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
01:15 AM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
01:30 AM	0	0	0	0	0	0	0	0	1	2	2	0	0	0	5	56-65	4
01:45 AM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
02:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
02:15 AM	0	0	0	0	0	0	0	1	0	2	0	1	0	0	4	51-60	2
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2	41-50	1
04:00 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
04:13 AM	0	0	0	0	0	0	0	0	1	2	2	0	0	0	5	56-65	4
04:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:00 AM	0	0	0	0	0	0	0	0	0	5	6		0	0	12	56-65	11
05:00 AM 05:15 AM	0	0	0	0	0	0	2	2	1	5	5	1	1	0	17	56-65	10
		0	0	0	0	0	0				2	1	0	2	11		
05:30 AM 05:45 AM	0 0	0	0	0	0	0	0	1	1 4	4		_	0	0	12	56-65 51-60	6 7
Day Total	U	U	U	U	U	U	U	2	4	3	2	1	U	U	12	21-00	/
Percent																	
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 51-60 06:15 AM 06:30 AM 56-65 06:45 AM 51-60 07:00 AM 56-65 07:15 AM 51-60 56-65 07:30 AM 61-70 07:45 AM 08:00 AM O 56-65 08:15 AM 56-65 08:30 AM 51-60 08:45 AM 51-60 09:00 AM 51-60 09:15 AM 51-60 09:30 AM 56-65 09:45 AM 46-55 10:00 AM 55-64 10:15 AM 46-55 10:30 AM O 51-60 10:45 AM 41-50 11:00 AM 36-45 11:15 AM 51-60 11:30 AM 56-65 11:45 AM 53-62 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236662 LOCATION: Plymouth Rd north of Rte 14 **SPECIFIC LOCATION: DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 56-65 41-50 12:30 PM 12:45 PM 56-65 01:00 PM 55-64 01:15 PM 51-60 01:30 PM 51-60 01:45 PM 55-64 02:00 PM O 51-60 02:15 PM 51-60 02:30 PM 51-60 02:45 PM 56-65 03:00 PM 46-55 03:15 PM 51-60 03:30 PM 51-60 03:45 PM 56-65 04:00 PM 51-60 04:15 PM 41-50 04:30 PM O 55-64 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 51-60 05:30 PM 51-60 05:45 PM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Plymouth Rd north of Rte 14 QC JOB #: 16236662 SPECIFIC LOCATION:

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
06:00 PM	0	0	0	0	0	0	1	1	3	2	0	0	0	0	7	51-60	5
06:15 PM	0	0	0	0	0	0	1	4	2	1	0	0	0	0	8	46-55	6
06:30 PM	0	0	0	0	0	0	0	1	2	3	1	1	0	0	8	51-60	5
06:45 PM	0	0	0	0	0	0	0	0	1	1	0	2	0	0	4	61-70	2
07:00 PM	0	0	0	0	0	0	0	0	1	1	3	0	0	0	5	56-65	4
07:15 PM	0	0	0	0	0	0	0	0	1	2	1	0	0	0	4	56-65	3
07:30 PM	0	0	0	0	0	0	0	0	2	2	2	0	0	0	6	51-60	4
07:45 PM	0	1	0	0	0	0	1	0	2	2	0	0	0	0	6	51-60	4
08:00 PM	0	0	0	0	0	0	0	1	1	3	2	0	0	0	7	56-65	5
08:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:30 PM	0	0	0	0	0	0	0	1	4	2	0	1	0	0	8	51-60	6
08:45 PM	0	0	0	0	0	2	0	1	1	3	1	1	0	0	9	53-62	4
09:00 PM	0	0	0	0	0	0	0	0	3	0	0	0	0	0	3	46-55	3
09:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	56-65	2
09:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
09:45 PM	0	0	0	1	0	0	0	2	0	0	0	0	0	0	3	41-50	2
10:00 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	2	31-40	2
10:15 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	2	41-50	2
10:30 PM	0	0	0	0	0	0	2	0	1	0	0	0	0	0	3	36-45	2
10:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	2	46-55	2
11:00 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
11:30 PM	0	0	0	1	2	0	0	0	1	0	0	0	0	0	4	26-35	3
11:45 PM	0	0	0	0	0	0	0	2	1	1	0	0	0	0	4	46-55	3
Day Total	1	1	0	3	4	17	43	78	159	216	108	32	10	5			
Percent	0.1%	0.1%	0%	0.4%	0.6%	2.5%	6.4%	11.5%	23.5%	31.9%	16%	4.7%	1.5%	0.7%	677	51-60	375
													_				
AM Peak	12:00 AM 0	12:00 AM 0	12:00 AM 0	11:45 AM 1	12:00 AM 0	10:30 AM 3	5:15 AM 2	10:45 AM 5	9:00 AM 7	11:15 AM 6	5:00 AM 6	7:45 AM 2	6:00 AM 2	5:30 AM 2	5:15 AM 17		
15-min Vol																	
PM Peak 15-min Vol	2:15 PM 1	7:45 PM 1	12:00 PM 0	9:45 PM 1	5:15 PM 2	2:00 PM 2	4:15 PM 3	3:00 PM 7	5:15 PM 8	2:00 PM 9	2:45 PM 5	5:30 PM 2	12:45 PM 1	3:15 PM 1	1:15 PM 18		

LOCATION: Ply	mouth R	ld north o	f Rte 14														#: 16236662
SPECIFIC LOCA	TION:															DI	RECTION: SB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1 15	16	21	26	31	36	41 45	46 50	51	56 60	61 65	66 70	71	76	Total	Pace Speed	Number in
speed hange		20	25	30	35	40			55				75	999	TOLAI	race speed	Pace
Grand Total	1	1	0	3	4	17	43	78	159	216	108	32	10	5	677	51-60	375
Percent	0.1%	0.1%	0%	0.4%	0.6%	2.5%	6.4%	11.5%	23.5%	31.9%	16%	4.7%	1.5%	0.7%	0//	31-00	3/3
Cumulative Percent	0.1%	0.3%	0.3%	0.7%	1.3%	3.8%	10.2%	21.7%	45.2%	77.1%	93.1%	97.8%	99.3%	100%			
ADT 677															Me	an Speed(Avera	ntile: 62 MPH age): 55 MPH dian: 55 MPH ode: 58 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236662 DIRECTION: SB DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
12:45 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
01:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
01:15 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
01:30 AM	0	2	0	0	1	0	0	1	1	0	0	0	0	0	5
01:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
02:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
02:15 AM	0	2	1	0	0	0	0	0	1	0	0	0	0	0	4
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
04:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
04:30 AM	0	2	1	0	1	0	0	1	0	0	0	0	0	0	5
04:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
05:00 AM	0	8	2	0	1	0	0	1	0	0	0	0	0	0	12
05:15 AM	0	4	7	2	3	0	0	0	1	0	0	0	0	0	17
05:30 AM	0	4	4	0	3	0	0	0	0	0	0	0	0	0	11
05:45 AM	0	3	5	0	3	0	0	1	0	0	0	0	0	0	12
Day Total Percent															
ADT							NVL			CINI					
677															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	1 6/20/20	222 7 50 484									COLIDER O	1:4	/	//	

SPECIFIC LOCATION:

QC JOB #: 16236662 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 202
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 AM	0	2	5	0	4	0	0	0	1	0	0	0	0	0	12
06:15 AM	0	0	3	0	4	0	0	1	0	0	0	0	0	0	8
06:30 AM	0	1	3	0	0	0	0	0	1	0	0	0	0	0	5
06:45 AM	0	5	0	0	4	0	0	0	0	0	0	0	0	0	9
07:00 AM	0	4	2	1	0	0	0	1	0	0	0	0	0	0	8
07:15 AM	0	2	3	1	1	0	0	0	0	0	0	0	0	0	7
07:30 AM	0	2	2	0	3	0	0	1	0	0	0	0	0	0	8
07:45 AM	0	4	0	0	2	0	0	0	1	0	0	0	0	0	7
08:00 AM	0	1	4	1	1	0	0	0	1	0	0	0	0	0	8
08:15 AM	0	2	3	0	1	0	0	3	0	0	0	0	0	0	9
08:30 AM	0	2	3	3	0	0	0	0	0	0	0	0	0	0	8
08:45 AM	0	3	3	0	0	0	0	0	0	0	0	0	0	0	6
09:00 AM	0	5	4	0	1	0	0	1	1	0	0	0	0	0	12
09:15 AM	0	4	5	1	4	0	0	1	0	0	0	0	0	0	15
09:30 AM	0	3	1	0	0	0	0	1	0	0	0	0	0	0	5
09:45 AM	0	3	4	1	2	0	0	0	0	0	0	0	0	0	10
10:00 AM	0	4	5	0	1	0	0	1	0	0	0	0	0	0	11
10:15 AM	0	2	0	0	1	0	0	3	0	0	0	0	0	0	6
10:30 AM	0	4	5	0	2	0	0	0	1	0	0	0	0	0	12
10:45 AM	0	4	1	0	0	0	0	2	0	0	0	0	0	0	7
11:00 AM	0	0	2	0	0	0	0	0	1	0	0	0	0	0	3
11:15 AM	0	6	1	0	0	0	0	0	0	0	0	0	0	0	7
11:30 AM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
11:45 AM	0	4	3	0	0	0	0	0	0	0	0	0	0	0	7
Day Total															
Percent				DATA	ATH	AT D	RIVF	SCC	MM	UNIT	TES				
ADT 677															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	L C /20 /20	22 7 50 414									COLUBEE O			//!:	

SPECIFIC LOCATION:

QC JOB #: 16236662 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 PM	0	6	4	0	1	0	0	0	1	0	0	0	0	0	12
12:15 PM	0	5	1	0	2	0	0	0	0	0	0	0	0	0	8
12:30 PM	0	3	3	1	3	0	0	0	0	0	0	0	0	0	10
12:45 PM	0	2	5	0	1	0	0	0	0	0	0	0	0	0	8
01:00 PM	0	1	6	1	2	0	0	1	0	0	0	0	0	0	11
01:15 PM	0	9	4	0	2	0	0	3	0	0	0	0	0	0	18
01:30 PM	0	5	6	0	2	0	0	0	0	0	0	0	0	0	13
01:45 PM	0	1	5	0	2	0	0	4	0	0	0	0	0	0	12
02:00 PM	0	3	7	0	4	0	0	0	3	0	0	0	0	0	17
02:15 PM	1	3	3	0	1	0	0	0	0	0	0	0	0	0	8
02:30 PM	0	6	4	0	0	0	0	0	0	0	0	0	0	0	10
02:45 PM	0	7	2	0	0	0	0	1	0	0	0	0	0	0	10
03:00 PM	0	5	9	0	1	0	0	1	0	0	1	0	0	0	17
03:15 PM	0	9	4	0	2	0	0	1	0	0	0	0	0	0	16
03:30 PM	0	4	5	0	1	0	0	4	0	0	0	0	0	0	14
03:45 PM	0	3	2	0	2	0	0	1	0	0	0	0	0	0	8
04:00 PM	0	12	3	0	1	0	0	1	0	0	0	0	0	0	17
04:15 PM	0	3	3	0	0	0	0	2	0	0	0	0	0	0	8
04:30 PM	0	6	5	0	1	0	0	1	1	0	0	0	0	0	14
04:45 PM	0	11	2	0	4	0	0	0	0	0	0	0	0	0	17
05:00 PM	0	10	3	1	0	0	00	1	1	0	0	0	0	0	16
05:15 PM	0	9	3	2	1	0	0	3	0	0	0	0	0	0	18
05:30 PM	0	5	3	0	1	0	0	0	0	0	0	0	0	0	9
05:45 PM	0	3	2	0	1	0	0	2	0	0	0	0	0	0	8
Day Total					a manual of		ESD /E			1 TK 112					
Percent				DAIA	AIHA	ALD	RIVE	SCC	MM	UNIT	11-5				
ADT 677															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
omments:															
	L C /20 /20	2227 50 414									COLUDEE O	Line Commun	116/11	. / /	4

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236662 DIRECTION: SB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	4	2	0	0	0	0	1	0	0	0	0	0	0	7
06:15 PM	0	2	2	1	1	0	0	2	0	0	0	0	0	0	8
06:30 PM	0	4	1	0	1	0	0	2	0	0	0	0	0	0	8
06:45 PM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
07:00 PM	0	2	0	0	1	0	0	2	0	0	0	0	0	0	5
07:15 PM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
07:30 PM	0	1	2	0	1	0	0	2	0	0	0	0	0	0	6
07:45 PM	0	2	2	0	0	0	0	2	0	0	0	0	0	0	6
08:00 PM	0	1	3	0	1	0	0	1	1	0	0	0	0	0	7
08:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
08:30 PM	0	5	0	2	1	0	0	0	0	0	0	0	0	0	8
08:45 PM	0	3	3	0	2	0	0	1	0	0	0	0	0	0	9
09:00 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
09:15 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
09:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	1	0	3
10:00 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
10:15 PM	0	1	0	0	0	0	0	0	1	0	0	0	0	0	2
10:30 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
10:45 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
11:00 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:30 PM	0	1	1	0	1	0	0	0	1	0	0	0	0	0	4
11:45 PM	0	1	1	0	1	0	0	1	0	0	0	0	0	0	4
Day Total	1	275	205	19	93	0	0	62	20	0	1	0	1	0	677
Percent	0.1%	40.6%	30.3%	2.8%	13.7%	0%	0%	9.2%	3%	0%	0.1%	0%	0.1%	0%	077
ADT 677															
AM Peak	12:00 AM	5:00 AM	5:15 AM	8:30 AM	6:00 AM	12:00 AM	12:00 AM	8:15 AM	1:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	5:15 AN
15-min Vol	0	8	7	3	4	0	0	3	1	0	0	0	0	0	17
PM Peak	2:15 PM	4:00 PM	3:00 PM	5:15 PM	2:00 PM	12:00 PM	12:00 PM	1:45 PM	2:00 PM	12:00 PM	3:00 PM	12:00 PM	9:45 PM	12:00 PM	1:15 PN
15-min Vol	1	12	9	2	4	0	0	4	3	0	1	0	1	0	18
mments:															

LOCATION: Plymouth Rd north of Rte 14

SPECIFIC LOCATION:

DIRECTION: SB

CITY/STATE: Benton, WA

DATE: Jun 13 2023

CITY/STATE: BEI	iton, wa													DATE:	Jun 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	1	275	205	19	93	0	0	62	20	0	1	0	1	0	677
Percent	0.1%	40.6%	30.3%	2.8%	13.7%	0%	0%	9.2%	3%	0%	0.1%	0%	0.1%	0%	0//
ADT 677															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

**DIRECTION:** SB **DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236662

CITY/STATE: Benton, WA

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		1				1			1	
12:45 AM		3				3			3	
01:00 AM		1				1			1	
01:15 AM		2				2			2	
01:30 AM		5				5			5	
01:45 AM		2				2			2	
02:00 AM		1				1			1	
02:15 AM		4				4			4	
02:30 AM		0				0			0	
02:45 AM		1				1			1	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		0				0			0	
04:00 AM		2				2			2	
04:15 AM		1			_   °.	1			1	
04:30 AM		5			21 1	5			5	
04:45 AM		1			461	1			1	
05:00 AM		12				12			12	
05:15 AM		17			TIATI	17 11	00 40 4		17	
05:30 AM		11			HALL	)	JIVIIVI		15 11	
05:45 AM		12				12			12	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662

**DIRECTION: SB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		12				12			12	
06:15 AM		8				8			8	
06:30 AM		5				5			5	
06:45 AM		9				9			9	
07:00 AM		8				8			8	
07:15 AM		7				7			7	
07:30 AM		8				8			8	
07:45 AM		7				7			7	
08:00 AM		8				8			8	
08:15 AM		9				9			9	
08:30 AM		8				8			8	
08:45 AM		6				6			6	
09:00 AM		12				12			12	
09:15 AM		15				15			15	
09:30 AM		5				5			5	
09:45 AM		10				10			10	
10:00 AM		11				11			11	
10:15 AM		6				6			6	
10:30 AM		12				12			12	
10:45 AM		7				7			7	
11:00 AM		3				3			3	
11:15 AM		7				7 0R/V5S C(	00000		7	
11:30 AM		5				JK / 5	JIVIIVI		<b>1 E 5</b>	
11:45 AM		7				7			7	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662 DIRECTION: SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		12				12			12	
12:15 PM		8				8			8	
12:30 PM		10				10			10	
12:45 PM		8				8			8	
01:00 PM		11				11			11	
01:15 PM		18				18			18	
01:30 PM		13				13			13	
01:45 PM		12				12			12	
02:00 PM		17				17			17	
02:15 PM		8				8			8	
02:30 PM		10				10			10	
02:45 PM		10				10			10	
03:00 PM		17				17			17	
03:15 PM		16				16			16	
03:30 PM		14				14			14	
03:45 PM		8				8			8	
04:00 PM		17				17			17	
04:15 PM		8				8			8	
04:30 PM		14				8 14			14	
04:45 PM		17				17			17	
05:00 PM		16				16			16	
05:15 PM		18				18	0 0 0 0 0		18	
05:30 PM		9				18 9	DIVIN		9	
05:45 PM		8				8			8	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236662 DIRECTION: SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		7				7			7	
06:15 PM		8				8			8	
06:30 PM		8				8			8	
06:45 PM		4				4			4	
07:00 PM		5				5			5	
07:15 PM		4				4			4	
07:30 PM		6				6			6	
07:45 PM		6				6			6	
08:00 PM		7				7			7	
08:15 PM		1				1			1	
08:30 PM		8				8			8	
08:45 PM		9				9			9	
09:00 PM		3				3			3	
09:15 PM		2				2			2	
09:30 PM		1				1			1	
09:45 PM		3				3	-		3	
10:00 PM		2				2			2	
10:15 PM		2				2			2	
10:30 PM		3				3		In.	3	
10:45 PM		2			261	2	$\cdot \cup \iota$	411	2	
11:00 PM		1				1			1	
11:15 PM		1				11			1	
11:30 PM		4			HALL	DRIV4S CO	DIVIN	UNII	4	
11:45 PM		4				4			4	
Day Total		677				677			677	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		5:15 AM				5:15 AM			5:15 AM	
15-min Vol		17				17			17	
PM Peak		1:15 PM				1:15 PM			1:15 PM	
15-min Vol		18				18			18	

CITY/STATE:	1	16	21	26	31	36	41	46	51	56	61	66	71	76		DATE: Jur	Numb
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	71 75	999	Total	Pace Speed	in Pa
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
			0					0		0		-		0	0		
04:45 AM	0 0	0 0	0	0	0	0	0		0		0	0	0		0	1-10	0
05:00 AM		-		0	0	0		0		0	0	0	0	0	_	1-10	_
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total Percent																	
AM Peak																	
.5-min Vol																	
PM Peak																	

CITY/STATE:			24	26	24	2.0	44	46	F4	F.C	C4		74	7.0	Ī	DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
11:45 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	2	36-45	1
Day Total	U	U	U	U	U	0		U	U	1	- 0	U	U	U	Z	30-43	1
Percent				DA	TA	TH	ATI	ORIN	/ES	CO	MN	1UN	IITIF	-5			
AM Peak																	
5-min Vol																	
PM Peak																	

CITY/STATE:																DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb
12:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	2	51-60	2
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 PM	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2	36-45	1
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
04:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
05:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	- 0	- 0	- 0	0	0	0	U	U	U	0	0	U	- 0	0	U	1-10	U
Percent				DA	TA	TH	ATI	DRI	/ES	CO	MN	1UN	IITIE	-5			
AM Peak																	
5-min Vol																	
PM Peak																	

QC JOB #: 16236663 LOCATION: Bofer Canyon Rd north of Coffin Rd **SPECIFIC LOCATION: DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number Start Time Total Pace Speed in Pace 06:00 PM 1-10 06:15 PM 46-55 06:30 PM 1-10 06:45 PM 1-10 07:00 PM 41-50 07:15 PM 1-10 07:30 PM 1-10 07:45 PM 1-10 08:00 PM O O 1-10 08:15 PM 1-10 08:30 PM 1-10 08:45 PM 1-10 09:00 PM 1-10 09:15 PM O 1-10 09:30 PM 1-10 09:45 PM 1-10 10:00 PM 1-10 10:15 PM 1-10 10:30 PM O 1-10 10:45 PM 1-10 11:00 PM 1-10 11:15 PM 1-10 11:30 PM 26-35 11:45 PM 1-10 O **Day Total** 51-60 0% 0% 0% 0% 4% 4% 8% 8% 40% 16% 20% 0% 0% 0% Percent AM Peak 12:00 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 1:15 AM 11:45 AM 12:00 AM 7:45 AM 10:15 AM 10:30 AM 12:00 AM 12:00 AM 12:00 AM 11:45 AM 15-min Vol PM Peak 12:00 PM 12:00 PM 12:00 PM 12:00 PM 11:30 PM 12:00 PM 1:45 PM 4:45 PM 12:00 PM 1:00 PM 12:15 PM 12:00 PM 12:00 PM 12:00 PM 1:00 PM 15-min Vol Comments:

LOCATION: Bo	fer Cany	on Rd nor	th of Coff	in Rd												QC JOB	#: 16236663
SPECIFIC LOCA	TION:															DII	RECTION: NB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Kange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	TOLAI	Pace Speed	Pace
Grand Total	0	0	0	0	1	1	2	2	10	4	5	0	0	0	25	51-60	14
Percent	0%	0%	0%	0%	4%	4%	8%	8%	40%	16%	20%	0%	0%	0%	25	31-60	14
Cumulative Percent	0%	0%	0%	0%	4%	8%	16%	24%	64%	80%	100%	100%	100%	100%			
ADT 25															Me	an Speed(Avera Med	ntile: 61 MPH age): 53 MPH dian: 53 MPH ode: 53 MPH
Comments:													-				

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236663 DIRECTION: NB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent				DATA		ALD	RIVE	SCC	IVIVI	UIVII	IES.				
ADT															
25															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: NB

CITY/STATE: Be	I WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 AxI	Not	un 13 2023
Start Time	Bikes	Cars & Trailers		Buses	Z Axie 6 Tire	3 Axie Single		<5 Axi Double	5 Axie Double	>6 AXI Double	<6 AXI Multi	ь Axie Multi	>6 AXI Multi	Classed	Total
			Long				Single								
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
Day Total															
Percent				DAI	1 11/	ALDI	RIVE	200	IIVIIVI	UIVII	15				
ADT															
25															
25															
ANA De als															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
												-			

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: NB

Start Time   Bike   Cars & 2 Avia   Buse   Cars & 1 Avia   Cars & 1 Avia   Salve   Sal	CITY/STATE: Be	IIIOII, WA		241		241.5	2.4.1	4 4 1					6 4 1			un 13 2023
Trailers   Long   Irae   Single   Single   Double   Double   Double   Double   Double   Multi   Multi   Multi   Classed	Start Time	Bikes			Buses											Total
12:15 PM																
12:36 PM																
12:45 PM				0	0				0	0	0			0		
01:0PM										0				0		
01:15 PM				-	_				-	0				•		_
01:30 PM				0	0				0	0	0			0	0	
01-15 PM		0	0	0	0	0	0		0	0	0	0	0	0	0	0
O2:05 PM		0	0	0	0	0			0	0	0	0	0	0	0	0
02:15 PM		0	1	1	0	0				0	0		0	0	0	2
02:45 PM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			_	0	0					0	-			0	-	0
02:45 PM			0	1	0	0				0	0			0	0	
03:00 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
O3:15 PM				0	0				0	0	0			0	0	0
03:30 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM			0	0	1					0	0		0	0	0	1
04:00 PM		0	1	0	0	0				0	0	-	0	0	0	1
04:15 PM		0	0	0	0	0			0	0	0	0	0	0	0	0
04:30 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM			<del>-</del>	1	0	-				0	0	-	-	0	0	2
05:15 PM		0	0	1	0				0	0	0		0	0	0	1
O5:30 PM				0	0				0	0				0	0	1
Os:45 PM			0	1										0	0	
Day Total Percent  ADT 25  AM Peak 15-min Vol PM Peak 15-min Vol PM Peak 15-min Vol																
Percent  ADT 25  AM Peak 15-min Vol  PM Peak 15-min Vol		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 25  AM Peak 15-min Vol  PM Peak 15-min Vol																1
AM Peak 15-min Vol PM Peak 15-min Vol	Percent				DAIA	AIHA		RIVE	500	MM	UNII	11-5				<u>                                       </u>
AM Peak 15-min Vol PM Peak 15-min Vol																
AM Peak 15-min Vol  PM Peak 15-min Vol																
15-min Vol PM Peak 15-min Vol	25															
15-min Vol PM Peak 15-min Vol																
PM Peak 15-min Vol																
15-min Vol																
omments:																
	Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: NB

	enton, WA	C 0	241		241.6	2.4.1.	4.4.1.	-F A '			.c a !	C A I			un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	12	8	2	3	0	0	0	0	0	0	0	0	0	25
Percent	0%	48%	32%	8%	12%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25
ADT 25															
AM Peak	12:00 AM	1:15 AM	10:30 AM	7:45 AM	10:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	11:45 AM
15-min Vol	0	1	1	1	1	0	0	0	0	0	0	0	0	0	2
PM Peak	12:00 PM	12:00 PM	6:15 PM	3:15 PM	1:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	1:00 PM
15-min Vol	0	1	2	1	1	0	0	0	0	0	0	0	0	0	2
Comments:															

LOCATION: Bofer Canyon Rd north of Coffin Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

Start Time

Bikes

Cars & 2 Axle

Buses

Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl 6 Axle >6 Axl Not

Total

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	12 48%	8 32%	2 8%	3 12%	0 0%	0 0%	0 0%	0 0%	0 0%	0 0%	0 0%	0 0%	0 0%	25
	0,0	1070	02,0	<u> </u>	12,0	• • • • • • • • • • • • • • • • • • • •	• • • • • • • • • • • • • • • • • • • •		• • • • • • • • • • • • • • • • • • • •	0,0	0,0	0,0	• • • • • • • • • • • • • • • • • • • •	0,0	
ADT 25															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM	0				0			0	
12:15 AM	0				0			0	
12:30 AM	0				0			0	
12:45 AM	0				0			0	
01:00 AM	0				0			0	
01:15 AM	1				1			1	
01:30 AM	0				0			0	
01:45 AM	0				0			0	
02:00 AM	0				0			0	
02:15 AM	0				0			0	
02:30 AM	0				0			0	
02:45 AM	0				0			0	
03:00 AM	0				0			0	
03:15 AM	0				0			0	
03:30 AM	0				0			0	
03:45 AM	0				0			0	
04:00 AM	0				0			0	
04:15 AM	0				0			0	
04:30 AM	0				0			0	
04:45 AM	0				0			0	
05:00 AM	0				0			0	
05:15 AM	0				DRIVOS CO	00 40 4		0	
05:30 AM	0					JIVIIVI		0	
05:45 AM	0				0			0	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak									
15-min Vol									
PM Peak									
15-min Vol									
Comments:									

QC JOB #: 16236663

**DIRECTION: NB** 

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236663

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		0				0			0	<u> </u>
06:00 AW 06:15 AM		0				0			0	
06:30 AM		0				0			0	
06:30 AW 06:45 AM		0				0			0	
07:00 AM		0				0			0	
07:00 AM 07:15 AM		0				0			0	
07:30 AM		0				0			0	
07:30 AM 07:45 AM		1				1			1	
07.43 AM 08:00 AM		0				0			0	
08:15 AM		0				0			0	
08:30 AM		0				0			0	
08:45 AM		0				0			0	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		0				0			0	
09:45 AM		0				0			0	
10:00 AM		0				0			0	
10:15 AM		1				1			1	
10:30 AM		1			311	1		IIO.	1	
10:45 AM		0			261	0	$\cdot \cup \iota$		0	
11:00 AM		0				0			0	
11:15 AM		0				0			0	
11:30 AM		1			HALL	DRIVIES CO	DIVIN	UNII	/E5 1	
11:45 AM		2				2			2	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236663

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		1				1			1	
12:15 PM		1				1			1	
12:30 PM		0				0			0	
12:45 PM		0				0			0	
01:00 PM		2				2			2	
01:15 PM		0				0			0	
01:30 PM		0				0			0	
01:45 PM		2				2			2	
02:00 PM		0				0			0	
02:15 PM		1				1			1	
02:30 PM		0				0			0	
02:45 PM		0				0			0	
03:00 PM		0				0			0	
03:15 PM		1				1			1	
03:30 PM		1				1			1	
03:45 PM		0				0			0	
04:00 PM		0				0			0	
04:15 PM		0				0			0	
04:30 PM		2				2		In.	2	
04:45 PM		1				<b>1 1 1</b>	$\sim$	411	1	
05:00 PM		1				1			1	
05:15 PM		1				DRIVES CO			1	
05:30 PM		0					DIVIN	UNII	0	
05:45 PM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

**DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236663

**DIRECTION: NB** 

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
Start Time		13 Jun 23				15-min Traffic			15-min Traffic	Average week i follie
06:00 PM		0				0			0	
06:15 PM		2				2			2	
06:30 PM		0				0			0	
06:45 PM		0				0			0	
07:00 PM		1				1			1	
07:15 PM		0				0			0	
07:30 PM		0				0			0	
07:45 PM		0				0			0	
08:00 PM		0				0			0	
08:15 PM		0				0			0	
08:30 PM		0				0			0	
08:45 PM		0				0			0	
09:00 PM		0				0			0	
09:15 PM		0				0			0	
09:30 PM		0				0			0	
09:45 PM		0				0			0	
10:00 PM		0				0			0	
10:15 PM		0				0			0	
10:30 PM		0				0			0	
10:45 PM		0				0			0	
11:00 PM		0				0			0	
11:15 PM		0					0.000		0	
11:30 PM		1			HALL	DRIVES CO	DIVIIVI		[E5 1	
11:45 PM		0				0			0	
Day Total		25				25			25	
% Weekday		1000/								
Average		100%								
% Week		1000/				1000/				
Average		100%				100%				
AM Peak		11:45 AM				11:45 AM			11:45 AM	
15-min Vol		2				2			2	
PM Peak		1:00 PM				1:00 PM			1:00 PM	
15-min Vol		2				2			2	
omments:										-

LOCATION: B	ATION:		orth of (	Coffin Rd												QC JOB #: 1	N: NB, SB
CITY/STATE:	Benton, 1	WA <b>16</b>	21	26	31	36	41	46	51	56	61	66	71	76		DATE: Jur	Number
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 AM Day Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Percent														-5			
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

LOCATION: E SPECIFIC LOC	ATION:		orth of (	Coffin Rd												QC JOB #: 1 DIRECTIO	N: NB, SB
CITY/STATE:	Benton,	WA														DATE: Jur	13 2023
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numbe in Pace
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
11:30 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
11:45 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	2	36-45	1
Day Total Percent														=5			
AM Peak 15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

CITY/STATE:	Benton,															DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12:00 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
01:00 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	2	51-60	2
01:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 PM	0	0	0	0	0	0	1	0	1	0	1	0	0	0	3	36-45	1
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
03:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
03:43 PM 04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		0
04:15 PM					•	•		_			•	•				1-10	
04:30 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
04:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
05:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total Percent														=5			
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	

LOCATION: Bofer Canyon Rd north of Coffin Rd QC JOB #: 16236663 **SPECIFIC LOCATION: DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number Start Time Total Pace Speed in Pace 06:00 PM 1-10 06:15 PM 46-55 06:30 PM 1-10 06:45 PM 46-55 07:00 PM 41-50 07:15 PM 1-10 07:30 PM 1-10 07:45 PM 1-10 08:00 PM O O 1-10 08:15 PM 36-45 08:30 PM 1-10 08:45 PM 1-10 09:00 PM 1-10 09:15 PM O 1-10 09:30 PM 1-10 09:45 PM 1-10 10:00 PM 1-10 10:15 PM 1-10 10:30 PM O 1-10 10:45 PM 1-10 11:00 PM 1-10 11:15 PM 1-10 11:30 PM 26-35 11:45 PM 1-10 O **Day Total** 51-60 0% 0% 0% 0% 2.6% 7.7% 7.7% 5.1% 35.9% 15.4% 25.6% 0% 0% 0% Percent AM Peak 12:00 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 1:15 AM 11:45 AM 12:00 AM 6:15 AM 10:15 AM 4:45 AM 12:00 AM 12:00 AM 12:00 AM 11:45 AM 15-min Vol PM Peak 12:00 PM 12:00 PM 12:00 PM 12:00 PM 11:30 PM 12:00 PM 1:45 PM 4:45 PM 6:45 PM 12:45 PM 12:00 PM 12:00 PM 12:00 PM 12:00 PM 1:45 PM 15-min Vol Comments:

LOCATION: Bo	fer Cany	on Rd nor	th of Coff	in Rd												QC JOB	#: 16236663
SPECIFIC LOCA	TION:															DIRECT	Γ <mark>ΙΟΝ</mark> : NB, SB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Kange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	Pace
Grand Total	0	0	0	0	1	3	3	2	14	6	10	0	0	0	39	51-60	20
Percent	0%	0%	0%	0%	2.6%	7.7%	7.7%	5.1%	35.9%	15.4%	25.6%	0%	0%	0%	39	31-60	20
Cumulative Percent	0%	0%	0%	0%	2.6%	10.3%	17.9%	23.1%	59%	74.4%	100%	100%	100%	100%			
ADT 39															Me	an Speed(Avera	ntile: 62 MPH nge): 53 MPH lian: 53 MPH ode: 53 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236663 DIRECTION: NB, SB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	TOLAI
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent				$)\Delta I \lambda$	ATH		RIVF	SCC	MM	UMH	11-5				
ADT															
39															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
Panart ganaratad	100 6/20/20	122 7.EU VVI									COLIDCE	iality Count	c 11 C /h++n,	//www.auali	tucquints not

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236663 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start rille	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
Day Total															
Percent				DAIL	AIHA		RIVE	266	IVIIVI	UIVII	15				
ADT															
39															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
COMMENTS.															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236663 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	enton, WA													DATE: J	un 13 2023
Chart Tire	Dilege	Cars &	2 Axle	D.co.	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
12:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
01:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
01:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
03:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
04:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
05:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent				DATA	ATHA		RIVF	5 ( (	IMM	UNIT	1-5				
ADT															
39															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: NB, SB

06:15 PM	Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06-35 PM	06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
G6-55 PM	06:15 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
07:00 PM	06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 PM	06:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
07:30 PM	07:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45 PM	07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:05 PM	07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 PM	07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	08:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
O9:05 PM	08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM         0<	11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total Percent         0         18         13         4         4         0	11:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Percent         0%         46.2%         33.3%         10.3%         10.3%         0% <td>11:45 PM</td> <td>0</td>	11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
ADT 39  AM Peak 12:00 AM 1:15 AM 4:45 AM 6:15 AM 10:15 AM 12:00 AM	Day Total	0	18	13	4	4	0	0	0	0	0	0	0	0	0	20
AM Peak 12:00 AM 1:15 AM 4:45 AM 6:15 AM 10:15 AM 12:00 A	Percent	0%	46.2%	33.3%	10.3%	10.3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	39
15-min Vol 0 1 1 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0																
PM Peak 12:00 PM 12:00 PM 1:45 PM 3:15 PM 12:45 PM 12:00	AM Peak	12:00 AM	1:15 AM	4:45 AM	6:15 AM	10:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	11:45 AN
	L5-min Vol	0	1	1	1	1	0	0	0	0	0	0	0	0	0	2
15-min Vol 0 2 2 1 1 0 0 0 0 0 0 0 0 0		12:00 PM	12:00 PM	1:45 PM	3:15 PM	12:45 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	1:45 PN
	15-min Vol	0	2	2	1	1	0	0	0	0	0	0	0	0	0	3

LOCATION: Bofer Canyon Rd north of Coffin Rd QC JOB #: 16236663 SPECIFIC LOCATION: **DIRECTION: NB, SB** CITY/STATE: Benton, WA **DATE:** Jun 13 2023 Cars & 2 Axle 2 Axle 6 4 Axle <5 Axl 5 Axle >6 Axl 3 Axle <6 Axl 6 Axle >6 Axl Not Start Time **Bikes Buses** Total **Trailers** Long Tire Single Single Double Double Double Multi Multi Multi Classed **Grand Total** 0 18 13 4 4 0 0 0 0 0 0 0 0 0 39 46.2% 10.3% 0% 0% 0% 0% 0% 0% 0% Percent 0% 33.3% 10.3% 0% 0% ADT 39

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: NB, SB DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		1				1			1	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		0				0			0	
04:15 AM		0				0			0	
04:30 AM		0				0			0	
04:45 AM		1				1			1	
05:00 AM		0				0			0	
05:15 AM		0				0 0 0	00 40 4		0	
05:30 AM		0				0	JIVIIVI		0	
05:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION: CITY/STATE: Benton, WA DIRECTION: NB, SB

DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
Start Time		13 Jun 23				15-min Traffic			15-min Traffic	Average vveek i follie
06:00 AM		0				0			0	
06:15 AM		1				1			1	
06:30 AM		0				0			0	
06:45 AM		0				0			0	
07:00 AM		0				0			0	
07:15 AM		0				0			0	
07:30 AM		0				0			0	
07:45 AM		1				1			1	
08:00 AM		0				0			0	
08:15 AM		1				1			1	
08:30 AM		0				0			0	
08:45 AM		0				0			0	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		1				1			1	
09:45 AM		0			-	0			0	
10:00 AM		0				0			0	
10:15 AM		1				1	-		1	
10:30 AM		1			21 1	1			1	
10:45 AM		0				0			0	
11:00 AM		0				0			0	
11:15 AM		1			TIATI	DRIVES CO	00 40 4		1	
11:30 AM		1			HALL		JIVIIVI		IES 1	
11:45 AM		2				2			2	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

QC JOB #: 16236663

SPECIFIC LOCATION: CITY/STATE: Benton, WA

**DIRECTION:** NB, SB **DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236663

12:00 PM 12:15 PM	13 Jun 23		Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:15 PM	2			2			2	
	1			1			1	
12:30 PM	0			0			0	
12:45 PM	2			2			2	
01:00 PM	2			2			2	
01:15 PM	1			1			1	
01:30 PM	0			0			0	
01:45 PM	3			3			3	
02:00 PM	0			0			0	
02:15 PM	1			1			1	
02:30 PM	0			0			0	
02:45 PM	0			0			0	
03:00 PM	0			0			0	
03:15 PM	1			1			1	
03:30 PM	1			1			1	
03:45 PM	1			1			1	
04:00 PM	0			0			0	
04:15 PM	0			0			0	
04:30 PM	2			2		In'	2	
04:45 PM	1			1		41 I	1	
05:00 PM	1			1			1	
05:15 PM	1				00000	T 18 117	1	
05:30 PM	0			0	JIVIIVI	UIVII	0	
05:45 PM	0			0			0	
Day Total								
% Weekday								
Average								
% Week								
Average								
AM Peak								
15-min Vol								
PM Peak								
15-min Vol								

CITY/STATE: Benton, WA

QC JOB #: 16236663 **DIRECTION: NB, SB** SPECIFIC LOCATION: **DATE:** Jun 13 2023 - Jun 13 2023

Thu Fri Mon Tue Wed Average Weekday Sat Sun Average Week **Average Week Profile Start Time** 15-min Traffic 13 Jun 23 15-min Traffic 06:00 PM 0 0 0 06:15 PM 2 2 2 06:30 PM 0 0 0 06:45 PM 2 2 2 07:00 PM 1 1 0 0 07:15 PM 07:30 PM 0 0 0 07:45 PM 0 08:00 PM 0 0 08:15 PM 1 1 08:30 PM 0 0 0 08:45 PM 0 0 0 09:00 PM 0 0 09:15 PM 0 0 0 09:30 PM 0 0 0 09:45 PM 0 0 0 10:00 PM 0 0 10:15 PM 0 10:30 PM 0 10:45 PM 0 11:00 PM 0 11:15 PM 0 11:30 PM 1 11:45 PM 0 Day Total 39 39 39 % Weekday 100% Average % Week 100% 100% Average 11:45 AM 11:45 AM 11:45 AM AM Peak 15-min Vol 2 2 2 1:45 PM 1:45 PM 1:45 PM PM Peak 15-min Vol 3 3 3 Comments:

LOCATION: E	ATION:		orth of (	Coffin Rd													TION: SB
CITY/STATE:			21	26	31	36	41	46	51	56	61	66	71	76	<u> </u>	DATE: Jur	
Start Time	1 15	16 20	25	30	35	40	41 45	50	55	60	65	70	71 75	999	Total	Pace Speed	Number in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total				DA	TA	TU	$\Lambda TI$	NDA	/EC	0	A // A	AL IN	IITII				
Percent				11	MA	$I \square I$	AII	JKI			IVIIV	IUIN					
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

QC JOB #: 16236663 LOCATION: Bofer Canyon Rd north of Coffin Rd SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 1-10 06:15 AM 46-55 06:30 AM 1-10 06:45 AM 1-10 07:00 AM 1-10 07:15 AM 1-10 07:30 AM 1-10 07:45 AM 1-10 08:00 AM O O 1-10 08:15 AM 56-65 08:30 AM 1-10 08:45 AM 1-10 09:00 AM 1-10 09:15 AM 1-10 09:30 AM 31-40 09:45 AM 1-10 10:00 AM 1-10 10:15 AM 1-10 10:30 AM O 1-10 10:45 AM 1-10 11:00 AM 1-10 11:15 AM 31-40 11:30 AM 1-10 11:45 AM 1-10 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: B SPECIFIC LOC	ATION:		north of	Coffin Rd													TION: SB
CITY/STATE:																DATE: Jun	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
12:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total Percent														=5			
AM Peak																	
15-min Vol PM Peak																	
15-min Vol																	
Comments:		0/2023 7:5														tn://www.guality	

 LOCATION: Bofer Canyon Rd north of Coffin Rd

 QC JOB #: 16236663

 SPECIFIC LOCATION:

 CITY/STATE: Benton, WA
 DATE: Jun 13 2023

 Start Time
 1
 16
 21
 26
 31
 36
 41
 46
 51
 56
 61
 66
 71
 76
 Total
 Pace Speed
 Number

Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numb
otait iiiie	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 ace Speed	in Pac
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2	46-55	2
07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	0	0	0	0	2	_1	0	4	2	5	0	0	0	14	56-65	7
Percent	0%	0%	0%	0%	0%	14.3%	7.1%	0%	28.6%	14.3%	35.7%	0%	0%	0%	14	30-03	,
AM Peak	12:00 AM		12:00 AM	12:00 AM		9:30 AM	12:00 AM	12:00 AM	6:15 AM	12:00 AM	4:45 AM	12:00 AM			4:45 AM		
15-min Vol	0	0	0	0	0	1	0	0	1	0	1	0	0	0	1		
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	8:15 PM 1	12:00 PM 0	6:45 PM 2	12:45 PM 1	12:00 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:45 PM 2		

LOCATION: Bot	fer Canyo	on Rd nor	th of Coff	fin Rd												QC JOB	#: 16236663
SPECIFIC LOCA	TION:															DI	RECTION: SB
CITY/STATE: Be	enton, W	<b>/</b> A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Kange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	TOTAL	Pace Speed	Pace
Grand Total	0	0	0	0	0	2	1	0	4	2	5	0	0	0	14	FC CF	7
Percent	0%	0%	0%	0%	0%	14.3%	7.1%	0%	28.6%	14.3%	35.7%	0%	0%	0%	14	56-65	/
Cumulative Percent	0%	0%	0%	0%	0%	14.3%	21.4%	21.4%	50%	64.3%	100%	100%	100%	100%			
ADT 14															Me	an Speed(Avera Med	ntile: 62 MPH nge): 56 MPH nge): 56 MPH nde: 63 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236663 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 202
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	О	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent				DATA	ATHA	ATD	RIVE	SCC	MM	UNIT	IES				
ADT 14															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	C /20 /20	2227 50 484									COLUBEE O		116/11	//!:	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: SB

,017.112. 50	nton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent				DATA	ALHA	ALD	RIVE	SCC	IVIIVI	UNH	11-5				
ADT 14															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: SB

CITY/STATE: Be	nton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 PM	0		0	0	0	0	0		0	0		0			1
12:00 PM 12:15 PM	0	<b>1</b> 0	0	0	0	0	0	0 0	0	0	0 0	0	0 0	0 0	1 0
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent															
ADT 14															
AM Peak 15-min Vol PM Peak															
15-min Vol															
omments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236663 DIRECTION: SB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	6	5	2	1	0	0	0	0	0	0	0	0	0	4.4
Percent	0%	42.9%	35.7%	14.3%	7.1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	14
ADT 14															
AM Peak	12:00 AM	8:15 AM	4:45 AM	6:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	4:45 AN
15-min Vol	0	1	1	1	0	0	0	0	0	0	0	0	0	0	1
PM Peak		12:00 PM	12:45 PM	3:45 PM	12:45 PM	12:00 PM			12:00 PM				12:00 PM	12:00 PM	12:45 PN
15-min Vol	0	1	1	1	1	0	0	0	0	0	0	0	0	0	2

LOCATION: Bofer Canyon Rd north of Coffin Rd QC JOB #: 16236663 SPECIFIC LOCATION: **DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Cars & 2 Axle 2 Axle 6 4 Axle <5 Axl 5 Axle >6 Axl 3 Axle <6 Axl 6 Axle >6 Axl Not Start Time **Bikes Buses** Total **Trailers** Long Tire Single Single Double Double Double Multi Multi Multi Classed **Grand Total** 0 6 5 2 1 0 0 0 0 0 0 0 0 0 14 42.9% 0% 0% 0% 0% 0% 0% 0% Percent 0% 35.7% 14.3% 7.1% 0% 0% ADT 14

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA

DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		0				0			0	
04:15 AM		0				0			0	
04:30 AM		0				0			0	
04:45 AM		1				L V 1			1	
05:00 AM		0				0			0	
05:15 AM		0					00000		0	
05:30 AM		0				0	DIVIIVI		/E5 0	
05:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

QC JOB #: 16236663

**DIRECTION: SB** 

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236663 DIRECTION: SB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		0				0			0	
06:15 AM		1				1			1	
06:30 AM		0				0			0	
06:45 AM		0				0			0	
07:00 AM		0				0			0	
07:15 AM		0				0			0	
07:30 AM		0				0			0	
07:45 AM		0				0			0	
08:00 AM		0				0			0	
08:15 AM		1				1			1	
08:30 AM		0				0			0	
08:45 AM		0				0			0	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		1				1			1	
09:45 AM		0				0			0	
10:00 AM		0				0			0	
10:15 AM		0				0			0	
10:30 AM		0				0			0	
10:45 AM		0				0			0	
11:00 AM		0				0			0	
11:15 AM		1				1	0000		1	
11:30 AM		0					DIVIN		(E) 0	
11:45 AM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236663

DIRECTION: SB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		1				1			1	
12:15 PM		0				0			0	
12:30 PM		0				0			0	
12:45 PM		2				2			2	
01:00 PM		0				0			0	
01:15 PM		1				1			1	
01:30 PM		0				0			0	
01:45 PM		1				1			1	
02:00 PM		0				0			0	
02:15 PM		0				0			0	
02:30 PM		0				0			0	
02:45 PM		0				0			0	
03:00 PM		0				0			0	
03:15 PM		0				0			0	
03:30 PM		0				0			0	
03:45 PM		1				1			1	
04:00 PM		0				0			0	
04:15 PM		0			_ 0	0			0	
04:30 PM		0				0			0	
04:45 PM		0			261	0			0	
05:00 PM		0				0			0	
05:15 PM		0			7 1 A T 7		00 40 4		0	
05:30 PM		0			HALL		DIVIN		0	
05:45 PM		0				0			0	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue	Wed Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
	13 Jun 23			15-min Traffic			15-min Traffic	_
06:00 PM	0			0			0	
06:15 PM	0			0			0	
06:30 PM	0			0			0	
06:45 PM	2			2			2	
07:00 PM	0			0			0	
07:15 PM	0			0			0	
07:30 PM	0			0			0	
07:45 PM	0			0			0	
08:00 PM	0			0			0	
08:15 PM	1			1			1	
08:30 PM	0			0			0	
08:45 PM	0			0			0	
09:00 PM	0			0			0	
09:15 PM	0			0			0	
09:30 PM	0			0			0	
09:45 PM	0			0			0	
10:00 PM	0			0			0	
10:15 PM	0			0	-		0	
10:30 PM	0			0			0	
10:45 PM	0			0			0	
11:00 PM	0			0			0	
11:15 PM	0			DRIVOES CO	3 A A A A A A A A A A A A A A A A A A A		0	
11:30 PM	0			0	JIVIIVIL		0	
11:45 PM	0			0			0	
Day Total	14			14			14	
% Weekday Average	100%							
% Week Average	100%			100%				
AM Peak	4:45 AM			4:45 AM			4:45 AM	
15-min Vol	1			1			1	
PM Peak	12:45 PM			12:45 PM			12:45 PM	
15-min Vol	2			2			2	
Comments:								

QC JOB #: 16236663

**DIRECTION: SB** 

SPECIFIC LO	CATION															QC JOB #: 1	TION: EI
CITY/STATE:		۱۸/۸														DATE: Jur	
JIII/SIAIL.	1	16	21	26	31	36	41	46	51	56	61	66	71	76		DATE. Jul	Numbe
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pac
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:45 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	21-30	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
05:15 AM	0	0	0	0	2	2	0	1	0	0	0	0	0	0	5	31-40	4
05:30 AM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	51-60	2
05:45 AM	0	0	0	0	0	0	0	0	2	1	1	0	0	0	4	51-60	3
Day Total Percent																	
Percent					MA	111/	7//		/ [ ]	LU	IVIIV	TUN					
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	
Comments:																	

QC JOB #: 16236664 LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 SPECIFIC LOCATION: **DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 56-65 06:15 AM 06:30 AM 51-60 06:45 AM 36-45 07:00 AM 56-65 07:15 AM 46-55 07:30 AM 56-65 07:45 AM 61-70 08:00 AM O 46-55 08:15 AM 56-65 08:30 AM 26-35 08:45 AM 46-55 09:00 AM 51-60 09:15 AM O 46-55 09:30 AM 26-35 09:45 AM 46-55 10:00 AM 51-60 10:15 AM 56-65 10:30 AM O 1-10 10:45 AM 46-55 11:00 AM 41-50 11:15 AM 51-60 11:30 AM 48-57 11:45 AM 46-55 Day Total Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236664 LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 SPECIFIC LOCATION: **DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 56-65 12:30 PM 1-10 12:45 PM 58-67 01:00 PM 51-60 01:15 PM 46-55 01:30 PM 56-65 01:45 PM 51-60 02:00 PM O 56-65 02:15 PM 51-60 02:30 PM 46-55 02:45 PM 51-60 03:00 PM 46-55 03:15 PM O 55-64 03:30 PM 41-50 03:45 PM 51-60 04:00 PM 51-60 04:15 PM 31-40 04:30 PM 41-50 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 60-69 05:30 PM 56-65 05:45 PM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION:

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
06:00 PM	0	0	0	0	0	0	0	0	3	5	3	3	1	0	15	51-60	8
06:15 PM	0	0	0	0	0	0	0	0	1	0	2	0	0	0	3	56-65	2
06:30 PM	0	0	0	0	0	0	0	0	2	4	3	1	1	0	11	56-65	7
06:45 PM	0	0	0	0	0	0	0	0	2	3	2	1	1	0	9	53-62	5
07:00 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	2	51-60	1
07:15 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
07:30 PM	0	0	0	0	1	1	1	0	2	2	1	2	0	0	10	51-60	4
07:45 PM	0	0	0	1	0	1	0	0	0	0	0	0	0	0	2	21-30	1
08:00 PM	0	0	0	0	0	0	0	0	0	3	1	2	1	1	8	56-65	4
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	66-75	2
08:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	71-80	0
09:15 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
09:30 PM	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	41-50	3
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:45 PM	0	0	0	0	0	0	0	2	3	0	0	0	0	0	5	46-55	5
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
11:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
Day Total	0	1	1	7	15	7	9	37	68	95	59	30	15	5			
Percent	0%	0.3%	0.3%	2%	4.3%	2%	2.6%	10.6%	19.5%	27.2%	16.9%	8.6%	4.3%	1.4%	349	51-60	163
AM Peak 15-min Vol	12:00 AM 0	12:00 AM 0	12:00 AM 0	4:45 AM 1	5:15 AM 2	5:15 AM 2	6:45 AM 1	10:45 AM 3	9:45 AM 3	5:30 AM 2	8:15 AM 3	7:30 AM 1	6:00 AM 1	8:45 AM 1	10:45 AM 10		
PM Peak 15-min Vol	12:00 PM 0	4:30 PM 1	4:30 PM 1	2:15 PM 1	4:15 PM 2	4:15 PM 1	3:30 PM 3	2:30 PM 3	4:45 PM 6	2:00 PM 5	5:15 PM 4	5:30 PM 4	8:15 PM 2	4:00 PM 1	4:45 PM 15		

SPECIFIC LOCATION: CITY/STATE: Benton, WA  Speed Range  1 16 21 26 31 36 41 46 51 56 61 66 71 76 999 Total 15 20 25 30 35 40 45 50 55 60 65 70 75 999 Total Percent  Owner in the percent  ADT 349  SPECIFIC LOCATION: CITY/STATE: Benton, WA  Speed Range  1 16 21 26 31 36 41 46 51 56 61 66 71 76 999 Total Pace Speed Number in Pace Pace Pace  Number in Pace Speed Number in Pace Pace Number in Pace Number in Pace Number in Pace Number in Pace Number in Pace Number in Pace Number in Numbe	LOCATION: Loc	cust Grov	ve Rd btw	n Nicosin	Rd and I-	82											QC JOB	#: 16236664
Speed Range   1   16   21   26   31   36   41   46   51   56   61   66   71   76   75   999   Total   Pace Speed   Number in Pace   Speed   Pace   Speed   S	SPECIFIC LOCA	TION:															DI	RECTION: EB
Speed Range   15   20   25   30   35   40   45   50   55   60   65   70   75   999   Total   Pace Speed   Pace	CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Grand Total Percent	Spood Pango	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Dago Spood	Number in
Percent         0%         0.3%         0.3%         2%         4.3%         2%         2.6%         10.6%         19.5%         27.2%         16.9%         8.6%         4.3%         1.4%         349         51-60         163           Cumulative Percent         0%         0.3%         0.6%         2.6%         6.9%         8.9%         11.5%         22.1%         41.5%         68.8%         85.7%         94.3%         98.6%         100%         Sth Percentile: 64 MP Mean Speed(Average): 56 MP Mean Speed(Average): 56 MP Mean Speed(Average): 56 MP Median:	Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	Pace
Percent	Grand Total	0	1	1	7	15	7	9	37	68	95	59	30	15	5	240	E1 60	162
Percent 0% 0.3% 0.6% 2.6% 6.9% 8.9% 11.5% 22.1% 41.5% 68.8% 85.7% 94.3% 98.6% 100% 85th Percentile: 64 MP Mean Speed(Average): 56 MP Median: 56 MP	Percent	0%	0.3%	0.3%	2%	4.3%	2%	2.6%	10.6%	19.5%	27.2%	16.9%	8.6%	4.3%	1.4%	349	31-00	103
ADT 349  Mean Speed(Average): 56 MP Median: 56 MP		0%	0.3%	0.6%	2.6%	6.9%	8.9%	11.5%	22.1%	41.5%	68.8%	85.7%	94.3%	98.6%	100%			
															_	Me	an Speed(Avera	age): 56 MPH dian: 56 MPH

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: EB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
05:15 AM	0	3	2	0	0	0	0	0	0	0	0	0	0	0	5
05:30 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
05:45 AM	0	0	2	0	2	0	0	0	0	0	0	0	0	0	4
Day Total															
Percent				DAIA	A I H	AD	RIVE	SCC	IMM	UNL	15				
ADT															
349															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
onart ganaratad	L = = C /20 /20	222 7.50 414									COLIDCE: O	l:4 C 4	- IIC/base.	//www.auali	

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: EB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
06:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
06:15 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
06:30 AM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
06:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:00 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
07:15 AM	0	1	0	0	1	0	0	0	1	0	0	0	0	0	3
07:30 AM	0	2	1	0	1	0	0	1	0	0	0	0	0	0	5
07:45 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
08:00 AM	0	2	0	0	2	0	0	0	1	0	0	0	0	0	5
08:15 AM	0	0	1	1	1	0	0	1	0	0	0	0	0	0	4
08:30 AM	0	1	1	0	2	0	0	0	0	0	0	0	0	0	4
08:45 AM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
09:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:15 AM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
09:30 AM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
09:45 AM	0	2	0	1	3	0	0	2	0	0	0	0	0	0	8
10:00 AM	0	1	0	0	3	0	0	0	0	0	0	0	0	0	4
10:15 AM	0	0	0	1	2	0	0	0	0	0	0	0	0	0	3
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	1	4	1	2	0	0	2	0	0	0	0	0	0	10
11:00 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
11:15 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
11:30 AM	0	1	4	0	0	0	0	0	0	0	0	0	0	0	5
11:45 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	2
Day Total															
Percent				DAIA	AIHA	ALD	RIVE	366	IIVIIVI	UNII	IES				
ADT															
349															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
Comments:	<u> </u>						<u> </u>								
onart ganaratad	l an 6/20/20	122 7.EO ANA									COLIDCE: O	iality Count	a IIC/b++n	//www.auali	tucquints r

SPECIFIC LOCATION:

QC JOB #: 16236664 DIRECTION: EB

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202 <b>Total</b>
	Billes	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	· Otal
12:00 PM	0	1	1	0	0	0	0	2	0	0	0	0	0	0	4
12:15 PM	0	1	2	1	0	0	0	0	0	0	0	0	0	0	4
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	1	0	1	4	0	0	0	0	0	0	0	0	0	6
01:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
01:15 PM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
01:30 PM	0	1	3	0	0	0	0	0	0	0	0	0	0	0	4
01:45 PM	0	0	1	1	2	0	0	1	0	0	0	0	0	0	5
02:00 PM	0	2	2	0	3	0	0	1	0	0	0	0	0	0	8
02:15 PM	0	0	1	1	3	0	0	0	0	0	0	0	0	0	5
02:30 PM	0	3	0	2	0	1	0	0	0	0	0	0	0	0	6
02:45 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
03:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:15 PM	0	1	1	1	2	0	0	1	0	0	0	0	0	0	6
03:30 PM	0	7	3	1	1	0	0	0	0	0	0	0	0	0	12
03:45 PM	0	2	3	1	4	0	0	3	0	0	0	0	0	0	13
04:00 PM	0	2	4	0	3	0	0	1	0	0	0	0	0	0	10
04:15 PM	0	4	2	1	2	0	0	0	0	0	0	0	0	0	9
04:30 PM	0	5	5	0	1	0	0	0	1	0	0	0	0	0	12
04:45 PM	0	9	2	1	2	0	0	0	1	0	0	0	0	0	15
05:00 PM	0	7	4	0	2	0	0	0	0	0	0	0	0	0	13
05:15 PM	0	6	2	1	2	0	0	0	0	0	0	0	0	0	11
05:30 PM	0	9	2	0	1	0	0	2	0	0	0	0	0	0	14
05:45 PM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
Day Total															
Percent				DAIA	4 1 1 1 1	AL LA	KIVE	3.66	<i>IIVIIVI</i>	ШИП	IES				
ADT 349															
343															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	L C/20/2/											11. 6 .		. / /	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: EB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	5	4	2	3	0	0	1	0	0	0	0	0	0	15
06:15 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
06:30 PM	0	5	3	1	2	0	0	0	0	0	0	0	0	0	11
06:45 PM	0	6	2	1	0	0	0	0	0	0	0	0	0	0	9
07:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
07:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
07:30 PM	0	8	0	0	1	0	0	1	0	0	0	0	0	0	10
07:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
08:00 PM	0	1	2	1	4	0	0	0	0	0	0	0	0	0	8
08:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
08:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:15 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
09:30 PM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	3
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	1	1	1	0	0	2	0	0	0	0	0	0	5
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
11:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
Day Total	0	137	83	24	78	1	0	22	4	0	0	0	0	0	349
Percent	0%	39.3%	23.8%	6.9%	22.3%	0.3%	0%	6.3%	1.1%	0%	0%	0%	0%	0%	343
ADT 349															
AM Peak	12:00 AM	6:15 AM	10:45 AM	8:15 AM	9:45 AM	12:00 AM	12:00 AM	9:45 AM	7:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	10:45 A
15-min Vol	0	4	4	1	3	0	0	2	1	0	0	0	0	0	10
PM Peak	12:00 PM	4:45 PM	4:30 PM	2:30 PM	12:45 PM	2:30 PM	12:00 PM	3:45 PM	4:30 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	4:45 PN
15-min Vol	0	9	5	2	4	1	0	3	1	0	0	0	0	0	15
omments:															

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

OC JOB #: 16236664

DIRECTION: EB

DATE: Jun 13 2023

CITT/STATE: BCI	itoli, wa													DAIL	Juli 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	0	137	83	24	78	1	0	22	4	0	0	0	0	0	240
Percent	0%	39.3%	23.8%	6.9%	22.3%	0.3%	0%	6.3%	1.1%	0%	0%	0%	0%	0%	349
ADT 349															
Comments:															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

QC JOB #: 16236664 **DIRECTION: EB** 

Start Time	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
		13 Jun 23				15-min Traffic			15-min Traffic	Average vacer i follie
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		1				1			1	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		0				0			0	
04:15 AM		0				0			0	
04:30 AM		0				0			0	
04:45 AM		1				1			1	
05:00 AM		1				1			1	
05:15 AM		5				5 2	00.00		5	
05:30 AM		2				2	DIVIIV		2	
05:45 AM		4				4			4	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

**SPECIFIC LOCATION:** 

QC JOB #: 16236664 **DIRECTION: EB** 

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023 Wed Thu Fri Average Weekday Average Week Mon Tue Sat Sun Average Week Profile **Start Time** 15-min Traffic 13 Jun 23 15-min Traffic 06:00 AM 2 2 2 06:15 AM 4 06:30 AM 4 06:45 AM 1 1 2 2 07:00 AM 3 3 07:15 AM 07:30 AM 5 5 07:45 AM 4 4 08:00 AM 5 5 08:15 AM 4 4 08:30 AM 4 08:45 AM 2 2 09:00 AM 1 1 3 09:15 AM 3 09:30 AM 09:45 AM 8 10:00 AM 10:15 AM 3 3 3 0 10:30 AM 0 10 10:45 AM 10 10 2 11:00 AM 2 11:15 AM 2 2 11:30 AM 5 11:45 AM 2 Day Total % Weekday Average % Week Average AM Peak 15-min Vol PM Peak 15-min Vol

Comments:

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DIRECTION: EB

DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236664

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		4				4			4	
12:15 PM		4				4			4	
12:30 PM		0				0			0	
12:45 PM		6				6			6	
01:00 PM		1				1			1	
01:15 PM		4				4			4	
01:30 PM		4				4			4	
01:45 PM		5				5			5	
02:00 PM		8				8			8	
02:15 PM		5				5			5	
02:30 PM		6				6			6	
02:45 PM		2				2			2	
03:00 PM		1				1			1	
03:15 PM		6				6			6	
03:30 PM		12				12			12	
03:45 PM		13				13			13	
04:00 PM		10				10			10	
04:15 PM		9				9			9	
04:30 PM		12				12		In.	12	
04:45 PM		15				15		411	15	
05:00 PM		13				13			13	
05:15 PM		11				11 14		LIKIT	11	
05:30 PM		14					JIVIIVI	UIVII	14	
05:45 PM		4				4			4	
Day Total										
% Weekday Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak 15-min Vol										
Comments:										

SPECIFIC LOC	ATION:	on na ana i	02						DIRECTION: EB
CITY/STATE:								DA	TE: Jun 13 2023 - Jun 13 2023
Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	15				15			15	
06:15 PM	3				3			3	
06:30 PM	11				11			11	
06:45 PM	9				9			9	
07:00 PM	2				2			2	
07:15 PM	1				1			1	
07:30 PM	10				10			10	
07:45 PM	2				2			2	
08:00 PM	8				8			8	
08:15 PM	2				2			2	
08:30 PM	1				1			1	
08:45 PM	1				1			1	
09:00 PM	1				1			1	
09:15 PM	2				2			2	
09:30 PM	3				3			3	
09:45 PM	0				0			0	
10:00 PM	0				0			0	
10:15 PM	1				1			1	
10:30 PM	1				1			1	
10:45 PM	5				5			5	
11:00 PM	0				0			0	
11:15 PM	0				0	00.000		0	
11:30 PM	3				3	DIVIN		3	
11:45 PM	1				1			1	
Day Total	349				349			349	
% Weekday Average	100%								
% Week Average	100%				100%				
AM Peak	10:45 AM				10:45 AM			10:45 AM	
15-min Vol	10				10			10	
PM Peak	4:45 PM				4:45 PM			4:45 PM	
15-min Vol	15				15			15	

Comments:

QC JOB #: 16236664

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 12:15 AM 1-10 12:30 AM 1-10 12:45 AM 46-55 01:00 AM 36-45 01:15 AM 46-55 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O O 1-10 02:15 AM 41-50 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 1-10 03:15 AM 1-10 03:30 AM 1-10 03:45 AM 1-10 04:00 AM 51-60 04:15 AM 46-55 04:30 AM O 51-60 04:45 AM 26-35 05:00 AM 51-60 05:15 AM 31-40 05:30 AM 51-60 05:45 AM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 56-65 06:15 AM 56-65 06:30 AM 51-60 06:45 AM 51-60 07:00 AM 56-65 07:15 AM 51-60 07:30 AM 56-65 07:45 AM 41-50 08:00 AM O 46-55 08:15 AM 56-65 08:30 AM 26-35 08:45 AM 36-45 09:00 AM 46-55 09:15 AM O 36-45 09:30 AM 51-60 09:45 AM 46-55 10:00 AM 51-60 10:15 AM 53-62 10:30 AM O 36-45 10:45 AM 46-55 11:00 AM 41-50 11:15 AM 51-60 11:30 AM 51-60 11:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

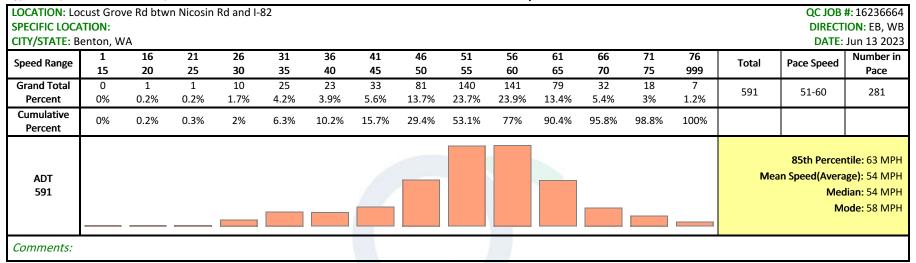
LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 51-60 46-55 12:30 PM 12:45 PM 59-68 01:00 PM 51-60 01:15 PM 46-55 01:30 PM 46-55 01:45 PM 51-60 02:00 PM O 56-65 02:15 PM 51-60 02:30 PM 46-55 02:45 PM 51-60 03:00 PM 46-55 03:15 PM O 54-63 03:30 PM 51-60 03:45 PM 51-60 04:00 PM 51-60 04:15 PM 31-40 04:30 PM 45-54 04:45 PM 51-60 05:00 PM 51-60 05:15 PM 60-69 05:30 PM 56-65 05:45 PM 51-60 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

QC JOB #: 16236664 **DIRECTION:** EB, WB

CITY/STATE:	Benton,	WA														DATE: Jun	13 2023
Start Time	1	16	21	26	31	36	41	46	51	56 60	61	66	71 75	76 999	Total	Pace Speed	Number
06.00.014	15	20	25	30	35	40	45	50	55	60	65	70	75		45	51.60	in Pace
06:00 PM	0	0	0	0	0	0	0	0	3	5	3	3	1	0	15	51-60	8
06:15 PM 06:30 PM	0	0	0	0 0	0 0	1 0	0	1 0	1	0	2	0	0	0	5 12	56-65	2
	0	0 0	0		-		0		2	5	3	1	1	0		56-65	8
06:45 PM	0	-	0	0	0	0	0	0	3	3	2	1	1	0	10	51-60	6
07:00 PM	0	0	0	0	0	0	0	1	0	1	0	1	1	0	4	66-75	2
07:15 PM	0	0	0	0	0	0	0	1	2	1	1	0	0	0	5	51-60	3
07:30 PM	0	0	0 0	0	1	1	1	2	6	2	2	2	0	0	17	48-57	8
07:45 PM	0	0 0	•	1 0	0	1	0	0	1	0	0	0	0	0	3	21-30	1
08:00 PM	0	-	0	-	•	0	0	0	1	3	2	2	1	2	11	56-65	5
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	66-75	2
08:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
08:45 PM	0	0	0	0 0	0	0	0	0	1	1	0	1	0	0	3	51-60	2
09:00 PM	0	0	0	_	•	0	0	1	0	0	0	0	0	1	2	41-50	1
09:15 PM	0	0	0	0	0	0	0	1	1	1	1	0	0	0	4	46-55	2
09:30 PM	0	0	0	0	0	0	0	4	0	1	0	0	0	0	5	41-50	4
09:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:45 PM	0	0	0	0	0	0	0	2	3	0	0	0	0	0	5	46-55	5
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:30 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
11:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
Day Total Percent	0 0%	1 0.2%	1 0.2%	10 1.7%	25 4.2%	23 3.9%	33 5.6%	81 13.7%	140 23.7%	141 23.9%	79 13.4%	32 5.4%	18 3%	7 1.2%	591	51-60	281
AM Peak 15-min Vol	12:00 AM 0	12:00 AM 0	12:00 AM 0	7:30 AM 2	5:15 AM 4	5:15 AM 8	5:00 AM 2	4:15 AM 5	6:30 AM 5	4:15 AM 4	8:15 AM 3	4:15 AM 1	6:00 AM 1	8:45 AM 1	5:15 AM 22		
PM Peak	12:00 PM	4:30 PM	4:30 PM	2:15 PM	4:45 PM	4:15 PM	3:30 PM	4:30 PM	4:45 PM	2:00 PM	12:45 PM	5:30 PM	8:15 PM	8:00 PM	3:45 PM		
15-min Vol	0	1	1	1	3	3	3	5	7	6	5	4	2	2	17		
Comments:																	
Comments.																to.//www.guality	

## **SUMMARY - Tube Count - Speed Data**



Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: EB, WB DATE: Jun 13 2023

Cars & 2 Axle 2 Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Bikes Start Time Buses Total **Trailers** Tire Single Single Double Double **Double** Multi Multi Multi Classed Long 12:00 AM 12:15 AM 12:30 AM 12:45 AM 01:00 AM 01:15 AM 01:30 AM 01:45 AM 02:00 AM 02:15 AM 02:30 AM 02:45 AM 03:00 AM 03:15 AM 03:30 AM 03:45 AM 04:00 AM 04:15 AM 04:30 AM 04:45 AM 05:00 AM 05:15 AM 05:30 AM 05:45 AM Day Total Percent ADT AM Peak 15-min Vol PM Peak 15-min Vol

Comments:

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: EB, WB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6 Tire	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl Double	<6 Axl	6 Axle	>6 Axl Multi	Not Classed	Total
		Trailers	Long			Single	Single	Double	Double		Multi	Multi			lotai
06:00 AM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
06:15 AM	0	5	3	0	0	0	0	0	0	0	0	0	0	0	8
06:30 AM	0	4	5	1	3	0	0	0	0	0	0	0	0	0	13
06:45 AM	0	4	1	0	0	0	0	0	0	0	0	0	0	0	5
07:00 AM	0	1	1	0	3	0	0	0	0	0	0	0	0	0	5
07:15 AM	0	4	1	0	2	0	0	1	1	0	0	0	0	0	9
07:30 AM	0	2	3	0	1	0	0	1	1	0	0	0	0	0	8
07:45 AM	0	4	2	0	1	0	0	2	0	0	0	0	0	0	9
MA 00:80	0	2	0	0	5	0	0	1	1	0	0	0	0	0	9
08:15 AM	0	1	2	1	1	0	0	1	0	0	0	0	0	0	6
08:30 AM	0	1	1	0	2	0	0	0	0	0	0	0	0	0	4
08:45 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
09:00 AM	0	2	0	0	3	0	0	0	0	0	0	0	0	0	5
09:15 AM	0	2	1	0	1	0	0	1	0	0	0	0	0	0	5
09:30 AM	0	3	2	0	1	0	0	1	0	0	0	0	0	0	7
09:45 AM	0	3	0	1	4	0	0	2	0	0	0	0	0	0	10
10:00 AM	0	1	1	0	4	0	0	0	0	0	0	0	0	0	6
10:15 AM	0	1	3	1	4	0	0	0	0	0	0	0	0	0	9
10:30 AM	0	0	1	0	1	0	0	1	0	0	0	0	0	0	3
10:45 AM	0	2	5	1	2	0	0	2	0	0	0	0	0	0	12
11:00 AM	0	2	0	0	0	0	0	2	0	0	0	0	0	0	4
11:15 AM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
11:30 AM	0	1	7	0	0	0	0	0	0	0	0	0	0	0	8
11:45 AM	0	2	0	1	2	0	0	1	0	0	0	0	0	0	6
Day Total															
Percent				DATA	AIHA	$\Delta ID$	RIVE	566	IMM	UNIT	11-5				
ADT															
591															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
omments:															
-															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: EB, WB

CITY/STATE: Be	enton, WA													DATÉ: J	un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl	5 Axle	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
		Trailers	Long	Duses				Double	Double						TOTAL
12:00 PM	0	1	3	0	0	0	0	2	1	0	0	0	0	0	7
12:15 PM	0	2	3	1	0	0	0	0	0	0	0	0	0	0	6
12:30 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
12:45 PM	0	1	1	1	7	0	0	0	0	0	0	0	0	0	10
01:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
01:15 PM	0	2	1	2	2	0	0	1	0	0	0	0	0	0	8
01:30 PM	0	3	3	0	0	0	0	1	0	0	0	0	0	0	7
01:45 PM	0	2	2	1	2	0	0	1	0	0	0	0	0	0	8
02:00 PM	0	2	3	0	4	0	0	1	0	0	0	0	0	0	10
02:15 PM	0	1	3	1	4	0	0	1	0	0	0	0	0	0	10
02:30 PM	0	3	5	2	2	1	0	0	0	0	0	0	0	0	13
02:45 PM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	3
03:00 PM	0	1	0	0	1	0	0	1	0	0	0	0	0	0	3
03:15 PM	0	2	2	1	2	0	0	1	0	0	0	0	0	0	8
03:30 PM	0	8	5	1	2	0	0	0	0	0	0	0	0	0	16
03:45 PM	0	3	4	1	5	0	0	4	0	0	0	0	0	0	17
04:00 PM	0	2	4	0	3	0	0	1	0	0	0	0	0	0	10
04:15 PM	0	6	4	1	2	0	0	0	0	0	0	0	0	0	13
04:30 PM	0	8	6	0	1	0	0	0	1	0	0	0	0	0	16
04:45 PM	0	10	2	1	2	0	0	1	1	0	0	0	0	0	17
05:00 PM	0	8	5	0	2	0	0	0	0	0	0	0	0	0	15
05:15 PM	0	8	2	1	2	0	0	0	0	0	0	0	0	0	13
05:30 PM	0	9	3	0	1	0	0	2	0	0	0	0	0	0	15
05:45 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
Day Total															
Percent				DAIA	1 17/	ALD	RIVE	200	IIVIIVI	UMI	153				
ADT															
591															
AM Peak 15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: EB, WB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	5	4	2	3	0	0	1	0	0	0	0	0	0	15
06:15 PM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
06:30 PM	0	5	3	1	3	0	0	0	0	0	0	0	0	0	12
06:45 PM	0	6	2	1	1	0	0	0	0	0	0	0	0	0	10
07:00 PM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
07:15 PM	0	1	2	0	2	0	0	0	0	0	0	0	0	0	5
07:30 PM	0	11	2	0	3	0	0	1	0	0	0	0	0	0	17
07:45 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
08:00 PM	0	3	2	1	5	0	0	0	0	0	0	0	0	0	11
08:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
08:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:45 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
09:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
09:15 PM	0	1	0	0	3	0	0	0	0	0	0	0	0	0	4
09:30 PM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
09:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	1	1	1	0	0	2	0	0	0	0	0	0	5
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
11:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
Day Total	0	232	159	27	128	1	0	38	6	0	0	0	0	0	591
Percent	0%	39.3%	26.9%	4.6%	21.7%	0.2%	0%	6.4%	1%	0%	0%	0%	0%	0%	591
ADT 591															
AM Peak	12:00 AM	5:15 AM	4:15 AM	6:30 AM	8:00 AM	12:00 AM	12:00 AM	7:45 AM	7:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	5:15 AM
15-min Vol	0	15	7	1	5	0	0	2	1	0	0	0	0	0	22
PM Peak	12:00 PM	7:30 PM	4:30 PM	1:15 PM	12:45 PM	2:30 PM	12:00 PM	3:45 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	3:45 PM
15-min Vol	0	11	6	2	7	1	0	4	1	0	0	0	0	0	17

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023

CITT/STATE: BEI	itori, WA													DATE	Juli 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	232 39.3%	159 26.9%	27 4.6%	128 21.7%	1 0.2%	0 0%	38 6.4%	6 1%	0 0%	0 0%	0 0%	0 0%	0 0%	591
ADT 591															
Comments:						77									

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: EB, WB

DATE: Jun 13 2023 - Jun 13 2023

13 Jun 23		Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	Average Week Profile
12:15 AM										_
12:30 AM										
12:45 AM									_	
01:00 AM		0				-			_	
01:15 AM		1								
01:30 AM		1							1	
01:45 AM		1								
02:00 AM		0							0	
02:15 AM		0							0	
02:30 AM		0				0	_		0	
02:45 AM		1				1			1	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM	03:30 AM	0				0			0	
04:15 AM     14     14     14       04:30 AM     6     6     6       04:45 AM     4     4     4       05:00 AM     9     9     9       05:15 AM     22     22     22       05:30 AM     8     8     8       05:45 AM     9     9     9       Day Total     9     9     9       % Weekday Average     9     9     9       % Week Average     9     9     9       AM Peak 15-min Vol     9     9     9     9       PM Peak     9     9     9     9     9	03:45 AM	0				0			0	
04:30 AM       6       6       6       4<	04:00 AM	6				6			6	
04:45 AM       4       4       4       9<	04:15 AM	14							14	
05:00 AM       9       9       9       22	04:30 AM	6				6		ın'	6	
05:15 AM       22       22       22         05:30 AM       8       8       8         05:45 AM       9       9       9         Day Total       9       9       9         % Weekday Average       1       1       1         % Week Average       4       1       1       1         AM Peak 15-min Vol       1       1       1       1       1         PM Peak       1       1       1       1       1       1       1	04:45 AM	4				4		411	4	
05:45 AM99Day Total99% Weekday Average11% Week Average11AM Peak 15-min Vol11PM Peak11PM Peak11PM Peak11PM Peak11	05:00 AM	9							9	
05:45 AM99Day Total99% Weekday Average11% Week Average11AM Peak 15-min Vol11PM Peak11PM Peak11PM Peak11PM Peak11	05:15 AM	22				22	00000	CONTRACTOR	22	
Day Total  Weekday Average  Week Average  AM Peak 15-min Vol  PM Peak	05:30 AM	8				8	DIVIN	UNII	8	
Weekday Average Week Average   Meek Average Section 1   AM Peak 15-min Vol Section 2   PM Peak Section 3   PM Peak Section 3   PM Peak Section 3	05:45 AM	9				9			9	
Average         Meek         Meek	Day Total									
% Week       Average       AM Peak       15-min Vol       PM Peak       15-min Vol	% Weekday									
Average	Average									
AM Peak           15-min Vol           PM Peak										
15-min Vol	Average									
PM Peak										
	15-min Vol									
15-min Vol										
	15-min Vol									

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236664 DIRECTION: EB, WB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		6				6			6	
06:15 AM		8				8			8	
06:30 AM		13				13			13	
06:45 AM		5				5			5	
07:00 AM		5				5			5	
07:15 AM		9				9			9	
07:30 AM		8				8			8	
07:45 AM		9				9			9	
08:00 AM		9				9			9	
08:15 AM		6				6			6	
08:30 AM		4				4			4	
08:45 AM		3				3			3	
09:00 AM		5				5			5	
09:15 AM		5				5			5	
09:30 AM		7				7			7	
09:45 AM		10				10			10	
10:00 AM		6				6			6	
10:15 AM		9				9			9	
10:30 AM		3				3			3	
10:45 AM		12				12			12	
11:00 AM		4				4			4	
11:15 AM		5				5 8	00000		5	
11:30 AM		8				8	DIVIN		8	
11:45 AM		6				6			6	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

CITY/STATE: Benton, WA

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION:** EB, WB **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		7				7			7	
12:15 PM		6				6			6	
12:30 PM		2				2			2	
12:45 PM		10				10			10	
01:00 PM		1				1			1	
01:15 PM		8				8			8	
01:30 PM		7				7			7	
01:45 PM		8				8			8	
02:00 PM		10				10			10	
02:15 PM		10				10			10	
02:30 PM		13				13			13	
02:45 PM		3				3			3	
03:00 PM		3				3			3	
03:15 PM		8				8			8	
03:30 PM		16				16			16	
03:45 PM		17				17			17	
04:00 PM		10				10			10	
04:15 PM		13			_	13			13	
04:30 PM		16				16		ın.	16	
04:45 PM		17				17		41 I	17	
05:00 PM		15				15			15	
05:15 PM		13			TIATI	13	00000	0.78.03	13	
05:30 PM		15			HALL	15	DIVIIVI	UNII	15	
05:45 PM		5				5			5	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon 13	Tue Jun 23	Wed	Thu	Fri	Average Week 15-min Traff		Sat	Sun	Average We 15-min Traf	Average Week Profile
06:00 PM		15				15				15	
06:15 PM		5				5				5	
06:30 PM		12				12				12	
06:45 PM		10				10				10	
07:00 PM		4				4				4	
07:15 PM		5				5				5	_
07:30 PM		17				17				17	
07:45 PM		3				3				3	
08:00 PM		11				11				11	
08:15 PM		2				2				2	
08:30 PM		1				1				1	-
08:45 PM		3				3				3	
09:00 PM		2				2				2	
09:15 PM		4				4	1			4	
09:30 PM		5				5				5	
09:45 PM		1				1				1	
10:00 PM		0				0				0	
10:15 PM		1				1	1			1	
10:30 PM		1				1			In.	1	
10:45 PM		5				5			411	5	
11:00 PM		0				0				0	
11:15 PM		0				0				0	
11:30 PM		3				3	C		UNII	3	
11:45 PM		1				1				1	
Day Total		591				591				591	
% Weekday		100%									
Average											
% Week Average		100%				100%					
AM Peak	5	:15 AM				5:15 AM				5:15 AM	
15-min Vol		22				22				22	
PM Peak	3	:45 PM				3:45 PM				3:45 PM	
15-min Vol		17				17				17	

QC JOB #: 16236664 DIRECTION: EB, WB

LOCATION: I		rove Rd b	twn Nic	osin Rd an	ıd I-82											QC JOB #: 1	
SPECIFIC LO	CATION:															DIRECT	ION: WB
CITY/STATE:	Benton,	WA														DATE: Jun	13 2023
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
01:15 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	1	0	4	1	0	0	0	0	6	51-60	5
04:15 AM	0	0	0	0	0	0	0	5	2	4	2	1	0	0	14	46-55	7
04:30 AM	0	0	0	0	0	1	0	0	2	3	0	0	0	0	6	51-60	5
04:45 AM	0	0	0	0	1	0	0	0	1	0	1	0	0	0	3	26-35	1
05:00 AM	0	0	0	0	1	1	2	0	3	1	0	0	0	0	8	51-60	4
05:15 AM	0	0	0	0	2	6	2	1	3	1	2	0	0	0	17	33-42	8
05:30 AM	0	0	0	0	0	0	0	1	3	2	0	0	0	0	6	51-60	5
05:45 AM	0	0	0	0	1	0	0	0	2	1	1	0	0	0	5	51-60	3
Day Total				-	A	-9-1 1	A === 1	200	/	00	0.70	#1 TK	11-9-11				
Percent				DA	MA	-1	ALL	DKI	/E3	(.()	MN	1UN	HHH	-5			
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	
Comments:																	
		. /											201100	- 0 II.			

QC JOB #: 16236664 LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 **SPECIFIC LOCATION: DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 56-65 06:15 AM 51-60 06:30 AM 51-60 06:45 AM 51-60 07:00 AM 41-50 07:15 AM 55-64 07:30 AM 21-30 07:45 AM 41-50 08:00 AM O 41-50 08:15 AM 26-35 08:30 AM 1-10 08:45 AM 36-45 09:00 AM 46-55 09:15 AM 36-45 09:30 AM 51-60 09:45 AM 41-50 10:00 AM 46-55 10:15 AM 51-60 10:30 AM O 36-45 10:45 AM 46-55 11:00 AM 41-50 11:15 AM 41-50 11:30 AM 51-60 11:45 AM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236664 LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 **SPECIFIC LOCATION: DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 26-35 12:15 PM 46-55 46-55 12:30 PM 12:45 PM 46-55 01:00 PM 1-10 01:15 PM 41-50 46-55 01:30 PM 01:45 PM 31-40 02:00 PM O 36-45 02:15 PM 51-60 02:30 PM 41-50 02:45 PM 51-60 03:00 PM 46-55 03:15 PM 31-40 03:30 PM 51-60 03:45 PM 36-45 04:00 PM 1-10 04:15 PM 36-45 04:30 PM O 46-55 04:45 PM 26-35 05:00 PM 41-50 05:15 PM 31-40 05:30 PM 51-60 05:45 PM 46-55 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

O

O

08:30 PM

08:45 PM

09:00 PM

09:15 PM

09:30 PM

09:45 PM

10:00 PM

10:15 PM

10:30 PM

10:45 PM

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 PM 1-10 06:15 PM 31-40 51-60 06:30 PM 06:45 PM 46-55 07:00 PM 41-50 07:15 PM 51-60 07:30 PM 46-55 07:45 PM 46-55 08:00 PM O O 46-55 08:15 PM 1-10 

1-10

46-55

41-50

46-55

41-50

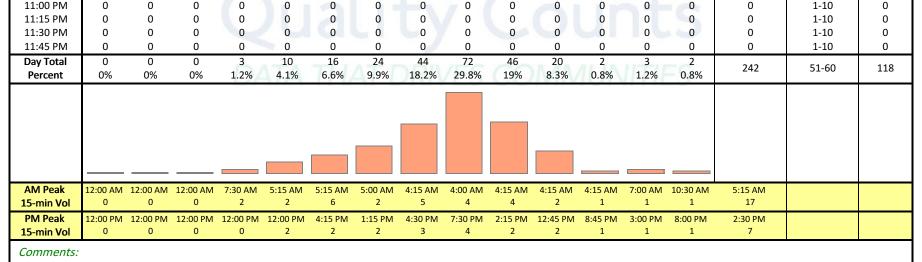
56-65

1-10

1-10

1-10

1-10



LOCATION: Loc	cust Grov	ve Rd btv	vn Nicosir	n Rd and I-	82											QC JOB	#: 16236664
SPECIFIC LOCA	TION:															DIR	ECTION: WB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Nange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 ace speed	Pace
Grand Total	0	0	0	3	10	16	24	44	72	46	20	2	3	2	242	51-60	118
Percent	0%	0%	0%	1.2%	4.1%	6.6%	9.9%	18.2%	29.8%	19%	8.3%	0.8%	1.2%	0.8%	242	31-00	110
Cumulative	0%	0%	0%	1.2%	5.4%	12%	21.9%	40.1%	69.8%	88.8%	97.1%	97.9%	99.2%	100%			
Percent	0,0	070	0,0	1.270	3.170	1270	21.570	10.170	03.070	00.070	37.170	37.370	33.270	10070			
ADT 242				. —											Me	an Speed(Avera Med	ntile: 58 MPH age): 51 MPH dian: 51 MPH ode: 53 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: WB

CITY/STATE: Be	IIIOII, WA	6. 0	241		2416	2.4.1	4.6.1	.e	5 A 1		.c. 4. 1	C A .			un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:30 AM	0	0	0	0	0	0	0	О	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	2	4	0	0	0	0	0	0	0	0	0	0	0	6
04:15 AM	0	5	7	0	1	0	0	1	0	0	0	0	0	0	14
04:30 AM	0	4	1	0	1	0	0	0	0	0	0	0	0	0	6
04:45 AM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	3
05:00 AM	0	7	1	0	0	0	0	0	0	0	0	0	0	0	8
05:15 AM	0	12	4	0	1	0	0	0	0	0	0	0	0	0	17
05:30 AM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
05:45 AM	0	3	0	0	2	0	0	0	0	0	0	0	0	0	5
Day Total															
Percent				DAIA	AIHA		RIVE	500	MM	UNIT	11-5				
ADT															
242															
ANA Dook															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: WB DATE: Jun 13 2023

Start Time	E: Jun 13 202														nton, WA	CITY/STATE: Be
06:00 AM	d Total	Not Classed										Buses			Bikes	Start Time
06:15 AM	4	0										0			0	06:00 AM
06:45 AM	4	0		0		0	0		0	0	0	0		1		
07:00 AM	9	0	0	0	0	0	0	0	0	0	2	1	3	3	0	06:30 AM
07:15 AM	4	0	0	0	0	0	0	0	0	0	0	0	1	3	0	06:45 AM
07:30 AM	3	0	0	0	0	0	0	0	0	0	2	0	1	0	0	07:00 AM
07:45 AM	6	0	0	0	0	0	0	1	0	0	1	0	1	3	0	07:15 AM
08:00 AM	3	0	0	0	0	0	1	0	0	0	0	0	2	0	0	07:30 AM
08:15 AM	5	0	0	0	0	0	0	2	0	0	1	0	0	2	0	07:45 AM
08:30 AM	4	0	0	0	0	0	0	1	0	0	3	0	0	0	0	08:00 AM
08:45 AM	2	0	0	0	0	0	0	0	0	0	0	0	1	1	0	
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	08:30 AM
09:15 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	
09:30 AM	4	0	0	0	0	0	0	0		0	3	0	0	1	0	
09:45 AM	2	0	0	0	0	0	0	1	0	0	1	0	0	0	0	
10:00 AM	3	0	0	0	0	0	0	1			0	0	1	1	0	09:30 AM
10:15 AM	2	0	0	0	0	0	0	0	0	0	1	0	0	1	0	09:45 AM
10:30 AM	2	0	0	0	0	0	0	0	0	0	1	0	1	0	0	10:00 AM
10:45 AM	6	0	0	0	0	0	0	0	0	0	2	0	3	1	0	10:15 AM
11:00 AM	3	0	0	0	0	0	0	1	0	0	1	0	1	0	0	
11:15 AM	2	0	0	0	0	0	0	0	0		0	0	1	1	0	10:45 AM
11:30 AM 0 0 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1:45 AM 0 1 0 0 2 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0	2	0	0	0			0	1	00			0	0	1	0	
11:45 AM 0 1 0 0 2 0 0 1 0 0 0 0 0 0 0 0 0 0 0 0	3	0	0	0				0	0				1	1	0	
Day Total Percent  ADT	3	0	0	0	0	0	0	0	0	0	0	0	3	0	0	11:30 AM
ADT ADT	4	0	0	0	0	0	0	1	0	0	2	0	0	1	0	
ADT																
					IES	JIMI	IVIIVI	3 ( )	RIVE	AL DI	A I ITIZ	DAIA				Percent
																ADT 242
AM Peak																
15-min Vol																
PM Peak 15-min Vol																
omments:																

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236664 DIRECTION: WB DATE: Jun 13 2023

															un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	0	0	2	0	0	0	0	0	1	0	0	0	0	0	3
12:15 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
12:30 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
12:45 PM	0	0	1	0	3	0	0	0	0	0	0	0	0	0	4
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 PM	0	0	0	2	1	0	0	1	0	0	0	0	0	0	4
01:30 PM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
01:45 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
02:00 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
02:15 PM	0	1	2	0	1	0	0	1	0	0	0	0	0	0	5
02:30 PM	0	0	5	0	2	0	0	0	0	0	0	0	0	0	7
02:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
03:00 PM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2
03:15 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
03:30 PM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
03:45 PM	0	1	1	0	1	0	0	1	0	0	0	0	0	0	4
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
04:30 PM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
04:45 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
05:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
05:15 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
Day Total															
Percent				DATA	AIHA		RIVE	SCC	MM	LIMII	11-5				
ADT															
242															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
omments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236664 DIRECTION: WB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
06:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
06:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
07:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
07:15 PM	0	1	1	0	2	0	0	0	0	0	0	0	0	0	4
07:30 PM	0	3	2	0	2	0	0	0	0	0	0	0	0	0	7
07:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:00 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
08:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
09:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:15 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
09:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
09:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	95	76	3	50	0	0	16	2	0	0	0	0	0	242
Percent	0%	39.3%	31.4%	1.2%	20.7%	0%	0%	6.6%	0.8%	0%	0%	0%	0%	0%	242
ADT 242															
AM Peak	12:00 AM	5:15 AM	4:15 AM	6:30 AM	8:00 AM	12:00 AM	12:00 AM	7:45 AM	7:30 AM				12:00 AM		5:15 AN
15-min Vol	0	12	7	1	3	0	0	2	1	0	0	0	0	0	17
PM Peak 15-min Vol	12:00 PM 0	4:30 PM 3	2:30 PM 5	1:15 PM 2	12:45 PM 3	12:00 PM 0	12:00 PM 0	1:15 PM 1	12:00 PM 1	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	2:30 PN 7
omments:	U					- 0	- 0			0	- 0	- 0	- 0	Ū	

LOCATION: Locust Grove Rd btwn Nicosin Rd and I-82 QC JOB #: 16236664 SPECIFIC LOCATION: **DIRECTION: WB** 

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	95 39.3%	76 31.4%	3 1.2%	50 20.7%	0 0%	0 0%	16 6.6%	2 0.8%	0 0%	0 0%	0 0%	0 0%	0 0%	242
ADT 242															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

DIRECTION: WB DATE: Jun 13 2023 - Jun 13 2023

QC JOB #: 16236664

CITY/STATE: Benton, WA

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		1				1			1	
01:15 AM		1				1			1	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		1				1			1	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		0				0			0	
03:30 AM		0				0			0	
03:45 AM		0				0			0	
04:00 AM		6				6			6	
04:15 AM		14			_ I º .	14	-		14	
04:30 AM		6				6			6	
04:45 AM		3			261	3			3	
05:00 AM		8				8			8	
05:15 AM		17			TIATI	17 6	30 40 A		17	
05:30 AM		6			MAI L		JIVIIVI		6	
05:45 AM		5				5			5	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE

QC JOB #: 16236664 DIRECTION: WB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		4				4			4	
06:15 AM		4				4			4	
06:30 AM		9				9			9	
06:45 AM		4				4			4	
07:00 AM		3				3			3	
07:15 AM		6				6			6	
07:30 AM		3				3			3	
07:45 AM		5				5			5	
08:00 AM		4				4			4	
08:15 AM		2				2			2	
08:30 AM		0				0			0	
08:45 AM		1				1			1	
09:00 AM		4				4			4	
09:15 AM		2				2			2	
09:30 AM		3				3			3	
09:45 AM		2				2			2	
10:00 AM		2				2			2	
10:15 AM		6			_ I ° .	6			6	
10:30 AM		3				3			3	
10:45 AM		2			<b>461</b>	2			2	
11:00 AM		2				2			2	
11:15 AM		3			LIATI	3	ON AN A	II IN IIT	1 E C 3	
11:30 AM		3			THI L		JIVIIVI	UNIT	3	
11:45 AM		4				4			4	
Day Total										
% Weekday Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236664 DIRECTION: WB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		3				3			3	
12:15 PM		2				2			2	
12:30 PM		2				2			2	
12:45 PM		4				4			4	
01:00 PM		0				0			0	
01:15 PM		4				4			4	
01:30 PM		3				3			3	
01:45 PM		3				3			3	
02:00 PM		2				2			2	
02:15 PM		5				5			5	
02:30 PM		7				7			7	
02:45 PM		1				1			1	
03:00 PM		2				2			2	
03:15 PM		2				2			2	
03:30 PM		4				4			4	
03:45 PM		4				4			4	
04:00 PM		0				0			0	
04:15 PM		4			_   •	4			4	
04:30 PM		4				4		In'	4	
04:45 PM		2			261	2		411	2	
05:00 PM		2				2			2	
05:15 PM		2					00.00.0	T TR TITE	2	
05:30 PM		1			HALL	DRIVES CO	DIVIIVI	UNII	IES 1	
05:45 PM		1				1			1	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236664 DIRECTION: WB DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		0				0			0	
06:15 PM		2				2			2	
06:30 PM		1				1			1	
06:45 PM		1				1			1	
07:00 PM		2				2			2	
07:15 PM		4				4			4	
07:30 PM		7				7			7	
07:45 PM		1				1			1	
08:00 PM		3				3			3	
08:15 PM		0				0			0	
08:30 PM		0				0			0	
08:45 PM		2				2			2	
09:00 PM		1				1			1	
09:15 PM		2				2			2	
09:30 PM		2				2			2	
09:45 PM		1				1			1	
10:00 PM		0				0			0	
10:15 PM		0				0		-	0	
10:30 PM		0				0		In.	0	
10:45 PM		0				0	$\sim$	411	0	
11:00 PM		0				0			0	
11:15 PM		0					01/11/	CONTRACTOR	0	
11:30 PM		0				0	DIVIN	UNII	<b>1 5</b> 0	
11:45 PM		0				0			0	
Day Total		242				242			242	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		5:15 AM				5:15 AM			5:15 AM	
15-min Vol		17				17			17	
PM Peak		2:30 PM				2:30 PM			2:30 PM	
15-min Vol		7				7			7	

LOCATION: R SPECIFIC LOC CITY/STATE:	ATION:		ine Cany	on Rd												QC JOB #: 1 DIREC DATE: Jur	TION: EB
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
12:15 AM	0	0	0	0	0	0	0	1	0	2	1	0	0	0	4	56-65	3
12:30 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2	41-50	2
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2	46-55	2
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	56-65	2
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
04:00 AM	0	0	0	0	0	0	0	1	0	1	1	1	0	0	4	56-65	2
04:15 AM	0	0	0	0	0	0	0	0	4	3	1	0	1	0	9	51-60	7
04:30 AM	0	0	0	0	0	0	0	0	0	6	8	3	2	0	19	56-65	14
04:45 AM	0	0	0	0	0	0	0	0	1	4	10	2	1	0	18	56-65	14
05:00 AM	0	0	0	0	0	0	0	0	0	3	0	1	0	0	4	51-60	3
05:15 AM	0	0	1	0	0	0	0	0	1	1	0	2	0	1	6	61-70	2
05:30 AM	0	0	0	0	0	0	0	0	1	5	3	0	0	0	9	56-65	8
05:45 AM	0	0	0	0	0	0	0	0	1	1	1	2	1	0	6	66-75	3
Day Total	0	0	- 0	-			A == 1		,		0.75	Z			· ·	00-73	3
Percent				DA	MA	IH	411	DKI	/ES	(.()	MIN	1UN		-5			
AM Peak 15-min Vol																	
PM Peak 15-min Vol																	
Comments:																	

tart Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
06:00 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	2	61-70	2
06:15 AM	0	0	0	0	0	0	0	0	0	3	5	0	0	0	8	56-65	8
06:30 AM	0	0	0	0	1	0	0	0	0	3	5	1	0	0	10	56-65	8
06:45 AM	0	0	0	0	0	0	0	0	0	4	1	1	0	0	6	56-65	5
07:00 AM	0	0	0	0	0	0	0	0	1	0	4	1	0	0	6	61-70	5
07:15 AM	0	0	0	0	0	0	0	0	2	4	2	1	1	0	10	56-65	6
07:30 AM	0	0	0	0	0	0	0	0	1	8	3	2	0	0	14	56-65	11
07:45 AM	0	0	0	0	0	0	0	0	2	2	6	2	0	0	12	58-67	8
08:00 AM	0	0	0	0	0	0	0	0	0	6	3	0	0	0	9	56-65	9
08:15 AM	0	0	0	0	0	0	0	0	0	2	3	0	0	0	5	56-65	5
08:30 AM	0	0	0	0	0	0	0	0	0	3	3	0	0	0	6	56-65	6
08:45 AM	0	0	0	0	0	0	0	0	4	2	3	2	0	0	11	51-60	6
09:00 AM	0	0	0	0	0	0	0	0	0	4	2	1	0	0	7	56-65	6
09:15 AM	0	0	0	0	0	0	0	0	1	2	1	0	1	0	5	56-65	3
9:30 AM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
9:45 AM	0	0	0	0	0	0	0	0	1	2	2	1	0	0	6	56-65	4
L0:00 AM	0	0	0	0	0	0	0	0	1	2	1	2	0	0	6	56-65	3
L0:15 AM	0	0	0	0	0	0	0	0	2	4	4	1	0	0	11	56-65	8
10:30 AM	0	0	0	0	0	0	0	2	1	5	2	0	0	0	10	56-65	7
10:45 AM	0	0	0	0	0	0	0	0	1	3	2	0	0	0	6	56-65	5
11:00 AM	0	0	0	0	0	0	0	1	<b>1</b>	2	2	2	2	0	10	56-65	4
11:15 AM	0	0	0	0	0	0	0	0	0	1	2	2	0	0	5	61-70	4
11:30 AM	0	0	0	0	0	0	0	0	1	4	4	0	0	0	9	56-65	8
11:45 AM	0	0	0	0	0	0	0	0	2	4	4	1	0	0	11	56-65	8
Day Total					TA	711	$\Lambda = I$	NDA	/FC	00	A // A	// IK	11711				
Percent				UF	MA		AII	JKI	/ED	CC	IVIIV	1UN					
AM Peak 5-min Vol																	
PM Peak 5-min Vol																	

QC JOB #: 16236665 LOCATION: Rte 397 West of Nine Canyon Rd **SPECIFIC LOCATION: DIRECTION: EB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 56-65 56-65 12:30 PM 12:45 PM 61-70 01:00 PM 56-65 01:15 PM 56-65 56-65 01:30 PM 01:45 PM 56-65 02:00 PM O 56-65 02:15 PM 56-65 02:30 PM 56-65 02:45 PM 61-70 03:00 PM 56-65 03:15 PM 56-65 03:30 PM 56-65 03:45 PM 56-65 04:00 PM 56-65 04:15 PM 56-65 04:30 PM O 56-65 04:45 PM 56-65 05:00 PM 56-65 05:15 PM 56-65 05:30 PM 60-69 05:45 PM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Rte 397 West of Nine Canyon Rd

QC JOB #: 16236665

SPECIFIC LOCATION:

DIRECTION: EB

CITY/STATE: Benton, WA DATE: Jun 13 202

CITI/STATE.	Benton,	WA														DATE: Jun	13 202
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
Start Tille	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	in Pac
06:00 PM	0	0	0	0	0	0	0	0	1	4	5	1	0	0	11	56-65	9
06:15 PM	0	0	0	0	0	0	0	0	0	1	4	2	0	1	8	61-70	6
06:30 PM	0	0	0	0	0	0	0	1	0	1	2	1	0	0	5	61-70	3
06:45 PM	0	0	0	0	0	0	0	0	1	2	1	0	0	0	4	56-65	3
07:00 PM	0	0	0	0	0	0	0	0	2	1	4	0	0	0	7	56-65	5
07:15 PM	0	0	0	0	0	0	0	0	0	2	2	1	0	0	5	56-65	4
07:30 PM	0	0	0	0	0	0	0	0	1	1	1	0	0	0	3	51-60	2
07:45 PM	0	0	0	0	0	0	0	0	0	2	1	0	1	0	4	56-65	3
08:00 PM	0	0	0	0	0	0	0	0	1	2	2	1	0	0	6	56-65	4
08:15 PM	0	0	0	0	0	0	0	0	2	2	4	1	0	0	9	56-65	6
08:30 PM	0	0	0	0	0	0	0	0	3	1	2	0	0	0	6	51-60	4
08:45 PM	0	0	0	0	0	0	0	0	1	2	1	0	0	0	4	56-65	3
09:00 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
09:15 PM	0	0	0	0	0	0	0	0	1	1	1	0	0	0	3	51-60	2
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:00 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
10:15 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	2	51-60	2
10:30 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
11:15 PM	0	0	0	0	0	0	0	2	1	1	0	0	0	0	4	46-55	3
11:30 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
11:45 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	2	4	41-50	1
Day Total	0	2	3	3	1	_1	2	17	81	232	229	72	20	7	670	56-65	461
Percent	0%	0.3%	0.4%	0.4%	0.1%	0.1%	0.3%	2.5%	12.1%	34.6%	34.2%	10.7%	3%	1%			
AM Peak 15-min Vol	12:00 AM 0	12:00 AM 0	5:15 AM 1	12:00 AM 0	6:30 AM 1	12:00 AM 0	12:00 AM 0	12:30 AM 2	4:15 AM 4	7:30 AM 8	4:45 AM 10	4:30 AM 3	4:30 AM 2	5:15 AM 1	4:30 AM 19		
PM Peak 15-min Vol	12:00 PM 0	5:00 PM 2	2:30 PM 1	2:30 PM 3	12:00 PM 0	2:30 PM 1	2:00 PM 1		12:15 PM 5	3:15 PM 10	4:15 PM 10	2:45 PM 3	12:45 PM 2	2:15 PM 2	2:30 PM 22		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Rte	397 We	est of Nine	e Canyon	Rd												QC JOB	#: 16236665
SPECIFIC LOCA	TION:															DI	RECTION: EB
CITY/STATE: Be	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Name	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	Pace
Grand Total	0	2	3	3	1	1	2	17	81	232	229	72	20	7	670	56-65	461
Percent	0%	0.3%	0.4%	0.4%	0.1%	0.1%	0.3%	2.5%	12.1%	34.6%	34.2%	10.7%	3%	1%	670	30-03	401
Cumulative Percent	0%	0.3%	0.7%	1.2%	1.3%	1.5%	1.8%	4.3%	16.4%	51%	85.2%	96%	99%	100%			
ADT 670															Me	an Speed(Avera Med	ntile: 64 MPH age): 59 MPH dian: 59 MPH ode: 58 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202 <b>Total</b>
	DIRCS	Trailers	Long	Duscs	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 AM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	3
12:15 AM	0	3	0	0	0	0	0	0	0	1	0	0	0	0	4
12:30 AM	0	0	0	1	0	0	0	1	0	0	0	0	0	0	2
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
04:00 AM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
04:15 AM	0	4	3	0	2	0	0	0	0	0	0	0	0	0	9
04:30 AM	0	10	9	0	0	0	0	0	0	0	0	0	0	0	19
04:45 AM	0	11	6	0	1	0	0	0	0	0	0	0	0	0	18
05:00 AM	0	3	1	0	0	0	0	0	0	0	0	0	0	0	4
05:15 AM	1	2	2	0	1	0	0	0	0	0	0	0	0	0	6
05:30 AM	0	4	4	1	0	0	0	0	0	0	0	0	0	0	9
05:45 AM	0	1	4	0	1	0	0	0	0	0	0	0	0	0	6
Day Total Percent															
refeelt				DALLA		-	UVL	3.00	ZIVIIVI	UIVIII	1153				
ADT 670															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	L C /20 /20	222 7 50 484									COLLBOE O	1:. 6	116/11	//	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB

CITY/STATE: Be	IIIOII, WA	C 0	2 4-1-		2 4-1- 6	2 4-1-	A A.da	4F A!	r A.d.	. C A!	4C A.J	C AI-	> C A!		un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
06:00 AM	0	0	0	0	1	0	0	1	0	0	0	0	0	0	2
06:15 AM	0	3	2	1	2	0	0	0	0	0	0	0	0	0	8
06:30 AM	0	2	5	0	2	0	0	1	0	0	0	0	0	0	10
06:45 AM	0	4	1	0	0	0	0	1	0	0	0	0	0	0	6
07:00 AM	0	2	1	1	2	0	0	0	0	0	0	0	0	0	6
07:15 AM	0	4	4	0	1	0	0	1	0	0	0	0	0	0	10
07:30 AM	0	10	3	1	0	0	0	0	0	0	0	0	0	0	14
07:45 AM	0	6	4	0	1	0	0	1	0	0	0	0	0	0	12
08:00 AM	0	4	2	2	1	0	0	0	0	0	0	0	0	0	9
08:15 AM	0	2	1	0	0	0	0	2	0	0	0	0	0	0	5
08:30 AM	0	3	1	0	2	0	0	0	0	0	0	0	0	0	6
08:45 AM	0	4	4	2	1	0	0	0	0	0	0	0	0	0	11
09:00 AM	0	4	2	0	1	0	0	0	0	0	0	0	0	0	7
09:15 AM	0	3	2	0	0	0	0	0	0	0	0	0	0	0	5
09:30 AM	0	0	0	0	2	0	0	1	0	0	0	0	0	0	3
09:45 AM	0	0	2	0	3	0	0	1	0	0	0	0	0	0	6
10:00 AM	0	1	2	0	1	0	0	2	0	0	0	0	0	0	6
10:15 AM	0	7	1	1	0	0	0	2	0	0	0	0	0	0	11
10:30 AM	0	3	1	2	3	0	0	1	0	0	0	0	0	0	10
10:45 AM	0	2	2	0	2	0	0	0	0	0	0	0	0	0	6
11:00 AM	0	2	3	1	3 1	0	0	1	0	0	0	0	0	0	10
11:15 AM	0	0	4	0		0	0	0	0	0	0	0	0	0	5
11:30 AM	0	1	5	0	3	0	0	0	0	0	0	0	0	0	9
11:45 AM	0	5	1	0	4	0	0	1	0	0	0	0	0	0	11
Day Total															1
Percent				DAIA	117	ALLI	KIVE	200	IIVIIVI	$\cup I M I I$	IES				
															1
															1
ADT															1
670															1
															1
															1
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB

Start Time   Bikes   Cars & 2 Axle   Puses   Tire   Single   Single   Double   Double   Double   Double   Double   Double   Double   Multi   Multi   Multi   Classe	
12:00 PM	Total
12:30 PM	9
12:45 PM	15
01:00 PM	13
01:15 PM         0         5         2         2         2         0<	7
01:30 PM         0         3         1         0         2         0         0         3         0<	12
01:45 PM         0         1         1         0         2         0         0         1         0<	9
02:00 PM         0         3         5         0<	9
02:15 PM         0         5         5         0         2         0<	5
02:30 PM         0         7         8         1         5         0         0         1         0<	11
02:45 PM         0         2         6         1         1         0<	12
03:00 PM         0         2         5         2         3         0         0         1         0<	22
03:15 PM         0         7         4         2         6         0         0         2         0<	10
03:30 PM         0         4         2         1         3         0         0         1         0<	13
03:45 PM         0         7         6         0         1         0         0         3         0<	21
04:00 PM         0         4         6         1         2         0         0         1         0<	11
04:15 PM         0         6         7         1         3         0         0         3         0<	17
04:30 PM         0         3         4         0         4         0         0         1         0<	14
04:45 PM         0         4         3         0         1         0<	20
05:00 PM         0         2         5         2         2         0         0         1         0<	12
05:15 PM         0         4         7         0         2         0         0         1         0         0         0         0         0         0           05:30 PM         0         5         7         0	8
<b>05:30 PM</b> 0 5 7 0 0 0 0 0 0 0 0 0 0 0	12
	14
<b>05:45 PM</b> 0 4 5 0 1 0 0 0 0 0 0 0 0 0	12
	10
Day Total Percent	
ADT	
AM Peak	
15-min Vol PM Peak	
15-min Vol	
comments:	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	7	3	0	1	0	0	0	0	0	0	0	0	0	11
06:15 PM	0	4	4	0	0	0	0	0	0	0	0	0	0	0	8
06:30 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
06:45 PM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
07:00 PM	0	4	2	0	0	0	0	1	0	0	0	0	0	0	7
07:15 PM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
07:30 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
07:45 PM	0	3	0	0	1	0	0	0	0	0	0	0	0	0	4
08:00 PM	0	2	1	0	1	0	0	2	0	0	0	0	0	0	6
08:15 PM	0	5	2	0	1	0	0	1	0	0	0	0	0	0	9
08:30 PM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	6
08:45 PM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
09:00 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
09:15 PM	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
10:15 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
10:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	2	1	0	0	0	0	1	0	0	0	0	0	0	4
11:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
11:45 PM	0	0	2	0	1	0	0	1	0	0	0	0	0	0	4
Day Total	1	264	216	33	102	0	0	53	0	1	0	0	0	0	670
Percent	0.1%	39.4%	32.2%	4.9%	15.2%	0%	0%	7.9%	0%	0.1%	0%	0%	0%	0%	070
ADT 670															
AM Peak	5:15 AM	4:45 AM	4:30 AM	3:15 AM	11:45 AM	12:00 AM	12:00 AM	8:15 AM	12:00 AM	12:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	4:30 AN
15-min Vol	1	11	9	2	4	0	0	2	0	1	0	0	0	0	19
PM Peak 15-min Vol	12:00 PM 0	12:15 PM 9	2:30 PM 8	12:30 PM 3	3:15 PM 6	12:00 PM 0	12:00 PM 0	12:00 PM 3	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	12:00 PM 0	2:30 PM
12-IIIII AOI	U	9	ð	3	0	U	U	3	U	U	U	U	U	U	22

LOCATION: Rte 397 West of Nine Canyon Rd

SPECIFIC LOCATION:

DIRECTION: EB
CITY/STATE: Benton, WA

DATE: Jun 13 2023

Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Grand Total	1	Trailers 264	Long 216	33	Tire 102	Single 0	Single 0	Double 53	<b>Double</b> 0	Double 1	<b>Multi</b> 0	Multi 0	Multi 0	Classed 0	670
Percent	0.1%	39.4%	32.2%	4.9%	15.2%	0%	0%	7.9%	0%	0.1%	0%	0%	0%	0%	0,0
ADT 670															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		3				3			3	
12:15 AM		4				4			4	
12:30 AM		2				2			2	
12:45 AM		0				0			0	
01:00 AM		2				2			2	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		1				1			1	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		0				0			0	
03:15 AM		2				2			2	
03:30 AM		0				0			0	
03:45 AM		1				1			1	
04:00 AM		4				4			4	
04:15 AM		9			_ I °.	9			9	
04:30 AM		19			21111	19			19	
04:45 AM		18			461	18			18	
05:00 AM		4				4			4	
05:15 AM		6			LIATI	6 9	00000		6	
05:30 AM		9			MALL		JIVIIVI		9	
05:45 AM		6				6			6	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

QC JOB #: 16236665 DIRECTION: EB

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DIRECTION: EB
DATE: Jun 13 2023 - Jun 13 2023
k
Average Week Profile

QC JOB #: 16236665

06:00 AM 2 2 2 2 0 06:15 AM 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:30 AM 0 10 10 10 10 10 10 10 10 10 10 10 10 1	06:00 AM		2				2			2	
0645 AM 6 6 6 6 6 6 6 70.700 AM 6 6 70.700 AM 10 10 10 10 10 10 10 10 10 10 10 10 10	06:15 AM		8				8			8	
07:00 AM	06:30 AM		10				10			10	
07:15 AM	06:45 AM		6				6			6	
07:30 AM			6								
07:45 AM     12     12     9       08:00 AM     9     9     9       08:15 AM     5     5     6       08:30 AM     6     6     6       08:45 AM     11     11     11       09:00 AM     7     7     7       09:15 AM     5     5     5       09:30 AM     3     3     3       09:45 AM     6     6     6       10:00 AM     6     6     6       10:15 AM     11     11     11       10:30 AM     10     10     10       10:45 AM     6     6     6       11:00 AM     10     10     10       11:15 AM     5     5     5       11:30 AM     9     9     11:45 AM     11     11       Day Total     8     9     9     11:45 AM     11     11     11       Week Average     8     9     9     10     10       AW Peak     15-min Vol     10     10     10     10       HP Peak     15-min Vol     10     10     10     10     10											
08:00 AM 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9											
08:15 AM			12								
08:30 AM 6 6 6 6 6 6 6 90:00 AM 11 11 11 11 11 11 11 11 11 11 11 11 11			9								
08:45 AM			5				5				
09:00 AM     7     7     7       09:15 AM     5     5     5       09:30 AM     3     3     3       09:45 AM     6     6     6       10:00 AM     6     6     6       10:15 AM     11     11     11       10:30 AM     10     10     10       10:45 AM     6     6     6       11:00 AM     10     10     10       11:30 AM     5     5     5       11:30 AM     9     9     9       11:45 AM     11     11     11       Day Total     *Weekday Average     *Weekday Average     *Weekday Average       AW Peak Average     *Weekday Average     *Weekday Average     *Weekday Average       PM Peak 15-min Vol     **Total Average Average     **Total Average Average Average Average     **Total Average A			_								
09:15 AM         5         5         5         930 AM         3         3         3         3         9.93 AM         6         6         6         6         6         6         6         6         6         10:00 AM         11         11         11         11         11         11         11         11         11         11         11         10:00 AM         10			11								
09:30 AM       3       3       3       3       3       9:45 AM       6       6       6       6       6       10:10 AM       10       11       11       11       11       11       11       11       11       11       11       11       11       11       11       11       11       11       11       10			7								
09:45 AM 6 6 6 6 6 6 6 10:15 AM 11 11 11 11 11 11 11 11 11 11 11 11 11			5								
10:00 AM			3							_	
10:15 AM			-								
10:30 AM											
10:45 AM						~ I .					
11:00 AM       10       10       10         11:15 AM       5       5       5         11:30 AM       9       9       9         11:45 AM       11       11       11         Day Total       Image: Company of the company of											
11:15 AM						761					
11:45 AM         11         <											
11:45 AM         11         <						LIATI	5	$\bigcirc \land \land \land \land \land$			
Day TotalMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday AverageMeekday 						$\square A \sqcap L$		DIVIIVI		F. Books Santa	
Weekday AverageWeek AverageSecond of the property of			11				11			11	
Average         Meek         Average         A											
% Week Average  AM Peak 15-min Vol  PM Peak 15-min Vol  To min Vol  PM Peak 15-min Vol  Region of the properties of the											
Average         AM Peak           15-min Vol         Company           PM Peak         Company           15-min Vol         Company											
AM Peak 15-min Vol Peak 15-min Vol Political PM Peak 15-min Vol Political											
15-min Vol       9M Peak       15-min Vol											
15-min Vol											
	Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236665

DIRECTION: EB
DATE: Jun 13 2023 - Jun 13 2023

CITI/STATE.	Mon	Tue	Wed	Thu	Fri	Average Weekday	Sat	Sun	Average Week	12. Juli 13 2023 - Juli 13 2023
Start Time	IVIOII	13 Jun 23	vveu	IIIu		15-min Traffic	Jai	Juli	15-min Traffic	Average Week Profile
12:00 PM		9				9			9	
12:15 PM		15				15			15	
12:30 PM		13				13			13	
12:45 PM		7				7			7	
01:00 PM		12				12			12	
01:15 PM		9				9			9	
01:30 PM		9				9			9	
01:45 PM		5				5			5	
02:00 PM		11				11			11	
02:15 PM		12				12			12	
02:30 PM		22				22			22	
02:45 PM		10				10			10	
03:00 PM		13				13			13	
03:15 PM		21				21			21	
03:30 PM		11				11			11	
03:45 PM		17				17			17	
04:00 PM		14				14			14	
04:15 PM		20				20			20	
04:30 PM		12				12			12	
04:45 PM		8				8			8	
05:00 PM		12				12	-		12	
05:15 PM		14				14	00.00.0		14	
05:30 PM		12				12	JIVIIVI		12	
05:45 PM		10				10			10	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236665 DIRECTION: EB

**DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		11				11			11	
06:15 PM		8				8			8	
06:30 PM		5				5			5	
06:45 PM		4				4			4	
07:00 PM		7				7			7	
07:15 PM		5				5			5	
07:30 PM		3				3			3	
07:45 PM		4				4			4	
08:00 PM		6				6			6	
08:15 PM		9				9			9	
08:30 PM		6				6			6	
08:45 PM		4				4			4	
09:00 PM		2				2			2	
09:15 PM		3				3			3	
09:30 PM		0				0			0	
09:45 PM		1				1			1	
10:00 PM		2				2			2	
10:15 PM		2			_   °	2			2	
10:30 PM		1				1		In.	1	
10:45 PM		1				1		411	1	
11:00 PM		0				0			0	
11:15 PM		4			TIATI	4	00000	1 TK 117	4	
11:30 PM		2			HALL	2	JIVIIVI	UIVII	2	
11:45 PM		4				4			4	
Day Total		670				670			670	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		4:30 AM				4:30 AM			4:30 AM	
15-min Vol		19				19			19	
PM Peak		2:30 PM				2:30 PM			2:30 PM	
15-min Vol		22				22			22	
omments:	<u> </u>									

QC JOB #: 16236665 LOCATION: Rte 397 West of Nine Canyon Rd **SPECIFIC LOCATION: DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 56-65 51-60 12:15 AM 12:30 AM 46-55 12:45 AM 1-10 01:00 AM 46-55 01:15 AM 56-65 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O 1-10 02:15 AM 56-65 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 61-70 03:15 AM 56-65 03:30 AM 51-60 03:45 AM 51-60 04:00 AM 56-65 04:15 AM 51-60 04:30 AM O 56-65 04:45 AM 56-65 05:00 AM 56-65 05:15 AM 61-70 05:30 AM 56-65 05:45 AM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236665 LOCATION: Rte 397 West of Nine Canyon Rd **SPECIFIC LOCATION: DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 53-62 56-65 06:15 AM 06:30 AM 56-65 06:45 AM 56-65 07:00 AM 56-65 07:15 AM 56-65 56-65 07:30 AM 07:45 AM 56-65 08:00 AM O 56-65 08:15 AM 56-65 08:30 AM 56-65 08:45 AM 56-65 09:00 AM 56-65 09:15 AM 51-60 09:30 AM 55-64 09:45 AM 56-65 10:00 AM 61-70 10:15 AM 51-60 10:30 AM 56-65 10:45 AM 56-65 11:00 AM 56-65 11:15 AM 56-65 11:30 AM 56-65 11:45 AM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Rte 397 West of Nine Canyon Rd QC JOB #: 16236665 **SPECIFIC LOCATION: DIRECTION: EB, WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 56-65 56-65 12:30 PM 12:45 PM 56-65 01:00 PM 56-65 01:15 PM 56-65 56-65 01:30 PM 01:45 PM 56-65 02:00 PM O 56-65 02:15 PM 56-65 02:30 PM 56-65 02:45 PM 56-65 03:00 PM 56-65 03:15 PM 56-65 03:30 PM 56-65 03:45 PM 56-65 04:00 PM 56-65 04:15 PM 56-65 04:30 PM O 60-69 04:45 PM 61-70 05:00 PM 56-65 05:15 PM 56-65 05:30 PM 56-65 05:45 PM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236665 DIRECTION: EB, WB

CITY/STATE:	Benton,	WA														DATE: Jun	13 2023
Start Time	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Numbe
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pace
06:00 PM	0	0	0	0	0	0	0	0	1	6	10	2	0	0	19	56-65	16
06:15 PM	0	0	0	0	0	0	0	0	0	4	9	3	0	1	17	56-65	13
06:30 PM	0	0	0	0	0	0	0	2	1	2	6	2	0	0	13	58-67	8
06:45 PM	0	0	0	0	0	0	0	0	1	3	3	1	1	1	10	56-65	6
07:00 PM	0	0	0	0	0	0	0	0	2	3	5	1	1	2	14	56-65	8
07:15 PM	0	0	0	0	0	0	0	1	0	3	4	1	1	0	10	56-65	7
07:30 PM	0	0	0	0	0	0	1	2	3	4	3	0	0	0	13	51-60	7
07:45 PM	0	0	0	0	0	0	0	0	2	5	3	0	1	0	11	56-65	8
08:00 PM	0	0	0	0	0	0	0	0	1	2	3	2	0	0	8	58-67	5
08:15 PM	0	0	0	0	0	0	0	1	4	3	5	1	0	2	16	56-65	8
08:30 PM	0	0	0	0	0	0	0	0	4	1	5	0	1	0	11	56-65	6
08:45 PM	0	0	0	0	0	0	0	0	1	2	2	1	0	0	6	56-65	4
09:00 PM	0	0	0	0	0	0	0	0	2	2	1	0	0	0	5	51-60	4
09:15 PM	0	0	0	0	0	0	0	0	2	1	2	0	1	0	6	51-60	3
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	2	2	1	0	1	0	6	51-60	4
10:00 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
10:15 PM	0	0	0	0	0	0	0	0	2	1	1	0	0	0	4	51-60	3
10:30 PM	0	0	0	0	0	0	0	0	0	3	1	0	0	0	4	56-65	4
10:45 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
11:00 PM	0	0	0	0	0	0	0	0	_ 1	0	0	0	0	0	1	46-55	1
11:15 PM	0	0	0	0	0	0	0	3	1	2	1	2	0	0	9	46-55	4
11:30 PM	0	0	0	0	0	0	0	0	2	1	3	0	0	0	6	56-65	4
11:45 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	2	4	41-50	1
Day Total	1	3	4	3	6	6	5	42	161	416	503	152	41	23	1266	FC CF	010
Percent	0.1%	0.2%	0.3%	0.2%	0.4%	0.4%	0.4%	3.1%	11.8%	30.5%	36.8%	11.1%	3%	1.7%	1366	56-65	919
AM Peak 15-min Vol	12:00 AM 0	5:30 AM 1	5:15 AM 1	12:00 AM 0	10:45 AM 2	10:15 AM 2	10:45 AM 2	10:45 AM 4	10:15 AM 8	7:30 AM 12	7:45 AM 12	7:30 AM 6	4:30 AM 2	6:15 AM 2	7:30 AM 28		
PM Peak 15-min Vol	1:30 PM 1	5:00 PM 2	2:30 PM 1	2:30 PM 3	3:00 PM 2	1:15 PM 3	2:00 PM 1	1:15 PM 3	12:15 PM 6	3:15 PM 12	4:15 PM 15	4:30 PM 7	12:45 PM 2	2:15 PM 2	2:30 PM 41		
Comments:																	

Type of report: Tube Count - Speed Data

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Rt	e 397 We	st of Nine	e Canyon	Rd												QC JOB	#: 16236665
SPECIFIC LOCA	ATION:															DIRECT	ION: EB, WB
CITY/STATE: B	enton, W	'A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Nullge	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 dec speed	Pace
Grand Total	1	3	4	3	6	6	5	42	161	416	503	152	41	23	1366	56-65	919
Percent	0.1%	0.2%	0.3%	0.2%	0.4%	0.4%	0.4%	3.1%	11.8%	30.5%	36.8%	11.1%	3%	1.7%	1300	30-03	919
Cumulative Percent	0.1%	0.3%	0.6%	0.8%	1.2%	1.7%	2%	5.1%	16.9%	47.4%	84.2%	95.3%	98.3%	100%			
ADT 1366															Mea	an Speed(Avera Med	ntile: 65 MPH age): 60 MPH dian: 60 MPH ode: 63 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236665 DIRECTION: EB, WB
DATE: Jun 13 2023

CITY/STATE: Be Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 202 <b>Total</b>
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	3
12:15 AM	0	4	0	0	0	0	0	1	0	1	0	0	0	0	6
12:30 AM	0	1	1	1	0	0	0	1	0	0	0	0	0	0	4
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	2	0	0	0	0	0	1	0	0	0	0	0	0	3
01:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	2	0	0	1	0	0	1	0	0	0	0	0	0	4
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	2
03:15 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
04:00 AM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
04:15 AM	0	4	5	0	3	0	0	0	0	0	0	0	0	0	12
04:30 AM	0	12	11	0	0	0	0	0	0	0	0	0	0	0	23
04:45 AM	0	12	8	0	3	0	0	1	0	0	0	0	0	0	24
05:00 AM	0	9	6	0	0	0	0	0	0	0	0	0	0	0	15
05:15 AM	1	2	6	0	4	0	0	0	0	0	0	0	0	0	13
05:30 AM	0	7	5	1	1	0	0	1	0	0	0	0	0	0	15
05:45 AM	0	4	8	0	2	0	0	2	0	0	0	0	0	0	16
Day Total															
Percent				DAIA	A I HA	AL LA	RIVE	566	IIVIIVI	UIVIII	IES				
ADT															
1366															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
											COLIDCE: O				

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB, WB

CITY/STATE: Be	inton, WA	Cars &	2 Avls		2 Axle 6	3 Axle	A Avla	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 AxI		un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle	Buses			4 Axle		5 Axie Double	>6 AXI Double	<6 AXI Multi	6 Axie Multi	>6 AXI Multi	Not Classed	Total
			Long		Tire	Single	Single	Double							
06:00 AM	0	2	2	0	3	0	0	2	0	0	0	0	0	0	9
06:15 AM	0	5	6	1	3	0	0	2	0	0	0	0	0	0	17
06:30 AM	0	5	9	1	3	0	0	1	0	0	0	0	0	0	19
06:45 AM	0	8	3	0	1	0	0	2	0	0	0	0	0	0	14
07:00 AM	0	9	4	1	3	0	0	4	0	0	0	0	0	0	21
07:15 AM	0	6	9	0	2	0	0	1	0	0	0	0	0	0	18
07:30 AM	0	15	8	2	1	0	0	2	0	0	0	0	0	0	28
07:45 AM	0	10	8	0	4	0	0	1	0	0	0	0	0	0	23
08:00 AM	0	4	10	3	2	0	0	0	0	0	0	0	0	0	19
08:15 AM	0	5	6	2	3	0	0	2	0	0	0	0	0	0	18
08:30 AM	0	6	3	0	2	0	0	0	0	0	0	0	0	0	11
08:45 AM	0	9	7	2	5	0	0	1	0	0	0	0	0	0	24
09:00 AM	0	6	8	0	4	0	0	3	0	0	0	0	0	0	21
09:15 AM	0	4	5	1	0	0	0	2	0	0	0	0	0	0	12
09:30 AM	0	5	1	0	3	0	0	1	0	0	0	0	0	0	10
09:45 AM	0	2	7	0	7	0	0	2	0	0	0	0	0	0	18
10:00 AM	0	2	6	1	3	0	0	2	0	0	0	0	0	0	14
10:15 AM	0	9	6	3	3	0	0	3	1	0	0	0	0	0	25
10:30 AM	0	8	7	4	4	0	0	2	0	0	0	0	0	0	25
10:45 AM	0	6	6	1	4	0	0	3	0	0	0	0	0	0	20
11:00 AM	0	6	8	1	5 2	0	0	2	0	0	0	0	0	0	22
11:15 AM	0	4	5	0		0	0	1	0	0	0	0	0	0	12
11:30 AM	0	2	8	0	4	0	0	1	0	0	0	0	0	0	15
11:45 AM	0	7	4	0	8	0	0	3	0	0	0	0	0	0	22
Day Total															
Percent				DAIZ	1 IHA		RIVE	566	IMIM	UNIT	11-5				
ADT															
1366															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
					_					_					

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236665 DIRECTION: EB, WB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA													DATE: J	un 13 2023
Chart Time -	Dilene	Cars &	2 Axle	Duna	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	Bikes	Trailers	Long	Buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 PM	0	4	6	0	1	0	0	5	0	0	0	0	0	0	16
12:15 PM	0	9	9	0	0	0	0	3	0	0	0	0	0	0	21
12:30 PM	0	10	3	3	3	0	0	2	0	0	0	0	0	0	21
12:45 PM	0	9	6	2	1	0	0	2	0	0	0	0	0	0	20
01:00 PM	0	8	5	0	7	0	0	2	0	0	0	0	0	0	22
01:15 PM	0	16	7	3	2	0	0	0	0	0	0	0	0	0	28
01:30 PM	0	10	3	2	7	0	0	3	0	0	0	0	0	0	25
01:45 PM	0	10	6	0	4	0	0	3	0	0	0	0	0	0	23
02:00 PM	0	7	11	0	0	0	0	3	0	0	0	0	0	0	21
02:15 PM	0	9	12	0	6	0	0	1	0	0	0	0	0	0	28
02:30 PM	0	16	13	1	8	0	0	3	0	0	0	0	0	0	41
02:45 PM	0	8	14	1	3	0	0	2	0	0	0	0	0	0	28
03:00 PM	0	4	8	3	6	0	0	4	0	0	0	0	0	0	25
03:15 PM	0	9	8	2	7	0	0	4	0	0	0	0	0	0	30
03:30 PM	0	7	9	2	5	0	0	2	0	0	0	0	0	0	25
03:45 PM	0	11	9	2	3	0	0	4	0	0	0	0	0	0	29
04:00 PM	0	8	9	1	3	0	0	7	0	0	0	0	0	0	28
04:15 PM	0	13	10	1	5	0	0	4	0	0	0	0	0	0	33
04:30 PM	0	6	10	0	5	0	0	2	0	0	0	0	0	0	23
04:45 PM	0	8	10	0	2	0	0	1	0	0	0	0	0	0	21
05:00 PM	0	7	6	2	5	0	0	1	0	0	0	0	0	0	21
05:15 PM	0	6	7	0	3	0	0	1	0	0	0	0	0	0	17
05:30 PM	0	10	8	0	0	0	0	0	0	0	0	0	0	0	18
05:45 PM	0	10	6	0	2	0	0	0	0	0	0	0	0	0	18
Day Total															
Percent				DATA	IHA		RIVF	500	IMM	UNIT	11-5				
ADT															
1366															
1000															
ANADaala															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB, WB

	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	10	6	1	2	0	0	0	0	0	0	0	0	0	19
06:15 PM	0	7	6	0	2	0	0	2	0	0	0	0	0	0	17
06:30 PM	0	7	3	0	3	0	0	0	0	0	0	0	0	0	13
06:45 PM	0	5	3	0	2	0	0	0	0	0	0	0	0	0	10
07:00 PM	0	5	7	0	0	0	0	2	0	0	0	0	0	0	14
07:15 PM	0	4	2	0	4	0	0	0	0	0	0	0	0	0	10
07:30 PM	0	4	3	2	1	0	0	2	1	0	0	0	0	0	13
07:45 PM	0	9	1	0	1	0	0	0	0	0	0	0	0	0	11
08:00 PM	0	3	2	0	1	0	0	2	0	0	0	0	0	0	8
08:15 PM	0	8	3	0	3	0	0	2	0	0	0	0	0	0	16
08:30 PM	0	8	2	0	1	0	0	0	0	0	0	0	0	0	11
08:45 PM	0	1	3	0	2	0	0	0	0	0	0	0	0	0	6
09:00 PM	0	2	1	0	2	0	0	0	0	0	0	0	0	0	5
09:15 PM	0	4	0	0	1	0	0	1	0	0	0	0	0	0	6
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	2	2	0	0	0	0	2	0	0	0	0	0	0	6
10:00 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
10:15 PM	0	1	2	0	0	0	0	1	0	0	0	0	0	0	4
10:30 PM	0	1	2	1	0	0	0	0	0	0	0	0	0	0	4
10:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
11:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
11:15 PM	0	4	3	0	1	0	0	1	0	0	0	0	0	0	9
11:30 PM	0	2	1	1	2	0	0	0	0	0	0	0	0	0	6
11:45 PM	0	0	2	0	1	0	0	1	0	0	0	0	0	0	4
Day Total	1	515	449	58	214	0	0	126	2	1	0	0	0	0	1366
Percent	0.1%	37.7%	32.9%	4.2%	15.7%	0%	0%	9.2%	0.1%	0.1%	0%	0%	0%	0%	1300
ADT 1366															
AM Peak	5:15 AM	7:30 AM	4:30 AM	10:30 AM	11:45 AM	12:00 AM	12:00 AM	7:00 AM	10:15 AM	12:15 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	7:30 AN
15-min Vol	1	15	11	4	8	0	0	4	1	1	0	0	0	0	28
PM Peak	12:00 PM	1:15 PM	2:45 PM	12:30 PM	2:30 PM	12:00 PM	12:00 PM	4:00 PM	7:30 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	2:30 PM
15-min Vol	0	16	14	3	8	0	0	7	1	0	0	0	0	0	41
omments:															

LOCATION: Rte 397 West of Nine Canyon Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023

	,														
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	1 0.1%	515 37.7%	449 32.9%	58 4.2%	214 15.7%	0 0%	0 0%	126 9.2%	2 0.1%	1 0.1%	0 0%	0 0%	0 0%	0 0%	1366
ADT 1366															
Comments:		·	·			1 7	·							·	·

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB, WB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		3				3			3	
12:15 AM		6				6			6	
12:30 AM		4				4			4	
12:45 AM		0				0			0	
01:00 AM		3				3			3	
01:15 AM		1				1			1	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		4				4	<b>\</b>		4	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		2				2	7		2	
03:15 AM		2				2			2	
03:30 AM		1				1			1	
03:45 AM		1				1			1	
04:00 AM		4				4			4	
04:15 AM		12				12			12	
04:30 AM		23			311'	23		In'	23	
04:45 AM		24			261	24		411	24	
05:00 AM		15				15			15	
05:15 AM		13				13			13	
05:30 AM		15			HALL	15	OMIN	UNII	15	
05:45 AM		16				16			16	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

SPECIFIC LOCATION:

**DIRECTION:** EB, WB CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		9				9			9	
06:15 AM		17				17			17	
06:30 AM		19				19			19	
06:45 AM		14				14			14	
07:00 AM		21				21			21	
07:15 AM		18				18			18	
07:30 AM		28				28			28	
07:45 AM		23				23			23	
08:00 AM		19				19			19	
08:15 AM		18				18			18	
08:30 AM		11				11			11	
08:45 AM		24				24			24	
09:00 AM		21				21			21	
09:15 AM		12				12			12	
09:30 AM		10				10			10	
09:45 AM		18				18			18	
10:00 AM		14				14			14	
10:15 AM		25				25			25	
10:30 AM		25				25			25	
10:45 AM		20				20			20	
11:00 AM		22				22			22	
11:15 AM		12				12	00 70 7		12	
11:30 AM		15				15	DIVIIVI		15	
11:45 AM		22				22			22	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

QC JOB #: 16236665

SPECIFIC LOCATION:

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM	16				16			16	
12:15 PM	21				21			21	
12:30 PM	21				21			21	
12:45 PM	20				20			20	
01:00 PM	22				22			22	
01:15 PM	28				28			28	
01:30 PM	25				25			25	
01:45 PM	23				23			23	
02:00 PM	21				21			21	
02:15 PM	28				28			28	
02:30 PM	41				41			41	
02:45 PM	28				28			28	
03:00 PM	25				25			25	
03:15 PM	30				30			30	
03:30 PM	25				25			25	
03:45 PM	29				29			29	
04:00 PM	28				28			28	
04:15 PM	33				33			33	
04:30 PM	23				23			23	
04:45 PM	21				21			21	
05:00 PM	21				21			21	
05:15 PM	17				17 18	00.000		17	
05:30 PM	18				18	DIVIIVI		18	
05:45 PM	18				18			18	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak									
15-min Vol									
PM Peak									
15-min Vol									
Comments:									

QC JOB #: 16236665

**DIRECTION:** EB, WB

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: EB, WB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		19				19			19	
06:15 PM		17				17			17	
06:30 PM		13				13			13	
06:45 PM		10				10			10	
07:00 PM		14				14			14	
07:15 PM		10				10			10	
07:30 PM		13				13			13	
07:45 PM		11				11			11	
08:00 PM		8				8			8	
08:15 PM		16				16			16	
08:30 PM		11				11			11	
08:45 PM		6				6			6	
09:00 PM		5				5			5	
09:15 PM		6				6			6	
09:30 PM		0				0			0	
09:45 PM		6				6			6	
10:00 PM		3				3			3	
10:15 PM		4				4			4	
10:30 PM		4				4		In	4	
10:45 PM		2				2			2	
11:00 PM		1				1			1	
11:15 PM		9				9			9	
11:30 PM		6				9 6	OMN	UNIT	6	
11:45 PM		4				4			4	
Day Total		1366				1366			1366	
% Weekday Average		100%								
% Week Average		100%				100%				
AM Peak		7:30 AM				7:30 AM			7:30 AM	
15-min Vol		28				28			28	
PM Peak		2:30 PM				2:30 PM			2:30 PM	
15-min Vol		41				41			41	

CITY/STATE:	1	16	21	26	31	36	41	46	51	56	61	66	71	76		DATE: Jun	Numb
Start Time	1 15	20	25	30	35	40	41 45	50	55	60	65	70	71 75	999	Total	Pace Speed	in Pac
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2	46-55	2
12:30 AM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
01:15 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	1	0	2	0	0	0	3	56-65	2
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	61-70	2
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	1	2	0	0	3	61-70	3
04:30 AM	0	0	0	0	0	0	0	0	0	0	3	1	0	0	4	61-70	4
04:45 AM	0	0	0	0	0	0	0	0	1	1	1	2	1	0	6	66-75	3
05:00 AM	0	0	0	0	0	0	0	0	1	1	7	2	0	0	11	61-70	9
05:15 AM	0	0	0	0	0	0	0	0	0	1	4	1	1	0	7	60-69	5
05:30 AM	0	1	0	0	0	0	0	0	1	4	0	0	0	0	6	51-60	5
05:45 AM	0	0	0	0	0	0	0	0	1	5	2	2	0	0	10	56-65	7
Day Total														-	10	30-03	,
Percent														-5			
AM Peak																	
L5-min Vol																	
PM Peak L5-min Vol																	

CITY/STATE:																DATE: Jur	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pac
06:00 AM	0	0	0	0	0	0	0	0	2	3	1	0	1	0	7	51-60	5
06:15 AM	0	0	0	0	0	0	0	0	0	1	5	1	0	2	9	59-68	6
06:30 AM	0	0	0	0	0	0	0	0	1	3	4	0	1	0	9	56-65	7
06:45 AM	0	0	0	0	0	0	0	0	0	4	3	1	0	0	8	56-65	7
07:00 AM	0	0	0	0	0	0	0	0	3	4	6	2	0	0	15	56-65	10
07:15 AM	0	0	0	0	0	0	0	0	0	2	5	0	1	0	8	56-65	7
07:30 AM	0	0	0	0	0	0	0	1	0	4	5	4	0	0	14	58-67	9
07:45 AM	0	0	0	0	0	0	0	0	0	2	6	1	0	2	11	56-65	8
08:00 AM	0	0	0	0	0	0	0	0	1	4	2	2	1	0	10	56-65	6
08:15 AM	0	0	0	0	0	0	0	0	2	2	6	3	0	0	13	61-70	9
08:30 AM	0	0	0	0	0	0	0	0	0	3	2	0	0	0	5	56-65	5
08:45 AM	0	0	0	0	0	0	0	0	2	3	8	0	0	0	13	56-65	11
09:00 AM	0	0	0	0	0	0	0	0	1	4	7	1	1	0	14	56-65	11
09:15 AM	0	0	0	0	0	0	0	0	2	3	1	1	0	0	7	51-60	5
09:30 AM	0	0	0	0	0	0	0	0	2	3	1	1	0	0	7	51-60	5
09:45 AM	0	0	0	0	0	0	0	0	2	2	5	2	1	0	12	60-69	7
10:00 AM	0	0	0	0	0	0	0	1	0	2	1	4	0	0	8	61-70	5
10:15 AM	0	0	0	0	0	2	0	0	6	3	2	0	1	0	14	51-60	9
10:30 AM	0	0	0	0	1	0	0	1	4	2	6	0	0	1	15	56-65	8
10:45 AM	0	0	0	0	2	0	2	4	1	2	2	0	0	1	14	41-50	6
11:00 AM	0	0	0	0	0	0	0	0	0	5	5	1	0	1	12	56-65	10
11:15 AM	0	0	0	0	0	0	0	0	1	3	2	0	1	0	7	56-65	5
11:30 AM	0	0	0	0	0	0	0	0	1	0	4	1	0	0	6	61-70	5
11:45 AM	0	0	0	0	0	0	0	0	2	1	6	1	0	1	11	56-65	7
Day Total			-	0	- 0	-	- 0	0			0		-			30-03	,
Percent														=5			
AM Peak L5-min Vol																	
PM Peak 5-min Vol																	

QC JOB #: 16236665 LOCATION: Rte 397 West of Nine Canyon Rd **SPECIFIC LOCATION: DIRECTION: WB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 56-65 12:15 PM 56-65 56-65 12:30 PM 12:45 PM 56-65 01:00 PM 56-65 01:15 PM 56-65 56-65 01:30 PM 01:45 PM 56-65 02:00 PM O 56-65 02:15 PM 56-65 02:30 PM 56-65 02:45 PM 56-65 03:00 PM 61-70 03:15 PM 56-65 03:30 PM 56-65 03:45 PM 60-69 04:00 PM 56-65 04:15 PM 56-65 04:30 PM O 61-70 04:45 PM 61-70 05:00 PM 56-65 05:15 PM 56-65 05:30 PM 55-64 05:45 PM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

LOCATION: Rte 397 West of Nine Canyon Rd QC JOB #: 16236665 SPECIFIC LOCATION: **DIRECTION: WB** 

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numbo in Pac
06:00 PM	0	0	0	0	0	0	0	0	0	2	5	1	0	0	8	56-65	7
06:15 PM	0	0	0	0	0	0	0	0	0	3	5	1	0	0	9	56-65	8
06:30 PM	0	0	0	0	0	0	0	1	1	1	4	1	0	0	8	60-69	5
06:45 PM	0	0	0	0	0	0	0	0	0	1	2	1	1	1	6	61-70	3
07:00 PM	0	0	0	0	0	0	0	0	0	2	1	1	1	2	7	56-65	3
07:15 PM	0	0	0	0	0	0	0	1	0	1	2	0	1	0	5	56-65	3
07:30 PM	0	0	0	0	0	0	1	2	2	3	2	0	0	0	10	53-62	5
07:45 PM	0	0	0	0	0	0	0	0	2	3	2	0	0	0	7	53-62	5
08:00 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	2	61-70	2
08:15 PM	0	0	0	0	0	0	0	1	2	1	1	0	0	2	7	51-60	3
08:30 PM	0	0	0	0	0	0	0	0	1	0	3	0	1	0	5	56-65	3
08:45 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	2	61-70	2
09:00 PM	0	0	0	0	0	0	0	0	1	2	0	0	0	0	3	51-60	3
09:15 PM	0	0	0	0	0	0	0	0	1	0	1	0	1	0	3	46-55	1
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:45 PM	0	0	0	0	0	0	0	0	2	1	1	0	1	0	5	51-60	3
10:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
10:15 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	2	46-55	1
10:30 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
10:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
11:00 PM	0	0	0	0	0	0	0	0	_ 1	0	0	0	0	0	1	46-55	1
11:15 PM	0	0	0	0	0	0	0	1	0	1	1	2	0	0	5	61-70	3
11:30 PM	0	0	0	0	0	0	0	0	2	0	2	0	0	0	4	46-55	2
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	1	1	1	0	5	5	3	25	80	184	274	80	21	16	-		
Percent	0.1%	0.1%	0.1%	0%	0.7%	0.7%	0.4%	3.6%	11.5%	26.4%	39.4%	11.5%	3%	2.3%	696	56-65	458
AM Peak	12:00 AM 0			12:00 AM						5:45 AM	8:45 AM	7:30 AM	4:45 AM	6:15 AM	7:00 AM		
15-min Vol		1	0	0	2	2	2	4	6	5	8	4	1	2	15		
PM Peak 15-min Vol	1:30 PM 1	12:00 PM 0	4:15 PM 1	12:00 PM 0	3:00 PM 2	1:15 PM 3	7:30 PM 1	1:15 PM 3	1:00 PM 2	1:45 PM 7	2:15 PM 9	4:30 PM 4	3:45 PM 2	7:00 PM 2	1:15 PM 19		

Type of report: Tube Count - Speed Data

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Rte	e 397 We	st of Nine	e Canyon	Rd												QC JOB	#: 16236665
SPECIFIC LOCA	TION:															DIR	ECTION: WB
CITY/STATE: Be	enton, W	<b>/</b> A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
speed hange	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	Pace
Grand Total	1	1	1	0	5	5	3	25	80	184	274	80	21	16	696	56-65	458
Percent	0.1%	0.1%	0.1%	0%	0.7%	0.7%	0.4%	3.6%	11.5%	26.4%	39.4%	11.5%	3%	2.3%	090	30-03	436
Cumulative Percent	0.1%	0.3%	0.4%	0.4%	1.1%	1.9%	2.3%	5.9%	17.4%	43.8%	83.2%	94.7%	97.7%	100%			
ADT 696															Me	an Speed(Avera Med	ntile: 65 MPH nge): 60 MPH lian: 60 MPH ode: 63 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: WB

CITY/STATE: Be	nton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Time	bikes	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	Total
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
12:30 AM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:15 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	1	0	0	1	0	0	1	0	0	0	0	0	0	3
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	2
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	2	0	1	0	0	0	0	0	0	0	0	0	3
04:30 AM	0	2	2	0	0	0	0	0	0	0	0	0	0	0	4
04:45 AM	0	1	2	0	2	0	0	1	0	0	0	0	0	0	6
05:00 AM	0	6	5	0	0	0	0	0	0	0	0	0	0	0	11
05:15 AM	0	0	4	0	3	0	0	0	0	0	0	0	0	0	7
05:30 AM	0	3	1	0	1	0	0	1	0	0	0	0	0	0	6
05:45 AM	0	3	4	0	1	0	0	2	0	0	0	0	0	0	10
Day Total															
Percent				DATA	ALHA		RIVF	566	IMM	UNIT	11-5				
ADT															
696															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: WB

Octoor   Start   Important   Start   Trailers   Long   Start   Long   Start   Long	CITY/STATE: Be	IIIOII, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 2023
06:00 AM	Start Time	Bikes			Buses											Total
06:15 AM	06:00 ANA	0			0											7
96:30 AM																
0645 AM															_	
07:00 AM										-				-	_	
07:15 AM			=							•				ŭ	_	
07:30 AM				-	_				-	Ū	_			-	-	
07:45 AM				•	Ū	_	_		_	-	•	_	-	·	•	
08:00 AM						_				•	_				_	
08:15 AM				•						_	_			-		
08:30 AM				_	_					_	-			•	_	
08:45 AM				•		•					_			-	_	
09:00 AM					-	•				U	•			•	•	
09:15 AM						-				0				-	_	
09:30 AM				_						0	_			-	_	
09:45 AM				3 1	_	-				0	•			•	-	
10:00 AM				Ε	Ū	-				Ū	•	-		•	_	_
10:15 AM				-	-	•					_			-	_	
10:30 AM				•	_									-	_	
10:45 AM				_						0	_			•	-	
11:00 AM			_	•	<del>-</del>	-				0	•	-	-	•	-	
11:30 AM			•							·	_			·	•	
11:30 AM						1	0							-	_	
11:45 AM														_	_	
ADT 696  AM Peak 15-min Vol  PM Peak 15-min Vol  AM Peak 15-min Vol  AM Peak 15-min Vol  PM Peak 15-min Vol															_	
ADT 696  AM Peak 15-min Vol PM Peak 15-min Vol PM Peak 15-min Vol		-		<u> </u>	0	<del></del>	U			U	-	U	- 0	U	0	11
ADT 696  AM Peak 15-min Vol PM Peak 15-min Vol																
AM Peak 15-min Vol PM Peak 15-min Vol  Tomin Vol  Tomin Vol Tomin	1 01 00110				7,241,2	1 11 10		UVL		71 011 01	CHULL					
15-min Vol  PM Peak 15-min Vol																
PM Peak 15-min Vol																
mments:																
	omments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: WB

CITY/STATE: Be	inton, WA		241		241.5	2.4.1						641			un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 PM	0	2	3	0	0	0	0	2	0	0	0	0	0	0	7
12:15 PM	0	0	3	0	0	0	0	3	0	0	0	0	0	0	6
12:30 PM	0	4	1	0	2	0	0	1	0	0	0	0	0	0	8
12:45 PM	0	8	5	0	0	0	0	0	0	0	0	0	0	0	13
01:00 PM	0	4	2	0	3	0	0	1	0	0	0	0	0	0	10
01:15 PM	0	11	5	1	2	0	0	0	0	0	0	0	0	0	19
01:30 PM	0	7	2	2	5	0	0	0	0	0	0	0	0	0	16
01:45 PM	0	9	5	0	2	0	0	2	0	0	0	0	0	0	18
02:00 PM	0	4	6	0	0	0	0	0	0	0	0	0	0	0	10
02:15 PM	0	4	7	0	4	0	0	1	0	0	0	0	0	0	16
02:30 PM	0	9	5	0	3	0	0	2	0	0	0	0	0	0	19
02:45 PM	0	6	8	0	2	0	0	2	0	0	0	0	0	0	18
03:00 PM	0	2	3	1	3	0	0	3	0	0	0	0	0	0	12
03:15 PM	0	2	4	0	1	0	0	2	0	0	0	0	0	0	9
03:30 PM	0	3	7	1	2	0	0	1	0	0	0	0	0	0	14
03:45 PM	0	4	3	2	2	0	0	1	0	0	0	0	0	0	12
04:00 PM	0	4	3	0	1	0	0	6	0	0	0	0	0	0	14
04:15 PM	0	7	3	0	2	0	0	1	0	0	0	0	0	0	13
04:30 PM	0	3	6	0	1	0	0	1	0	0	0	0	0	0	11
04:45 PM	0	4	7	0	1	0	0	1	0	0	0	0	0	0	13
05:00 PM	0	5	1	0	3 1	0	0	0	0	0	0	0	0	0	9
05:15 PM	0	2	0	0		0	0	0	0	0	0	0	0	0	3
05:30 PM	0	5	1	0	0	0	0	0	0	0	0	0	0	0	6
05:45 PM	0	6	1	0	1	0	0	0	0	0	0	0	0	0	8
Day Total															
Percent				DAIA	ALHA		RIVF	500	MM	UNIT	1-5				
ADT															
696															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: WB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	3	3	1	1	0	0	0	0	0	0	0	0	0	8
06:00 PM 06:15 PM	0	3	2	0	2	0	0	2	0	0	0	0	0	0	9
06:30 PM	0	5 5	1	0	2	0	0	0	0	0	0	0	0	0	8
06:45 PM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
07:00 PM	0	1	5	0	0	0	0	1	0	0	0	0	0	0	7
07:15 PM	0	1	1	0	3	0	0	0	0	0	0	0	0	0	5
07:30 PM	0	2	2	2	1	0	0	2	1	0	0	0	0	0	10
07:45 PM	0	6	1	0	0	0	0	0	0	0	0	0	0	0	7
08:00 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
08:15 PM	0	3	1	0	2	0	0	1	0	0	0	0	0	0	7
08:30 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
08:45 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
09:00 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
09:15 PM	0	1	0	0	1	0	0	1	0	0	0	0	0	0	3
09:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 PM	0	1	2	0	0	0	0	2	0	0	0	0	0	0	5
10:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
10:15 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
10:30 PM	0	1	1	1	0	0	0	0	0	0	0	0	0	0	3
10:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
11:15 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
11:30 PM	0	1	1	1	1	0	0	0	0	0	0	0	0	0	4
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	251	233	25	112	0	0	73	2	0	0	0	0	0	606
Percent	0%	36.1%	33.5%	3.6%	16.1%	0%	0%	10.5%	0.3%	0%	0%	0%	0%	0%	696
ADT 696															
AM Peak	12:00 AM	7:00 AM	8:00 AM	8:15 AM	8:45 AM	12:00 AM	12:00 AM	7:00 AM	10:15 AM				12:00 AM		7:00 AN
15-min Vol	0	7	8	2	4	0	0	4	1	0	0	0	0	0	15
PM Peak	12:00 PM	1:15 PM	2:45 PM	1:30 PM	1:30 PM	12:00 PM		4:00 PM	7:30 PM						1:15 PN
15-min Vol Comments:	0	11	8	2	5	0	0	6	1	0	0	0	0	0	19

LOCATION: Rte 397 West of Nine Canyon Rd

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023

CITY/STATE: Bei	itori, WA													DATE:	Jun 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	0 0%	251 36.1%	233 33.5%	25 3.6%	112 16.1%	0 0%	0 0%	73 10.5%	2 0.3%	0 0%	0 0%	0 0%	0 0%	0 0%	696
ADT 696															
Comments:	•		•	•		1 /				•	•	•		•	

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236665 DIRECTION: WB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		2				2			2	
12:30 AM		2				2			2	
12:45 AM		0				0			0	
01:00 AM		1				1			1	
01:15 AM		1				1			1	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		3				3			3	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		2				2			2	
03:15 AM		0				0			0	
03:30 AM		1				1			1	
03:45 AM		0				0			0	
04:00 AM		0				0			0	
04:15 AM		3				3			3	
04:30 AM		4			311'	4		In	4	
04:45 AM		6			<b>a</b> L I	6	-	411	6	
05:00 AM		11				11			11	
05:15 AM		7				7	0.000		7	
05:30 AM		6			HALL	6	DIVIN	UNII	6	
05:45 AM		10				10			10	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023

Thu Fri Average Weekday Mon Tue Wed Sat Sun Average Week **Average Week Profile Start Time** 15-min Traffic 13 Jun 23 15-min Traffic 06:00 AM 7 06:15 AM 9 9 9 06:30 AM 9 9 9 06:45 AM 8 8 8 07:00 AM 15 15 15 8 8 07:15 AM 8 07:30 AM 14 14 14 07:45 AM 11 11 11 08:00 AM 10 10 10 08:15 AM 13 13 13 08:30 AM 5 5 5 08:45 AM 13 13 13 09:00 AM 14 14 14 7 7 09:15 AM 7 7 7 7 09:30 AM 09:45 AM 12 12 12 10:00 AM 8 8 8 10:15 AM 14 14 14 15 10:30 AM 15 15 14 10:45 AM 14 14 12 11:00 AM 12 12 11:15 AM 7 7 11:30 AM 6 11:45 AM 11 11 11 Day Total % Weekday Average % Week Average AM Peak 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236665

**DIRECTION: WB** 

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236665 DIRECTION: WB

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM	7				7			7	
12:15 PM	6				6			6	
12:30 PM	8				8			8	
12:45 PM	13				13			13	
01:00 PM	10				10			10	
01:15 PM	19				19			19	
01:30 PM	16				16			16	
01:45 PM	18				18			18	
02:00 PM	10				10			10	
02:15 PM	16				16			16	
02:30 PM	19				19			19	
02:45 PM	18				18			18	
03:00 PM	12				12			12	
03:15 PM	9				9			9	
03:30 PM	14				14			14	
03:45 PM	12				12			12	
04:00 PM	14				14			14	
04:15 PM	13				13			13	
04:30 PM	11				11			11	
04:45 PM	13				13			13	
05:00 PM	9				9			9	
05:15 PM	3				3 6	00.000		3	
05:30 PM	6				6	JIVIIVI		6	
05:45 PM	8				8			8	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak									
15-min Vol									
PM Peak									
15-min Vol									
Comments:									

CITY/STATE: Benton, WA

LOCATION: Rte 397 West of Nine Canyon Rd QC JOB #: 16236665 SPECIFIC LOCATION: **DIRECTION: WB** 

**DATE:** Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		8				8			8	
06:15 PM		9				9			9	
06:30 PM		8				8			8	
06:45 PM		6				6			6	
07:00 PM		7				7			7	
07:15 PM		5				5			5	
07:30 PM		10				10			10	
07:45 PM		7				7			7	
08:00 PM		2				2			2	
08:15 PM		7				7			7	
08:30 PM		5				5			5	
08:45 PM		2				2			2	
09:00 PM		3				3			3	
09:15 PM		3				3			3	
09:30 PM		0				0			0	
09:45 PM		5				5			5	
10:00 PM		1				1			1	
10:15 PM		2				2			2	
10:30 PM		3			3     '	3		In.	3	
10:45 PM		1				1		41 I	1	
11:00 PM		1				1			1	
11:15 PM		5			7 I A T 1	5 4 5	00.00.0	17 18 117	5	
11:30 PM		4			HALL	4 5 (	DIVIN	UNII	4	
11:45 PM		0				0			0	
Day Total		696				696			696	
% Weekday		100%								
Average		100%								
% Week		100%				100%				
Average										
AM Peak		7:00 AM				7:00 AM			7:00 AM	
15-min Vol		15				15			15	
PM Peak		1:15 PM				1:15 PM			1:15 PM	
15-min Vol		19				19			19	
Comments:										

CITY/STATE:			21	26	31	36	41	46	51	56	61	66	71	76		DATE: Jun	Numbe
Start Time	1 15	16 20	25	30	35	40	41 45	50	55	60	65	70	71 75	999	Total	Pace Speed	in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2	71-80	1
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
Day Total	0	-			-			-	-						0	1-10	
Percent				DA	TA	$TH_{\nu}$	$\Delta T I$	ORIN	/ES	CO	MN	1UN	IITIE	-5			
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	

LOCATION: I		yon Rd so	outh of F	Rte 397												QC JOB #: 1	
SPECIFIC LO																	TION: NB
CITY/STATE:	Benton,	WA														DATE: Jur	13 2023
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
07:30 AM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	2	0	1	0	3	56-65	2
08:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
08:45 AM	0	2	0	0	0	0	0	0	0	3	0	0	0	0	5	51-60	3
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
10:15 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	1	3	51-60	1
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
10:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
11:00 AM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
11:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	56-65	1
Day Total					TA	711	A 77 7	SOA	ITC		A 7 A	#1 1K	IITII				
Percent				UF	MA	IH	411	JKI	/E3	CC	IVIIV	U					
AM Peak																	
15-min Vol																	
PM Peak																	
15-min Vol																	
Comments:																	
enort generati	od on 6/20	0/2022 7.5	1 1 1 1										COLIDO	E. Quality	Counts IIC/ht	tn·//www.auality	(counts no

QC JOB #: 16236666 LOCATION: Nine Canyon Rd south of Rte 397 **SPECIFIC LOCATION: DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 PM 51-60 12:15 PM 71-80 56-65 12:30 PM 12:45 PM 61-70 01:00 PM 66-75 01:15 PM 56-65 56-65 01:30 PM 01:45 PM 56-65 02:00 PM O 51-60 02:15 PM 61-70 02:30 PM 51-60 02:45 PM 56-65 03:00 PM 1-10 03:15 PM 56-65 03:30 PM 1-10 03:45 PM 51-60 56-65 04:00 PM 04:15 PM 56-65 04:30 PM O 51-60 04:45 PM 51-60 05:00 PM 61-70 05:15 PM 61-70 05:30 PM 66-75 05:45 PM 56-65 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236666 LOCATION: Nine Canyon Rd south of Rte 397 SPECIFIC LOCATION: **DIRECTION: NB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number Start Time Total Pace Speed in Pace 06:00 PM 51-60 06:15 PM 56-65 06:30 PM 1-10 06:45 PM 41-50 07:00 PM 1-10 07:15 PM 16-25 46-55 07:30 PM 07:45 PM 1-10 08:00 PM O O 1-10 08:15 PM 46-55 08:30 PM 56-65 08:45 PM 1-10 09:00 PM 1-10 09:15 PM O 46-55 09:30 PM 56-65 09:45 PM 46-55 10:00 PM 41-50 10:15 PM 1-10 10:30 PM O 1-10 10:45 PM 36-45 11:00 PM 36-45 11:15 PM 51-60 11:30 PM 1-10 11:45 PM 1-10 **Day Total** 56-65 0% 1.7% 0.8% 0% 0% 0.8% 1.7% 6.7% 10.8% 22.5% 25.8% 15% 8.3% 5.8% Percent AM Peak 12:00 AM 8:45 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 7:30 AM 12:00 AM 8:45 AM 7:45 AM 6:45 AM 5:15 AM 5:15 AM 8:45 AM 15-min Vol PM Peak 12:00 PM 12:00 PM 7:15 PM 12:00 PM 12:00 PM 1:15 PM 10:45 PM 12:15 PM 2:00 PM 4:45 PM 1:15 PM 5:00 PM 12:00 PM 12:15 PM 1:15 PM 15-min Vol

Comments:

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Nir	ne Canyo	n Rd sou	th of Rte 3	397												QC JOB	#: 16236666
SPECIFIC LOCA	TION:															DIF	RECTION: NB
CITY/STATE: Be	DATE: Jun 13 2023																
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Range	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	race speed	Pace
Grand Total	0	2	1	0	0	1	2	8	13	27	31	18	10	7	120	56-65	58
Percent	0%	1.7%	0.8%	0%	0%	0.8%	1.7%	6.7%	10.8%	22.5%	25.8%	15%	8.3%	5.8%	120	30-03	56
Cumulative Percent	0%	1.7%	2.5%	2.5%	2.5%	3.3%	5%	11.7%	22.5%	45%	70.8%	85.8%	94.2%	100%			
ADT 120												Me	an Speed(Avera Med	ntile: 69 MPH age): 61 MPH dian: 61 MPH ode: 63 MPH			
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236666 DIRECTION: NB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 AM	0	0	Long 0	0	0	o 0	Single 0	0	0	0	0	0	0	0	0
12:00 AM 12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AW 01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:15 AM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2
05:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total															
Percent															
ADT 120															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
	1 C/20/2/	222 7 52 484									COLIDER O	1:4	- IIC/L.	//	4 4

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
Start Hime	DIKES	Trailers	Long	buses	Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	iotai
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2
07:45 AM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
08:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	2	2	1	0	0	0	0	0	0	0	0	0	0	5
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
10:15 AM	0	0	1	2	0	0	0	0	0	0	0	0	0	0	3
10:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:45 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:00 AM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
11:15 AM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
11:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:45 AM	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
Day Total															
Percent				DAIL	AIHA		RIVE	200	IVIIVI	UIVIII	15				
ADT															
120															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton WA QC JOB #: 16236666 DIRECTION: NB
DATE: Jun 13 2023

CITY/STATE: Be	enton, WA														un 13 202
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:00 PM	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2
12:00 PM	0	0	2	0	1	0	0	0	0	0	0	0	0	0	3
12:30 PM	0	2	1	2	0	0	0	0	0	0	0	0	0	0	5
12:45 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
01:00 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
01:15 PM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	6
01:30 PM	0	1	0	0	2	0	0	0	0	0	0	0	0	0	3
01:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
02:00 PM	0	3	1	1	0	0	0	0	0	0	0	0	0	0	5
02:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
02:30 PM	0	1	1	0	0	0	0	1	0	0	0	0	0	0	3
02:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:00 PM	0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
04:15 PM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
04:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
04:45 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
05:00 PM	0	2	2	0	1	0	0	0	0	0	0	0	0	0	5
05:15 PM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	2
05:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
05:45 PM	0	2	4	0	0	0	0	0	0	0	0	0	0	0	6
Day Total															
Percent				DAIA	ALHA	ALD	RIVE	500	IVIIVI	UIVII	IES				
ADT															
120															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
mments:															
ort conorotoo	L == C /20 /2/	022 7.50 414									COLUDEE O	!:4 Caa4	- IIC/b	. / /	

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
OC-00 DN4	0			0											1
06:00 PM	0 0	0	1	0 0	0	0	0	0	0 0	0	0	0	0	0 0	1
06:15 PM	0	0 0	0	0	1 0	0 0	0 0	0	0	0	0 0	0 0	0	0	1
06:30 PM		-	0 0	0	0 1	0	0	•	0	0	0	0	0	·	0
06:45 PM 07:00 PM	0 0	0 0	0	0	0	0	0	0	0	0	0	0	0	0	1 0
07:00 PM 07:15 PM	0	0	1	0	0	0	0	0	0	1	0	0	0	0	2
07:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
07:30 PM 07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07.43 PM 08:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:00 PM 08:15 PM	0	3	0	0	1	0	0	0	0	0	0	0	0	0	4
08:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:30 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
09:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:00 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
11:00 PM	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2
11:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Day Total	0	45	36	11	25	0	0	2	0	1	0	0	0	0	
Percent	0%	37.5%	30%	9.2%	20.8%	0%	0%	1.7%	0%	0.8%	0%	0%	0%	0%	120
ADT 120															
AM Peak	12:00 AM	7:45 AM	8:45 AM	10:15 AM	11:45 AM	12:00 AM			12:00 AM		12:00 AM				8:45 AN
15-min Vol	0	2	2	2	2	0	0	1	0	0	0	0	0	0	5
PM Peak	12:00 PM	1:15 PM	5:45 PM	12:30 PM	1:30 PM	12:00 PM		2:30 PM	12:00 PM	7:15 PM	12:00 PM		12:00 PM		1:15 PM
15-min Vol	0	6	4	2	2	0	0	1	0	1	0	0	0	0	6

LOCATION: Nine Canyon Rd south of Rte 397

SPECIFIC LOCATION:

DIRECTION: NB
CITY/STATE: Repton, WA

DATE: Jun 13 2023

CITT/STATE: BEI	Itoli, WA													DATE	Juli 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	0	45	36	11	25	0	0	2	0	1	0	0	0	0	120
Percent	0%	37.5%	30%	9.2%	20.8%	0%	0%	1.7%	0%	0.8%	0%	0%	0%	0%	120
ADT 120															

Report generated on 6/20/2023 7:50 AM

Comments:

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

CITY/STATE: Benton, WA DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon Tue 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM	0				0			0	
12:15 AM	0				0			0	
12:30 AM	0				0			0	
12:45 AM	0				0			0	
01:00 AM	0				0			0	
01:15 AM	0				0			0	
01:30 AM	0				0			0	
01:45 AM	0				0			0	
02:00 AM	0				0			0	
02:15 AM	0				0			0	
02:30 AM	0				0			0	
02:45 AM	0				0			0	
03:00 AM	0				0			0	
03:15 AM	0				0			0	
03:30 AM	0				0			0	
03:45 AM	0				0			0	
04:00 AM	0				0			0	
04:15 AM	0				0			0	
04:30 AM	0				0			0	
04:45 AM	0				0			0	
05:00 AM	0				0			0	
05:15 AM	2				$\frac{2}{0}$ S C	00 40 4		2	
05:30 AM	0					DIVIIVI		/E5 0	
05:45 AM	0				0			0	
Day Total									
% Weekday									
Average									
% Week									
Average									
AM Peak									
15-min Vol									
PM Peak									
15-min Vol									
Comments:									

QC JOB #: 16236666

**DIRECTION: NB** 

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236666

**DIRECTION: NB** 

G600 AM	Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:30 AM	06:00 AM		0				0			0	
06:45 AM	06:15 AM		0				0			0	
07:00 AM	06:30 AM		0				0			0	
07:15 AM	06:45 AM		1				1			1	
07:30 AM	07:00 AM		0				0			0	
07:45 AM	07:15 AM		0				0			0	
08:00 AM	07:30 AM		2				2			2	
08:15 AM	07:45 AM		3				3			3	
08:30 AM	08:00 AM		1				1			1	
08:45 AM     5     5     5       09:00 AM     0     0     0       09:15 AM     0     0     0       09:30 AM     1     1     1       09:45 AM     0     0     0       10:00 AM     1     1     1       10:15 AM     3     3     3       10:30 AM     1     1     1       11:04 AM     1     1     1       11:15 AM     1     1     1       11:30 AM     1     1     1       11:45 AM     2     2     2       Day Total       % Week Average     **Week Average       AM Peak       15-min Vol	08:15 AM		0				0			0	
09:00 AM	08:30 AM		0				0			0	
09:15 AM	08:45 AM		5				5			5	
09:30 AM	09:00 AM		0				0			0	
09:45 AM	09:15 AM		0				0			0	
10:00 AM	09:30 AM		1				1			1	
10:15 AM	09:45 AM		0				0	-		0	
10:30 AM 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10:00 AM		1				1			1	
10:45 AM	10:15 AM		3							3	
11:00 AM	10:30 AM		1				1		In.	1	
11:15 AM	10:45 AM		1				L V 1	-	411	1	
11:45 AM         2         2         2           Day Total         % Weekday Average         AVERAGE </td <td>11:00 AM</td> <td></td> <td>2</td> <td></td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td>2</td> <td></td>	11:00 AM		2				2			2	
11:45 AM         2         2         2           Day Total         % Weekday Average         AVERAGE </td <td>11:15 AM</td> <td></td> <td>1</td> <td></td> <td></td> <td></td> <td>11</td> <td></td> <td></td> <td>1</td> <td></td>	11:15 AM		1				11			1	
11:45 AM         2         2         2           Day Total         % Weekday Average         AVERAGE </td <td>11:30 AM</td> <td></td> <td>1</td> <td></td> <td></td> <td></td> <td>DRIVES CO</td> <td>DIVIN</td> <td>UNII</td> <td>/ES 1</td> <td></td>	11:30 AM		1				DRIVES CO	DIVIN	UNII	/ES 1	
% Weekday Average % Week Average   AM Peak 15-min Vol PM Peak	11:45 AM		2				2			2	
Average         Meek         ————————————————————————————————————	Day Total										
% Week       Average	% Weekday										
Average	Average										
AM Peak 15-min Vol  PM Peak	% Week										
15-min Vol	Average										
PM Peak	AM Peak										
	15-min Vol										
15-min Vol											
	15-min Vol										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236666

**DIRECTION: NB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		2				2			2	
12:15 PM		3				3			3	
12:30 PM		5				5			5	
12:45 PM		3				3			3	
01:00 PM		5				5			5	
01:15 PM		6				6			6	
01:30 PM		3				3			3	
01:45 PM		1				1			1	
02:00 PM		5				5			5	
02:15 PM		1				1			1	
02:30 PM		3				3			3	
02:45 PM		1				1			1	
03:00 PM		0				0			0	
03:15 PM		4				4			4	
03:30 PM		0				0			0	
03:45 PM		1				1			1	
04:00 PM		4				4			4	
04:15 PM		6			_   °.	6			6	
04:30 PM		2				2	$\cap$	In:	2	
04:45 PM		3				3		<i>.</i> 411	3	
05:00 PM		5				5			5	
05:15 PM		2			TIATI	2 1 S C	CA AAA	11 18 117	2	
05:30 PM		1			HALL		JIVIIVI	UNII	1	
05:45 PM		6				6			6	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

**DATE:** Jun 13 2023 - Jun 13 2023

QC JOB #: 16236666

**DIRECTION: NB** 

CITY/STATE: Benton, WA

Start Time	Mon Tue 13 Jun 23	Wed Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	1			1			1	
06:15 PM	1			1			1	
06:30 PM	0			0			0	
06:45 PM	1			1			1	
07:00 PM	0			0			0	
07:15 PM	2			2			2	
07:30 PM	1			1			1	
07:45 PM	0			0			0	
08:00 PM	0			0			0	
08:15 PM	4			4			4	
08:30 PM	2			2			2	
08:45 PM	0			0			0	
09:00 PM	0			0			0	
09:15 PM	1			1			1	
09:30 PM	1			1			1	
09:45 PM	1			1			1	
10:00 PM	2			2			2	
10:15 PM	0			0			0	
10:30 PM	0			0			0	
10:45 PM	1			1			1	
11:00 PM	2			2			2	
11:15 PM	1				00.00.01		1	
11:30 PM	0			0	JIVIIVIL		0	
11:45 PM	0			0			0	
Day Total	120			120			120	
% Weekday Average	100%							
% Week Average	100%			100%				
AM Peak	8:45 AM			8:45 AM			8:45 AM	
15-min Vol	5			5			5	
PM Peak	1:15 PM			1:15 PM			1:15 PM	
15-min Vol	6			6			6	
Comments:								

LOCATION: I			outh of F	Rte 397												QC JOB #: 1 DIRECTIO	
CITY/STATE:																DATE: Jur	,
CITI/STATE.	1	16	21	26	31	36	41	46	51	56	61	66	71	76		DATE. Jul	Number
Start Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	Pace Speed	in Pace
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:30 AM	0	0	0	0	0	0	0	0	0	0	2	0	1	0	3	56-65	2
03:45 AM	0	0	0	0	0	0	0	1	0	0	2	0	0	0	3	56-65	2
04:00 AM	0	0	0	0	0	0	0	0	1	0	2	2	0	0	5	61-70	4
04:15 AM	0	0	0	0	0	0	1	2	0	1	2	0	0	0	6	41-50	3
04:30 AM	0	0	0	1	0	0	0	0	2	1	0	0	0	0	4	51-60	3
04:45 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	2	51-60	1
05:00 AM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	61-70	2
05:15 AM	0	0	0	0	0	0	0	0	0	0	0	1	2	1	4	66-75	3
05:30 AM	0	0	0	0	0	0	3	0	0	0	1	1	0	0	5	36-45	3
05:45 AM	0	0	0	0	0	0	0	1	0	0	1	1	0	0	3	61-70	2
Day Total Percent																	
AM Peak																	
15-min Vol PM Peak																	
15-min Vol																	
Comments:																	
eport generat	ad an 6/20	0/2022 7.5	1 1 1 1										COLIDC	E. Quality	Counts IIC (ht	tp://www.gualit	veerinte ne

LOCATION: N SPECIFIC LOC CITY/STATE:	ATION:		outh of R	te 397												QC JOB #: 1 DIRECTIOI DATE: Jun	N: NB, SB
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pace
06:00 AM	0	0	0	0	0	0	1	0	1	1	0	0	0	0	3	51-60	2
06:15 AM	0	0	0	0	0	0	0	0	1	1	0	1	0	0	3	51-60	2
06:30 AM	0	0	0	0	0	0	1	1	0	2	3	1	0	0	8	56-65	5
06:45 AM	0	0	0	0	0	0	0	0	1	2	1	1	0	0	5	56-65	3
07:00 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
07:15 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
07:30 AM	0	0	0	0	0	0	0	1	0	0	3	0	0	0	4	56-65	3
07:45 AM	0	0	0	0	0	0	2	0	0	2	2	0	1	0	7	56-65	4
08:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	1	0	2	51-60	1
08:15 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
08:30 AM	0	0	0	0	0	0	1	1	0	0	1	0	0	0	3	41-50	2
08:45 AM	0	2	0	0	0	0	0	2	0	3	0	0	0	0	7	51-60	3
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:30 AM	0	0	0	0	0	0	1	0	1	0	1	0	0	0	3	36-45	1
09:30 AM 09:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
10:00 AM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
				0	0	0		0		1	0		0				
10:15 AM	0	0	0		•	_	0		0	_	-	1	-	1	3	51-60	1
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
10:45 AM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
11:00 AM	0	0	0	0	0	0	0	0	0	2	1	1	0	0	4	56-65	3
11:15 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
11:30 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2	41-50	1
11:45 AM	0	0	0	0	0	0	0	1	0	1	1	0	2	0	5	66-75	2
Day Total Percent																	
AM Peak 15-min Vol PM Peak																	
15-min Vol Comments:																	

CITY/STATE:	Benton,	WA														DATE: Jur	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	1	0	2	51-60	1
12:15 PM	0	0	0	0	0	0	0	1	0	1	0	0	1	1	4	71-80	1
12:30 PM	0	0	0	0	0	0	0	0	0	2	1	2	0	0	5	56-65	3
12:45 PM	0	0	0	0	0	0	0	0	1	1	2	1	0	0	5	61-70	3
01:00 PM	0	0	2	0	0	0	0	0	1	0	2	2	1	1	9	61-70	4
01:15 PM	0	0	0	0	0	1	0	1	0	1	3	0	0	0	6	56-65	4
01:30 PM	0	0	0	0	0	1	0	0	0	2	1	1	0	0	5	56-65	3
01:45 PM	0	0	0	0	0	0	0	1	0	1	1	0	0	0	3	56-65	2
02:00 PM	0	0	0	0	0	0	0	0	3	2	0	1	0	0	6	51-60	5
02:15 PM	0	0	0	0	0	0	0	1	0	0	1	1	0	0	3	61-70	2
02:30 PM	0	0	0	0	0	0	0	0	1	3	0	0	1	0	5	51-60	4
02:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
03:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
03:15 PM	0	0	0	0	0	0	0	0	0	2	1	1	0	1	5	56-65	3
03:30 PM	0	0	0	0	0	0	0	0	1	1	0	1	0	0	3	51-60	2
03:45 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
03.43 FW	0	0	0	0	0	0	0	0	0	1	1	1	0	1	4	56-65	2
04:00 PM	0	0	0	0	0	0	0	0	2	1	3	0	0	0	6	56-65	4
04:30 PM	0	0	0	0	0	0	0	0	0	1	1	0	1	0	3	56-65	2
04:30 PM 04:45 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	3	51-60	3
	0	0	4	0		0	0	_	0	0		_			9		
05:00 PM			-		0			0			1	3 1	0	1	2	16-25	4
05:15 PM	0	0	0	0	0	0	0	0	0	0	1		0	0		61-70	2
05:30 PM	0	0	0	0	0	0	0	1	0	1	1	0	1	0	4	56-65	2
05:45 PM Day Total	0	0	0	0	0	0	1	1	1	0	3	1	1	1	9	61-70	4
Percent																	
AM Peak																	
15-min Vol																	
PM Peak 15-min Vol																	

LOCATION: Nine Canyon Rd south of Rte 397 QC JOB #: 16236666 SPECIFIC LOCATION: **DIRECTION:** NB, SB

CITY/STATE:	Benton,	WA														DATE: Jun	13 202
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Number in Pac
06:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
06:15 PM	0	0	0	0	0	0	1	0	0	0	1	0	1	0	3	36-45	1
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
06:45 PM	0	0	0	0	0	0	0	2	0	0	0	0	0	0	2	41-50	2
07:00 PM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
07:15 PM	0	0	1	0	0	0	0	1	0	0	1	0	0	0	3	16-25	1
07:30 PM	0	0	0	0	0	0	0	2	2	0	0	0	0	0	4	46-55	4
07:45 PM	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
08:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
08:15 PM	0	0	0	0	0	0	0	1	3	0	1	0	0	0	5	46-55	4
08:30 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	0	3	56-65	3
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
09:00 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
09:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
09:30 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	56-65	2
09:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
10:00 PM	0	0	0	0	0	0	0	1	0	1	1	0	0	0	3	56-65	2
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
10:30 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
10:45 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1	36-45	1
11:00 PM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	2	36-45	1
11:15 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	2	51-60	2
11:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
Day Total	0	2	7	1	0	3	14	28	28	53	62	32	15	8			
Percent	0%	0.8%	2.8%	0.4%	0%	1.2%	5.5%	11.1%	11.1%	20.9%	24.5%	12.6%	5.9%	3.2%	253	56-65	115
AM Peak	12:00 AM 0	8:45 AM 2	12:00 AM 0	4:30 AM 1	12:00 AM 0	12:00 AM 0	5:30 AM 3	4:15 AM 2	4:30 AM 2	8:45 AM 3	6:30 AM 3	4:00 AM 2	5:15 AM 2	5:15 AM 1	6:30 AM 8		
15-min Vol																	
PM Peak 15-min Vol	12:00 PM 0	12:00 PM 0	5:00 PM 4	12:00 PM 0	12:00 PM 0	1:15 PM 1	5:45 PM 1	6:45 PM 2	2:00 PM 3	2:30 PM 3	1:15 PM 3	5:00 PM 3	12:00 PM 1	12:15 PM 1	1:00 PM 9		

## **SUMMARY - Tube Count - Speed Data**

LOCATION: Nin	ne Canyo	n Rd sou	th of Rte 3	397												QC JOB	#: 16236666
SPECIFIC LOCA	TION:															DIREC	TION: NB, SB
CITY/STATE: B	enton, W	/A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Name	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 dec speed	Pace
Grand Total	0	2	7	1	0	3	14	28	28	53	62	32	15	8	253	56-65	115
Percent	0%	0.8%	2.8%	0.4%	0%	1.2%	5.5%	11.1%	11.1%	20.9%	24.5%	12.6%	5.9%	3.2%	233	30-03	113
Cumulative Percent	0% 0.8% 3.6% 4% 4% 5.1% 10.7% 21.7% 32.8% 53.8% 78.3% 90.9% 96.8% 100%																
ADT 253															Mea	an Speed(Avera Med	ntile: 67 MPH age): 59 MPH dian: 59 MPH ode: 63 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB, SB DATE: Jun 13 2023

CITY/STATE: Be	illoll, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	2	0	0	0	0	0	0	0	0	0	3
03:45 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
04:00 AM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
04:15 AM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	6
04:30 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
04:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:15 AM	0	1	0	1	2	0	0	0	0	0	0	0	0	0	4
05:30 AM	0	2	0	0	3	0	0	0	0	0	0	0	0	0	5
05:45 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
Day Total															
Percent				DAIL	AIHZ	$\lambda L L L L$	RIVE	266	IVIIVI	U	IES				
ADT															
253															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:	C /20 /2/	222750444									COLUBEE	Ita C	- 11 C /l-1:	//	tycounts not)

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB, SB DATE: Jun 13 2023

Cars & 2 Axle 2 Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Bikes Start Time Buses Total **Trailers** Tire Single Single Double Double **Double** Multi Multi Multi Classed Long 06:00 AM 06:15 AM 06:30 AM 06:45 AM 07:00 AM 07:15 AM 07:30 AM 07:45 AM 08:00 AM 08:15 AM 08:30 AM 08:45 AM 09:00 AM 09:15 AM 09:30 AM 09:45 AM 10:00 AM 10:15 AM 10:30 AM 10:45 AM 11:00 AM 11:15 AM 11:30 AM 11:45 AM Day Total Percent ADT AM Peak 15-min Vol PM Peak 15-min Vol Comments:

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB, SB DATE: Jun 13 2023

Start time																
12:00 PM	Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
12:30 PM	12:00 PM	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2
1245 PM	12:15 PM	0	0	3	0	1	0	0	0	0	0	0	0	0	0	4
01:0PM	12:30 PM	0	2	1	2	0	0	0	0	0	0	0	0	0	0	5
01:15 PM	12:45 PM	0	3	2	0	0	0	0	0	0	0	0	0	0	0	5
01:30 PM	01:00 PM	1	2	3	0	2	0	0	0	1	0	0	0	0	0	9
01:45 PM 0 0 2 2 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0	01:15 PM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	6
02:15 PM 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	01:30 PM	0	2	0	0	2	0	0	1	0	0	0	0	0	0	5
02:15 PM		0	0	2	0	1	0	0	0	0	0	0	0	0	0	3
02:30 PM	02:00 PM	0	3	1	1	1	0	0	0	0	0	0	0	0	0	6
02:45 PM		0	2	0	0	0	0	0		0	0	0	0	0	0	3
03:00 PM		0	3	1	0	0	0	0	1	0	0	0	0	0	0	5
03:15 PM		0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:30 PM		0	0	0	0	1	0	0	0	0	0	0	0	0	0	
03:45 PM		0		2	0	1	0	0	0	0	0	0	0	0	0	
04:00 PM	03:30 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
04:15 PM         0         3         2         0         1         0<		0		0	0	1	0	0	0	0	0	0	0	0	0	2
04:30 PM         0         2         0         0         1         0<		0	2	1	0	1	0	0	0	0	0	0	0	0	0	4
04:45 PM	04:15 PM	0	3	2	0	1	0	0	0	0	0	0	0	0	0	6
05:00 PM	04:30 PM	0	2	0	0	1	0	0	0	0	0	0	0	0	0	3
05:15 PM	04:45 PM	0	2	0	0	1		0	0	0	0	0	0	0	0	3
05:30 PM		0	3	4	0			0	1	0		0	0	0	0	9
Digital   Digi	05:15 PM	0	0					0	0	0		0		0	0	2
Day Total Percent  ADT 253  AM Peak 15-min Vol		0	0	3	0	1	0	0	0	0	0	0	0	0	0	4
ADT 253  AM Peak 15-min Vol	05:45 PM	0	3	5	1	0	0	0	0	0	0	0	0	0	0	9
ADT 253  AM Peak 15-min Vol	Day Total							- n /-								
AM Peak 15-min Vol	Percent				$)\Delta IZ$	AIHA		RIVE	500	MM	UNII	11-5				
15-min Vol																
	15-min Vol															
PM Peak	PM Peak															
15-min Vol	15-min Vol															
'amments:	Comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB, SB DATE: Jun 13 2023

Cars & 2 Axle 2 Axle 6 3 Axle 4 Axle <5 Axl 5 Axle >6 Axl <6 Axl 6 Axle >6 Axl Not Bikes Start Time **Buses** Total **Trailers** Tire Single Single **Double** Double **Double** Multi Multi Multi Classed Long 06:00 PM 06:15 PM 06:30 PM 06:45 PM 07:00 PM 07:15 PM 07:30 PM 07:45 PM 08:00 PM 08:15 PM 08:30 PM 08:45 PM 09:00 PM 09:15 PM 09:30 PM 09:45 PM 10:00 PM 10:15 PM 10:30 PM 10:45 PM 11:00 PM 11:15 PM 11:30 PM 11:45 PM **Day Total** Percent 0.4% 41.9% 27.3% 7.1% 20.6% 0% 0% 2% 0.4% 0.4% 0% 0% 0% 0% ADT **AM Peak** 12:00 AM 4:15 AM 6:30 AM 6:45 AM 11:45 AM 12:00 AM 12:00 AM 10:00 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 12:00 AM 6:30 AM 15-min Vol 1:00 PM 1:15 PM 5:45 PM 12:30 PM 1:00 PM 12:00 PM 12:00 PM 1:30 PM 1:00 PM 7:15 PM 12:00 PM 12:00 PM 12:00 PM 12:00 PM PM Peak 1:00 PM 15-min Vol Comments:

LOCATION: Nine Canyon Rd south of Rte 397

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023

CITY/STATE: BEI	iton, WA													DATE:	Jun 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total Percent	1 0.4%	106 41.9%	69 27.3%	18 7.1%	52 20.6%	0 0%	0 0%	5 <b>2</b> %	1 0.4%	1 0.4%	0 0%	0 0%	0 0%	0 0%	253
ADT 253															
Comments:						7			·	·				·	·

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: NB, SB

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		1				1			1	
03:15 AM		0				0			0	
03:30 AM		3				3			3	
03:45 AM		3				3			3	
04:00 AM		5				5			5	
04:15 AM		6				6			6	
04:30 AM		4				4			4	
04:45 AM		2				2		411	2	
05:00 AM		2				2			2	
05:15 AM		4				4	0 0 0 0 0		4	
05:30 AM		5				5	DIVIN	UNII	5	
05:45 AM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
omments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA **DATE:** Jun 13 2023 - Jun 13 2023

Average	Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:30 AM	06:00 AM		3				3			3	
0643 AM 5 5 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	06:15 AM		3				3			3	
07:00 AM	06:30 AM		8				8			8	
07:15 AM	06:45 AM		5				5			5	
07:30 AM	07:00 AM		1				1			1	
07:45 AM	07:15 AM		1				1			1	
08:00 AM 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	07:30 AM		4				4			4	
08:15 AM	07:45 AM		7				7			7	
08:30 AM	08:00 AM		2				2			2	
08:45 AM	08:15 AM		1				1			1	
09:00 AM	08:30 AM		3				3			3	
09:15 AM       0       0       0       0       0       0       09:30 AM       3       3       3       3       3       09:45 AM       1 <td>08:45 AM</td> <td></td> <td>7</td> <td></td> <td></td> <td></td> <td>7</td> <td></td> <td></td> <td>7</td> <td></td>	08:45 AM		7				7			7	
09:30 AM       3       3       3       3       3       9:45 AM       1	09:00 AM		0				0			0	
09:45 AM       1<	09:15 AM		0				0			0	
10:00 AM	09:30 AM		3				3			3	
10:15 AM	09:45 AM		1				1			1	
10:30 AM 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	10:00 AM		2				2			2	
10:45 AM	10:15 AM		3							3	
11:00 AM	10:30 AM		1				1		ın		
11:15 AM	10:45 AM		2				2		411	2	
11:45 AM         5         5           Day Total         Weekday         Company of the part of the p	11:00 AM		4				•			4	
11:45 AM         5         5           Day Total         Weekday         Company of the part of the p	11:15 AM		1				200/200	00.000	T TR TIP		
11:45 AM         5         5           Day Total         Weekday         Company of the part of the p	11:30 AM		2				2	DIVIIVI	UNII	2	
Weekday Average  Week Average  AM Peak 15-min Vol  PM Peak	11:45 AM		5				5			5	
Average         Meek	Day Total										
Week Average  AM Peak 15-min Vol  PM Peak	% Weekday										
Average	Average										
AM Peak         15-min Vol         PM Peak	% Week										
15-min Vol	Average										
PM Peak											
	15-min Vol										
15-min Vol											
	15-min Vol										

QC JOB #: 16236666

**DIRECTION:** NB, SB

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		2				2			2	
12:15 PM		4				4			4	
12:30 PM		5				5			5	
12:45 PM		5				5			5	
01:00 PM		9				9			9	
01:15 PM		6				6			6	
01:30 PM		5				5			5	
01:45 PM		3				3			3	
02:00 PM		6				6			6	
02:15 PM		3				3			3	
02:30 PM		5				5			5	
02:45 PM		1				1			1	
03:00 PM		1				1			1	
03:15 PM		5				5			5	
03:30 PM		3				3			3	
03:45 PM		2				2			2	
04:00 PM		4				4			4	
04:15 PM		6				6			6	
04:30 PM		3				3			3	
04:45 PM		3				3			3	
05:00 PM		9				9			9	
05:15 PM		2				2 4 5	70000		2	
05:30 PM		4					JIVIIVI		4	
05:45 PM		9				9			9	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak 15-min Vol										
PM Peak										
15-min Vol										
Comments:										

QC JOB #: 16236666

**DIRECTION:** NB, SB

SPECIFIC LOCATION:

CITY/STATE: Benton, WA **DATE**: Jun 13 2023 - Jun 13 2023

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM		1				1			1	
06:15 PM		3				3			3	
06:30 PM		0				0			0	
06:45 PM		2				2			2	
07:00 PM		2				2			2	
07:15 PM		3				3			3	
07:30 PM		4				4			4	
07:45 PM		1				1			1	
08:00 PM		1				1			1	
08:15 PM		5				5			5	
08:30 PM		3				3			3	
08:45 PM		0				0			0	
09:00 PM		1				1			1	
09:15 PM		1				1			1	
09:30 PM		2				2			2	
09:45 PM		1				1			1	
10:00 PM		3				3			3	
10:15 PM		0				0			0	
10:30 PM		1				1			1	
10:45 PM		1				1	$\cdot \cup \cup$		1	
11:00 PM		2				2			2	
11:15 PM		2				DR/\\\^2_1\S\C(			2	
11:30 PM		1				DRIVES CO	DIVINI		/E5 1	
11:45 PM		1				1			1	
Day Total		253				253			253	
% Weekday		100%								
Average		100%								
% Week		100%				100%				
Average		100%				100%				
AM Peak		6:30 AM				6:30 AM			6:30 AM	
15-min Vol		8				8			8	
PM Peak		1:00 PM				1:00 PM			1:00 PM	
15-min Vol		9				9			9	

QC JOB #: 16236666

**DIRECTION:** NB, SB

QC JOB #: 16236666 LOCATION: Nine Canyon Rd south of Rte 397 **SPECIFIC LOCATION: DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 12:00 AM 1-10 12:15 AM 1-10 12:30 AM 1-10 12:45 AM 1-10 01:00 AM 1-10 01:15 AM 1-10 01:30 AM 1-10 01:45 AM 1-10 02:00 AM O 1-10 02:15 AM 1-10 02:30 AM 1-10 02:45 AM 1-10 03:00 AM 56-65 03:15 AM 1-10 03:30 AM 56-65 03:45 AM 56-65 61-70 04:00 AM 04:15 AM 41-50 04:30 AM O 51-60 04:45 AM 51-60 05:00 AM 61-70 05:15 AM 66-75 05:30 AM 36-45 05:45 AM 61-70 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

QC JOB #: 16236666 LOCATION: Nine Canyon Rd south of Rte 397 **SPECIFIC LOCATION: DIRECTION: SB** CITY/STATE: Benton, WA **DATE: Jun 13 2023** Number **Start Time** Total Pace Speed in Pace 06:00 AM 51-60 51-60 06:15 AM 06:30 AM 56-65 06:45 AM 56-65 07:00 AM 46-55 07:15 AM 51-60 56-65 07:30 AM 07:45 AM 36-45 08:00 AM O 66-75 08:15 AM 56-65 08:30 AM 41-50 08:45 AM 41-50 09:00 AM 1-10 09:15 AM 1-10 36-45 09:30 AM 09:45 AM 56-65 10:00 AM 56-65 10:15 AM 1-10 10:30 AM O 1-10 10:45 AM 51-60 11:00 AM 51-60 11:15 AM 1-10 11:30 AM 41-50 11:45 AM 41-50 **Day Total** Percent **AM Peak** 15-min Vol PM Peak 15-min Vol Comments:

CITY/STATE:																DATE: Jun	
Start Time	1 15	16 20	21 25	26 30	31 35	36 40	41 45	46 50	51 55	56 60	61 65	66 70	71 75	76 999	Total	Pace Speed	Numb in Pac
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
12:45 PM	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	56-65	2
01:00 PM	0	0	2	0	0	0	0	0	0	0	1	0	0	1	4	16-25	2
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
01:30 PM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	2	31-40	1
01:45 PM	0	0	0	0	0	0	0	1	0	1	0	0	0	0	2	41-50	1
02:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
02:15 PM	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
02:30 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	2	51-60	2
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
03:15 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
03:30 PM	0	0	0	0	0	0	0	0	1	1	0	1	0	0	3	51-60	2
03:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
04:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0			56-65	
		-			0	-					_	0		0	1		1
04:45 PM	0	0	0	0		0	0	0	0	0	0		0	0	0	1-10	0
05:00 PM	0	0	4	0	0	0	0	0	0	0	0	0	0	0	4	16-25	4
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
05:30 PM	0	0	0	0	0	0	0	1	0	1	1	0	0	0	3	56-65	2
05:45 PM	0	0	0	0	0	0	1	1	0	0	0	1	0	0	3	41-50	2
Day Total Percent																	
AM Peak 15-min Vol																	
PM Peak																	
L5-min Vol																	

LOCATION: Nine Canyon Rd south of Rte 397

SPECIFIC LOCATION:

DIRECTION: SB

CITY/STATE: Benton, WA DATE: Jun 13 3

1 15	16 20	21	26	31	36	41	46	51				74	7.0			
	20				30		46		56	61	66	71	76	Total	Pace Speed	Numbe
	20	25	30	35	40	45	50	55	60	65	70	75	999	iotai	race speed	in Pac
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	1	0	0	0	0	0	1	0	2	36-45	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	41-50	1
0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
0	0	0	0	0	0	0	2	1	0	0	0	0	0	3	46-55	3
0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	41-50	1
0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	51-60	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	56-65	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	31-40	1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1-10	0
0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46-55	1
0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	61-70	1
0	0	6	1	0	2	12	20	15	26	31	14	5	1	122	FC CF	F-7
0%	0%	4.5%	0.8%	0%	1.5%	9%	15%	11.3%	19.5%	23.3%	10.5%	3.8%	0.8%	133	56-65	57
						5:30 AM	4:15 AM	4:30 AM	6:30 AM	6:30 AM	4:00 AM			6:30 AM		
U	U	4	U	U	1	1	2	1	1	1	1	1	1	4		
	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0 1 0 0 0 0	0 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 1 0 0 0 1 0	0 0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 0 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 1 0 0 0 0	0 0 0 0 0 0 0 0 0 0 0 1 0 0 0 1 0 0 0 0

LOCATION: Nir	ne Canyo	n Rd sou	ith of Rte 3	397												QC JOB	#: 16236666
SPECIFIC LOCA	TION:															DI	RECTION: SB
CITY/STATE: Be	enton, W	'A														DATE:	Jun 13 2023
Speed Range	1	16	21	26	31	36	41	46	51	56	61	66	71	76	Total	Pace Speed	Number in
Speed Name	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total	1 ace Speed	Pace
Grand Total	0	0	6	1	0	2	12	20	15	26	31	14	5	1	133	56-65	57
Percent	0%	0%	4.5%	0.8%	0%	1.5%	9%	15%	11.3%	19.5%	23.3%	10.5%	3.8%	0.8%	133	30-03	37
Cumulative	0%	0%	4.5%	5.3%	5.3%	6.8%	15.8%	30.8%	42.1%	61.7%	85%	95.5%	99.2%	100%			
Percent	070	070	4.570	3.370	3.370	0.070	13.670	30.070	42.170	01.770	0370	JJ.J/0	33.270	10070			
ADT 133															Me	an Speed(Avera Me	ntile: 65 MPH age): 57 MPH dian: 57 MPH ode: 63 MPH
Comments:																	

Report generated on 6/20/2023 7:51 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: SB

CITY/STATE: Be	IIIOII, WA	Corr. 0	2 4-1-		2 4.4- 6	2 4-1-	A Al.	<5 Axl	F A1-	>6 Axl	ac Al	C A1-	. C AI		un 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle	Buses	2 Axle 6 Tire	3 Axle	4 Axle	<5 Axı Double	5 Axle Double	>6 AXI Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
			Long			Single	Single								
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
03:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 AM	0	1	0	0	2	0	0	0	0	0	0	0	0	0	3
03:45 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
04:00 AM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	5
04:15 AM	0	6	0	0	0	0	0	0	0	0	0	0	0	0	6
04:30 AM	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4
04:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:00 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
05:15 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
05:30 AM	0	2	0	0	3	0	0	0	0	0	0	0	0	0	5
05:45 AM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	3
Day Total															
Percent				DAIL	AIHA	ALLI	RIVE	366	IVIIVI	ШИШ	IES				
ADT															
133															
AM Peak															
15-min Vol															
PM Peak															
15-min Vol															
Comments:															
comments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: SB

Start Time   Bikes   Trailers   Long   Buses   Time   Single   Single   Single   South   South   South   South   South   Multi   Multi   Classed   Total   Single   Single   Single   South   South   South   South   Multi   Multi   Classed   Single   Single   Single   South   South   South   South   Multi   Classed   Single   South   CITY/STATE: Be	inton, WA	Cars &	2 Axle		2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	un 13 2023	
06:00 AM	Start Time	Bikes			Buses											Total
06:15 AM 0 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 3 3 6 0 0 0 0	06:00 AM	0			1											3
06-30 AM			3													
06:55 AM					0					0	0			0	0	8
07:00 AM				1	2	0			0	0	0		0	0	0	4
07:30 AM	07:00 AM		1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:45 AM	07:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
08:00 AM	07:30 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
08:35 AM	07:45 AM	0	1	2	0	1	0	0	0	0	0	0	0	0	0	4
08:30 AM	08:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
08:45 AM		0	0	0	0	1	0	0		0	0	0	0	0	0	1
09:00 AM		0	2	0	1	0	0	0	0	0	0	0	0	0	0	3
09:15 AM		0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
09:30 AM		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 AM				0	0	-				0	0			0	-	
10:00 AM		_		-	1	•				·	•	_		•	•	
10:15 AM					0	_					_			0	_	
10:30 AM				-	•									-	_	
10:45 AM			-	•	•	-				0	_			•	_	-
11:00 AM			-	_	·	•				0	ŭ	-	-	•	•	
11:15 AM		_								O	_			Ū	·	
11:30 AM														-	_	
11:45 AM														_	_	
Day Total Percent  ADT 133  AM Peak 15-min Vol  PM Peak 15-min Vol															_	
ADT 133  AM Peak 15-min Vol PM Peak 15-min Vol		0	1	0	0	2	0	0	0	0	0	0	0	0	0	3
ADT 133  AM Peak 15-min Vol  PM Peak 15-min Vol																
133  AM Peak 15-min Vol  PM Peak 15-min Vol	Percent				DAIA	A 11714	AL DI	RIVE	366	IIVIIVI	UIVIII	IES				
AM Peak 15-min Vol  PM Peak 15-min Vol																
15-min Vol  PM Peak 15-min Vol	133															
PM Peak 15-min Vol																
Comments:	PM Peak															
	comments:															

SPECIFIC LOCATION: CITY/STATE: Benton. WA QC JOB #: 16236666 DIRECTION: SB

CITY/STATE: Be	enton, WA														un 13 2023
Start Time	Bikes	Cars &	2 Axle	Buses	2 Axle 6	3 Axle	4 Axle	<5 Axl	5 Axle	>6 Axl	<6 Axl	6 Axle	>6 Axl	Not	Total
		Trailers	Long		Tire	Single	Single	Double	Double	Double	Multi	Multi	Multi	Classed	
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	1	1	0	0	0	0	0	0	0	0	0	0	0	2
01:00 PM	1	0	1	0	1	0	0	0	1	0	0	0	0	0	4
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:30 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
01:45 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	2
02:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
02:15 PM	0	1	0	0	0	0	0	1	0	0	0	0	0	0	2
02:30 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
03:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
03:30 PM	0	2	1	0	0	0	0	0	0	0	0	0	0	0	3
03:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00 PM	0	1	2	0	0	0	00	1	0	0	0	0	0	0	4
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	2	0	1	0	0	0	0	0	0	0	0	0	3
05:45 PM	0	1	1	1	0	0	0	0	0	0	0	0	0	0	3
Day Total				_											
Percent															
ADT 133															
AM Peak 15-min Vol															
PM Peak 15-min Vol															
omments:															

SPECIFIC LOCATION: CITY/STATE: Benton, WA QC JOB #: 16236666 DIRECTION: SB

Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 PM	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:00 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
07:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
07:30 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	3
07:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
08:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
09:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
09:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
11:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
11:45 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Day Total	1	61	33	7	27	0	0	3	1	0	0	0	0	0	122
Percent	0.8%	45.9%	24.8%	5.3%	20.3%	0%	0%	2.3%	0.8%	0%	0%	0%	0%	0%	133
ADT 133															
AM Peak	12:00 AM	4:15 AM	6:30 AM	6:45 AM	5:30 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	12:00 AM	6:30 AN
15-min Vol	0	6	4	2	3	0	0	0	0	0	0	0	0	0	8
PM Peak	1:00 PM	2:30 PM	7:30 PM	5:45 PM	1:00 PM	12:00 PM	12:00 PM	1:30 PM	1:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	12:00 PM	1:00 PM
15-min Vol	1	2	3	1	1	0	0	1	1	0	0	0	0	0	4
omments:															

LOCATION: Nine Canyon Rd south of Rte 397

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

DATE: Jun 13 2023

CITY STATE: BC	10011, 4471													D/ (TE. )	Juli 13 2023
Start Time	Bikes	Cars & Trailers	2 Axle Long	Buses	2 Axle 6 Tire	3 Axle Single	4 Axle Single	<5 Axl Double	5 Axle Double	>6 Axl Double	<6 Axl Multi	6 Axle Multi	>6 Axl Multi	Not Classed	Total
Grand Total	1	61	33	7	27	0	0	3	1	0	0	0	0	0	122
Percent	0.8%	45.9%	24.8%	5.3%	20.3%	0%	0%	2.3%	0.8%	0%	0%	0%	0%	0%	133
ADT 133															
Comments:															

Report generated on 6/20/2023 7:50 AM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net)



SPECIFIC LOCATION:

**DIRECTION: SB** 

QC JOB #: 16236666

CITY/STATE: I			\A/+ -1	TL	F :	A	C -	C. ·		TE: Jun 13 2023 - Jun 13 2023
Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 AM		0				0			0	
12:15 AM		0				0			0	
12:30 AM		0				0			0	
12:45 AM		0				0			0	
01:00 AM		0				0			0	
01:15 AM		0				0			0	
01:30 AM		0				0			0	
01:45 AM		0				0			0	
02:00 AM		0				0			0	
02:15 AM		0				0			0	
02:30 AM		0				0			0	
02:45 AM		0				0			0	
03:00 AM		1				1			1	
03:15 AM		0				0			0	
03:30 AM		3				3			3	
03:45 AM		3				3			3	
04:00 AM		5				5	-		5	
04:15 AM		6				6			6	
04:30 AM		4				4			4	
04:45 AM		2				2			2	
05:00 AM		2				2			2	
05:15 AM		2				2 5	00.00		2	
05:30 AM		5				DRIV55 C	DIVIIV		5	
05:45 AM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236666

**DIRECTION: SB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 AM		3				3			3	
06:15 AM		3				3			3	
06:30 AM		8				8			8	
06:45 AM		4				4			4	
07:00 AM		1				1			1	
07:15 AM		1				1			1	
07:30 AM		2				2			2	
07:45 AM		4				4			4	
08:00 AM		1				1			1	
08:15 AM		1				1			1	
08:30 AM		3				3			3	
08:45 AM		2				2			2	
09:00 AM		0				0			0	
09:15 AM		0				0			0	
09:30 AM		2				2			2	
09:45 AM		1				1			1	
10:00 AM		1				1			1	
10:15 AM		0			_	0	-		0	
10:30 AM		0				0			0	
10:45 AM		1				1		$\mathcal{A}$	1	
11:00 AM		2				2			2	
11:15 AM		0			TIATI	DRIVIES CO	3 A A A A	11 18 117	0	
11:30 AM		1			MALL		DIVIIVI	UNII	IE5 1	
11:45 AM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA

QC JOB #: 16236666

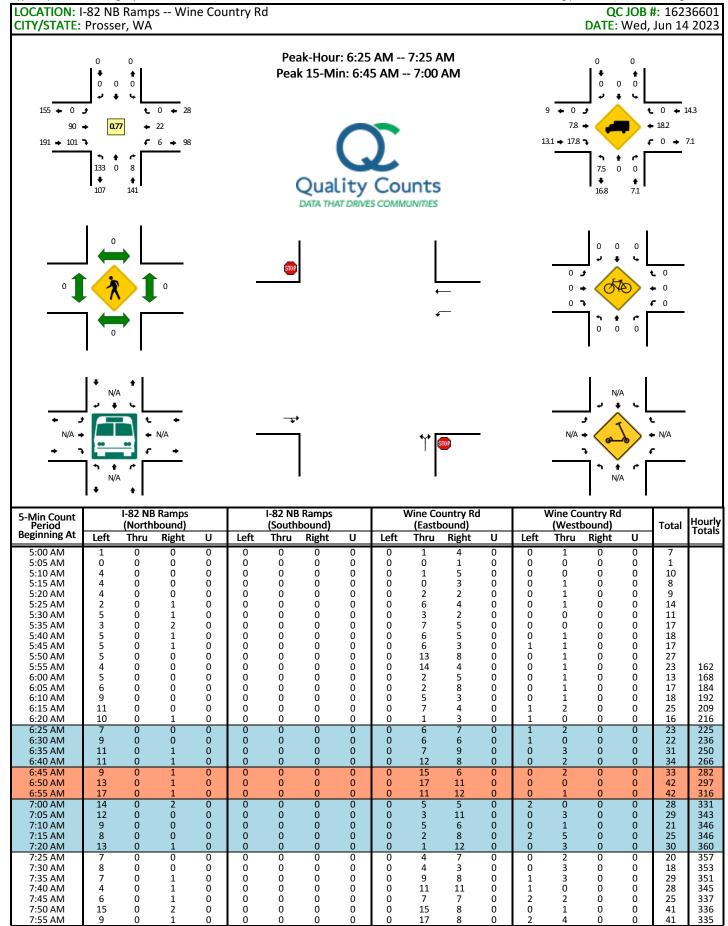
**DIRECTION: SB** 

Start Time	Mon	<b>Tue</b> 13 Jun 23	Wed	Thu	Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
12:00 PM		0				0				<u> </u>
12:00 PM 12:15 PM		1				1			0 1	
12:15 PM 12:30 PM		0				0			0	
12:45 PM		2				2			2	
01:00 PM		4				4			4	
01:00 PM		0				0			0	
01:30 PM		2				2			2	
01:45 PM		2				2			2	
02:00 PM		1				1			1	
02:15 PM		2				2			2	
02:30 PM		2				2			2	
02:45 PM		0				0			0	
03:00 PM		1				1			1	
03:15 PM		1				1			1	
03:30 PM		3				3			3	
03:45 PM		1				1			1	
04:00 PM		0				0			0	
04:15 PM		0			_ 0	0			0	
04:30 PM		1			21 1	1	$\cap$	In'	1	
04:45 PM		0				0		411	0	
05:00 PM		4				4			4	
05:15 PM		0			TIATI	0 3 5	O A // A /	11.18.117	0	
05:30 PM		3			HALL		JIVIIV	UNII	3	
05:45 PM		3				3			3	
Day Total										
% Weekday										
Average										
% Week										
Average										
AM Peak										
15-min Vol										
PM Peak										
15-min Vol										
Comments:										

SPECIFIC LOCATION:

CITY/STATE: Benton, WA DAT

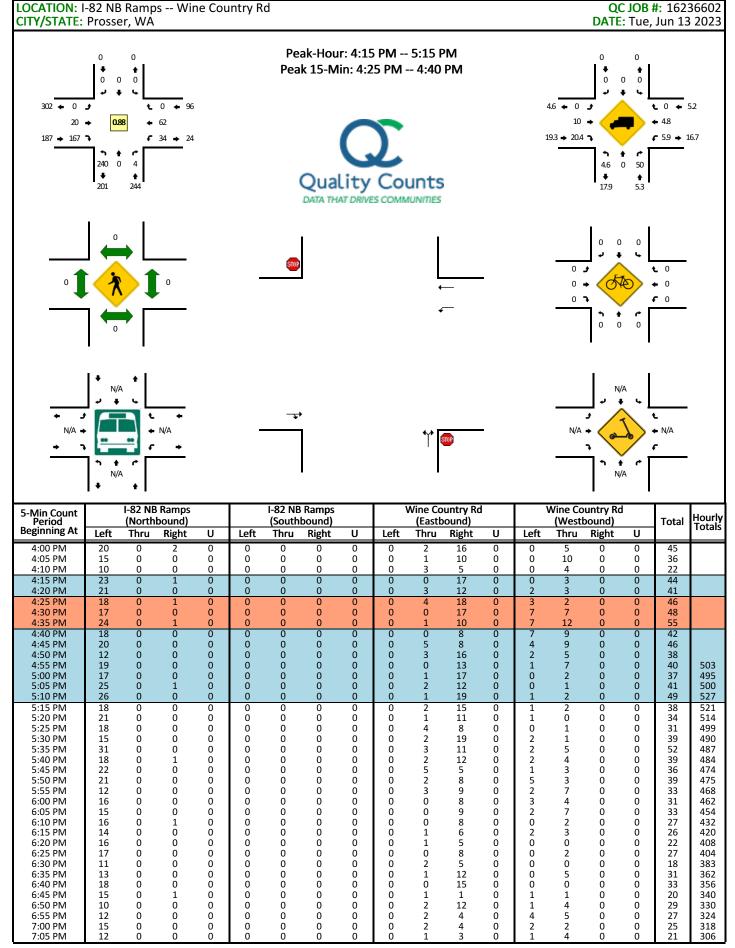
Start Time	Mon Tue 13 Jun 23	<b>W</b> ed Thu	ı Fri	Average Weekday 15-min Traffic	Sat	Sun	Average Week 15-min Traffic	Average Week Profile
06:00 PM	0			0			0	
06:15 PM	2			2			2	
06:30 PM	0			0			0	
06:45 PM	1			1			1	
07:00 PM	2			2			2	
07:15 PM	1			1			1	
07:30 PM	3			3			3	
07:45 PM	1			1			1	
08:00 PM	1			1			1	
08:15 PM	1			1			1	
08:30 PM	1			1			1	
08:45 PM	0			0			0	
09:00 PM	1			1			1	
09:15 PM	0			0			0	
09:30 PM	1			1			1	
09:45 PM	0			0			0	
10:00 PM	1			1			1	
10:15 PM	0			0			0	
10:30 PM	1			1			1	
10:45 PM	0			0			0	
11:00 PM	0			0			0	
11:15 PM	1			DRIVES CO	0 0 0 0 0		1	
11:30 PM	1			JKIVI S C	JIVIIVI		/E5 1	
11:45 PM	1			1			1	
Day Total	133			133			133	
% Weekday Average	100%							
% Week Average	100%			100%				
AM Peak	6:30 AM			6:30 AM			6:30 AM	
15-min Vol	8			8			8	
PM Peak	1:00 PM			1:00 PM			1:00 PM	
15-min Vol	4			4			4	



Peak 15-Min Flowrates		North	bound		Southbound			Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	156	0	12	0	0	0	0	0	0	172	116	0	0	12	0	0	468
Heavy Trucks	12	0	0		0	0	0		0	8	24		0	0	0		44
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

Report generated on 6/29/2023 1:28 PM

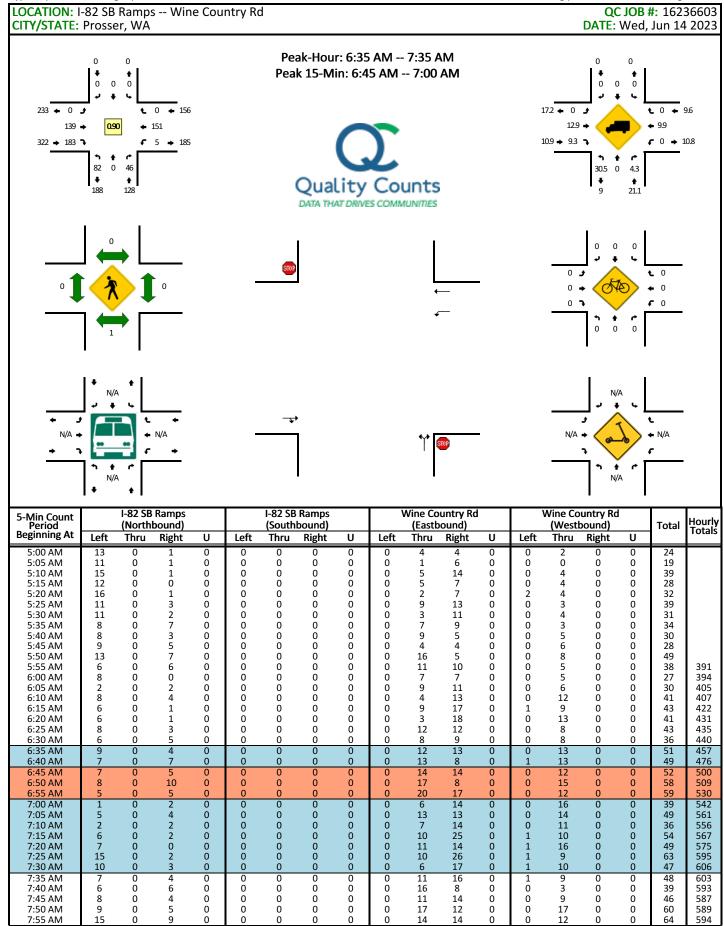
SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212



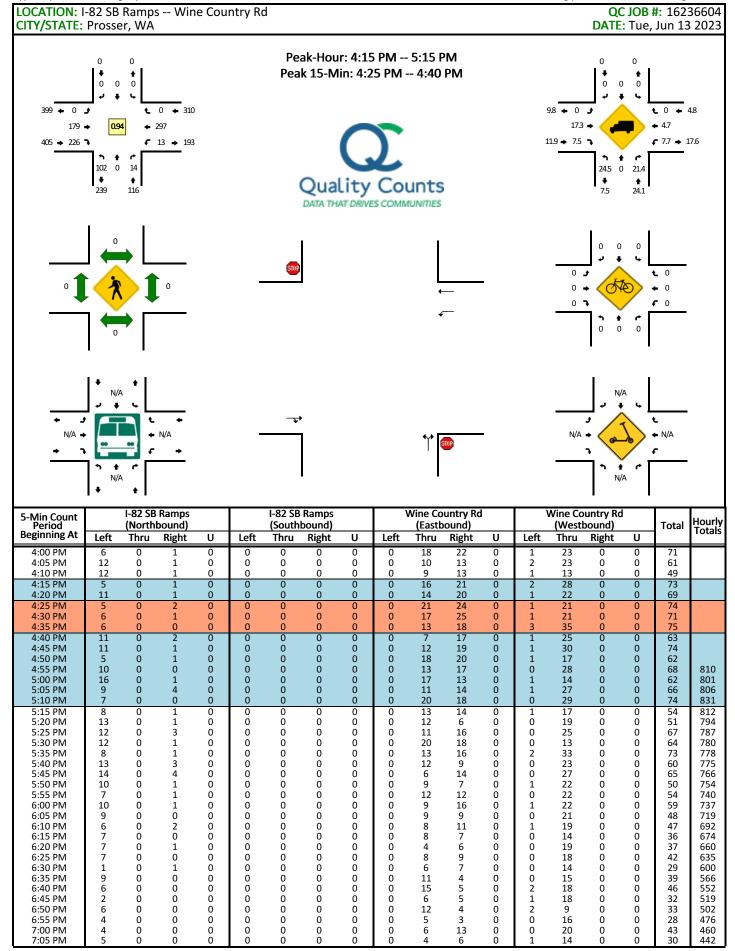
5-Min Count Period			Ramps bound)		I-82 NB Ramps (Southbound)				Wine Country Rd (Eastbound)				Wine Country Rd (Westbound)				Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	9	0	0	0	0	0	0	0	0	1	6	0	0	0	0	0	16	295
7:15 PM	13	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0	20	289
7:20 PM	15	0	0	0	0	0	0	0	0	1	7	0	0	1	0	0	24	291
7:25 PM	5	0	0	0	0	0	0	0	0	3	9	0	0	1	0	0	18	282
7:30 PM	14	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	16	280
7:35 PM	5	0	0	0	0	0	0	0	0	1	5	0	0	0	0	0	11	260
7:40 PM	12	0	0	0	0	0	0	0	0	1	2	0	0	0	0	0	15	242
7:45 PM	11	0	0	0	0	0	0	0	0	1	6	0	1	1	0	0	20	242
7:50 PM	11	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0	15	228
7:55 PM	8	0	0	0	0	0	0	0	0	1	5	0	0	2	0	0	16	217
Peak 15-Min	Northbound				Southbound					Eastb	ound		Westbound				Total	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tai
All Vehicles	236	0	8	0	0	0	0	0	0	20	180	0	68	84	0	0	59	96
Heavy Trucks Buses	8	0	0		0	0	0		0	0	32		0	0	0		40	
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		

Report generated on 6/29/2023 1:28 PM

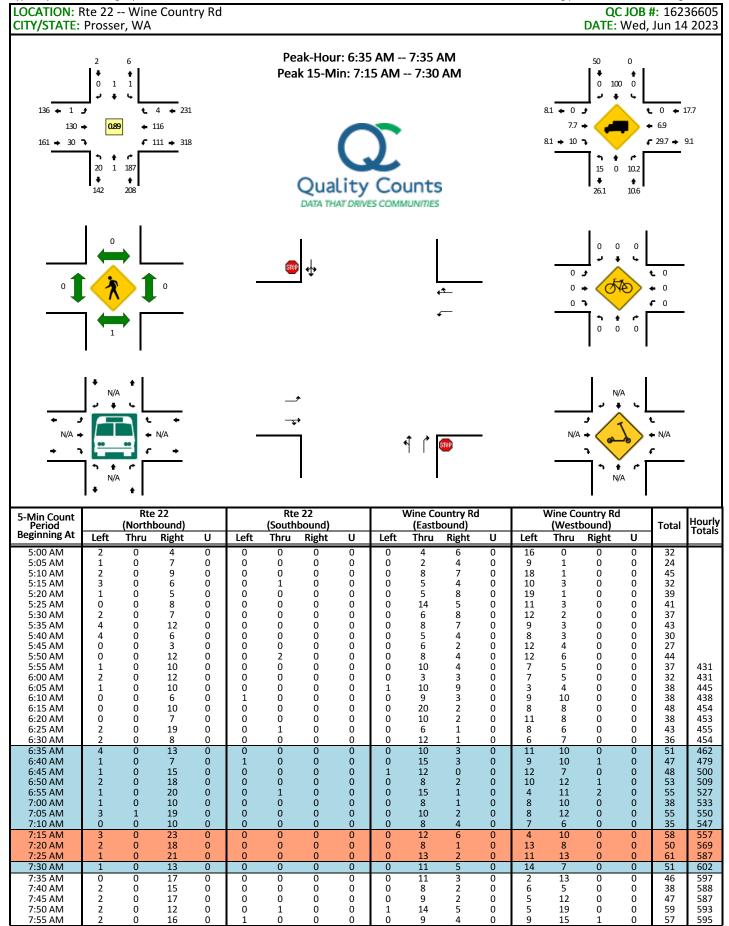
SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212



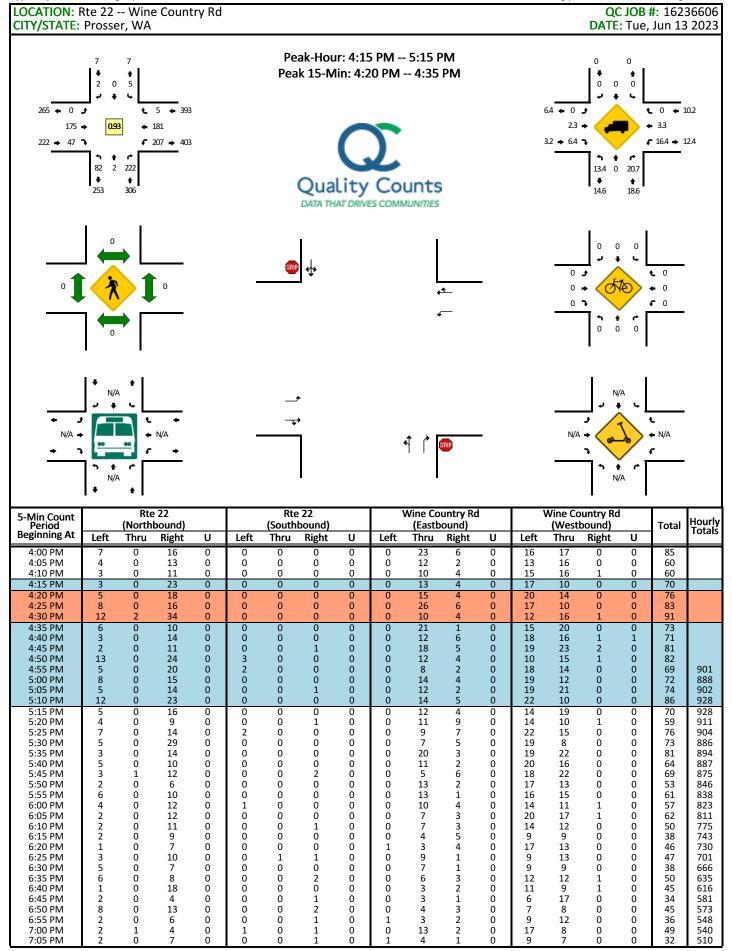
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOTAL
All Vehicles	80	0	80	0	0	0	0	0	0	204	156	0	0	156	0	0	676
Heavy Trucks	16	0	8		0	0	0		0	36	16		0	12	0		88
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



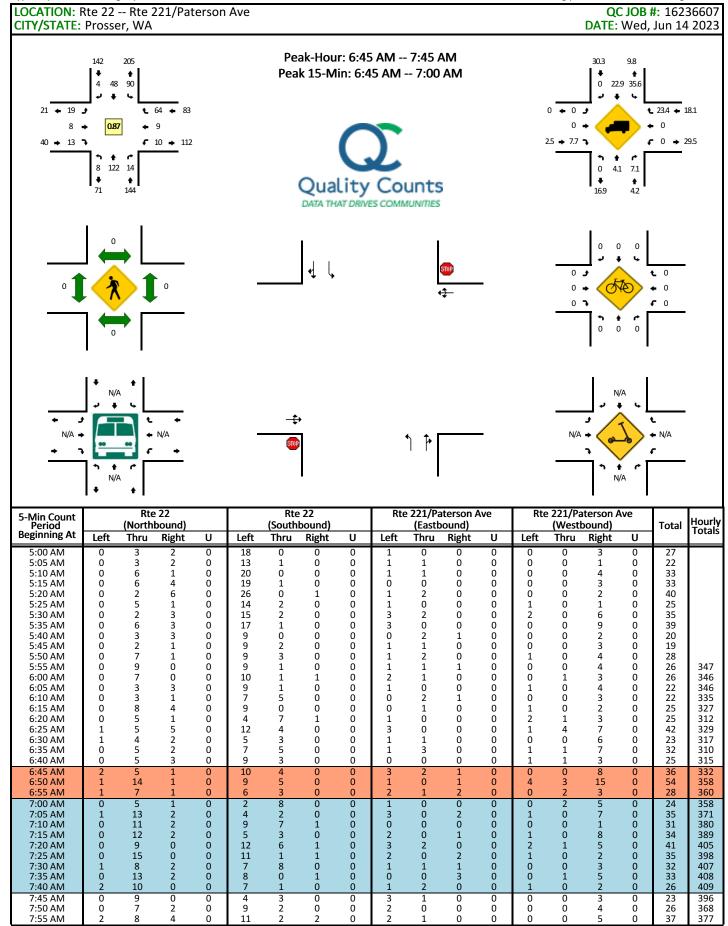
5-Min Count Period			Ramps bound)				Ramps bound)		'		ountry Rd oound)		,		ountry Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	2	0	0	0	0	0	0	0	0	7	7	0	0	9	0	0	25	420
7:15 PM	4	0	0	0	0	0	0	0	0	7	4	0	1	13	0	0	29	413
7:20 PM	4	0	0	0	0	0	0	0	0	8	3	0	0	16	0	0	31	407
7:25 PM	5	0	2	0	0	0	0	0	0	10	9	0	0	7	0	0	33	398
7:30 PM	4	0	0	0	0	0	0	0	0	1	11	0	1	12	0	0	29	398
7:35 PM	6	0	0	0	0	0	0	0	0	6	5	0	0	8	0	0	25	384
7:40 PM	4	0	0	0	0	0	0	0	0	3	4	0	0	10	0	0	21	359
7:45 PM	4	0	1	0	0	0	0	0	0	6	7	0	0	12	0	0	30	357
7:50 PM	3	0	0	0	0	0	0	0	0	3	9	0	0	12	0	0	27	351
7:55 PM	7	0	0	0	0	0	0	0	0	6	5	0	0	9	0	0	27	350
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	A - I
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	То	tai
All Vehicles	68	0	12	0	0	0	0	0	0	204	268	0	20	308	0	0	88	30
Heavy Trucks	8	0	0		0	0	0		0	36	20		0	12	0		7	6
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



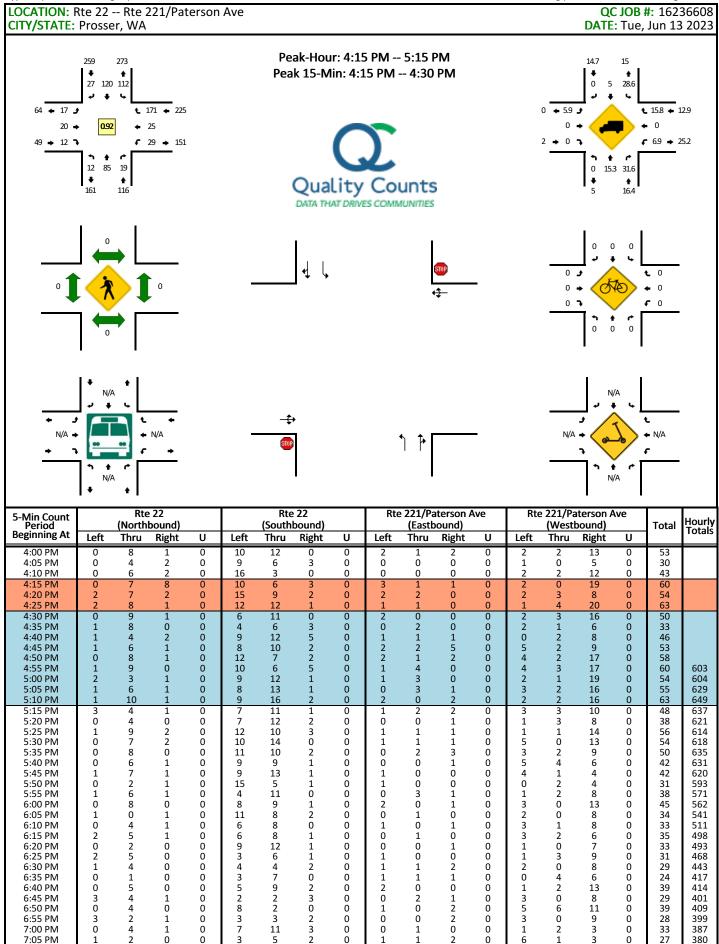
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	24	0	248	0	0	0	0	0	0	132	36	0	112	124	0	0	676
Heavy Trucks	8	0	24		0	0	0		0	8	0		40	12	0		92
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



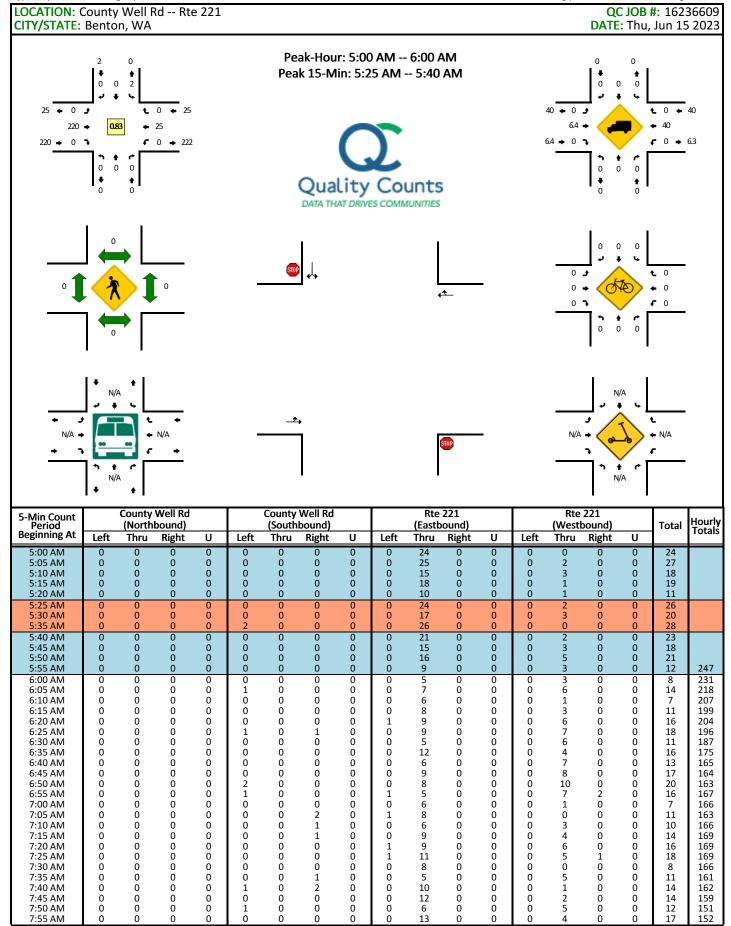
5-Min Count Period			22 bound)				22 bound)		'		ountry Rd oound)		'		ountry Ro bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	5	0	8	0	2	0	1	0	0	5	2	0	6	8	0	0	37	497
7:15 PM	4	0	10	0	0	1	0	0	0	1	2	0	11	7	0	0	36	495
7:20 PM	3	0	6	0	0	0	0	0	0	5	4	0	12	9	0	0	39	488
7:25 PM	2	0	11	0	0	0	0	0	0	5	2	0	6	3	0	0	29	470
7:30 PM	2	0	6	0	0	0	0	0	0	7	1	0	8	8	0	0	32	464
7:35 PM	2	0	7	0	0	0	0	0	0	4	2	0	7	6	1	0	29	443
7:40 PM	3	0	2	0	0	0	0	0	0	5	2	0	7	6	0	0	25	423
7:45 PM	1	0	9	0	0	0	0	0	0	4	0	0	9	9	0	0	32	421
7:50 PM	1	0	6	0	0	0	0	0	0	6	1	0	6	8	0	0	28	404
7:55 PM	5	0	8	0	0	0	0	0	0	3	2	0	8	8	0	0	34	402
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	a - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	100	8	272	0	0	0	0	0	0	204	56	0	196	160	4	0	10	00
Heavy Trucks	20	0	60		0	0	0		0	4	8		40	8	0		14	40
Buses																		_
Pedestrians	_	0				0				0				0				)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



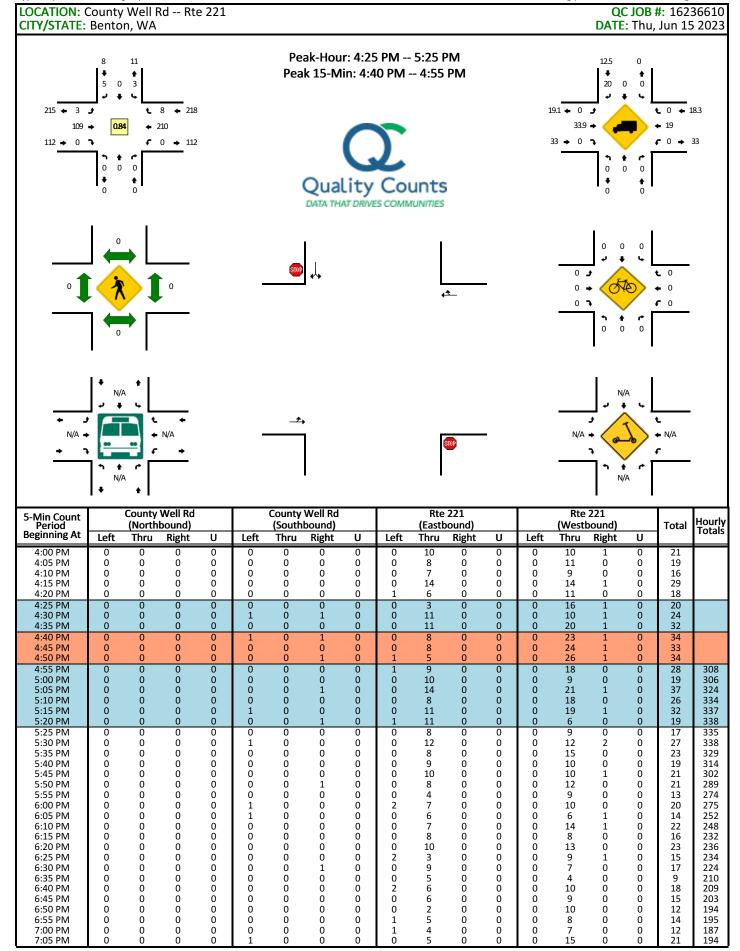
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	16	104	12	0	100	48	0	0	24	12	16	0	16	20	104	0	472
Heavy Trucks	0	0	0		24	20	0		0	0	0		0	0	20		64
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



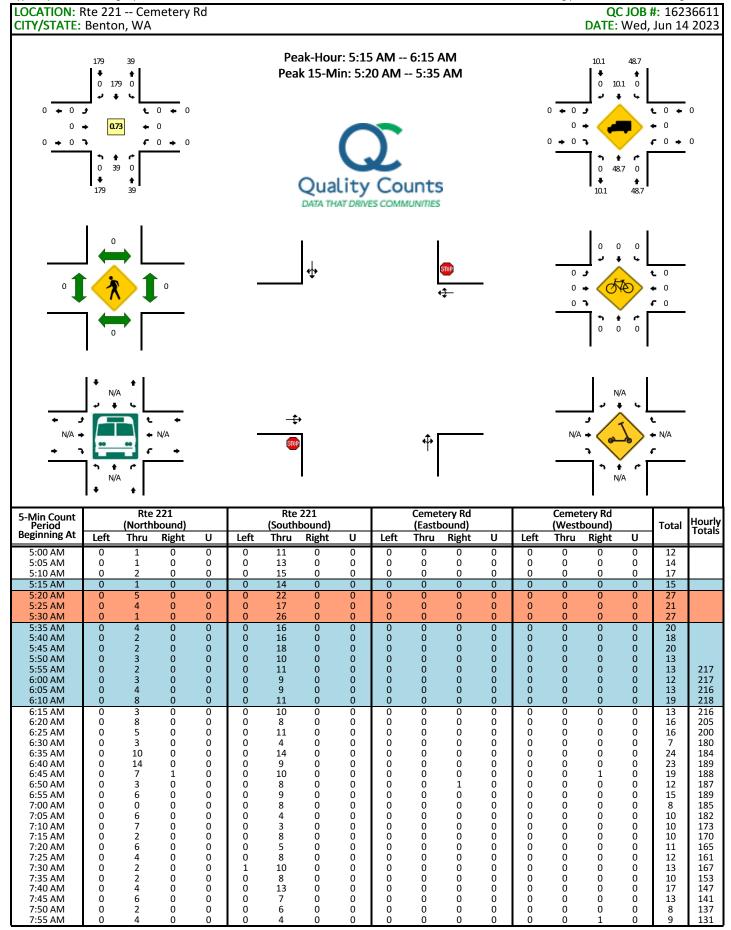
5-Min Count Period			22 bound)				e 22 bound)		Rte		aterson A bound)	ve	Rte		aterson A bound)	ve	Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		iotais
7:10 PM	0	5	0	0	8	1	2	0	1	0	0	0	0	1	9	0	27	374
7:15 PM	1	5	1	0	3	5	2	0	3	1	1	0	0	0	5	0	27	366
7:20 PM	1	5	3	0	2	12	1	0	0	1	1	0	0	1	6	0	33	366
7:25 PM	2	3	1	0	5	5	0	0	1	2	0	0	1	1	7	0	28	363
7:30 PM	1	3	1	0	3	6	1	0	1	0	0	0	2	0	3	0	21	355
7:35 PM	0	0	0	0	4	6	0	0	0	1	0	0	1	0	2	0	14	345
7:40 PM	0	0	0	0	5	4	0	0	0	0	3	0	0	2	5	0	19	325
7:45 PM	3	5	1	0	5	5	0	0	0	2	0	0	0	1	2	0	24	320
7:50 PM	2	2	0	0	3	5	0	0	2	0	1	0	1	0	3	0	19	300
7:55 PM	1	3	0	0	3	3	1	0	0	2	1	0	0	0	6	0	20	292
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	h - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	То	tai
All Vehicles	16	88	44	0	148	108	24	0	24	16	4	0	20	28	188	0	70	)8
Heavy Trucks	0	20	12		44	0	0		0	0	0		0	0	40		13	L6
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



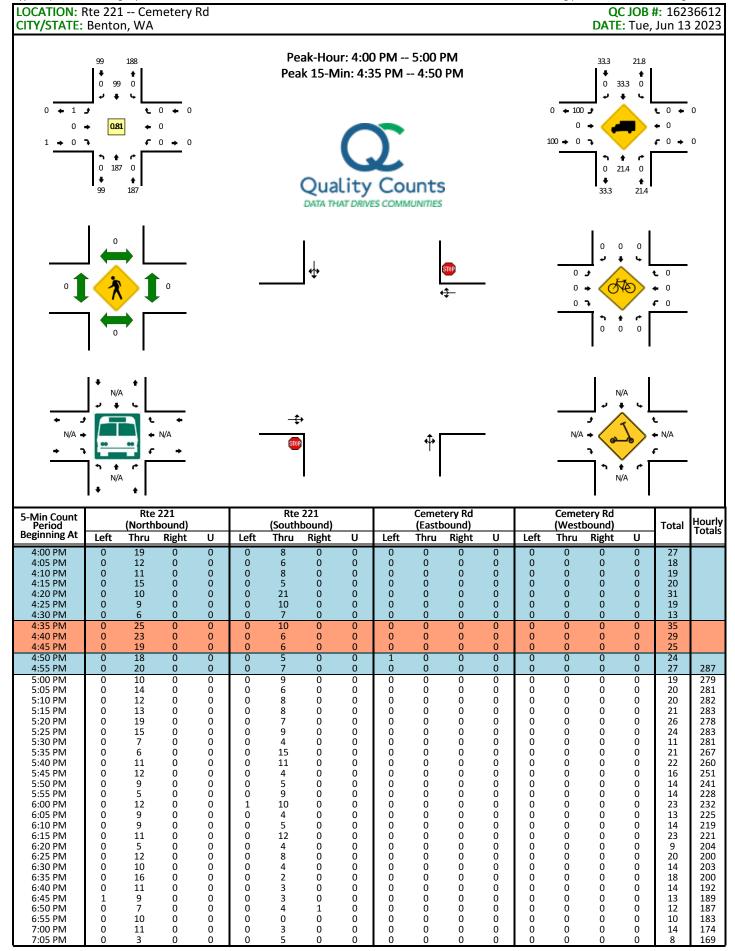
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	8	0	0	0	0	268	0	0	0	20	0	0	296
Heavy Trucks	0	0	0		0	0	0		0	16	0		0	12	0		28
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



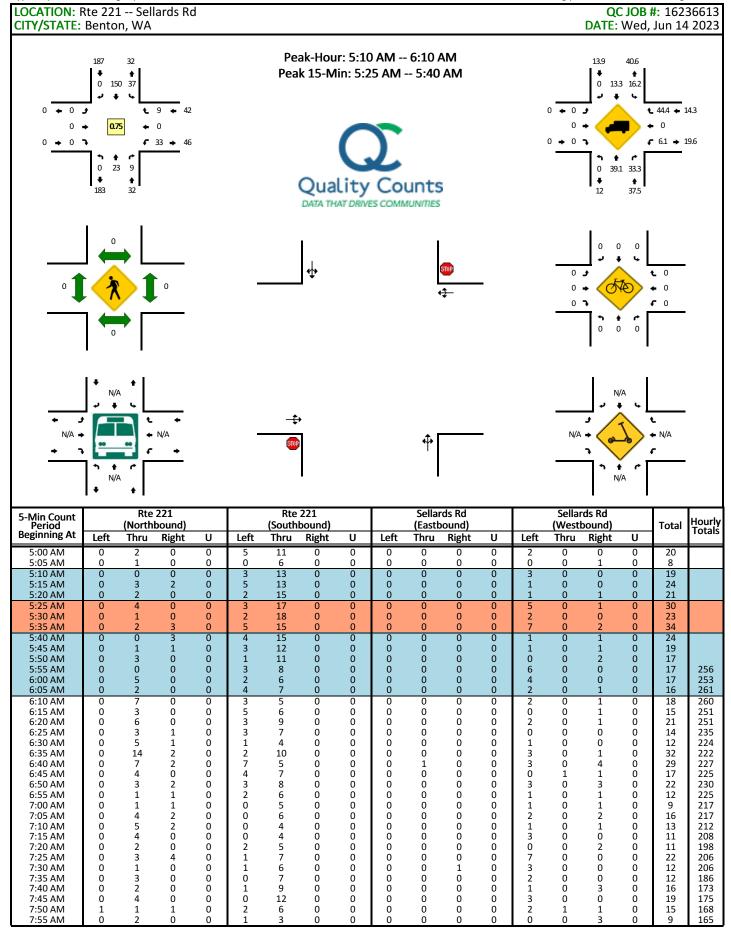
5-Min Count Period			Well Rd bound)				Well Rd bound)				221 oound)				221 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	0	0	0	0	0	0	0	0	7	0	0	0	3	0	0	10	182
7:15 PM	0	0	0	0	0	0	0	1	0	5	0	0	0	5	0	0	11	177
7:20 PM	0	0	0	0	0	0	1	0	1	3	0	0	0	5	0	0	10	164
7:25 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	3	0	0	5	154
7:30 PM	0	0	0	0	0	0	0	0	0	5	0	0	0	4	0	0	9	146
7:35 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	6	0	0	9	146
7:40 PM	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	0	8	136
7:45 PM	0	0	0	0	0	0	0	0	0	6	0	0	0	9	0	0	15	136
7:50 PM	0	0	0	0	0	0	0	0	0	8	0	0	0	3	0	0	11	135
7:55 PM	0	0	0	0	0	0	0	0	0	5	0	0	0	2	0	0	7	128
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	a - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	0	0	0	4	0	8	0	4	84	0	0	0	292	12	0	40	04
Heavy Trucks	0	0	0		0	0	4		0	32	0		0	28	0		6	4
Buses																		
Pedestrians		0	_		•	0				0	•			0				)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



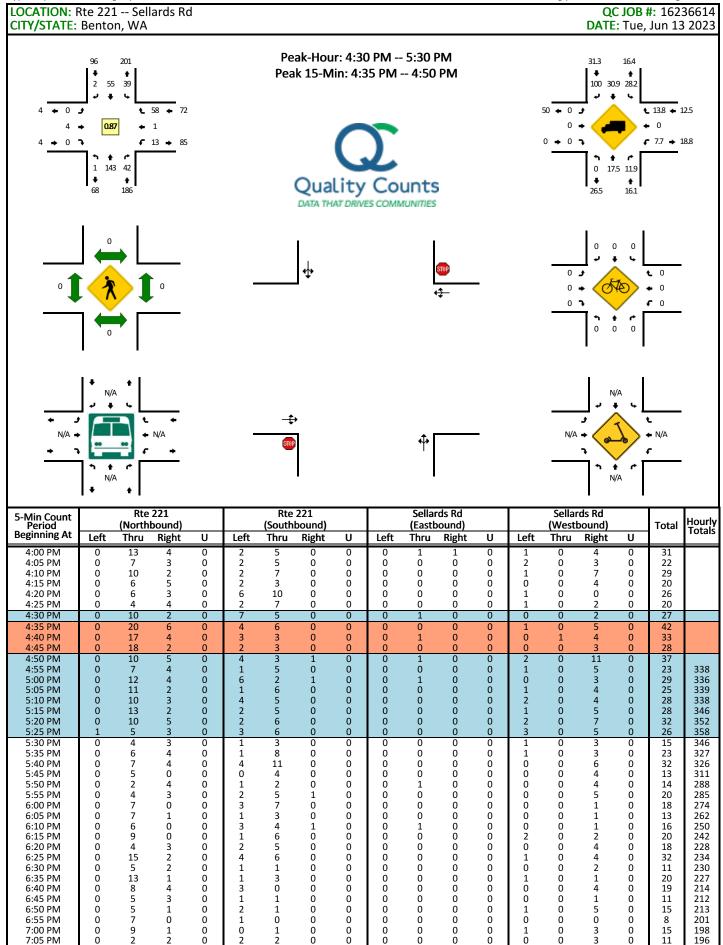
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	40	0	0	0	260	0	0	0	0	0	0	0	0	0	0	300
Heavy Trucks	0	12	0		0	4	0		0	0	0		0	0	0		16
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



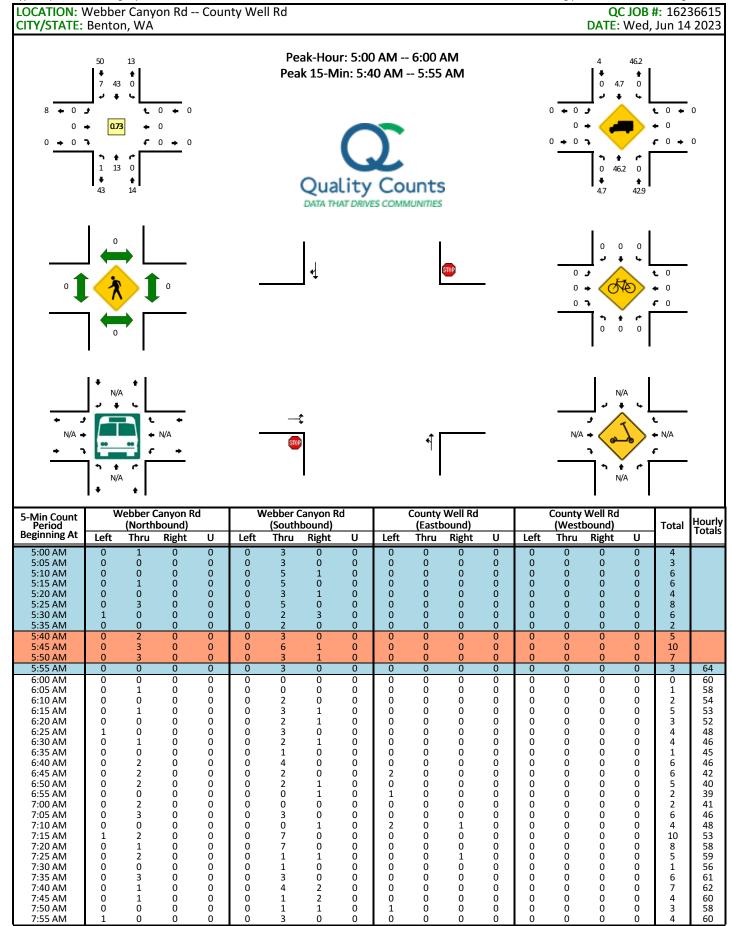
5-Min Count Period			221 bound)				221 bound)				tery Rd oound)				ery Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	5	0	0	0	4	0	0	0	0	0	0	0	0	0	0	9	164
7:15 PM	0	7	0	0	0	2	0	0	0	0	0	0	0	0	0	0	9	150
7:20 PM	0	5	0	0	0	6	0	0	0	0	0	0	0	0	0	0	11	152
7:25 PM	0	3	0	0	0	1	0	0	0	0	0	0	0	0	0	0	4	136
7:30 PM	0	5	0	0	0	2	0	0	0	0	0	0	0	0	0	0	7	129
7:35 PM	0	3	0	0	0	7	0	0	0	0	0	0	0	0	0	0	10	121
7:40 PM	0	6	0	0	0	3	0	0	0	0	0	0	0	0	0	0	9	116
7:45 PM	0	2	0	0	0	4	0	0	0	0	0	0	0	0	0	0	6	109
7:50 PM	0	3	0	0	0	4	0	0	0	0	0	0	0	0	0	0	7	104
7:55 PM	0	2	0	0	0	4	0	0	0	0	0	0	0	0	0	0	6	100
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	a - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	268	0	0	0	88	0	0	0	0	0	0	0	0	0	0	3.	56
Heavy Trucks	0	16	0		0	32	0		0	0	0		0	0	0		4	.8
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



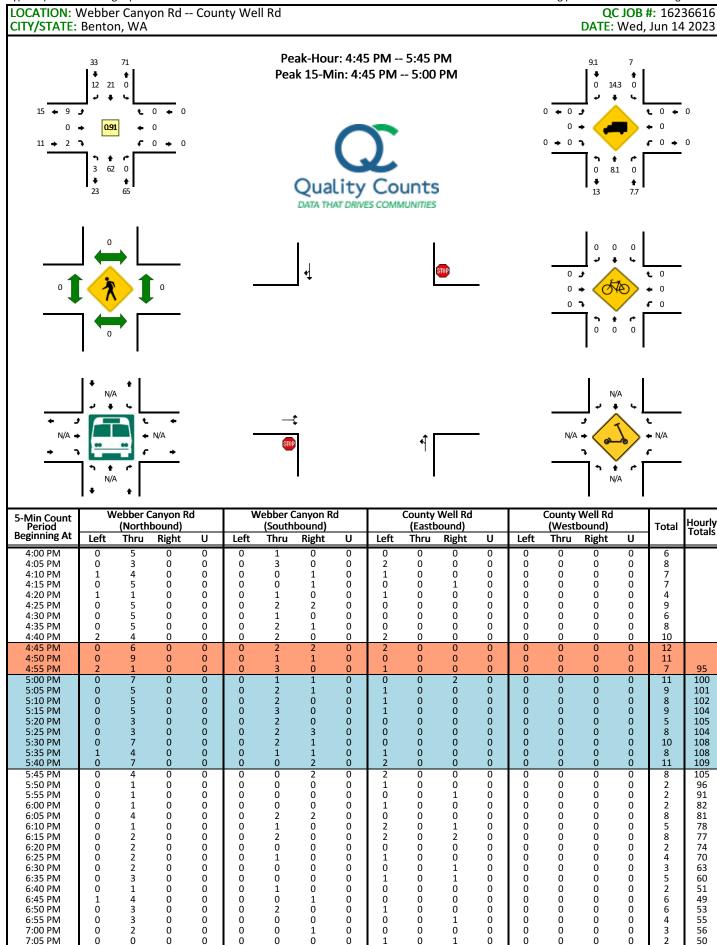
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	bound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOTAL
All Vehicles	0	28	12	0	40	200	0	0	0	0	0	0	56	0	12	0	348
Heavy Trucks	0	12	0		8	28	0		0	0	0		0	0	8		56
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



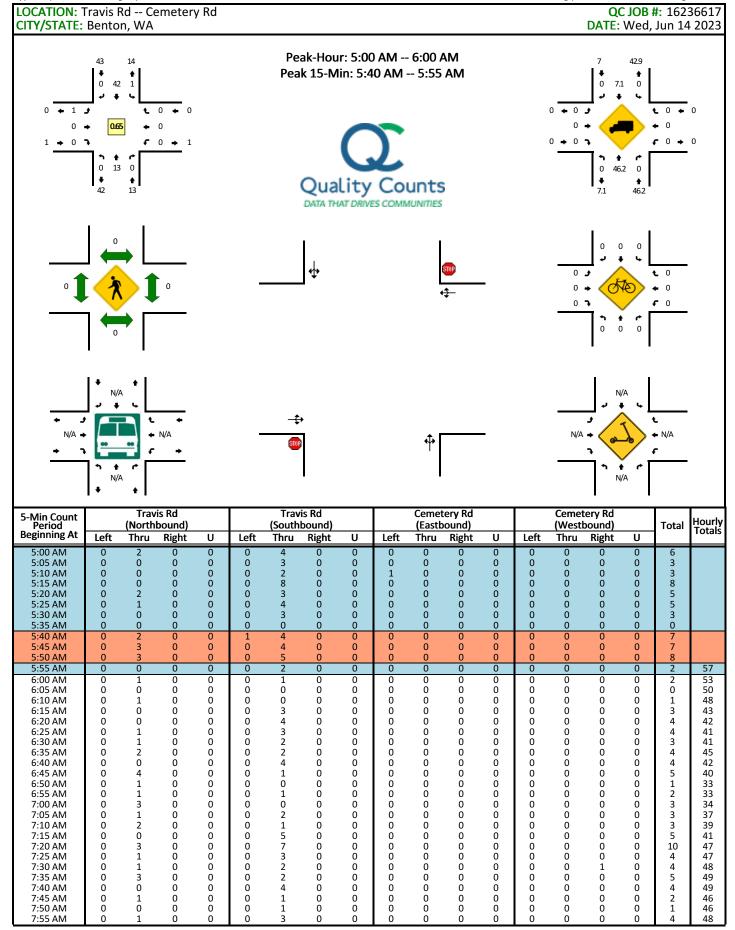
5-Min Count Period			221 bound)				221 bound)				rds Rd oound)				rds Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	6	1	0	1	5	0	0	0	0	0	0	0	0	0	0	13	193
7:15 PM	0	5	2	0	3	1	0	0	0	0	0	0	0	0	1	0	12	185
7:20 PM	0	2	0	0	4	1	0	0	0	0	0	0	1	0	1	0	9	176
7:25 PM	0	1	1	0	2	1	0	0	0	0	0	0	1	0	2	0	8	152
7:30 PM	0	3	1	0	1	0	0	0	0	0	0	0	0	0	3	0	8	149
7:35 PM	0	5	1	0	3	4	0	0	0	0	0	0	0	0	1	0	14	143
7:40 PM	0	4	1	0	1	3	0	0	0	0	0	0	0	0	1	0	10	134
7:45 PM	0	1	0	0	0	3	0	0	0	0	0	0	0	0	0	0	4	127
7:50 PM	0	2	1	0	1	4	0	0	0	0	0	0	0	0	0	0	8	120
7:55 PM	0	3	0	0	0	2	0	0	0	0	0	0	0	0	1	0	6	118
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	a - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	220	48	0	36	48	0	0	0	4	0	0	4	4	48	0	4:	12
Heavy Trucks	0	16	0		8	28	0		0	0	0		0	0	0		5	2
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



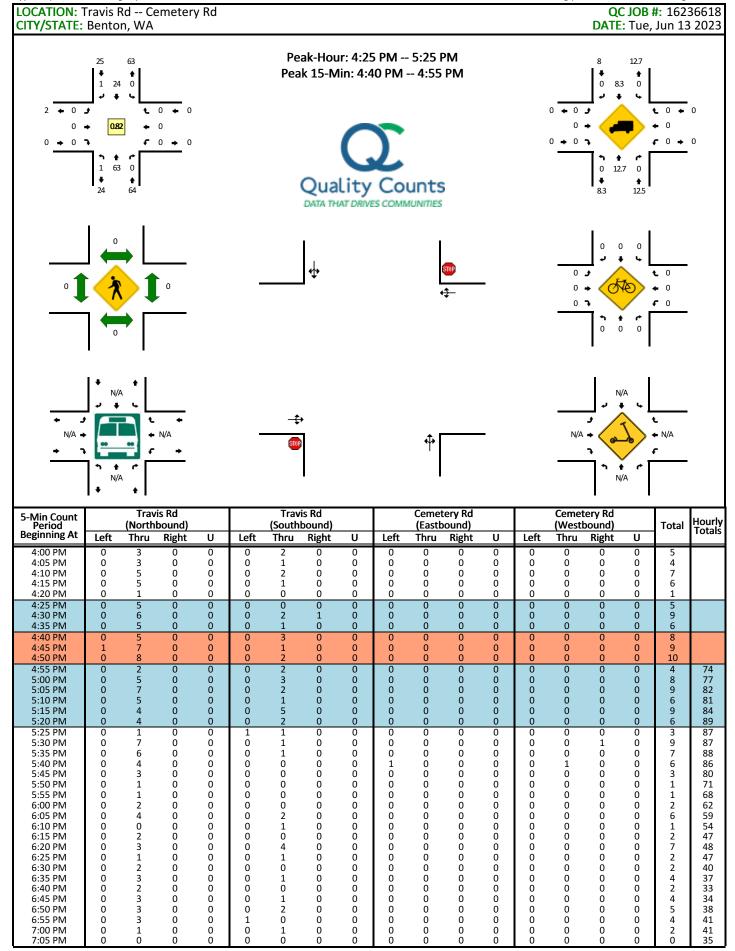
Peak 15-Min		North	bound		Southbound					Eastb	ound			West	Total		
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	32	0	0	0	48	8	0	0	0	0	0	0	0	0	0	88
Heavy Trucks	0	20	0		0	4	0		0	0	0		0	0	0		24
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



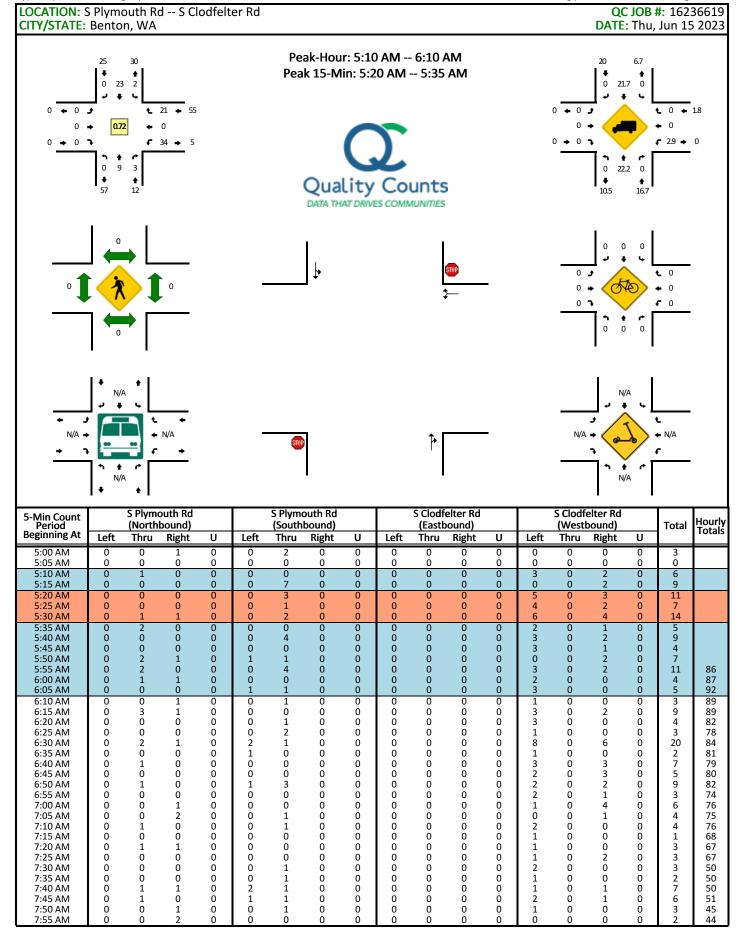
5-Min Count Period	W		Canyon Ro bound)	d	W		Canyon R bound)	d			Well Rd ound)				Well Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	46
7:15 PM	0	0	0	0	0	0	0	0	0	0	1	2	0	0	0	0	3	41
7:20 PM	1	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	3	42
7:25 PM	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	40
7:30 PM	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	38
7:35 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	34
7:40 PM	5	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	7	39
7:45 PM	1	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	35
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	29
7:55 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	26
Peak 15-Min		North	bound			Southbound Eastbound Westbound												
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	8	64	0	0	0	24	12	0	12	0	0	0	0	0	0	0	12	20
Heavy Trucks	0	0	0		0	4	0		0	0	0		0	0	0		4	4
Buses																		
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



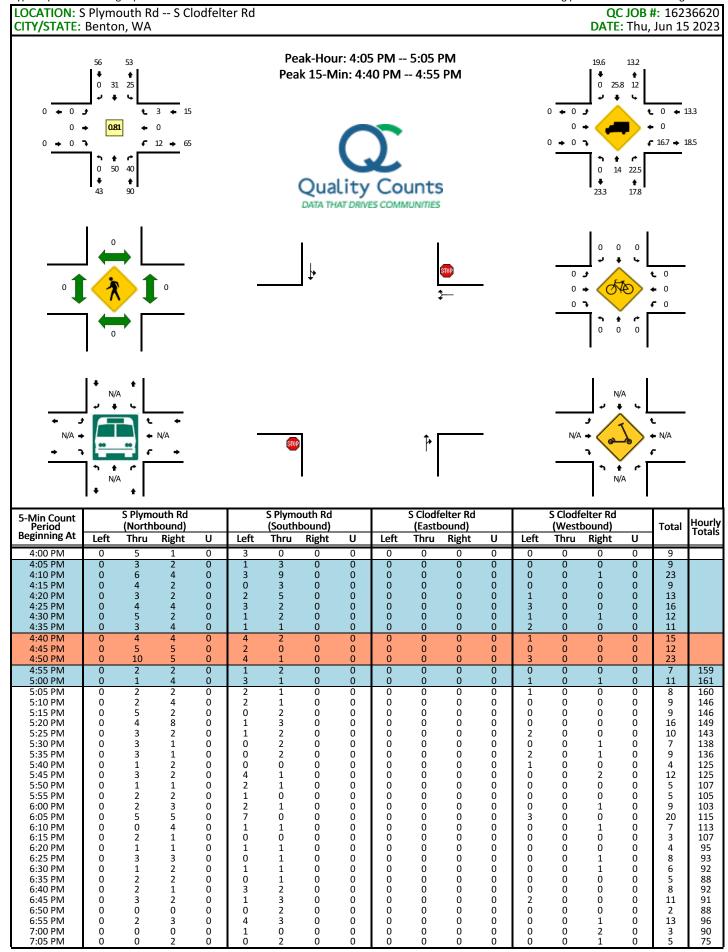
Peak 15-Min		North	bound		Southbound					Eastb	ound			West	Total		
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	32	0	0	4	52	0	0	0	0	0	0	0	0	0	0	88
Heavy Trucks	0	20	0		0	4	0		0	0	0		0	0	0		24
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



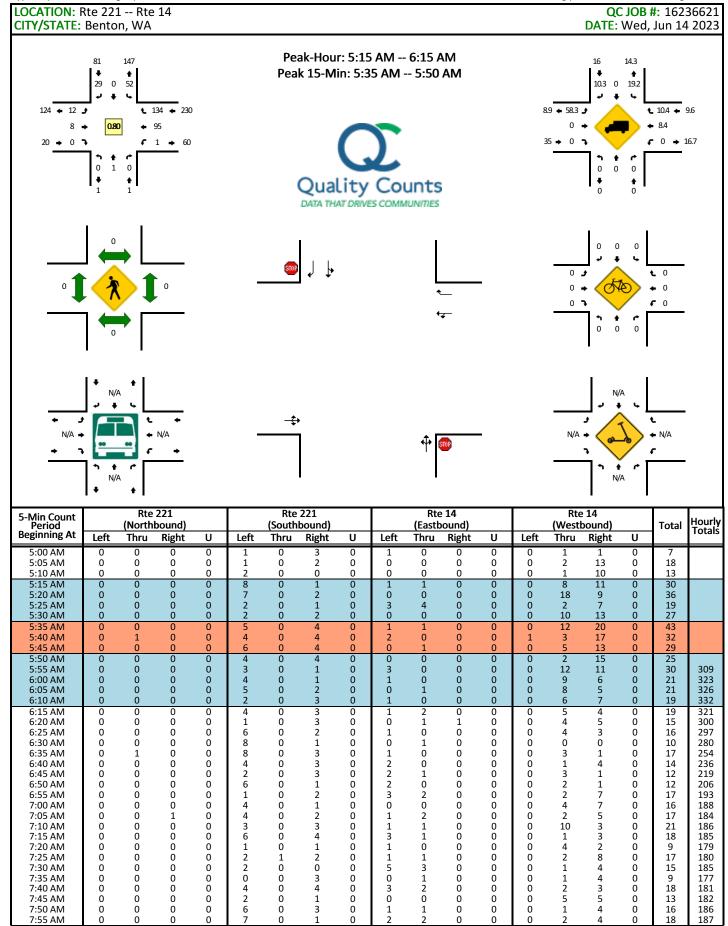
5-Min Count Period		Travis Rd (Northbound)				Travis Rd (Southbound)					tery Rd oound)			Cemet (Westl	Total	Hourly Totals		
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	2	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3	37
7:15 PM	0	1	0	0	1	2	0	0	0	0	0	0	0	0	0	0	4	39
7:20 PM	0	2	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3	35
7:25 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	34
7:30 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	1	0	3	35
7:35 PM	0	2	0	0	2	0	0	0	0	0	0	0	0	0	2	0	6	37
7:40 PM	0	3	0	0	1	1	0	0	0	0	0	0	0	1	0	0	6	41
7:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	38
7:50 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0	2	35
7:55 PM	0	1	0	0	1	0	0	0	1	0	0	0	0	0	0	0	3	34
Peak 15-Min		North	bound			South	bound		Eastbound Westbound						т.	s - 1		
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	То	taı
All Vehicles	4	80	0	0	0	24	0	0	0	0	0	0	0	0	0	0	10	)8
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



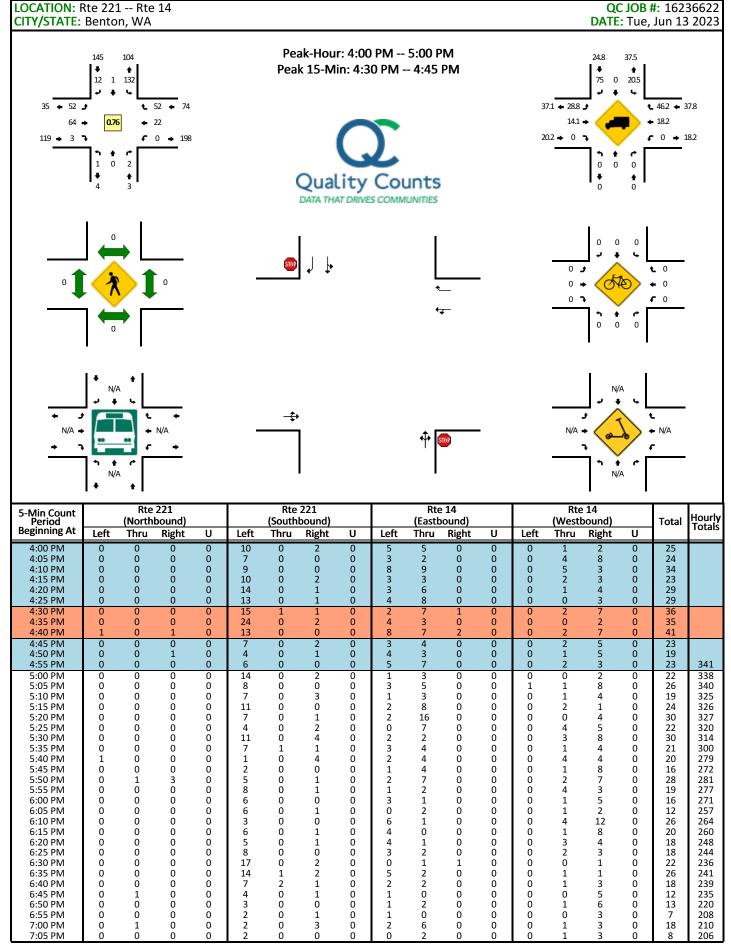
Peak 15-Min Flowrates		North	bound		Southbound				Eastb	ound			West	Total			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	4	4	0	0	24	0	0	0	0	0	0	60	0	36	0	128
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	0		0
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



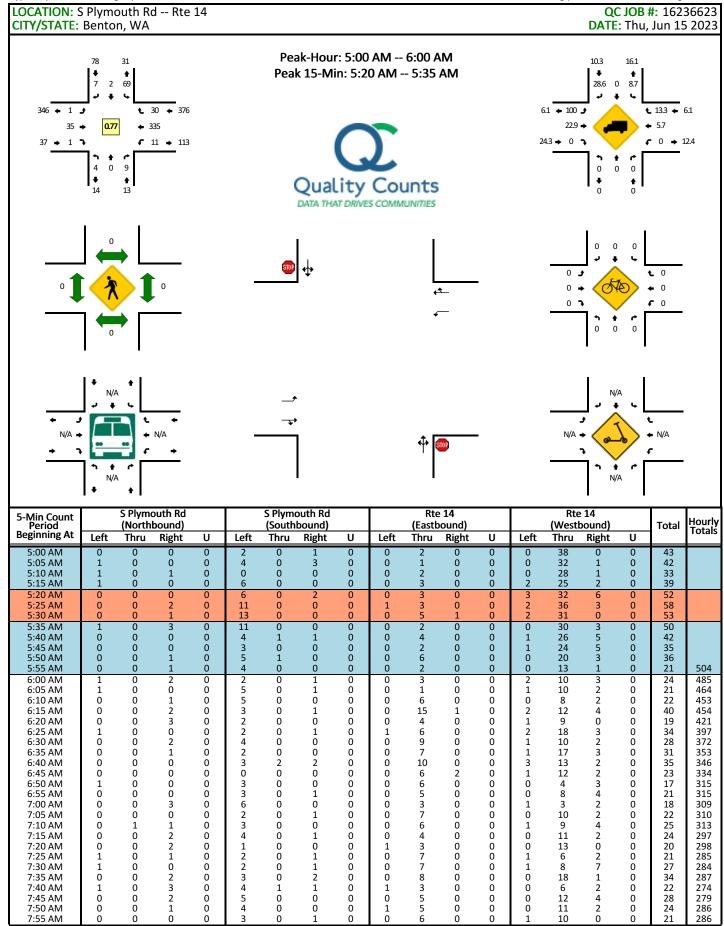
5-Min Count Period			outh Rd bound)				outh Rd bound)				elter Rd ound)				elter Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOTALS
7:10 PM	0	1	1	0	1	0	0	0	0	0	0	0	0	0	0	0	3	71
7:15 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	69
7:20 PM	0	1	0	0	3	3	0	0	0	0	0	0	0	0	0	0	7	72
7:25 PM	0	2	0	0	2	2	0	0	0	0	0	0	0	0	0	0	6	70
7:30 PM	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	4	68
7:35 PM	0	0	0	0	0	4	0	0	0	0	0	0	1	0	0	0	5	68
7:40 PM	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	4	64
7:45 PM	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	2	55
7:50 PM	0	3	0	0	1	0	0	0	0	0	0	0	0	0	0	0	4	57
7:55 PM	0	1	1	0	0	2	0	0	0	0	0	0	1	0	0	0	5	49
Peak 15-Min		North	bound		Southbound					Eastb	ound			Westl	bound		Total	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tai
All Vehicles	0	76	56	0	40	12	0	0	0	0	0	0	16	0	0	0	20	00
Heavy Trucks Buses	0	12	12		0	0	0		0	0	0		0	0	0		2	4
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0			)
Comments:																		



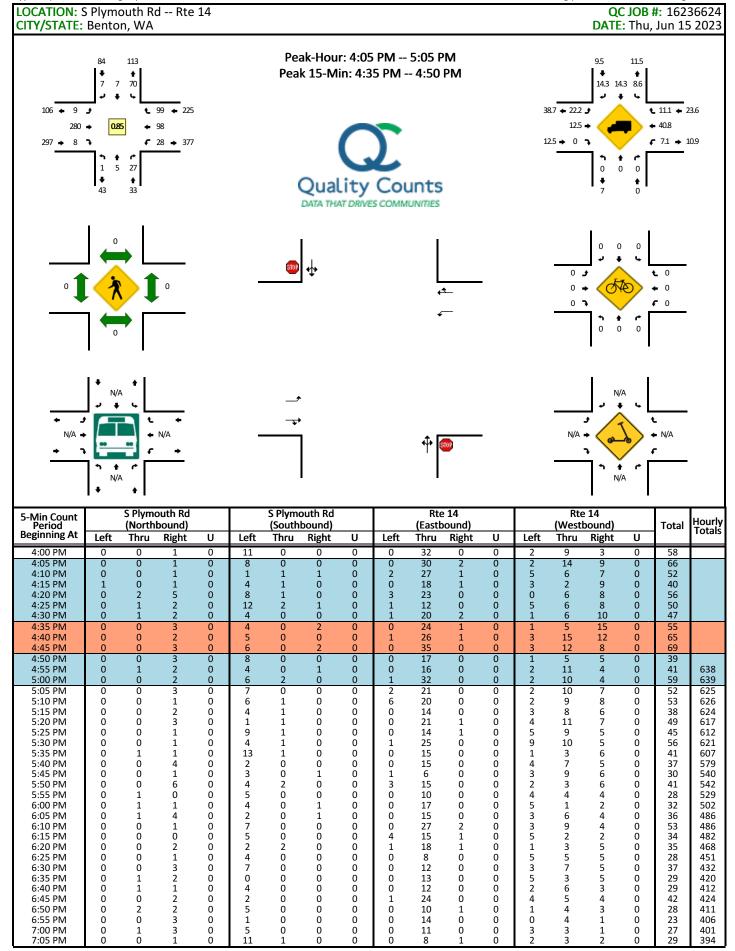
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	4	0	0	60	0	48	0	12	8	0	0	4	80	200	0	416
Heavy Trucks	0	0	0		4	0	0		4	0	0		0	12	20		40
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



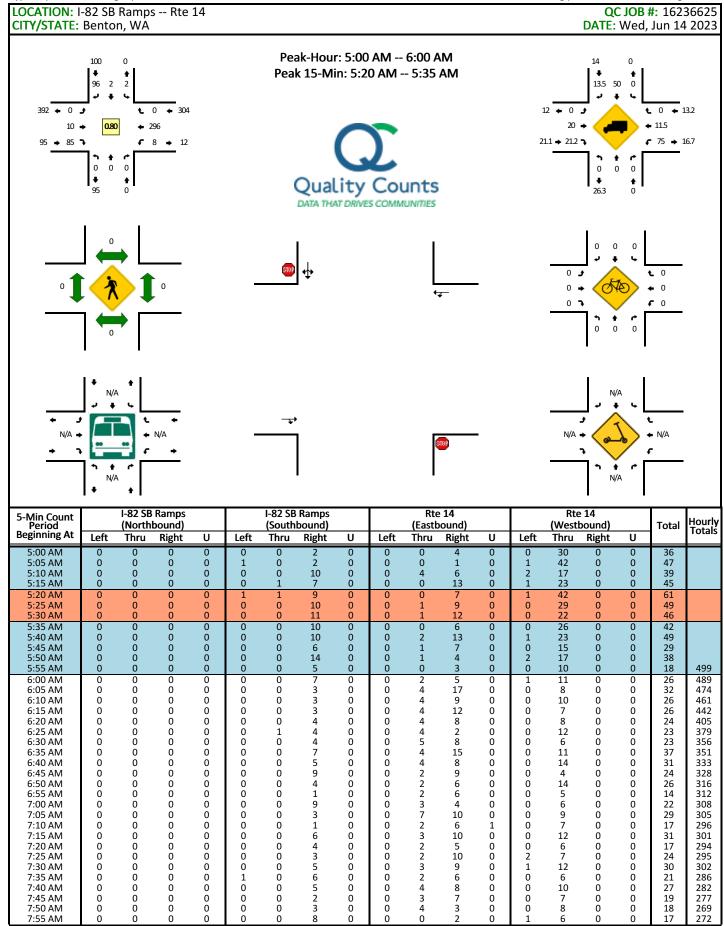
5-Min Count Period			221 bound)				221 bound)				e 14 oound)				e 14 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	1	0	1	0	1	1	0	0	0	0	2	0	6	186
7:15 PM	0	0	0	0	3	0	0	0	2	0	0	0	0	0	1	0	6	172
7:20 PM	0	0	0	0	4	0	0	0	2	2	0	0	0	0	3	0	11	165
7:25 PM	0	0	0	0	1	0	1	0	2	1	0	0	0	1	2	0	8	155
7:30 PM	0	0	0	0	1	0	0	0	0	2	0	0	0	0	3	0	6	139
7:35 PM	0	0	0	0	1	0	0	0	1	0	0	0	0	2	2	0	6	119
7:40 PM	0	0	0	0	0	0	0	0	1	1	0	0	0	0	5	0	7	108
7:45 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	2	1	0	5	101
7:50 PM	0	0	0	0	2	0	0	0	0	0	0	0	0	1	1	0	4	92
7:55 PM	0	0	0	0	3	0	0	0	3	0	0	0	0	1	0	0	7	92
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	4	0	4	0	208	4	12	0	56	68	12	0	0	16	64	0	44	48
Heavy Trucks	0	0	0		28	0	4		12	12	0		0	4	36		9	16
Buses																		
Pedestrians		0				0				0				0				0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



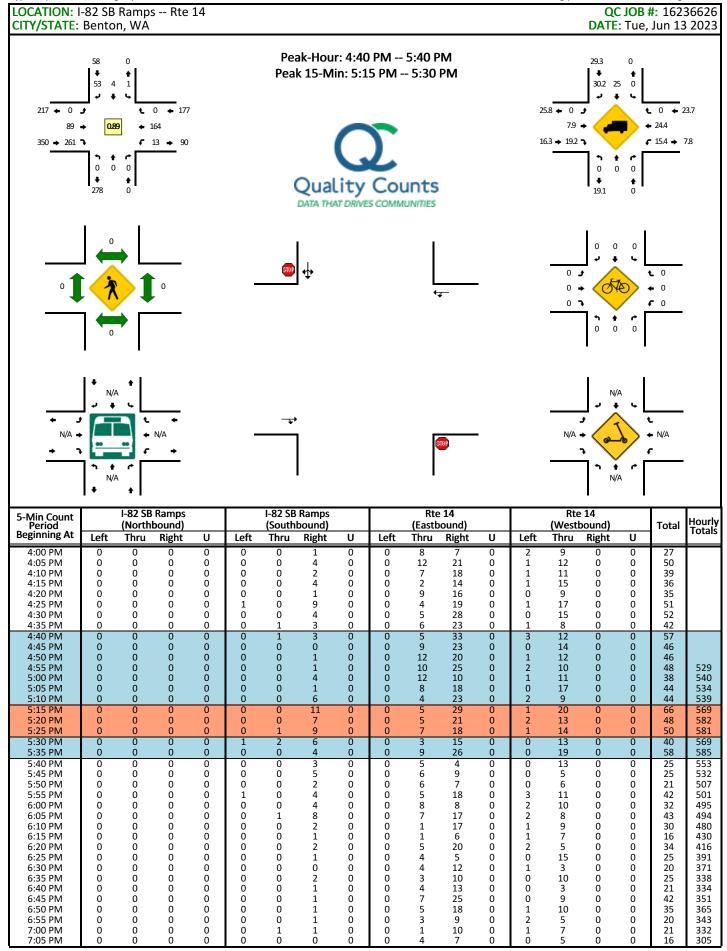
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	12	0	120	0	8	0	4	44	4	0	28	396	36	0	652
Heavy Trucks	0	0	0		8	0	4		4	4	0		0	20	8		48
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



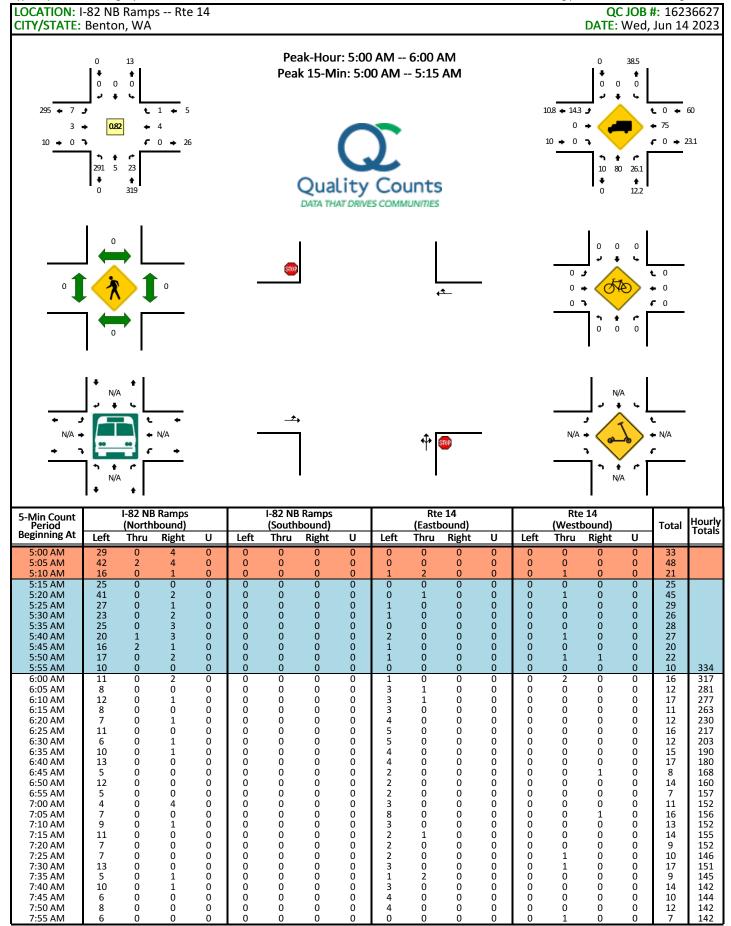
5-Min Count Period			outh Rd bound)				outh Rd bound)				e 14 oound)				e 14 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOLAIS
7:10 PM	0	0	0	0	3	0	0	0	0	6	0	0	0	4	5	0	18	359
7:15 PM	0	0	0	0	0	0	0	0	0	2	0	0	3	4	2	0	11	336
7:20 PM	0	0	1	0	1	0	0	0	0	7	0	0	4	7	4	0	24	325
7:25 PM	0	0	2	0	1	0	0	0	0	6	0	0	3	4	0	0	16	313
7:30 PM	0	0	1	0	4	0	1	0	0	4	0	0	3	4	1	0	18	294
7:35 PM	0	0	0	0	4	0	0	0	0	2	0	0	0	2	3	0	11	276
7:40 PM	0	0	1	0	4	0	0	0	1	7	0	0	2	6	4	0	25	272
7:45 PM	0	0	1	0	6	0	0	0	0	5	0	0	3	0	4	0	19	249
7:50 PM	0	0	0	0	2	0	0	0	0	3	0	0	2	0	0	0	7	228
7:55 PM	0	1	2	0	2	0	0	0	0	4	0	0	2	2	0	0	13	218
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	4-1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	1 10	tal
All Vehicles	0	0	32	0	60	0	16	0	4	340	8	0	28	128	140	0	7.	56
Heavy Trucks	0	0	0		4	0	0		0	24	0		0	40	16		8	4
Buses																		
Pedestrians		0				0				0				0				)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



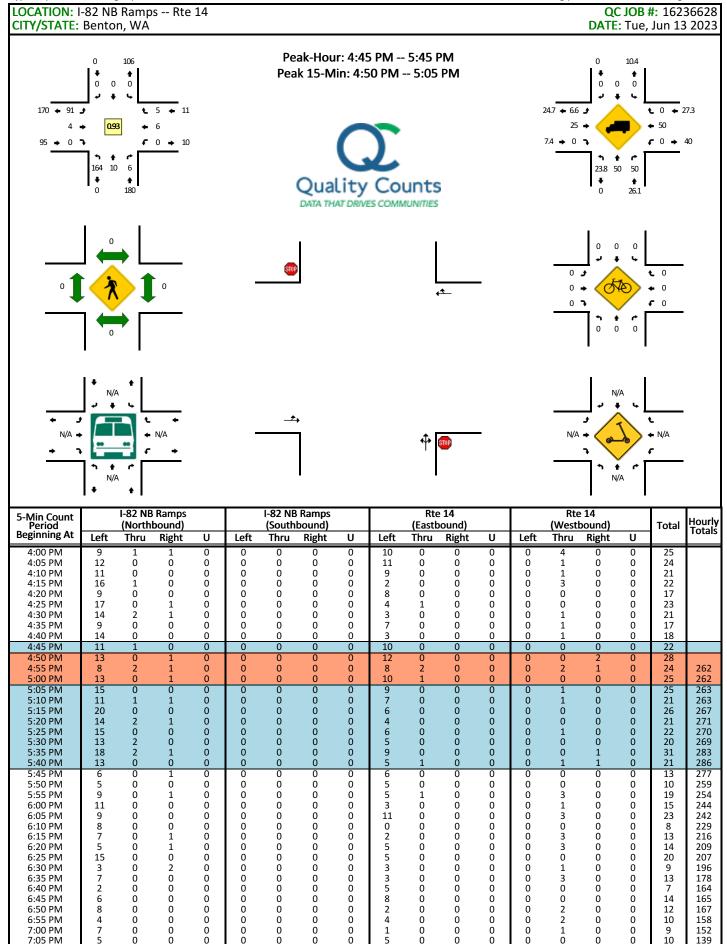
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	4	4	120	0	0	8	112	0	4	372	0	0	624
Heavy Trucks	0	0	0		0	0	20		0	0	24		4	24	0		72
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



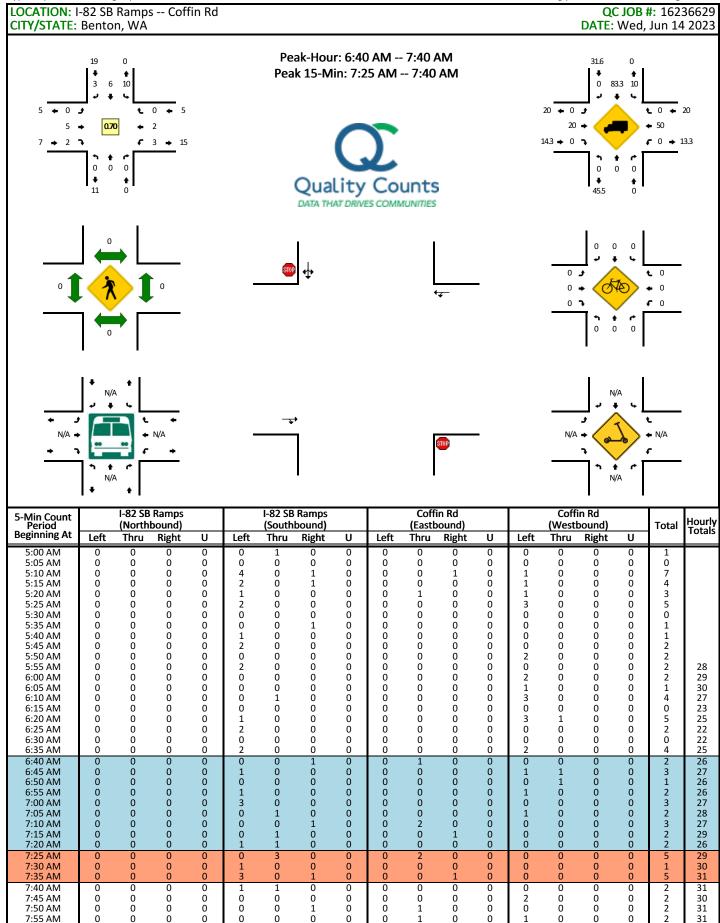
5-Min Count Period			Ramps bound)				Ramps bound)				e 14 bound)				e 14 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	0	0	0	2	7	0	0	7	0	0	16	291
7:15 PM	0	0	0	0	0	0	2	0	0	8	12	0	0	9	0	0	31	306
7:20 PM	0	0	0	0	0	0	3	0	0	0	6	0	0	5	0	0	14	286
7:25 PM	0	0	0	0	0	0	0	0	0	4	3	0	2	4	0	0	13	274
7:30 PM	0	0	0	0	0	0	3	0	0	0	7	0	0	5	0	0	15	269
7:35 PM	0	0	0	0	0	1	4	0	0	2	10	0	0	7	0	0	24	268
7:40 PM	0	0	0	0	1	0	0	0	0	3	7	0	0	5	0	0	16	263
7:45 PM	0	0	0	0	0	0	2	0	0	0	6	0	0	1	0	0	9	230
7:50 PM	0	0	0	0	0	0	0	0	0	1	2	0	1	4	0	0	8	203
7:55 PM	0	0	0	0	0	0	2	0	0	0	7	0	0	4	0	0	13	196
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	a - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	0	0	0	0	4	108	0	0	68	272	0	16	188	0	0	65	56
Heavy Trucks	0	0	0		0	0	36		0	0	44		4	36	0		12	20
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



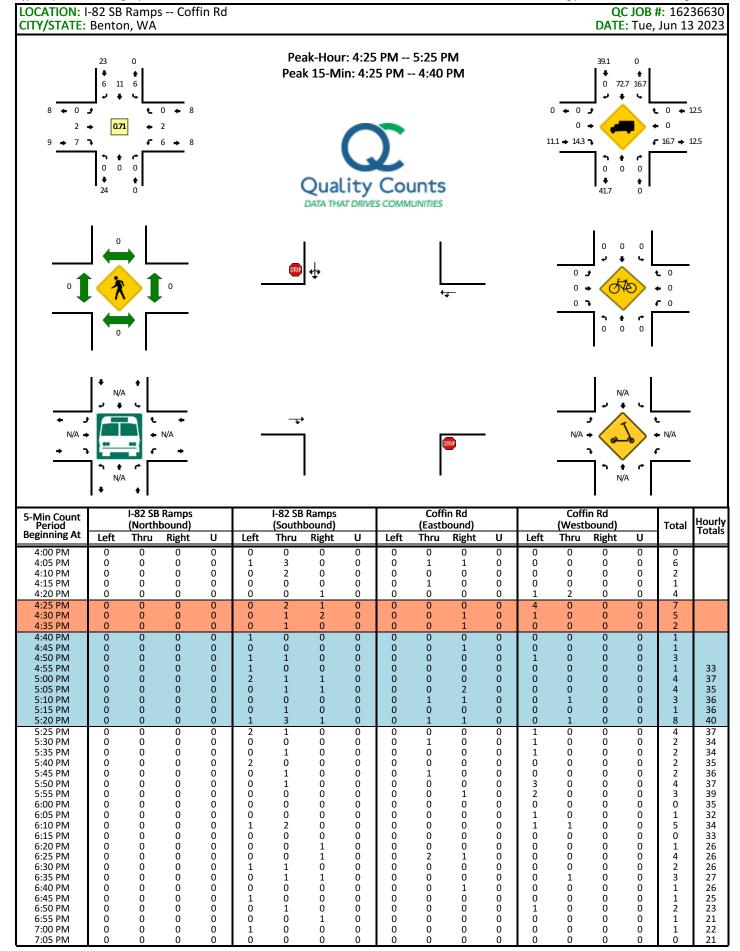
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOTAL
All Vehicles	348	8	36	0	0	0	0	0	4	8	0	0	0	4	0	0	408
Heavy Trucks Buses	28	4	4		0	0	0		4	0	0		0	0	0		40
Pedestrians		0				0				0				0			0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		0
Comments:																	



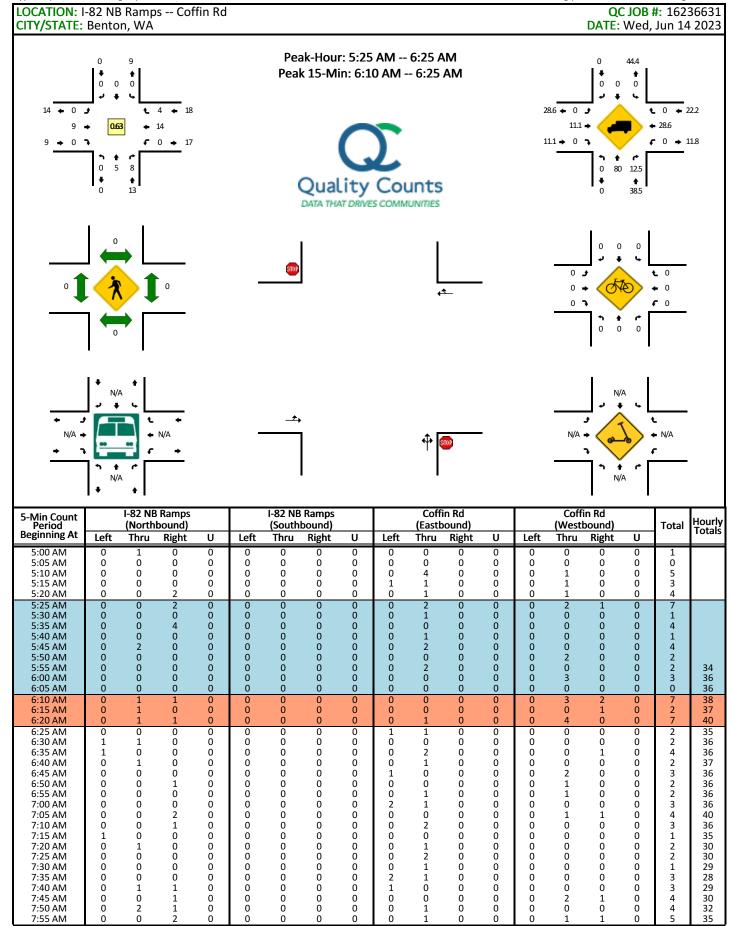
5-Min Count Period			Ramps bound)				Ramps bound)				e 14 bound)				e 14 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	7	0	1	0	0	0	0	0	1	1	0	0	0	0	0	0	10	141
7:15 PM	10	0	0	0	0	0	0	0	7	0	0	0	0	0	0	0	17	145
7:20 PM	5	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	6	137
7:25 PM	5	0	0	0	0	0	0	0	4	0	0	0	0	1	0	0	10	127
7:30 PM	5	0	1	0	0	0	0	0	1	0	0	0	0	0	0	0	7	125
7:35 PM	6	2	1	0	0	0	0	0	2	0	0	0	0	0	0	0	11	123
7:40 PM	6	0	0	0	0	0	0	0	3	0	0	0	0	0	0	0	9	125
7:45 PM	1	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	2	113
7:50 PM	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	106
7:55 PM	3	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	4	100
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	A - I
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	136	8	12	0	0	0	0	0	120	12	0	0	0	8	12	0	30	08
Heavy Trucks	44	4	4		0	0	0		8	4	0		0	8	0		7	2
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



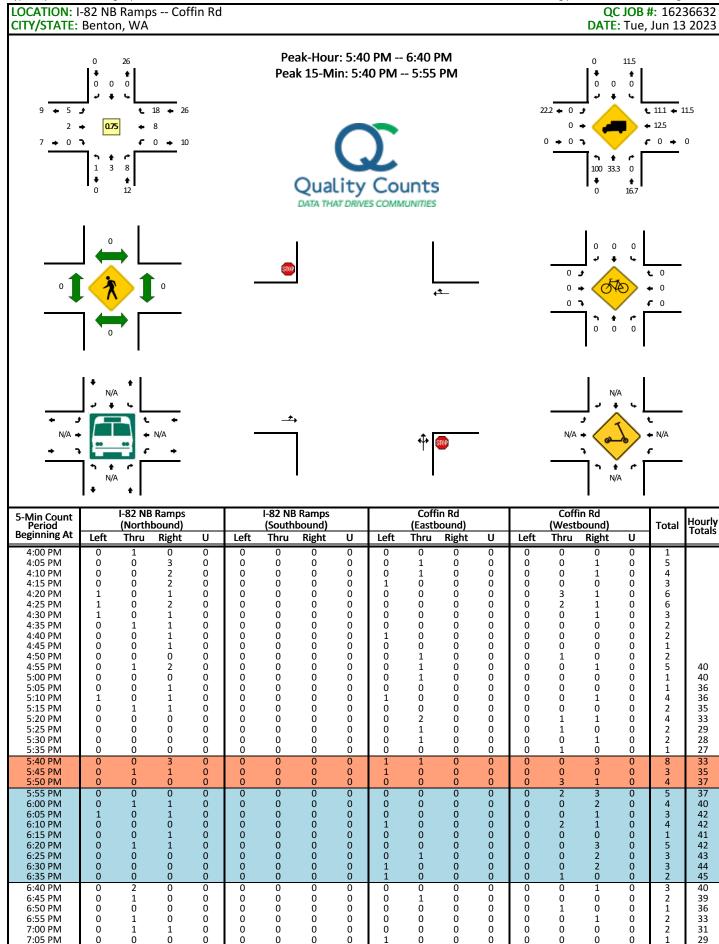
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	16	12	4	0	0	8	4	0	0	0	0	0	44
Heavy Trucks	0	0	0		4	8	0		0	0	0		0	0	0		12
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



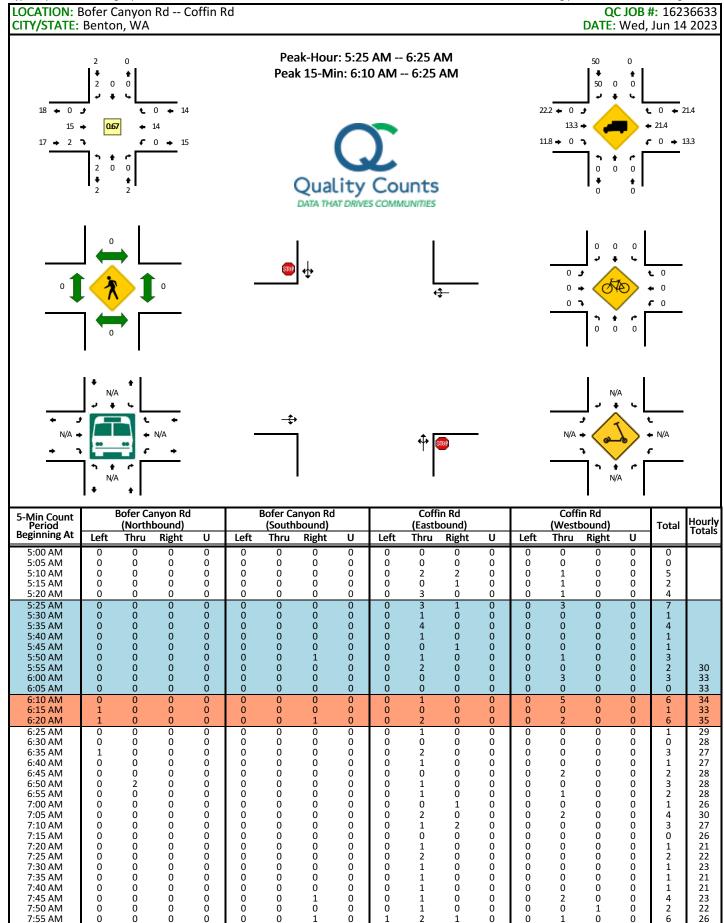
5-Min Count Period			Ramps bound)				Ramps bound)				in Rd oound)				in Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	2	18
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17
7:25 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	14
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12
7:35 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1	10
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
7:45 PM	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3	11
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
7:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	4-I
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	1 10	tal
All Vehicles	0	0	0	0	0	16	12	0	0	0	8	0	20	0	0	0	5	6
Heavy Trucks	0	0	0		0	12	0		0	0	4		4	0	0		2	20
Buses																		
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	12	8	0	0	0	0	0	0	4	0	0	0	28	12	0	64
Heavy Trucks	0	8	0		0	0	0		0	4	0		0	8	0		20
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

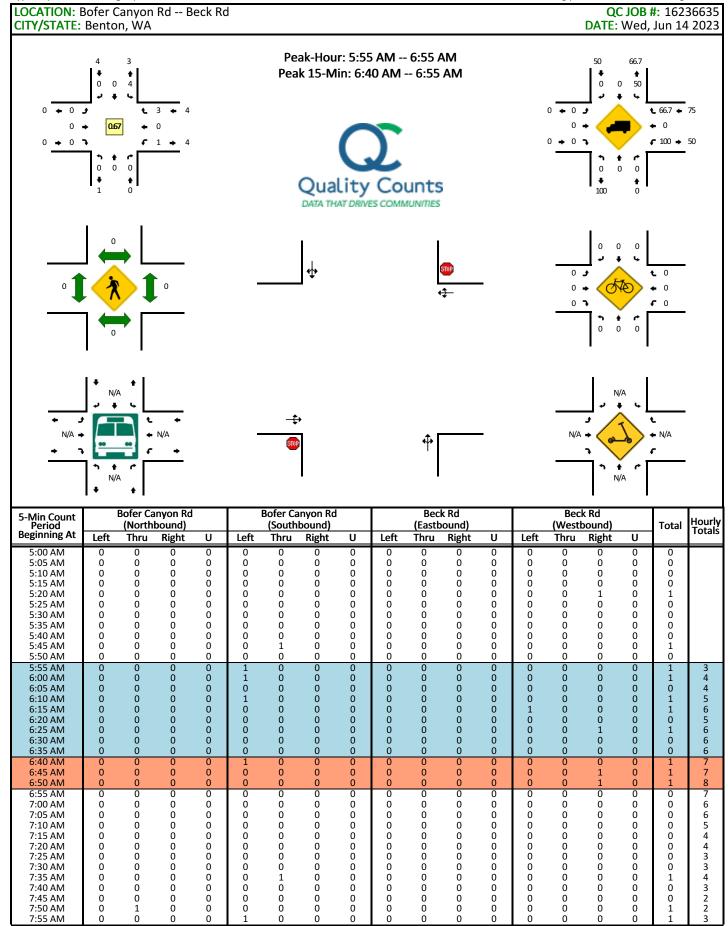


5-Min Count Period			Ramps bound)				Ramps bound)				in Rd ound)				in Rd bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	2	27
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	26
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	22
7:25 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	20
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	17
7:35 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	16
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13
7:45 PM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	13
7:50 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	13
7:55 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	12
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	4-I
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	4	16	0	0	0	0	0	8	4	0	0	0	12	16	0	6	0
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	8		8	3
Buses Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	Ö	0		0	0	0		0	Ö	0		0	0	0			Ď

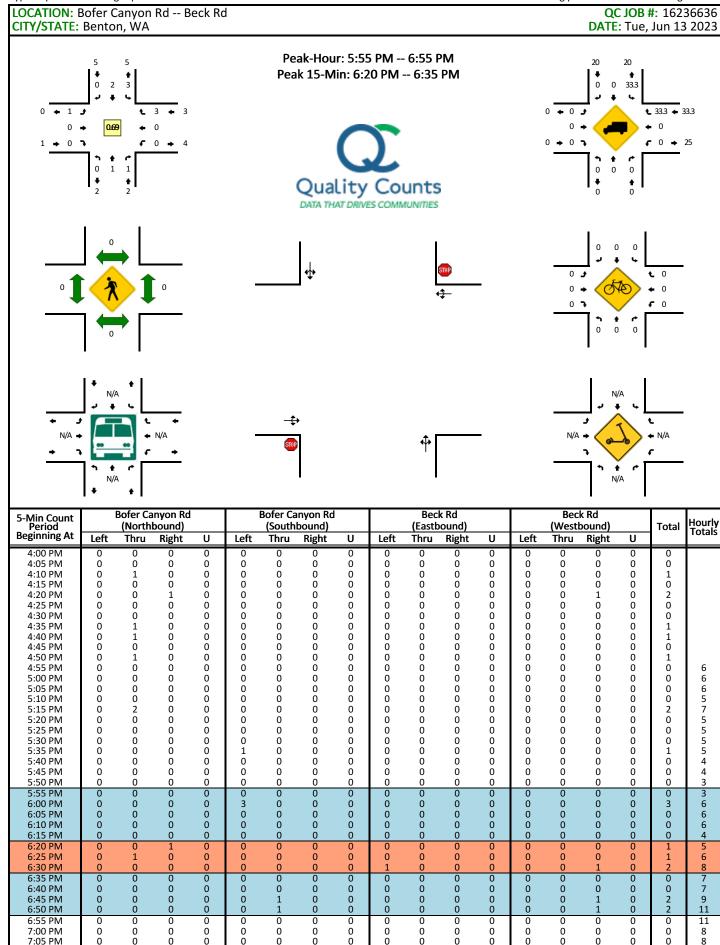


Peak 15-Min	Northbound				Southbound			Eastbound					West	Total			
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	8	0	0	0	0	0	4	0	0	12	0	0	0	28	0	0	52
Heavy Trucks	0	0	0		0	0	4		0	4	0		0	4	0		12
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

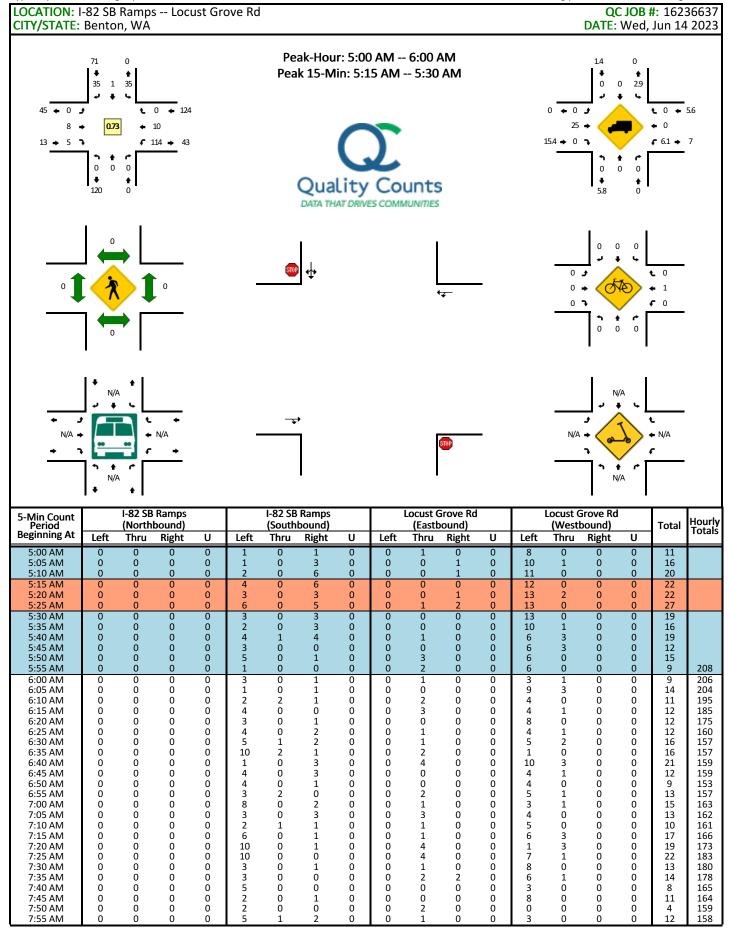
5-Min Count Period			nyon Rd bound)				anyon Rd bound)				in Rd oound)				in Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	2	21
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	20
7:20 PM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	17
7:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	11
7:35 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	10
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
7:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
Peak 15-Min		North	bound		Southbound					Eastb	ound		Westbound				Total	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	1 10	tai
All Vehicles	8	4	0	0	0	0	0	0	0	4	0	0	0	36	0	0	5	2
Heavy Trucks	4	0	0		0	0	0		0	0	0		0	0	0			4
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



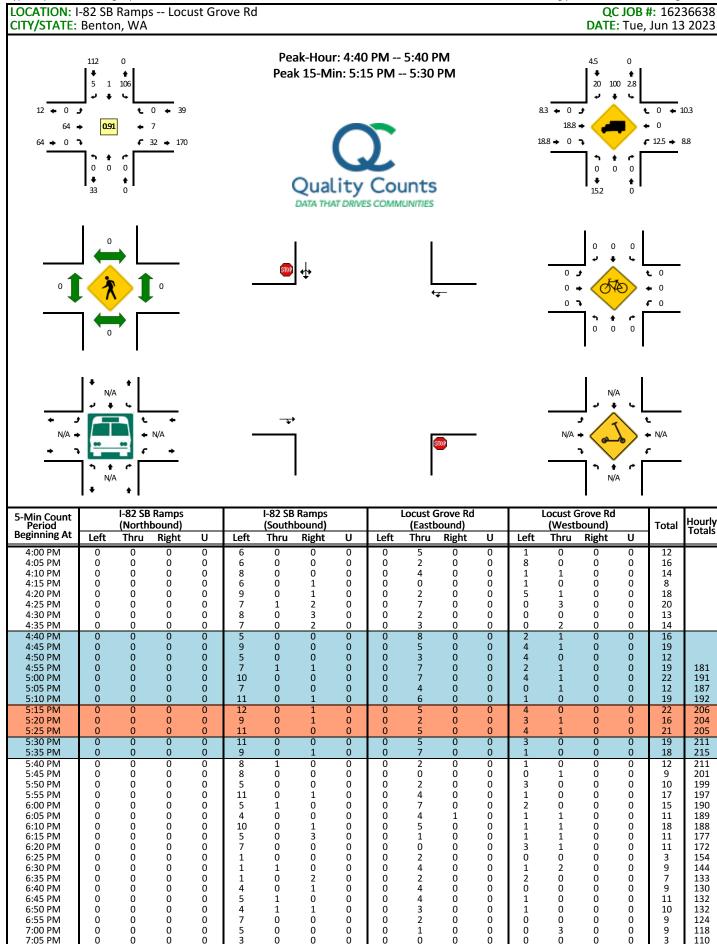
Peak 15-Min	Northbound				Southbound			Eastbound					West	Total			
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOTAL
All Vehicles	0	0	0	0	4	0	0	0	0	0	0	0	0	0	8	0	12
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	4		4
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



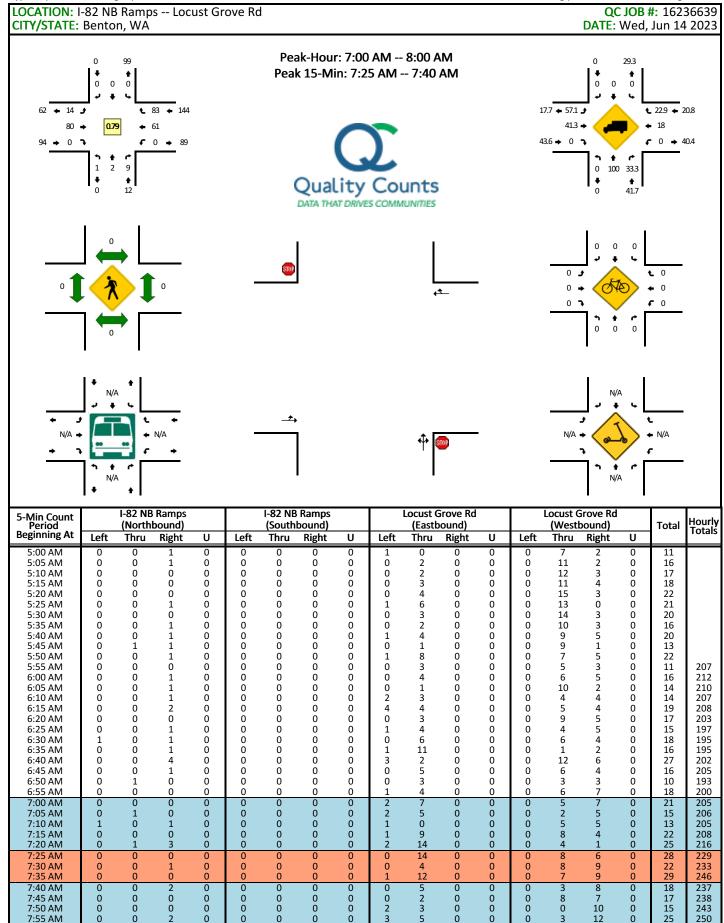
5-Min Count Period			nyon Rd bound)				anyon Rd bound)				k Rd oound)				k Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	9
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8
7:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
7:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Peak 15-Min		North	bound		Southbound					Eastb	ound		Westbound				Total	
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	1 10	tai
All Vehicles	0	4	4	0	0	0	0	0	4	0	0	0	0	0	4	0	1	.6
Heavy Trucks	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



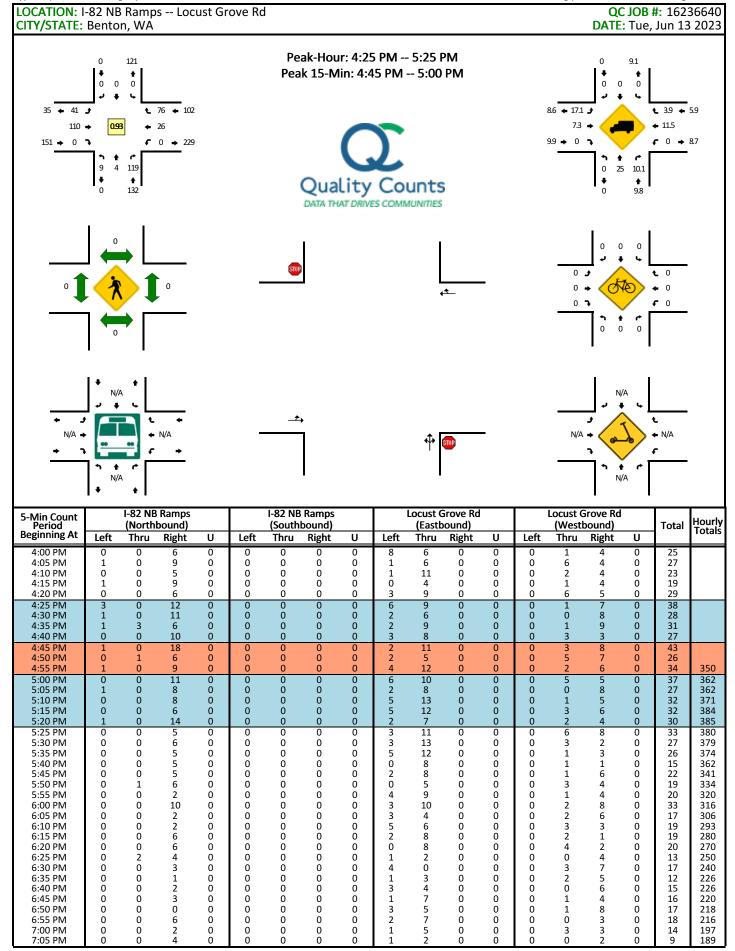
Peak 15-Min	Northbound				Southbound				Eastbound					West	Total		
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	52	0	56	0	0	4	12	0	152	8	0	0	284
Heavy Trucks	0	0	0		4	0	0		0	0	0		12	0	0		16
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



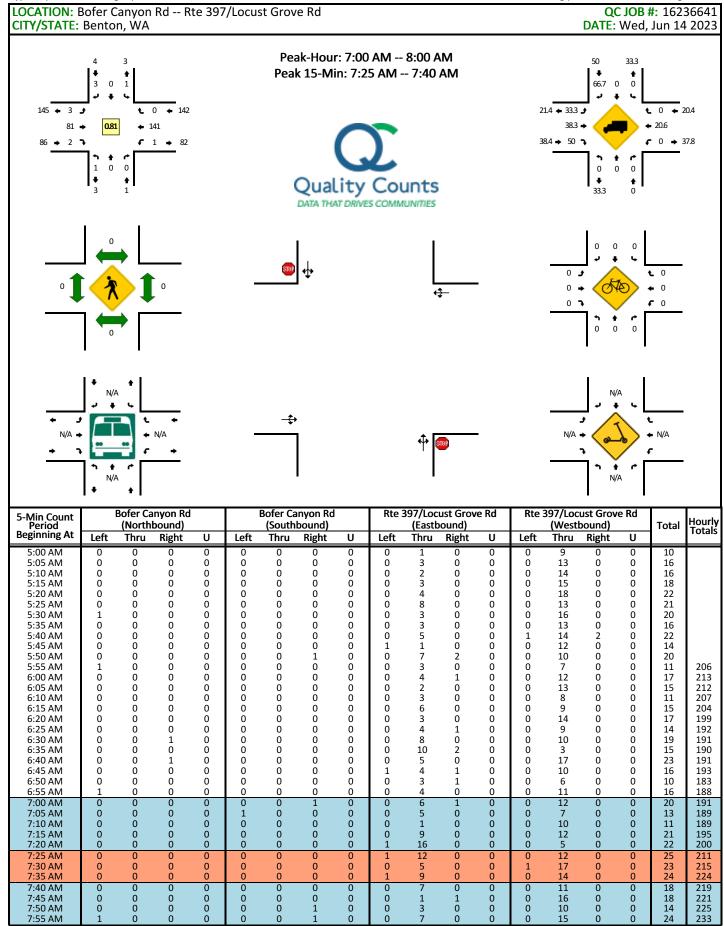
5-Min Count Period			Ramps bound)				Ramps bound)				Grove Rd Dound)				Frove Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	5	1	1	0	0	1	0	0	3	0	0	0	11	103
7:15 PM	0	0	0	0	4	0	1	0	0	0	0	0	2	0	0	0	7	99
7:20 PM	0	0	0	0	7	0	1	0	0	1	0	0	1	0	0	0	10	98
7:25 PM	0	0	0	0	1	0	0	0	0	0	0	0	1	3	0	0	5	100
7:30 PM	0	0	0	0	3	0	1	0	0	2	0	0	0	1	0	0	7	98
7:35 PM	0	0	0	0	6	0	1	0	0	7	0	0	4	3	0	0	21	112
7:40 PM	0	0	0	0	1	0	0	0	0	2	0	0	1	3	0	0	7	110
7:45 PM	0	0	0	0	4	0	1	0	0	0	0	0	1	1	0	0	7	106
7:50 PM	0	0	0	0	2	0	1	0	0	3	0	0	4	0	0	0	10	106
7:55 PM	0	0	0	0	5	0	1	0	0	0	0	0	0	0	0	0	6	103
Peak 15-Min		North	bound		Southbound					Eastb	ound		Westbound				т.	A-1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	0	0	0	128	0	8	0	0	48	0	0	44	8	0	0	23	36
Heavy Trucks	0	0	0		4	0	0		0	8	0		12	0	0		2	4
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	4	0	0	0	0	0	4	120	0	0	0	92	96	0	316
Heavy Trucks	0	0	0		0	0	0		4	68	0		0	12	24		108
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

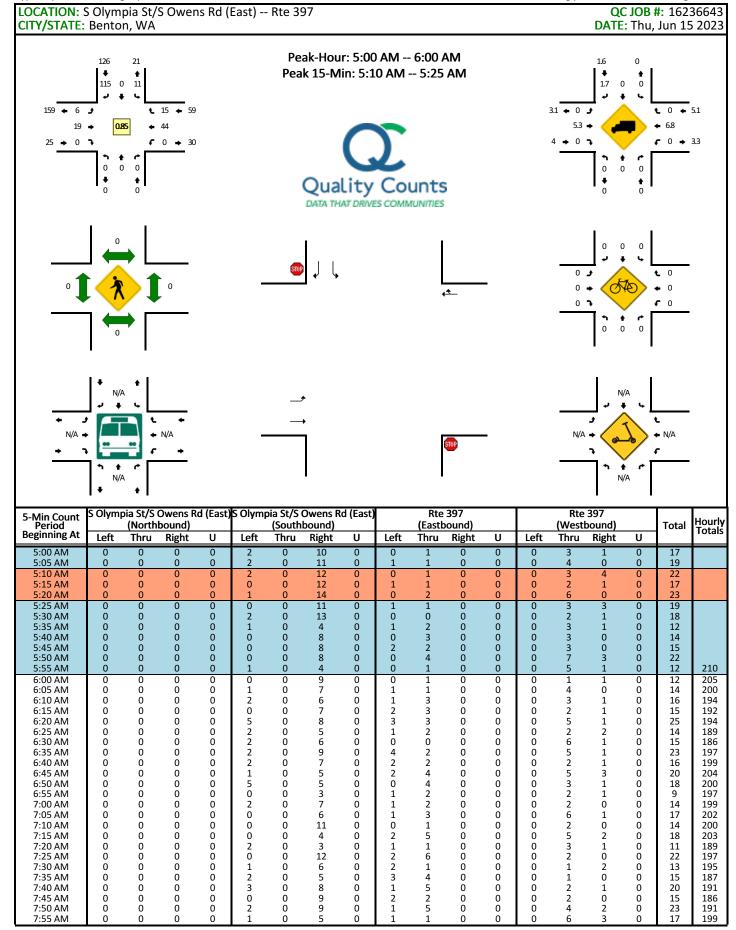


5-Min Count Period			Ramps bound)				Ramps bound)				Grove Rd Dound)				rove Rd cound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	3	0	0	0	0	0	0	5	0	0	0	3	0	0	11	181
7:15 PM	0	0	3	0	0	0	0	0	1	3	0	0	0	2	0	0	9	171
7:20 PM	0	0	3	0	0	0	0	0	1	6	0	0	0	1	3	0	14	165
7:25 PM	1	0	1	0	0	0	0	0	0	3	0	0	0	3	1	0	9	161
7:30 PM	0	0	0	0	0	0	0	0	0	4	0	0	0	1	10	0	15	159
7:35 PM	0	0	0	0	0	0	0	0	4	8	0	0	0	6	3	0	21	168
7:40 PM	0	0	2	0	0	0	0	0	0	2	0	0	0	4	4	0	12	165
7:45 PM	0	0	2	0	0	0	0	0	1	5	0	0	0	2	3	0	13	162
7:50 PM	0	0	0	0	0	0	0	0	2	1	0	0	0	4	3	0	10	155
7:55 PM	0	0	1	0	0	0	0	0	1	6	0	0	0	0	4	0	12	149
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	tal
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	itai
All Vehicles	8	4	132	0	0	0	0	0	32	112	0	0	0	40	84	0	4:	12
Heavy Trucks Buses	0	4	16		0	0	0		8	8	0		0	4	4		4	14
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		

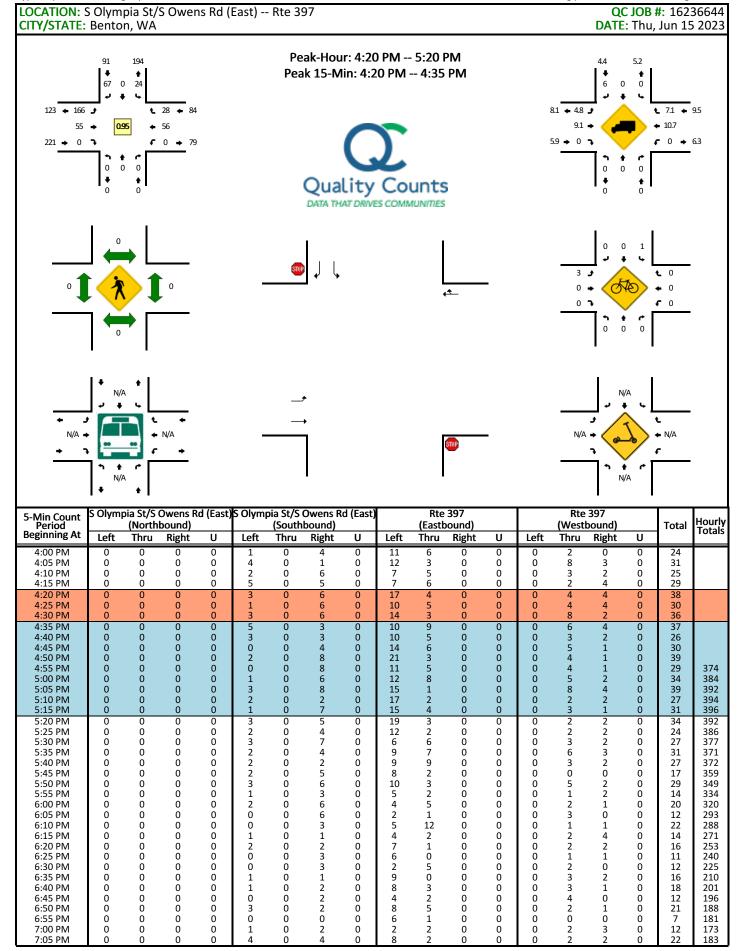


Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	0	0	0	0	8	104	0	0	4	172	0	0	288
Heavy Trucks	0	0	0		0	0	0		4	52	0		0	24	0		80
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

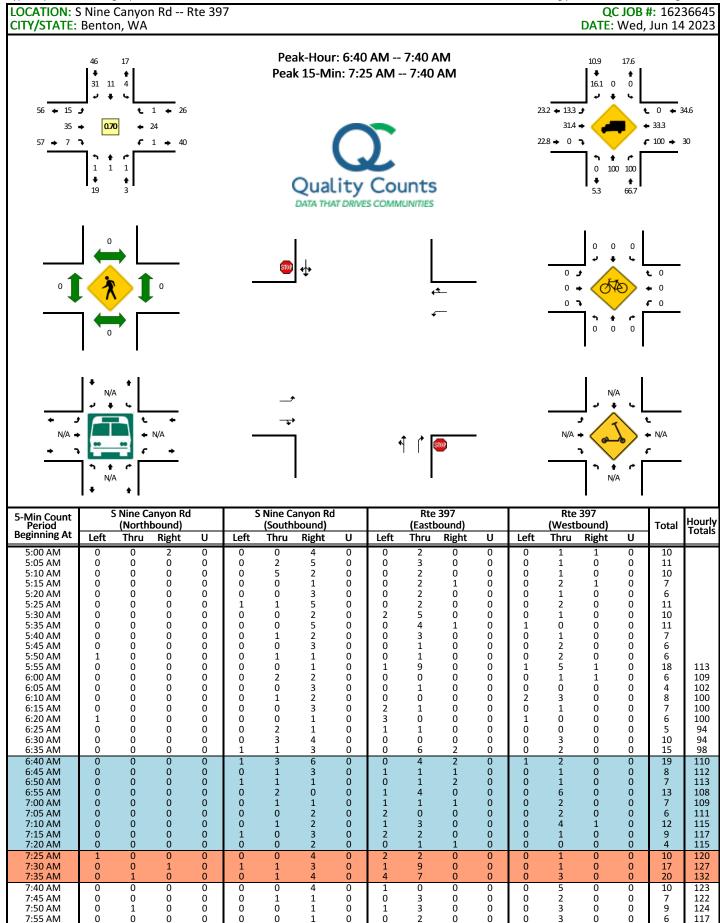
5-Min Count Period			nyon Rd bound)				anyon Rd bound)		Rte 3		ust Grove oound)	e Rd	Rte 3		ust Grov bound)	e Rd	Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	0	0	0	8	0	0	0	4	0	0	12	161
7:15 PM	0	1	0	0	0	0	0	0	0	7	0	0	0	2	0	0	10	156
7:20 PM	0	0	0	0	0	0	1	0	0	10	0	0	0	3	0	0	14	152
7:25 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	6	0	0	8	150
7:30 PM	0	0	0	0	0	0	0	0	0	5	0	0	0	8	0	0	13	149
7:35 PM	0	0	0	0	0	0	0	0	0	9	0	0	0	9	0	0	18	155
7:40 PM	0	0	0	0	0	0	0	0	0	4	0	0	0	8	0	0	12	151
7:45 PM	0	0	0	0	0	0	0	0	0	6	0	0	0	4	0	0	10	148
7:50 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	8	0	0	10	146
7:55 PM	0	0	0	0	0	0	0	0	0	8	0	0	0	4	0	0	12	138
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	s - 1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	0	4	0	0	0	4	0	4	236	0	0	0	132	0	0	38	30
Heavy Trucks	0	0	0		0	0	4		0	24	0		0	4	0		3	2
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



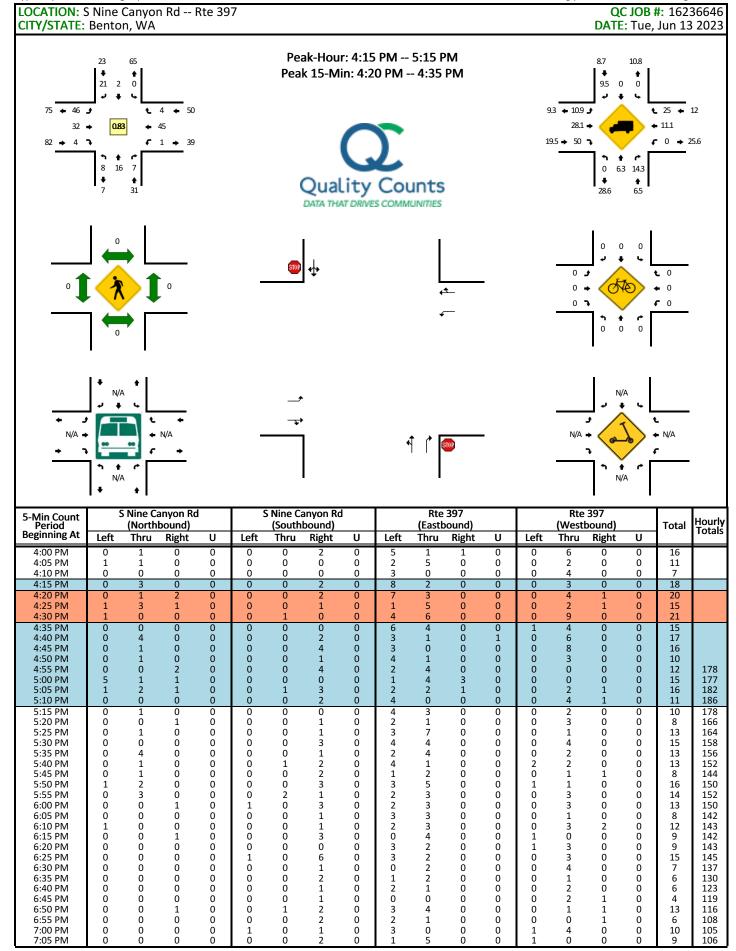
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	12	0	152	0	4	16	0	0	0	44	20	0	248
Heavy Trucks	0	0	0		0	0	4		0	0	0		0	4	0		8
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



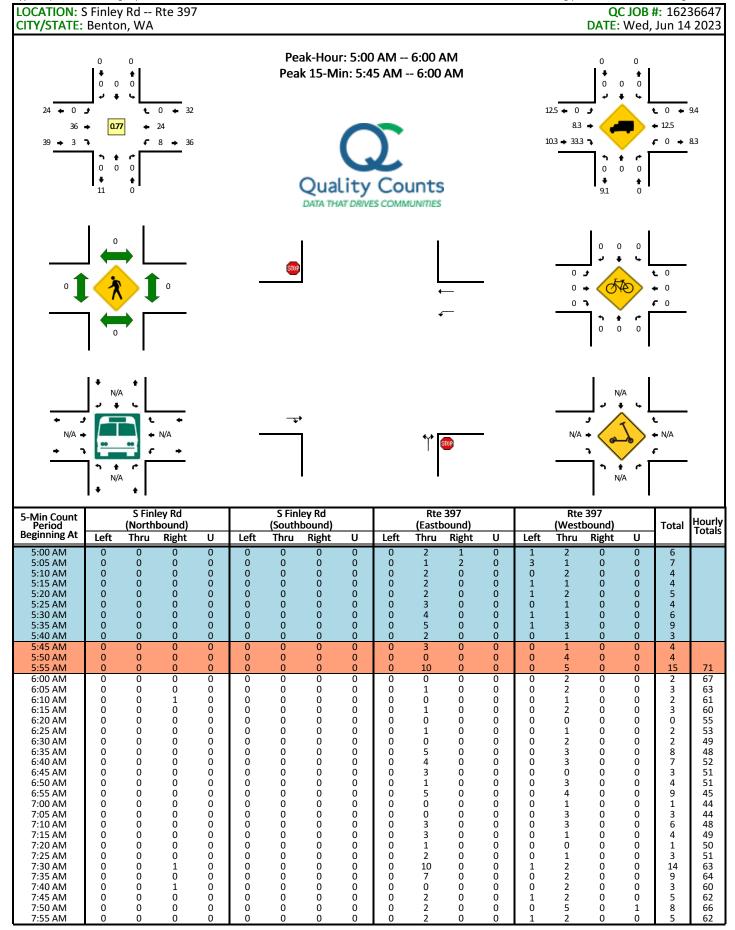
Period	S Olym		Owens R bound)	d (East)	S Olym <sub>i</sub>		Owens R bound)	d (East)			397 ound)				397 bound)		Total	Hourly
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	2	0	7	2	0	0	0	4	0	0	15	176
7:15 PM	0	0	0	0	3	0	1	0	1	0	0	0	0	7	2	0	14	176
7:20 PM	0	0	0	0	1	0	2	0	3	0	0	0	0	1	1	0	8	168
7:25 PM	0	0	0	0	4	0	1	0	3	1	0	0	0	1	0	0	10	167
7:30 PM	0	0	0	0	1	0	1	0	3	0	0	0	0	0	1	0	6	161
7:35 PM	0	0	0	0	3	0	3	0	2	1	0	0	0	0	1	0	10	155
7:40 PM	0	0	0	0	4	0	1	0	7	4	0	0	0	1	1	0	18	155
7:45 PM	0	0	0	0	1	0	0	0	4	1	0	0	0	1	3	0	10	153
7:50 PM	0	0	0	0	1	0	0	0	2	2	0	0	0	1	0	0	6	138
7:55 PM	0	0	0	0	1	0	2	0	7	2	0	0	0	1	2	0	15	146
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	4-1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	0	0	0	28	0	72	0	164	48	0	0	0	64	40	0	4:	16
Heavy Trucks	0	0	0		0	0	4		4	4	0		0	12	4		2	8
Buses																		
Pedestrians		0				0				0				0				)
Bicycles Scooters	0	0	0		0	0	0		12	0	0		0	0	0		1	.2
Comments:																		



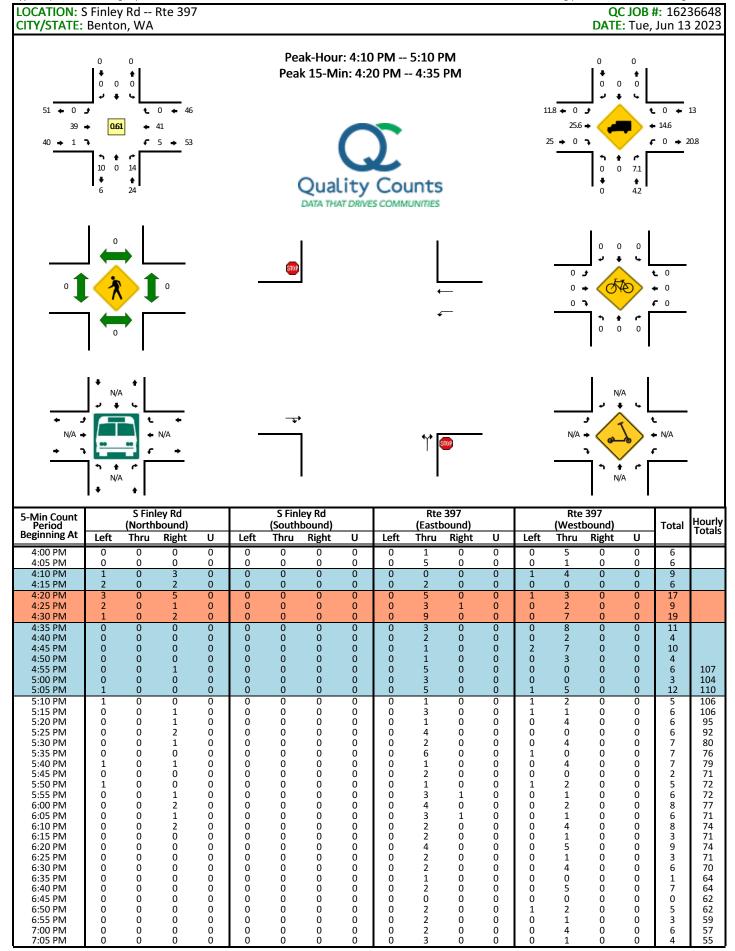
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	4	4	4	0	4	8	44	0	28	72	0	0	0	20	0	0	188
Heavy Trucks	0	4	4		0	0	8		8	20	0		0	8	0		52
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



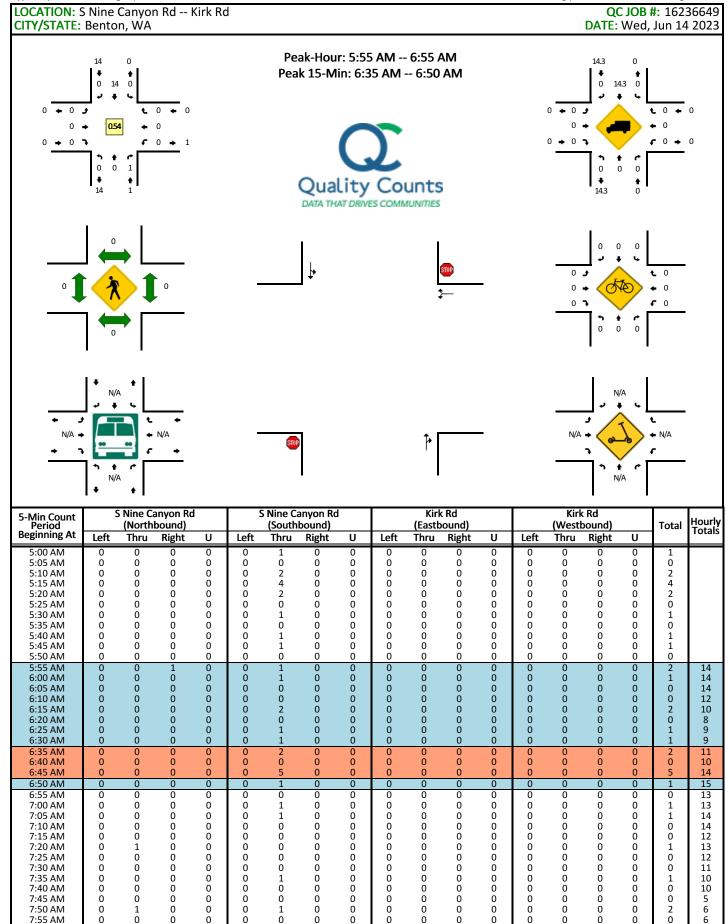
5-Min Count Period	9		anyon Rd bound)		9		anyon Ro bound)				397 oound)				397 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	0	0	0	0	0	2	0	3	1	0	0	0	0	0	0	6	100
7:15 PM	0	0	0	0	0	0	3	0	1	0	0	0	0	0	0	0	4	95
7:20 PM	0	1	1	0	0	0	0	0	0	1	0	0	0	1	0	0	4	90
7:25 PM	0	1	0	0	0	0	2	0	2	1	1	0	0	2	0	0	9	84
7:30 PM	0	0	0	0	0	0	0	0	1	2	0	0	0	3	0	0	6	83
7:35 PM	0	0	0	0	0	0	3	0	0	0	0	0	2	2	0	0	7	84
7:40 PM	1	0	0	0	0	0	1	0	1	1	1	0	0	1	0	0	6	84
7:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	81
7:50 PM	0	0	0	0	0	1	0	0	1	4	0	0	0	2	0	0	8	76
7:55 PM	0	0	0	0	0	0	1	0	0	3	0	0	0	4	2	0	10	80
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	bound		т.	<b>.</b>
Peak 15-Min Flowrates	Left	North Thru	bound Right	U	Left	South Thru	bound Right	U	Left	Eastb Thru	ound Right	U	Left	Westl Thru	bound Right	U	То	tal
	Left 8			0	Left 0			U 0	Left 48			<b>U</b>	Left 0			U 0		tal 24
All Vehicles Heavy Trucks		Thru	Right			Thru	Right			Thru	Right			Thru	Right		22	
All Vehicles Heavy Trucks Buses	8	Thru 16	Right 12		0	Thru 4 0	Right 12		48	Thru 56	Right 0		0	Thru 60	Right 8		22	24 4
All Vehicles Heavy Trucks Buses Pedestrians	8	16 0 0	Right 12 0		0	4 0 0	Right 12 0		48 8	56 8 0	Right 0 0		0	60 4 0	Right 8 4		22	24 4
All Vehicles Heavy Trucks Buses	8	Thru 16	Right 12		0	Thru 4 0	Right 12		48	Thru 56	Right 0		0	Thru 60	Right 8		22	24 4
All Vehicles Heavy Trucks Buses Pedestrians Bicycles	8	16 0 0	Right 12 0		0	4 0 0	Right 12 0		48 8	56 8 0	Right 0 0		0	60 4 0	Right 8 4		22	24 4



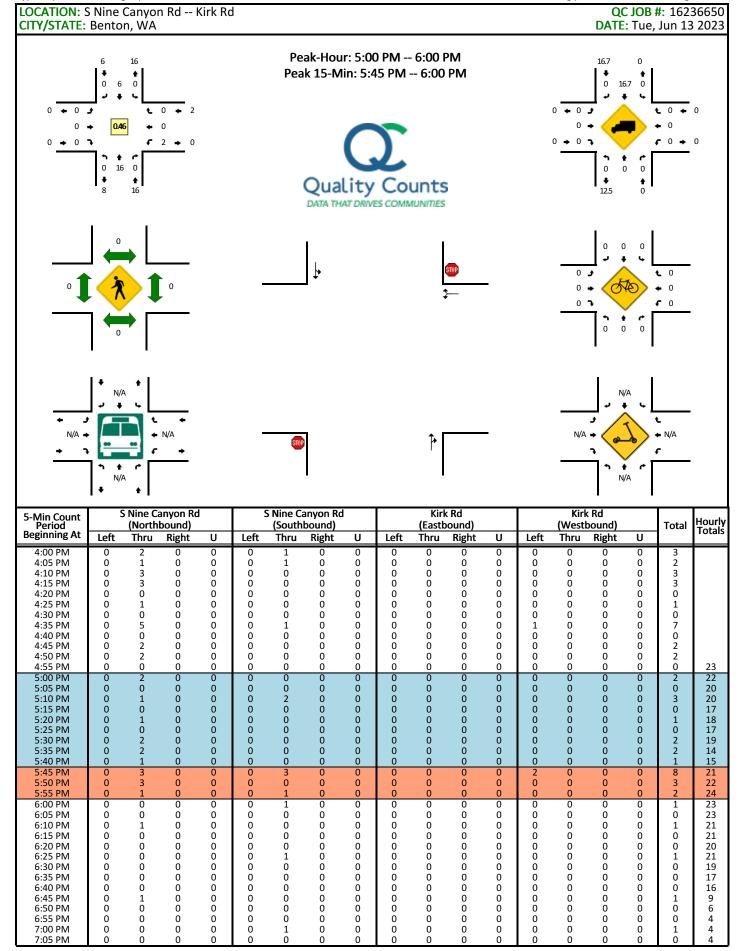
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	0	0	0	0	0	52	0	0	0	40	0	0	92
Heavy Trucks	0	0	0		0	0	0		0	4	0		0	8	0		12
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



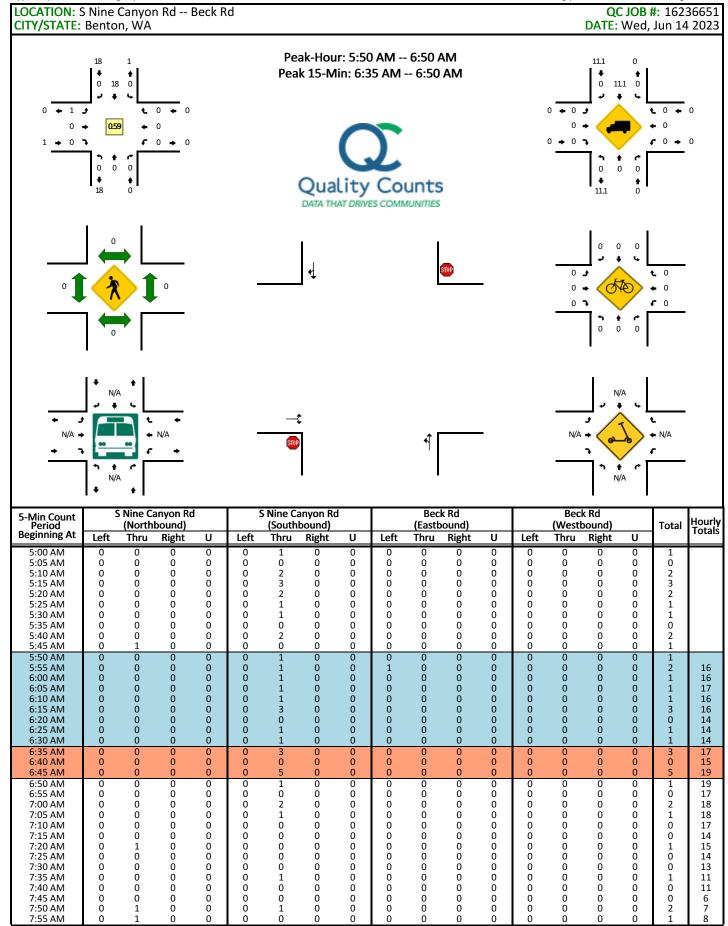
5-Min Count Period			ley Rd bound)				ley Rd bound)				397 ound)				397 bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	0	3	50
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	47
7:20 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2	40
7:25 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	3	0	0	4	41
7:30 PM	0	0	0	0	0	0	0	0	0	3	0	0	0	1	0	0	4	39
7:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	42
7:40 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2	37
7:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	2	39
7:50 PM	0	0	0	0	0	0	0	0	0	2	1	0	0	3	0	0	6	40
7:55 PM	1	0	0	0	0	0	0	0	0	4	0	0	0	2	0	0	7	44
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	I
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	1 10	tal
All Vehicles	24	0	32	0	0	0	0	0	0	68	4	0	4	48	0	0	18	80
Heavy Trucks	0	0	4		0	0	0		0	12	0		0	8	0		2	24
Buses																		
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



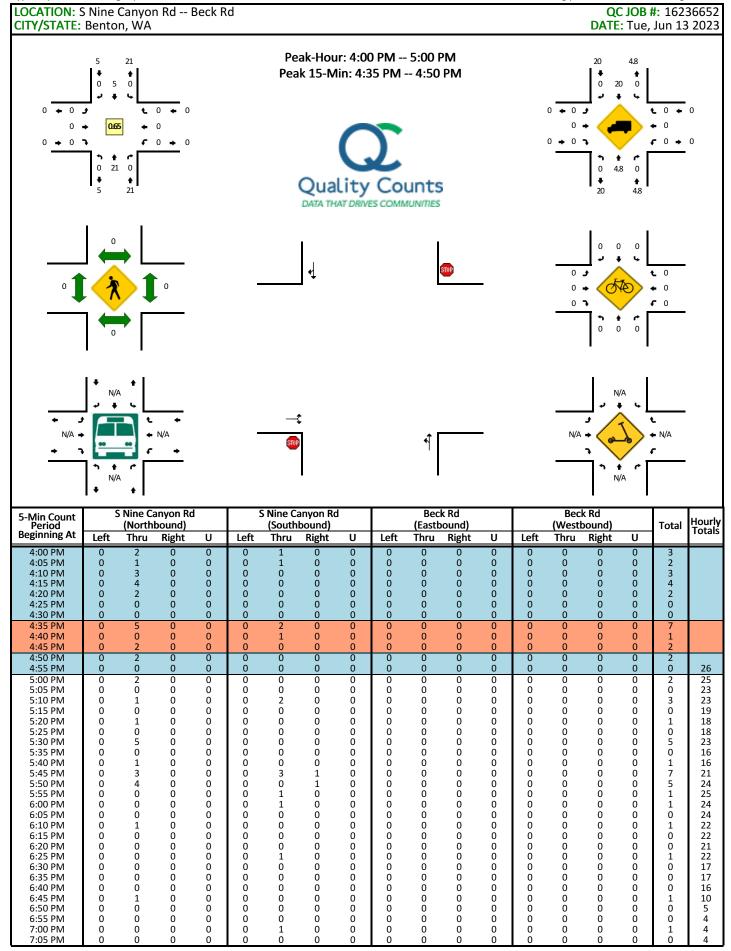
Peak 15-Min		North	bound			South	bound			Eastb	ound			West	oound		Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	0	28	0	0	0	0	0	0	0	0	0	0	28
Heavy Trucks	0	0	0		0	4	0		0	0	0		0	0	0		4
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



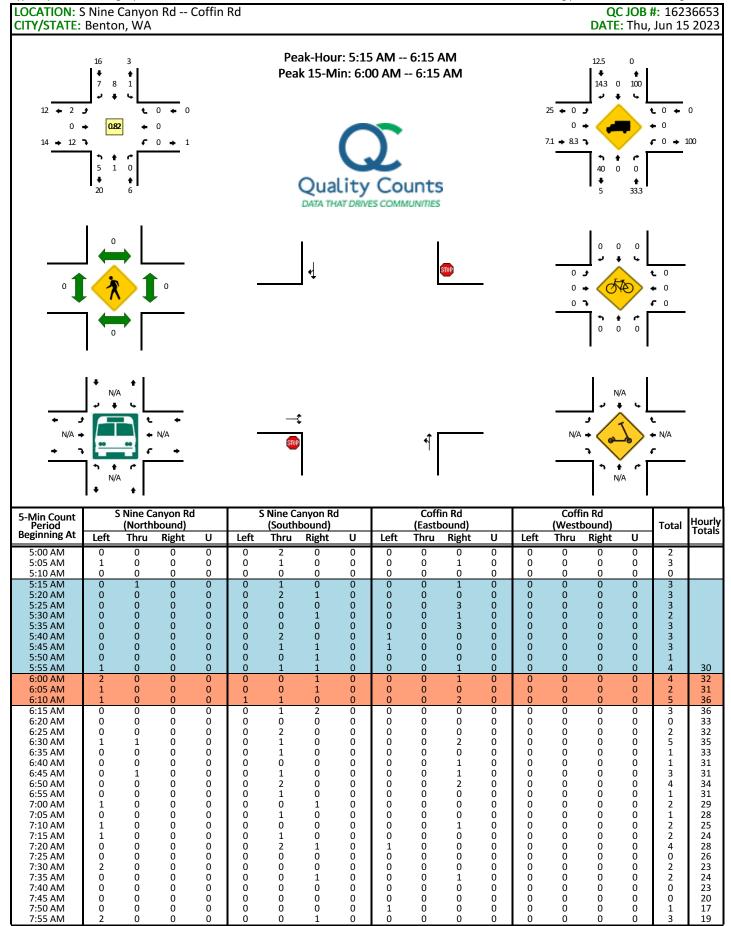
5-Min Count Period	9		anyon Rd bound)		9		anyon Ro bound)				c Rd oound)				Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	4
7:15 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2	6
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
7:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	5
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	<b>.</b>
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	28	0	0	0	16	0	0	0	0	0	0	8	0	0	0	5	52
Heavy Trucks	0	0	0		0	4	0		0	0	0		0	0	0		4	4
Buses																		
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



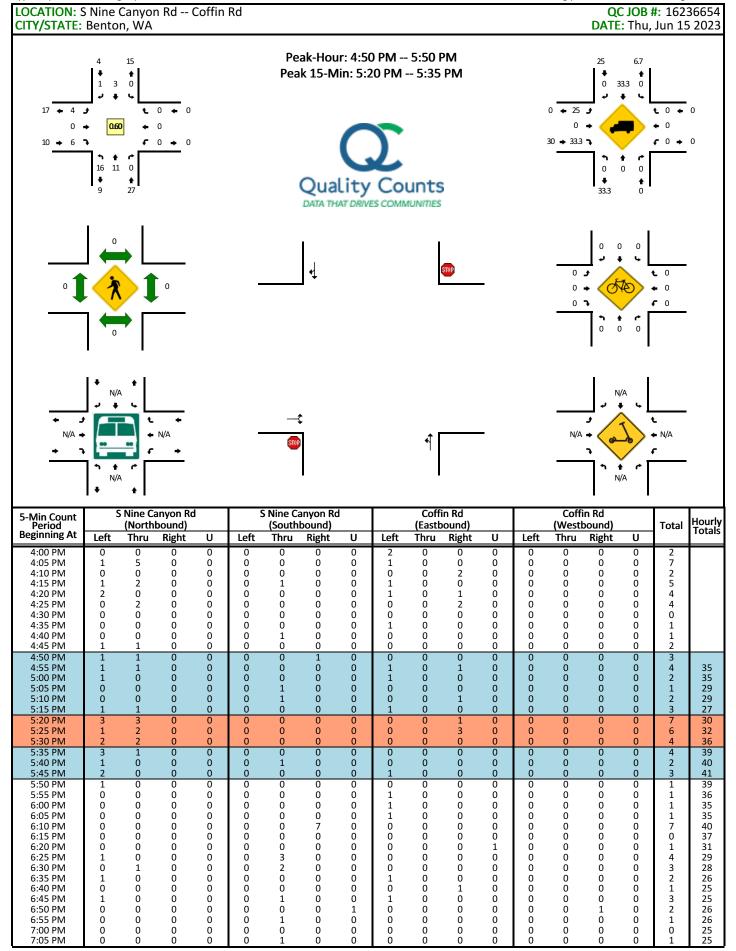
Peak 15-Min Flowrates		North	bound		Southbound				Eastb	ound			West	Total			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	0	0	0	0	32	0	0	0	0	0	0	0	0	0	0	32
Heavy Trucks	0	0	0		0	4	0		0	0	0		0	0	0		4
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



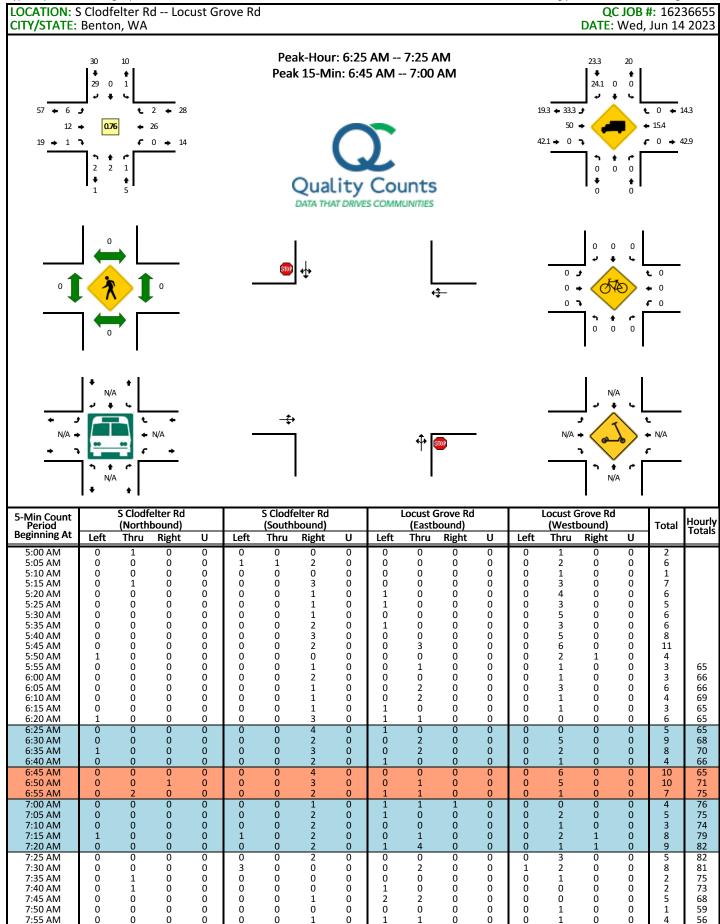
5-Min Count Period	9		anyon Rd bound)		9		anyon Ro bound)				k Rd oound)				k Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	4
7:15 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	2	6
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
7:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	5
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
7:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
Peak 15-Min		North	bound			South	uthbound Eastbound Westbound							т.				
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	28	0	0	0	12	0	0	0	0	0	0	0	0	0	0	4	10
Heavy Trucks	0	4	0		0	0	0		0	0	0		0	0	0			4
Buses																		
Pedestrians		0				0				0				0			(	0
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	0
Comments:																		



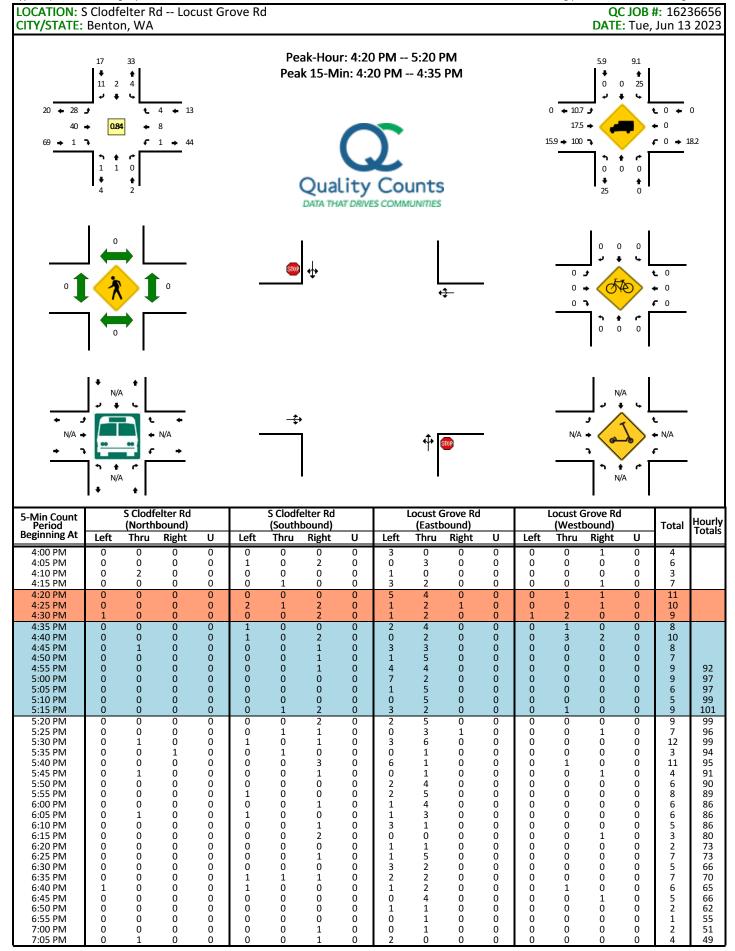
Peak 15-Min Flowrates		North	bound		Southbound				Eastb	ound			West	Total			
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOTAL
All Vehicles	16	0	0	0	4	4	8	0	0	0	12	0	0	0	0	0	44
Heavy Trucks	8	0	0		4	0	0		0	0	4		0	0	0		16
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



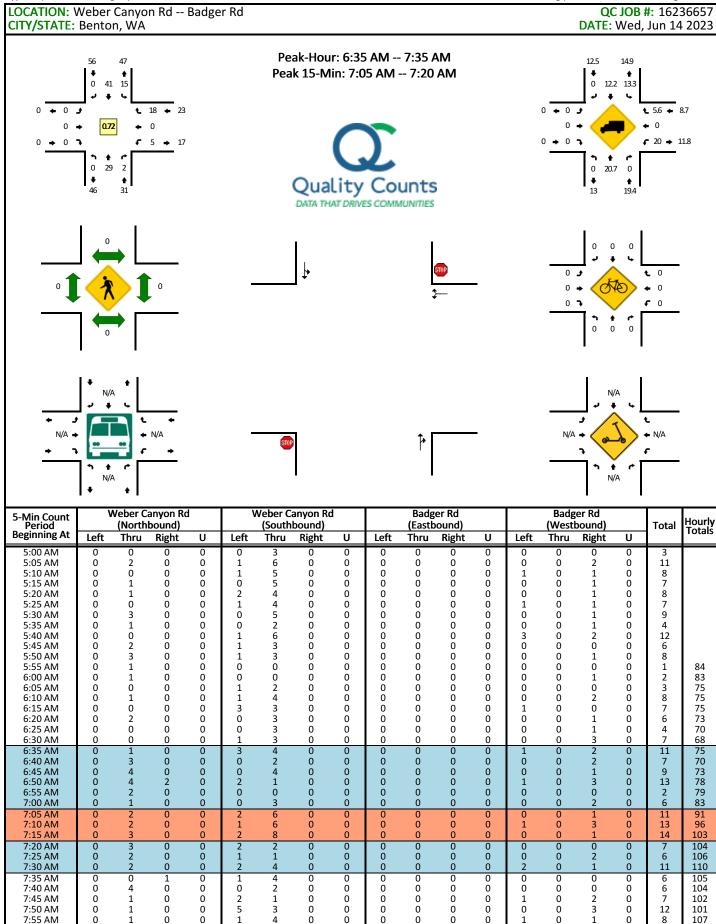
5-Min Count Period			anyon Rd bound)		9		anyon Ro bound)				in Rd oound)				in Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		TOtals
7:10 PM	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	2	20
7:15 PM	0	1	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	22
7:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	21
7:25 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1	18
7:30 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	16
7:35 PM	1	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	3	17
7:40 PM	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	2	18
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15
7:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13
7:55 PM	0	0	0	0	0	1	1	0	0	0	1	0	0	0	0	0	3	15
Peak 15-Min		North	bound		Southbound					Eastb	ound			Westl	Total			
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tai
All Vehicles	24	28	0	0	0	0	0	0	0	0	16	0	0	0	0	0	6	8
Heavy Trucks Buses	0	0	0		0	0	0		0	0	8		0	0	0		8	3
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



Peak 15-Min Flowrates		North	bound		Southbound					Eastb	ound			West	Total		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	8	4	0	0	0	36	0	4	8	0	0	0	48	0	0	108
Heavy Trucks	0	0	0		0	0	4		4	0	0		0	16	0		24
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	



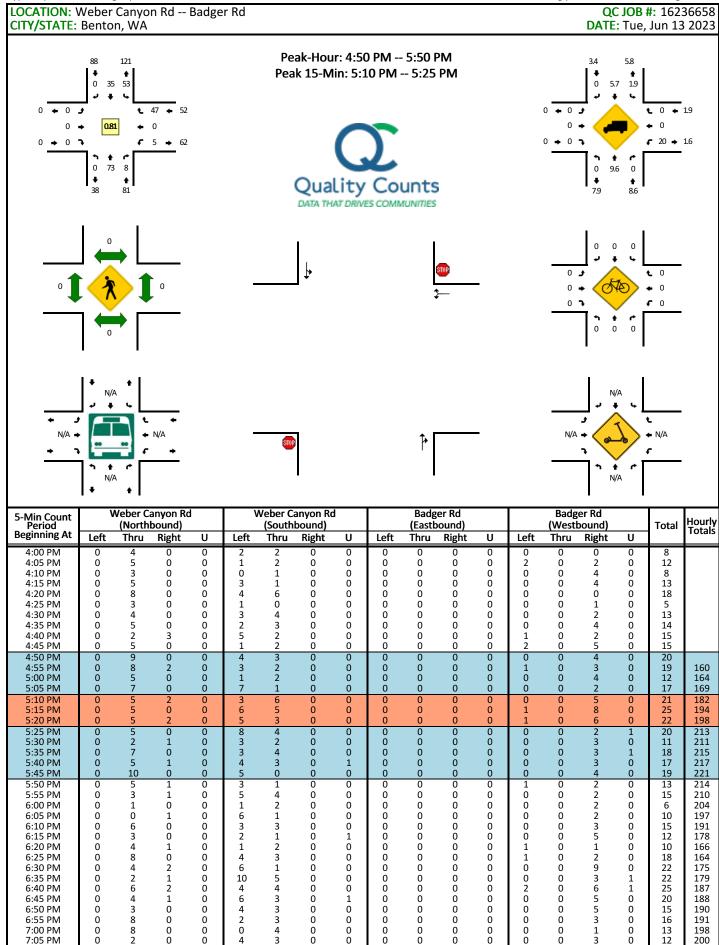
5-Min Count Period			elter Rd bound)				elter Rd bound)				Grove Rd Dound)				Frove Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	0	0	0	0	0	1	0	1	1	0	0	0	0	0	0	3	47
7:15 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	45
7:20 PM	0	0	0	0	0	0	1	0	2	1	0	0	0	1	1	0	6	49
7:25 PM	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0	2	44
7:30 PM	0	1	0	0	0	0	1	0	2	2	0	0	0	2	1	0	9	48
7:35 PM	0	0	0	0	2	0	0	0	1	0	0	0	0	1	0	0	4	45
7:40 PM	0	0	0	0	0	0	1	0	3	1	0	0	0	1	1	0	7	46
7:45 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2	0	3	44
7:50 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	2	0	3	45
7:55 PM	0	1	0	0	0	0	2	0	2	1	0	0	0	0	0	0	6	50
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	т.	s - 1		
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	То	taı
All Vehicles	4	0	0	0	8	4	16	0	28	32	4	0	4	12	8	0	12	20
Heavy Trucks	0	0	0		0	0	0		4	4	4		0	0	0		1	2
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		



Peak 15-Min		North	bound			South	bound			Eastb	ound			Westbound			Total
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	TOLAI
All Vehicles	0	28	0	0	20	80	0	0	0	0	0	0	4	0	20	0	152
Heavy Trucks	0	4	0		8	12	0		0	0	0		0	0	0		24
Buses																	
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Scooters																	
Comments:																	

Report generated on 6/29/2023 1:29 PM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212



5-Min Count Period	٧		anyon Ro bound)		١		anyon Ro bound)	i			er Rd ound)				er Rd bound)		Total	Hourly Totals
Beginning At	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		Totals
7:10 PM	0	1	2	0	0	0	0	0	0	0	0	0	0	0	0	1	4	189
7:15 PM	0	2	2	0	0	1	0	0	0	0	0	0	0	0	0	1	6	183
7:20 PM	0	1	1	1	1	1	0	0	0	0	0	0	0	0	0	0	5	178
7:25 PM	0	1	0	0	1	3	0	0	0	0	0	0	0	0	0	1	6	166
7:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	1	0	0	1	4	148
7:35 PM	0	0	2	0	1	0	0	0	0	0	0	0	0	0	1	0	4	130
7:40 PM	0	0	1	0	1	0	0	0	0	0	0	0	1	0	0	0	3	108
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	89
7:50 PM	0	0	0	0	0	1	0	0	0	0	0	0	2	0	1	0	4	78
7:55 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	0	2	64
Peak 15-Min		North	bound			South	bound			Eastb	ound			Westl	oound		т.	4-1
Flowrates	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	10	tal
All Vehicles	0	60	16	0	56	56	0	0	0	0	0	0	8	0	76	0	2	72
Heavy Trucks	0	4	0		4	4	0		0	0	0		0	0	0		1	.2
Buses																		
Pedestrians		0				0				0				0			(	)
Bicycles Scooters	0	0	0		0	0	0		0	0	0		0	0	0		(	)
Comments:																		

Report generated on 6/29/2023 1:29 PM

SOURCE: Quality Counts, LLC (http://www.qualitycounts.net) 1-877-580-2212

# APPENDIX C: CRASH ANALYSIS SUMMARY

Tetra Tech																			MV DRIVER CONTRIBUTIN NG	MV DRIVER CONTRIBUTI NG	MV DRIVER CONTRIBUTI NG	WA STATE	WA STATE	LAT	LONG
Intersection ID#	Tetra Tech Intersection Name	DATE	TIME	MOST SEVERE INJURY TYPE	# FA VI	# # # E PE BIK H DS ES	VEHICLE 1 TYPE	VEHICLE 2 TYPE	JUNCTION RELATIONSHIP	WEATHER	ROADWAY SURFACE CONDITION	LIGHTING CONDITION	FIRST COLLISION TYPE / OBJECT STRUCK	VEHICLE 1 ACTION	VEHICLE 2 ACTION	VEHICLE 1 COMPASS DIRECTION FROM	VEHICLE 1 COMPASS DIRECTION TO	VEHICLE 2 COMPASS DIRECTION FROM	VEHICLE 2 G CIRCUMSTA COMPASS CIRCUMSTAN NCE 2 (UNIT DIRECTION TO CE 1 (UNIT 1) 1)	CIRCUMSTA T NCE 3 (UNIT 1)	CIRCUMSTA FIRST IMPACT LOCATION FIRST IMPACT LOCATION (City, County & Misc Trafficways - 2010 forward)	PLANE SOUTH P - X 2010 - FORWARD	PLANE SOUTH - Y 2010 - FORWARD	DAI .	Lord
1	Wine Country Road at I-82 WB Ramps	02/02/2019	12:16	No Apparent Injury	0 0	1 0 0	Passenger Car		At Intersection and Related	Fog or Smog or Smoke	Wet	Daylight	Concrete Barrier/Jersey Barrier - Leading End	Making Right Turn		South	East		Exceeding Reas. Safe Speed		Left Shoulder On Ramp Decreasing Milepost Side of Mainline	1832473.25	322501.13	46.2151814	-119.7412937
	Wine Country Road at I-82 WB Ramps	09/01/2022	15:31	No Apparent Injury	0 0	2 0 0	) Passenger Car	Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Making Left Turn	Going Straight Ahead	South	West	East	West Did Not Grant RW to Vehicle		None Lane 1 LX Increasing Milepost ( Prior to 2002 Impact Location Code was not lane specific)	1832501.56	322520.47	46.2152337	-119.7411811
1	Wine Country Road at I-82 WB Ramos	11/11/2022	07:20	Possible Injury	1 0	2 0 0	Passenger Car	Pickup, Panel Truck or Vanett under 10,000 lb	Intersection Related but Not at Intersection	Overcast	Dry	Daylight	From same direction - both going straight - both moving - rear-end	Going Straight Ahead	Slowing	East	West	East	West Follow Too Closely		None Lane 1 Off Ramp Decreasing Milepost Side of Mainline	1832523.63	322433.64	46.214995	-119.7410972
2	Wine Country Road at 1-82 EB Ramps	04/04/2018	15:41	Suspected Minor Injury	2 0	1 0 0	Passenger Car		At Intersection and Related	Raining	Wet	Daylight	Concrete Barrier/Jersey Barrier - Leading End	Making Left Turn		North	East		Exceeding Reas, Safe Speed		Left Shoulder On Ramp Increasing Milepost Side of Mainline	1831413.39	322208.79	46.2144076	-119.7454914
2	Wine Country Road at 1-82 EB Ramps	01/05/2019	06:03	No Apparent Injury	0 0	2 0 0	Pickup,Panel Truck or Vanett under 10,000 lb	e Passenger Car	At Intersection and Related	Fog or Smog or Smoke	Dry	Dark-Street Lights On	From same direction - all others	Backing	Stopped for Traffic	Vehicle Stopped	Vehicle Backing	Vehicle Stopped	Wehicle Stopped Inattention		None Lane 1 Off Ramp Increasing Milepost Side of Mainline	1831454.56	322156.49	46.2142631	-119.7453307
2	Wine Country Road at 1-82 EB Ramps	11/27/2019	06:40	Unknown	0 0	1 0 0	Truck Tractor & Semi-Trailer		At Intersection and Related	Clear	Dry	Dark-Street Lights On	Wood Sign Post	Making Left Turn		East	South		Unknown Distraction		Left Shoulder On Ramp Increasing Milepost Side of Mainline	1831414.14	322208.65	46.2144072	-119.7454884
2	Wine Country Road at 1-82 EB Ramps	11/24/2021	06:06	No Apparent Injury	0 0	1 0 0	Pickup,Panel Truck or Vanett under 10,000 lb		Not at Intersection and Not Related	Clear	Ice	Dark-Street Lights On	Guardrail - Face	Going Straight Ahead		West	East		Exceeding Reas. Safe Speed		Right Shoulder Increasing Milepost	1831668.40	322279.91	46.214596	-119.7444813
2	Wine Country Road at 1-82 EB Ramps	11/17/2022	03:10	No Apparent Injury	0 0	1 0 0	Truck & Trailer		At Intersection and Related	Overcast	Dry	Dark-Street Lights On	Metal Sign Post	Making Left Turn		South	West		Improper Turn/Merge		Left Shoulder Off Ramp Increasing Milepost Side of Mainline	1831414.94	322209.26	46.2144089	-119.7454852
		06/27/2018	16:00	Possible Injury	1 0	2 0 0	Pickup,Panel Truck or Vanett under 10,000 lb	e Pickup, Panel Truck or Vanett under 10,000 lb	e At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	From same direction - both going straight - one stopped - rear-end	Slowing	Stopped at Signal or Stop Sign	West	East	Vehicle Stopped	Vehicle Stopped Driver Follow Too Interacting Closely with Passengers,		None Lane 1 Increasing Milepost	1830894.21	322048.29	46.2139811	-119.7475482
3	Wine Country Road at Route 22 Wine Country Road at Route 22	02/10/2019	15:03	No Apparent Injury	0 0	2 0 0	Truck Tractor & Semi-Trailer	Passenger Car	At Intersection and Related	Snowing	Snow/Slush	Daylight	Same direction — both turning right — both	Making Right Turn	Making Right Turn	South	East	South	Anim East Inattention		None Lane 1 Increasing Milepost	1830886.21	322053.90	46.2139967	-119.7475796
2	Wine Country Road at Route 22	08/19/2019	15:48	Possible Injury	2 0	2 0 0	Passenger Car	Pickup, Panel Truck or Vanett under 10,000 lb	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Going Straight Ahead	Going Straight Ahead	South	North	East	West Did Not Grant RW to Vehicle		None Lane 1 Decreasing Milepost	1830879.45	322058.64	46.2140099	-119.7476061
3	Wine Country Road at Route 22  Wine Country Road at Route 22	11/12/2019	09:32	No Apparent Injury	0 0	2 0 0	Passenger Car	Passenger Car	At Intersection and Related	Raining	Wet	Daylight	From same direction - both going straight - one stopped - rear-end	Going Straight Ahead	Stopped at Signal or Stop Sign	West	East	Vehicle Stopped	Vehicle Stopped Operating Defective Equipment		None Lane 1 Increasing Milepost	1830879.45	322058.64	46.2140099	-119.7476061
3	Wine Country Road at Route 22	11/27/2019	03:02	Suspected Serious Injury	1 0	1 0 0	Passenger Car		Not at Intersection and Not Related	Clear or Partly Cloudy	Dry	Dark-Street Lights On	Railroad Tracks (ie. Run off the road and hit the tracks)	Going Straight Ahead		South	North		Apparently Exceeding Asleep or Reas, Safe Fatigued Speed		Past the Outside Shoulder of Primary Trafficway	1830816.30	322206.87	46.214418	-119.74785
3	Wine Country Road at Route 22	06/18/2020 09/30/2022	19:39 14:25	Possible Injury No Apparent Injury	1 0	2 0 0	Pickup, Panel Truck or Vanett under 10,000 lb Pickup, Panel Truck or Vanett under 10,000 lb	e Pickup, Panel Truck or Vanett under 10,000 lb e Pickup, Panel Truck or Vanett under 10,000 lb	e At Intersection and Related  e At Intersection and Related	Clear or Partly Cloudy  Clear or Partly Cloudy	Dry	Daylight Daylight	From same direction - both going straight - one stopped - rear-end Entering at angle	Going Straight Ahead Making Left Turn	Stopped at Signal or Stop Sign Going Straight Ahead	West	East West	Vehicle Stopped East	Vehicle Stopped Other Distractions West Did Not Grant		None Left Turn Lane Decreasing Milepost None Lane 1 Decreasing Milepost	1830880.77 1830883.12	322057.72 322056.07	46.2140073	-119.747601
3	Wine Country Road at Route 22	11/29/2018	14:35	No Apparent Injury	0 0	2 0 0	Pickup, Panel Truck or Vanett	under 10,000 lb e Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Starting in Traffic Lane	Going Straight Ahead	West	East	West	RW to Vehicle East Did Not Grant		None Lane 1 Increasing Milepost	1830658.75	319162.24	46.2140027	-119.7475917
4	Route 22 at Route 221 and Paterson Ave	05/23/2019	13:11	Possible Injury	1 0	2 0 0	under 10,000 lb Passenger Car	Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Going Straight Ahead	Going Straight Ahead	West	East	East	RW to Vehicle West Did Not Grant		None Lane 1 Increasing Milepost	1830658.75	319162.24	46.2060733	-119.7485869
4	Route 22 at Route 221 and Paterson Ave Route 22 at Route 221 and Paterson Ave	08/21/2019	07:56	No Apparent Injury	0 0	2 0 0	Passenger Car	Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Going Straight Ahead	Going Straight Ahead	South	North	East	RW to Vehicle West Inattention		Driver Not Lane 1 Decreasing Milepost	1830662.77	319161.56	46.2060733 46.2060713	-119.7485869 -119.7485711
4	Route 22 at Route 221 and Paterson Ave	11/20/2019	05:51	No Apparent Injury	0 0	1 0 0	Truck & Trailer		At Intersection and Related	Clear	Dry	Dark-Street Lights On	Vehicle overturned	Making Left Turn		East	South		Exceeding Reas. Safe		Right Shoulder Increasing Milepost	1830719.69	319137.98	46.2060051	-119.7483472
4	Route 22 at Route 221 and Paterson Ave	02/21/2020	15:25	No Apparent Injury	0 0	2 0 0	Passenger Car	Pickup, Panel Truck or Vanett under 10,000 lb	e At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Starting in Traffic Lane	Going Straight Ahead	South	North	East	West Did Not Grant RW to Vehicle		None Lane 1 Decreasing Milepost	1830658.75	319162.24	46.2060733	-119.7485869
4	Route 22 at Route 221 and Paterson Ave	09/24/2020	14:30 09:12	No Apparent Injury No Apparent Injury	0 0	2 0 0	Passenger Car Truck Tractor & Semi-Trailer	Passenger Car	At Intersection and Related  At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight Daylight	From same direction - all others  Vehicle overturned	Stopped in Roadway Making Left Turn	Backing	Wehicle Stopped East	Vehicle Stopped South	Vehicle Backing	Vehicle Backing None Exceeding		Improper Backing Lane 1 Increasing Milepost Lane 1 Increasing Milepost	1830658.75 1830658.75	319162.24 319162.24	46.2060733	-119.7485869
4	Route 22 at Route 221 and Paterson Ave	09/07/2021	13:18	No Apparent Injury	0 0	1 0 0		.0	Intersection Related but Not	Clear	Dry	Daylight	Utility Box	Making Left Turn		North	West		Exceeding Reas. Safe Speed Exceeding		Intersecting Trafficway	1830574.19	319165.32	46.2060733	-119.7485869
4	Route 22 at Route 221 and Paterson Ave	09/05/2022	17:04	No Apparent Injury	0 0	2 0 0	Pickup,Panel Truck or Vanett under 10,000 lb Passenger Car	Pickup, Panel Truck or Vanett under 10,000 lb	Intersection Related but Not at Intersection  At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Making Left Turn	Going Straight Ahead	West	North	East	Reas. Safe Speed West Did Not Grant		None Lane 1 Decreasing Milepost	1830658.75	319162.24	46.2060839	-119.7489208
4 5	Route 22 at Route 221 and Paterson Ave Route 221 at County Well Road							under 10,000 lb					N	O CRASHES REPORTED					RW to Vehicle					46.2060733	-119.7485869
6	Route 221 at Cemetery Road  Route 221 at Sellants Road	07/02/2018	12:17	No Apparent Injury	0 0	2 0 0	Other	Truck Tractor & Semi-Trailer	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	From same direction - one left turn - one	O CRASHES REPORTED  Overtaking and Passing	Making Left Turn	North	South	North	Southeast Inattention		None Lane 1 Increasing Milepost	1868181.05	292205.14	46.1310769	-119.6016068
7	Route 221 at Sellards Road	09/06/2019	03:55	No Apparent Injury	0 0	1 0 0	Pickup,Panel Truck or Vanett under 10,000 lb	e	At Intersection and Related	Clear or Partly Cloudy	Dry	Dark-No Street Lights	Utility Pole	Making Left Turn		East	South		Disregard Stop Sign - Flashing		Past Right Shoulder Decreasing Milepost	1868179.51	292203.19	46.1310716	-119.601613
7	Route 221 at Sellards Road	09/18/2019	17:48	Suspected Minor Injury	1 0		Motorcycle		At Intersection and Related	Clear or Partly Cloudy	Dry	Dusk	Vehicle overturned	Going Straight Ahead Making Left Turn	Color Carrieda Abrard	North	South	Country	None None		Lane 1 Decreasing Milepost	1868191.35	292205.04	46.1310763	-119.6015662
7	Route 221 at Sellards Road  Route 221 at Sellards Road	11/23/2020	12:20	No Apparent Injury	0 0		Truck Tractor & Semi-Trailer Truck Tractor & Semi-Trailer	Pickup, Panel Truck or Vanett under 10,000 lb Truck Tractor & Semi-Trailer	At Intersection and Related	Overcast	Wet	Daylight	straight From opposite direction - all others	Making Left Turn Stopped for Traffic	Going Straight Ahead	North	East	South	North Improper Turn/Merge North None		None Lane 1 Decreasing Milepost	1868178.96	292204.46	46.1310769 46.1310751	-119.6016093 -119.6016151
7	Route 221 at Sellards Road	08/08/2021	20:15	Dead at Scene	0 2	2 0 0	Pickup,Panel Truck or Vanett under 10,000 lb	e Passenger Car	At Intersection and Related	Clear	Dry	Dusk	Entering at angle	Going Straight Ahead	Going Straight Ahead	East	West	South	North Under Disregard Influence of Traffic Sign Drugs and Signals		None Lane 1 Increasing Milepost	1868180.56	292205.14	46.1310769	-119.6016087
7	Route 221 at Sellards Road	08/10/2021 12/01/2021	11:09 04:02	No Apparent Injury No Apparent Injury	0 0	2 0 0	Passenger Car Pickup, Panel Truck or Vanett	Pickup, Panel Truck or Vanett under 10,000 lb le Truck - Double Trailer	At Intersection and Related  At Intersection and Related	Clear or Partly Cloudy Clear	Dry	Daylight  Dark-No Street Lights	From same direction - both going straight - one stopped - rear-end Entering at angle	Going Straight Ahead Going Straight Ahead	Stopped at Signal or Stop Sign Going Straight Ahead	East East	West	Vehicle Stopped South	Vehicle Stopped Follow Too Closely North Disregard		None Intersecting Road Increasing Milepost None Lane 1 Decreasing Milepost	1868178.96 1868178.96	292204.46 292204.46	46.1310751	-119.6016151
7	Route 221 at Sellards Road	05/09/2022	17:09	Suspected Serious Injury	2 0	2 0 0	under 10,000 lb Pickup,Panel Truck or Vanett	Combinations be Passenger Car	At Intersection and Related	Clear	Dry	Daylight	Entering at angle	Going Straight Ahead	Going Straight Ahead	West	East	South	Traffic Sign and Signals North Did Not Grant		None Lane 1 Increasing Milepost	1868178.96	292204.46	46.1310751	-119.6016151
7	Route 221 at Sellards Road	08/25/2022	15:45	No Apparent Injury	0 0	2 0 0	under 10,000 lb	Truck - Double Trailer Combinations	At Intersection and Related	Clear or Partly Cloudy	Dry	Daylight	Entering at angle	Making Left Turn	Stopped at Signal or Stop Sign	South	East	East	RW to Vehicle  West Improper		Other Intersecting Road Increasing	1868178.96	292204.46	46.1310751	-119.6016151
7	Route 221 at Sellards Road	on lor i	43.55				Combinations	combinations		Classes Death Class				Making Right Turn	Color Carrishs About	-			Turn/Merge		Contributing Milepost Circ Not Listed	4000430	202204 - 2	46.1310751	-119.6016151
7	Route 221 at Sellards Road	12/09/2022	15:55	No Apparent Injury	0 0		Pickup,Panel Truck or Vanett under 10,000 lb	Birkun Basal Tourk or Verness	At Intersection and Related	Clear or Partly Cloudy Fog or Smog or Smoke	Samultina	Doublehe	Entering at angle	Making Right Turn  Overtaking and Passing	Stongard at Singal or Street	Eur	redtill Mort	Mobiela Stone	North Did Not Grant RW to Vehicle		Later 1 Increasing Milepost	10001/8.96	292204.46	46.1310751	-119.6016151
7 8	Route 221 at Sellards Road Webber Carryon Road at County Well Rd	12/03/2022	06:52	no epairent injury		2 0 0		Pickup, Panel Truck or Vanett under 10,000 lb	e per mod section and related	- of or suink or swore	Snowysiash	- saleRur	From same direction - both going straight - one stopped - rear-end N	O CRASHES REPORTED	woodway ar silkura ou srob silku	-mel	west	ve-stre stopped	Passing Passing	<u> </u>	Milepost	1000111.98	*35550.76	46.1311184	-119.6014407
9	Cemetery Road at Travis Road (north of intersection)	06/26/2019 10/09/2022	21:50	No Apparent Injury No Apparent Injury	0 0	1 0 0	Pickup,Panel Truck or Vanett under 10,000 lb Passenger Car		Not at Intersection and Not Related At Intersection and Not	Overcast	Dry	Dark-No Street Lights	Vehicle Strikes Deer Earth Bank or Ledge	Going Straight Ahead Going Straight Ahead		South	North South		None Overcorrection		Lane of Primary Trafficway  Page the Outside Shouldow of	1905441.34 1905466.84	306326.57	46.1685397	-119.4539148
9	Cemetery Road at Travis Road	,071022	35.30						Related	Fog or Smog or Smoke	[	Dark-No Street Lights	Earth Bank or Ledge	Going Straight Ahead					g / Oversteering		Primary Trafficway	4022077	202720.5	46.157706	-119.454021
10	S. Plymouth Road at Locust Grove Road	07/17/2022	07/20	No Apparent Injury	0 0	1 0 0	Truck - Double Trailer		At Intersection and Related	rog or smog or smoke Clear or Partly Cloudy	Dry	Davieht	Vehicle overturned	Making Left Turn		North	East		Exceeding Reas. Safe Speed Exceeding		Other Location (City/County/Misc. Trafficway) Past Right Shoulder	1868674 6P	221397 37	46.116293	-119.3696745
11	Route 221 at Route 14	12/20/2018	18:24	No Apparent Injury	0 0	2 0 0	Combinations  Passenger Car	Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry	Dark-Street Lights (%)	From same direction - hoth onine ctrainbt	Going Straight Ahead	Stopped at Signal or Stop Sign	North	South	Vehicle Stonner	Exceeding Reas. Safe Speed Whicle Stopped Exceeding		Trafficway)  Past Right Shoulder Increasing Milepost  None Lane 1 Decreasing Milepost	1868674 AP	221397 32	45.9368991	-119.603033
11	Route 221 at Route 14	12/20/2018	13:35	No Apparent Injury	0 0	1 0 0	Passenger Car		At Intersection and Not	Clear or Partly Cloudy	Dry	Daylight	From same direction - both going straight - one stopped - rear-end Fire started in vehicle	Going Straight Ahead	The area of the second	West	East	stopped	Vehicle Stopped Exceeding Reas. Safe Speed None		Lane 1 Increasing Milepost	1868624.68	221397.33	45.9368991	-119.603033
11	Route 221 at Route 14	02/23/2019	19:04	Possible Injury	1 0	1 0 0	Truck Tractor & Semi-Trailer		Related At Intersection and Related	Clear	Dry	Dark-Street Lights On	Jackknife Trailer	Making Right Turn	1	East	North		Apparently Asleep or		Past Right Shoulder Increasing Milepost	1868628.16	221398.38	45.9368991 45.9369019	-119.603033 -119.6030193
11	Route 221 at Route 14				Ш	Ш	1				1	I							Fatigued	1	The state of the s				

																		MV DRIVER CONTRIBUTION	ONTRIBUTI CONTR			WA STATE	WA STATE		
Tetra Tech Intersection			MOST SEVERE INJURY	# FA VE	a a PE BIK				ROADWAY					VEHICLE 1 COMPASS	VEHICLE 1 COMPASS	VEHICLE 2 COMPASS	VEHICLE 2 COMPASS	G GRCUMSTAN	IRCUMSTA CIRCUI	MSTA CIRCUMSTA FIRST	v. County & Misc	LANE SOUTH	PLANE SOUTH - Y 2010 -	LAT	LONG
IDII	Tetra Tech Intersection Name	DATE TIME 05/17/2019 08:58	TYPE No Apparent Injury	INJ T H	DS ES VEHICLE 17  0 0 Pickup,Panel Truck		JUNCTION RELATIONSHIP At Intersection and Related	WEATHER Clear or Partly Cloudy	CONDITION LIGHTING Dry Daylight	CONDITION	FIRST COLLISION TYPE / OBJECT STRUCK Earth Bank or Ledge	VEHICLE 1 ACTION Going Straight Ahead	VEHICLE 2 ACTION	DIRECTION FROM East	DIRECTION TO West	DIRECTION FROM	DIRECTION TO	CE 1 (UNIT 1) Other	1) 1)		rays - 2010 forward) ht Shoulder	FORWARD 1868624.68	FORWARD 221397.33		
					under 10,000 lb													Contributing Circ Not Listed		Decreas	ing Milepost			45.9368991	-119.603033
11	Route 221 at Route 14	10/10/2019 08:10	Suspected Minor Injury	1 0 1	0 0 Truck Tractor & Se	ii-Trailer	At Intersection and Related	Clear or Partly Cloudy	Dry Daylight		Vehicle overturned	Making Right Turn		West	North			Inattention E	xceeding eas. Safe	Intersec	ting Road Decreasing	1868624.68	221397.33		
11	Route 221 at Route 14																	S	eas. Safe peed					45.9368991	-119.603033
11	Route 221 at Route 14	03/23/2020 14:25	5 No Apparent Injury	0 0 1	0 0 Truck - Double Trail Combinations	r	At Intersection and Related	Severe Crosswind	Dry Daylight		Vehicle overturned	Making Left Turn		West	North			Exceeding Reas. Safe		Past Rigi Decreas	ht Shoulder ing Milepost	1868624.68	221397.33	45.9368991	-119.603033
	NDULE 221 at NDULE 14	12/11/2020 12:31	1 No Apparent Injury	0 0 1	0 Truck Tractor & Se	ii-Trailer	At Intersection and Related	Clear	Dry Daylight		Vehicle overturned	Making Left Turn		North	East			Operating Defective		Intersec Milepos	ting Road Increasing	1868653.89	221404.53	45.9369179	-119.6029179
11	Route 221 at Route 14	02/07/2022 05:39	No Annarent Injury	0 0 2	0 0 Truck Tractor & Se	ii.Trailer   Pirkun Panel Truck or Va	natte åt Intercertion and Related	Clear or Partly Cloudy	Dry Dark-Stree	t Lights On	Entering at angle	Making Left Turn	Going Straight Ahead	North	Fact	Fact	West	Equipment Did Not Grant		None Lane 1 F	terreasing Milennst	1868624 68	22139733	43.3303173	113.0023173
11	Route 221 at Route 14	1 1,1,1	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			under 10,000 lb		, , , , , , , , , , , , , , , , , , , ,	.,									RW to Vehicle						45.9368991	-119.603033
		10/02/2022 13:19	No Apparent Injury	0 0 2	0 O Pickup, Panel Truck under 10,000 lb	or Vanette Truck & Trailer	At Intersection and Related	Clear or Partly Cloudy	Dry Daylight		Entering at angle	Making Left Turn	Going Straight Ahead	North	East	East	West	Did Not Grant RW to Vehicle		None Lane 1 E	lecreasing Milepost	1868629.19	221396.04	45.9368954	-119.6030154
11	Route 221 at Route 14	02/09/2018 16:08	Suspected Minor Injury	2 0 2	0 0 Passenger Car	Passenger Car	At Intersection and Related	Clear or Partly Cloudy	Dry Daylight		From same direction - both going straight -	Going Straight Ahead	Stopped for Traffic	East	West	Vehicle Stopped	Vehicle Stopped	Exceeding		None Lane 1 E	ecreasing Milepost	1932612.32	225050.55		
12	Route 14 at S. Plymouth Rd									1	one stopped - rear-end							Reas. Safe Speed						45.9446412	-119.3513535
12	Route 14 at S. Plymouth Rd	03/16/2018 12:57 07/24/2018 19:34	Possible Injury Possible Injury	1 0 2	0 0 Pickup, Panel Truck under 10,000 lb 0 0 Passenger Car	or Vanette Passenger Car Pickup, Panel Truck or Va	Intersection Related but No at Intersection nette At Intersection and Related	Clear or Partly Cloudy Clear or Partly Cloudy	Dry Daylight		From same direction - both going straight - both moving - rear-end From same direction - both going straight -	Going Straight Ahead Going Straight Ahead	Stopped for Traffic	East	West	East	West	Inattention E	ollow Too losely		ecreasing Milepost	1932664.52	225044.33	45.944622	-119.3511487
12	Route 14 at S. Plymouth Rd	08/31/2018 13:49		0 0 2		under 10,000 lb	natto At Intersection and Related	Clear or Partly Cloudy	Day Daylight		one stopped - rear-end From same direction - both going straight -	Going Straight Ahead	Stopped for Traffic	Mort	Eart	Vericle Stopped	Vehicle Stopped	Inattention				1932012.32	225050.55	45.9446412	-119.3513535
12	Route 14 at S. Plymouth Rd	08/14/2019 09:25	No Apparent Injury No Apparent Injury	0 0 2	0 O Pickup Panel Truck	Pickup, Panel Truck or Va under 10,000 lb or Vanette Pickup, Panel Truck or Va	nette At Intersection and Related	Clear or Partly Cloudy	Dry Daylight		one stopped - rear-end From same direction - one left turn - one	Overtaking and Passing	Making Left Turn	East	West	East	South	Inattention II	ngroper		ncreasing Milepost ncreasing Milepost	1932612.32	225050.55	45.9446353	-119.351319
12	Route 14 at S. Plymouth Rd	04/03/2020 12:08	Possible Injury	1 0 2	under 10,000 lb  0 Motorcycle	or Vanette Pickup, Panel Truck or Va under 10,000 lb Motorcycle	At Intersection and Related	Clear	Dry Daylight		straight From same direction - both goine straight -	Going Straight Ahead	Slowing	East	West	East	West	Distractions	assing		lecreasing Milepost	1932602.66	225050.89	45.9446412	-119.3513535
12	Route 14 at S. Plymouth Rd									I	both moving - rear-end							Outside Vehicle						45.9446425	-119.3513914
12	Route 14 at S. Plymouth Rd		Possible Injury	2 0 2	0 0 Pickup,Panel Truck under 10,000 lb		At Intersection and Related	Clear or Partly Cloudy	Dry Daylight	1	From same direction - all others	Backing	Stopped for Traffic	Vehicle Stopped	Vehicle Backing	Vehicle Stopped	Vehicle Stopped	Improper Backing		None Intersec Milepos	ting Road Decreasing t	1932609.21	225050.93	45.9446423	-119.3513657
12	Route 14 at S. Plymouth Rd	05/05/2021 05:51 09/24/2021 18:20	Possible Injury	3 0 2		or Vanette Pickup, Panel Truck or Va under 10,000 lb Truck & Trailer	nette At Intersection and Related	Clear or Partly Cloudy	Dry Daylight		From same direction - one left turn - one straight	Overtaking and Passing	Making Left Turn	East	West	East	South	Improper Passing		None Lane 1 li	ncreasing Milepost	1932609.38	225052.35	45.9446462	-119.3513649
		09/24/2021 18:20	No Apparent Injury	0 0 2	0 O Passenger Car	Truck & Trainer	At Intersection and Related	Clear	Dry Daysignt		From same direction - one left turn - one straight	Going Straight Ahead	Making Left Turn	West	East	West	North	Operating E Defective 8 Equipment S	eas. Safe Reckles peed Aggress	ng None Lane 1 L	lecreasing Milepost	1932602.73	225050.10	45.9446403	-119.3513912
12	Route 14 at S. Plymouth Rd																			,				43.5440403	-119.3313912
		01/01/2022 12:15	No Apparent Injury	0 0 1	0 0 Truck Tractor & Se	ii-Trailer	At Intersection and Related	Clear	Ice Daylight		Roadway Ditch	Making Left Turn		North	East			Exceeding Reas. Safe			ht Shoulder ng Milepost	1932612.20	225050.53	45.9446411	-119.3513539
12	Route 14 at S. Plymouth Rd	<b>05/11/2022</b> 04:52	2 Possible Injury	2 0 2	0 0 Truck Tractor & Se	ii-Trailer Truck Tractor & Semi-Tra	iller At Intersection and Related	Clear	Dry Dawn		Entering at angle	Going Straight Ahead	Going Straight Ahead	North	South	West	East	Speed Other	isregard raffic Sign	None Lane 1 li	ncreasing Milepost	1932609.21	225050.93		
12	Route 14 at S. Plymouth Rd																	Distractions T	raffic Sign nd Signals					45.9446423	-119.3513657
	Day 14 14 C Married D4	07/22/2022 04:27	Dead at Scene	4 1 2	0 0 Pickup,Panel Truck under 10,000 lb	or Vanette Pickup, Panel Truck or Va under 10,000 lb	nette At Intersection and Related	Clear	Dry Dawn	1	Entering at angle	Going Straight Ahead	Going Straight Ahead	North	South	West	East	Did Not Grant RW to Vehicle		None Lane 1 I	ncreasing Milepost	1932602.73	225050.10	45.9446403	-119.3513912
12	Route 14 at S. Plymouth Rd	01/13/2020 17:25	5 No Apparent Injury	0 0 2	0 Truck Tractor & Se	il-Trailer Truck Tractor & Semi-Tra	iller At Intersection and Related	Snowing	Ice Dark-Stree	t Lights On	Entering at angle	Going Straight Ahead	Going Straight Ahead	West	East	North	South	Exceeding C Reas Safe T	isregard raffic Sign	None Lane 1 C	ecreasing Milepost	1936449.74	225334.09	45.9452644	-119.336254
13	I-82 Eastbound Ramps at Route 14	02/08/2020 19:36	5 No Apparent Injury	0 0 1	0 0 Passenger Car		At Intercerting and Related	Clear	Wet Dark-Stree	t Lights On	Wood Sign Post	Making Right Turn		West	South			Speed a	nd Signals	Part Rie	ht Shoulder	1936464 74	225314.46	43.5432044	-119.330234
13	I-82 Eastbound Ramps at Route 14	1	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,															Exceeding Reas. Safe Speed		Increasi	ng Milepost			45.94521	-119.3361962
		12/30/2021 08:31	No Apparent Injury	0 0 1	0 0 Truck Tractor & Se	i-Trailer	At Intersection and Related	Snowing	ice Daylight		Street Light Pole or Base	Making Right Turn		West	South			Exceeding Reas. Safe		Past Rigi Increasi	ht Shoulder ng Milepost	1936461.57	225323.44	45.9452347	-119.3362081
13 14	I-82 Eastbound Ramps at Route 14 Route 14 at I-82 NB Ramps											O CRASHES REPORTED						Speed							
15 16	Coffin Road at I-82 SB Ramps Coffin Road at I-82 NB Ramps											O CRASHES REPORTED  O CRASHES REPORTED													
17	Coffin Road at Bofer Canyon Road Bofer Canyon Road at Beck Road											O CRASHES REPORTED O CRASHES REPORTED													
18 19	Locust Grove Road at I-82 SB Ramps	11/22/2019 21/20										O CRASHES REPORTED													
20	Locust Grove Road at I-82 WB Ramps	11/22/2019 21:36	6 No Apparent Injury	0 0 1	0 O'Truck Tractor & Se	i-Trailer	At Intersection and Related	Fog or Smog or Smoke	Dry Dark-Stree	t Lights On	Roadway Ditch	Going Straight Ahead		East	West			Inattention E	eas. Safe	Milepos	ting Road Decreasing t	1969686.57	297752.35	46.1424102	-119.2009296
20	LOCUS GIOTE ROSO SE PAZ WO REINDS	12/04/2020 21:52	No Apparent Injury	0 0 1	0 Truck & Trailer		At Intersection and Related	Fog or Smog or Smoke	Wet Dark-Stree	t Lights On	Roadway Ditch	Going Straight Ahead		East	West			Exceeding C Reas, Safe T	isregard raffic Sign	Past Rigi Decreas	ht Shoulder ing Milepost	1969684.83	297752.95	46.1424119	-119.2009364
20	Locust Grove Road at I-82 WB Ramps	08/22/2022 08:48	Suspected Minor Injury	1 0 2	0 0 Truck Tractor & Se	ii-Trailer Motorcycle	At Intersection and Related	Clear or Partly Cloudy	Dry Daylight	1	Entering at angle	Going Straight Ahead	Going Straight Ahead	East	West	North	South	Speed a Other	nd Signals	Other Lane 1 D	lecreasing Milepost	1969684.06	297750.69	.0.1727119	-13.2003304
											-		-					Contributing Circ Not Listed		Contributing Circ Not				46.1424058	-119.2009396
20	Locust Grove Road at I-82 WB Ramps	12/10/2020 05:29	9 Unknown	0 0 1	0 0 Passenger Car		At Intersection and Related	Fog or Smag or Smake	Wet Dark-No Si	reet Lights	Metal Sign Post	Going Straight Ahead		South	North			Exceeding L Reas Safe D	inknown		ting Road Decreasing	1969823.18	297777.75		
21	Route 397/Locust Grove Road at Bofer Canyon Rd			ШШ										1				Speed C	istraction	Milepos				46.1424737	-119.2003891
22	Route 397 at S. Olympia Street  Route 397 at S. Nine Carryon Road	10/17/2018 16:56	S No Apparent Injury	0 0 2	0 Truck Tractor & Se	ii-Trailer Truck Tractor & Semi-Tra	iller At Intersection and Related	Clear or Partly Cloudy	Dry Daylight	1	From same direction - both going straight -	Going Straight Ahead	Stopped at Signal or Stop Sign	South	North	Vehicle Stopped	Vehicle Stopped	Inattention			ting Road Increasing	2003710.62	292702.87	46.1269493	-119.0670844
23	Houte 397 at 5. Nine Carryon Hoad	05/25/2019 00:39	No Apparent Injury	0 0 1	0 0 Passenger Car		At Intersection and Related	Overcast	Dry Dark-No Si	reet Lights	one stopped - rear-end Tree or Stump (stationary)	Going Straight Ahead		South	North			Disregard Stop E Sign - Flashing S	sceeding tated	Milepos Past Rigi Decreas	t ht Shoulder ing Milepost	2003719.88	292697.08		
23	Route 397 at S. Nine Carryon Road																	Red S	peed Limit	Secress				46.126933	-119.0670483
	Marie and a second second second	09/14/2022 09:40	No Apparent Injury	0 0 1	0 0 Truck (Flatbad, Van	etc)	At Intersection and Related	Clear	Dry Daylight		Vehicle overturned	Making Right Turn		South	East			Other Contributing		Right Sh Milepos	oulder Decreasing t	2009521.98	291295.73		
24	Route 397 at S. Finley Road			ЩП										<u> </u>				Circ Not Listed						46.1227994	-119.0442706
25 26	Nine Carryon Road at Kirk Road Nine Carryon Road at Beck Road											O CRASHES REPORTED													
27	Nine Carryon Road at Coffin Road	00 (34 (3030)	distance.		al alamona		Terrane and a	In	Inc. Incr.			O CRASHES REPORTED	ı	Free	literate			I		le	Autorita de 1	1045557 12	207704		
28	Locust Grove at S. ClodFelter Road	09/21/2020 16:00	Oriendwh	0 0 1	0 0 Passenger Car		At Intersection and Related	Cital	Daylight Daylight	ľ	Metal Sign Post	Making Right Turn		cest	worth			Improper Turn/Merge		Primary	Outside Shoulder of Trafficway	1945bb/.42	297701.77	46.1433167	-119.2956802
29	Webber Canyon Road south of Badger Road	10/23/2019 20:50	Possible Injury	1 0 1	0 0 Passenger Car		Not at Intersection and Not Related	Clear or Partly Cloudy	Dry Dark-No Si	reet Lights	Guardrail - Through, Over or Under	Going Straight Ahead		East	West			Inattention		Outside Traffice	Shoulder of Primary ay	1905171.10	310035.30	46.178719	-119.4547873

2 of 2

### **Intersection Crash Rate Worksheet**

Horse Heaven Wind Farm Benton County, Washington

Tetra Tech Intersection ID#	Tetra Tech Intersection Name	Total PM Peak Hourly Approach Volume <sup>1</sup>	"K" Factor <sup>2</sup>	INTERSECTION ADT (V) = TOTAL DAILY APPROACH VOLUME:	Total # of Crashes <sup>3</sup>	# of Years	Average # Crashes per Year	Crash Rate Calculation⁴
1	Wine Country Road at I-82 WB Ramps	527	0.10	5270	3	5	0.6	0.31
2	Wine Country Road at I-82 EB Ramps	831	0.10	8310	5	5	1	0.33
3	Wine Country Road at Route 22	928	0.10	9280	7	5	1.4	0.41
4	Route 22 at Route 221 and Paterson Ave	649	0.10	6490	9	5	1.8	0.76
5	Route 221 at County Well Road	338	0.10	3380	0	5	0	0.00
6	Route 221 at Cemetery Road	287	0.10	2870	0	5	0	0.00
7	Route 221 at Sellards Road	358	0.10	3580	12	5	2.4	1.84
8	Webber Canyon Road at County Well Rd	109	0.10	1090	0	5	0	0.00
9	Cemetery Road at Travis Road	89	0.10	890	2	5	0.4	1.23
10	S. Plymouth Road at Locust Grove Road	161	0.10	1610	1	5	0.2	0.34
11	Route 221 at Route 14	341	0.10	3410	10	5	2	1.61
12	Route 14 at S. Plymouth Rd	639	0.10	6390	12	5	2.4	1.03
13	I-82 Eastbound Ramps at Route 14	585	0.10	5850	3	5	0.6	0.28
14	Route 14 at I-82 NB Ramps	286	0.10	2860	0	5	0	0.00
15	Coffin Road at I-82 SB Ramps	40	0.10	400	0	5	0	0.00
16	Coffin Road at I-82 NB Ramps	45	0.10	450	0	5	0	0.00
17	Coffin Road at Bofer Canyon Road	39	0.10	390	0	5	0	0.00
18	Bofer Canyon Road at Beck Road	11	0.10	110	0	5	0	0.00
19	Locust Grove Road at I-82 SB Ramps	215	0.10	2150	0	5	0	0.00
20	Locust Grove Road at I-82 WB Ramps	385	0.10	3850	3	5	0.6	0.43
21	Route 397/Locust Grove Road at Bofer Canyon Rd	339	0.10	3390	1	5	0.2	0.16
22	Route 397 at S. Olympia Street	396	0.10	3960	0	5	0	0.00
23	Route 397 at S. Nine Canyon Road	186	0.10	1860	2	5	0.4	0.59
24	Route 397 at S. Finley Road	110	0.10	1100	1	5	0.2	0.50
25	Nine Canyon Road at Kirk Road	24	0.10	240	0	5	0	0.00
26	Nine Canyon Road at Beck Road	26	0.10	260	0	5	0	0.00
27	Nine Canyon Road at Coffin Road	41	0.10	410	0	5	0	0.00
28	Locust Grove at S. ClodFelter Road	101	0.10	1010	1	5	0.2	0.54
29	Webber Canyon Road at Badger Road	221	0.10	2210	1	5	0.2	0.25

#### Notes:

- 1) Based on peak period turning movement count data collected on June 13th, 2023 & June 14th, 2023.
  2) PM Peak Hour K Factor was calculated using ATR Data collected throughout the study area.

- 3) Crash Data provided by WSDOT.4) Crash Rate calculated per million entering vehicles.

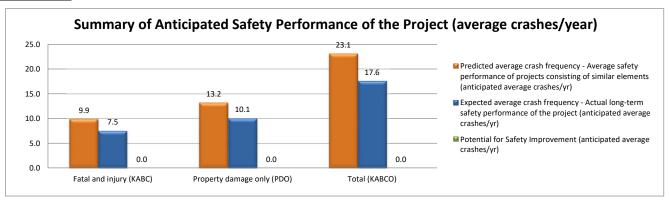
### PROJECT SAFETY PERFORMANCE SUMMARY REPORT

### **General Information**

Project Name
Project Name
Project Description
Reference Number
Analyst
Agency/Company
Contact Email
Contact Phone
Horse Heaven Wind Farm
Safety Analysis
143-67639-23007
James Vorosmarti
WSDOT
James.vorosmarti@tetratech.com

Date Completed 07/17/23 Years of crash data incorporated into the analysis: 5

### PROJECT SUMMARY



		Total Crashes/yr (KABCO)		Fata	l and Injury Crasho (KABC)	es/yr	Propert	y Damage Only Cra (PDO)	ashes/yr
Project Element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
INDIVIDUAL INTERSECTIONS									
Intersection 1	1.0	0.7	0.0	0.4	0.3	0.0	0.6	0.4	0.0
Intersection 2	1.0	1.0	0.0	0.4	0.4	0.0	0.6	0.6	0.0
Intersection 3	3.7	1.8	0.0	1.6	0.8	0.0	2.1	1.0	0.0
Intersection 4	2.2	1.9	0.0	1.0	0.8	0.0	1.3	1.1	0.0
Intersection 5	0.3	0.2	0.0	0.1	0.1	0.0	0.2	0.1	0.0
Intersection 6	0.1	0.1	0.0	0.0	0.0	0.0	0.1	0.0	0.0
Intersection 7	1.5	2.1	0.6	0.6	0.9	0.3	0.8	1.2	0.3
Intersection 8	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0
Intersection 9	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.1	0.0
Intersection 10	0.2	0.2	0.0	0.1	0.1	0.0	0.1	0.1	0.0
Intersection 11	1.7	1.9	0.2	0.7	0.8	0.1	1.0	1.1	0.1
Intersection 12	1.3	1.7	0.4	0.6	0.7	0.2	0.7	1.0	0.2
Intersection 13	1.7	1.0	0.0	0.7	0.4	0.0	1.0	0.5	0.0
Intersection 14	2.0	0.6	0.0	0.9	0.3	0.0	1.1	0.3	0.0
Intersection 15	0.2	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0
Intersection 16	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0
Intersection 17	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0
Intersection 18	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Intersection 19	1.3	0.5	0.0	0.5	0.2	0.0	0.7	0.3	0.0
Intersection 20	2.0	1.0	0.0	0.9	0.4	0.0	1.1	0.6	0.0
Intersection 21	0.5	0.4	0.0	0.2	0.2	0.0	0.3	0.2	0.0
Intersection 22	0.6	0.2	0.0	0.2	0.1	0.0	0.3	0.1	0.0
Intersection 23	0.4	0.4	0.0	0.2	0.2	0.0	0.2	0.2	0.0
Intersection 24	0.1	0.1	0.0	0.0	0.1	0.0	0.1	0.1	0.0
Intersection 25	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Intersection 26	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Intersection 27	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0
Intersection 28	0.3	0.3	0.0	0.1	0.1	0.0	0.2	0.2	0.0
Intersection 29	0.5	0.8	0.3	0.2	0.3	0.1	0.3	0.5	0.2
COMBINED (sum of column)	23.1	17.6	0.0	9.9	7.5	0.0	13.2	10.1	0.0

### PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Rural 2-Lane Roads

	N predicted(PROJECT)	N expected (PROJECT)	N potential for improvement (PROJECT)
Crash severity level	Predicted average crash frequency - Average safety performance of projects consisting of similar elements (anticipated average crashes/yr)	Expected average crash frequency - Actual long-term safety performance of the project (anticipated average crashes/yr)	Potential for Safety Improvement (anticipated average crashes/yr)
Fatal and injury (KABC)	9.9	7.5	N/A
Property damage only (PDO)	13.2	10.1	N/A
Total (KABCO)	23.1	17.6	N/A

HSM1 Extended Spreadsheet for Part C Chapter 10 v.9.1

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Inter	sections	
	Location Information		
	Roadway	0	
	Intersection	Wine Country Road	at I-82 NB Ramps
	Jurisdiction	WSDOT	
	Analysis Year	2023	
	Site Conditions		Base Conditions

Intersection type (3ST, 4ST, 4SG)					3ST		
AADT <sub>major</sub> (veh/day)							
AADT <sub>minor</sub> (veh/day)	(veh/day)		2	2,440			
Intersection skew angle (degrees) [If 4ST, does skew di	iffer for minor legs? Else, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled approaches with a left-	turn lane (0, 1, 2, 3, 4)				1		0
Number of signalized or uncontrolled approaches with a right		0				0	
Intersection lighting (present/not present)		Present				Not Present	
Calibration Factor, C <sub>i</sub>		1.00				1.00	

# Average Annual Crash History (3 or 5-yr average)

James Vorosmarti WSDOT

07/17/23

Intersection 1

Unsignalized

General Information

Agency or Company

Date Performed

Signalized/Unsignalized

Analyst

Intersection

Input Data

Intersection crashes	KABC	Fatal and Injury Only	0.2
intersection crashes	PDO	Property Damage Only	0.4

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)								
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF								
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>								
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)								
1.00	0.56	1.00	0.90	0.50								

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections												
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)						
	N	Overdispersion	Crash Severity	N <sub>spf 3ST, 4ST or 4SG</sub> by Severity	Combined CMFs		Predicted average crash frequency, N						
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, G	predicted int						
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, C	(5)*(6)*(7)						
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5) (6) (7)						
Total	2.079	0.54	1.000	2.079	0.50	1.00	1.049						
Fatal and Injury (FI)			0.415	0.863	0.50	1.00	0.435						
Property Damage Only (PDO)			0.585	1.216	0.50	1.00	0.614						

	Worksheet 2D Crashes by Severity Level and Collision Type for Rural Two-Lane Two-Way Road Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)					
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C					
Total	1.000	1.049	1.000	0.435	1.000	0.614					
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO					
			SINGLE-VEHICL	.E							
Collision with animal	0.019	0.020	0.008	0.003	0.026	0.016					
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001					
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001					
Overturned	0.013	0.014	0.022	0.010	0.007	0.004					
Ran off road	0.244	0.256	0.240	0.105	0.247	0.152					
Other single-vehicle collision	0.016	0.017	0.011	0.005	0.020	0.012					
Total single-vehicle crashes	0.294	0.309	0.283	0.123	0.302	0.185					
			MULTIPLE-VEHIO	CLE							
Angle collision	0.237	0.249	0.275	0.120	0.210	0.129					
Head-on collision	0.052	0.055	0.081	0.035	0.032	0.020					
Rear-end collision	0.278	0.292	0.260	0.113	0.292	0.179					
Sideswipe collision	0.097	0.102	0.051	0.022	0.131	0.080					
Other multiple-vehicle collision	0.042	0.044	0.050	0.022	0.033	0.020					
Total multiple-vehicle crashes	0.706	0.741	0.717	0.312	0.698	0.428					

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.0						
Fatal and Injury (FI)	0.415	0.4						
Property Damage Only (PDO)	0.585	0.6						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
Total Crashes/yr Fatal and Injury Crashes/yr Property Damage Only Crashes/yr									ishes/yr	
	(KABCO)				(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.0	0.7	0.0	0.4	0.3	0.0	0.6	0.4	0.0	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Works	sheet 2A Ger	eral Information and	Input Data for Rural	íwo-Lane Two-V	Vay Roadway Inter	sections	
General Information					Location Informati	on			
Analyst	James Vorosmarti				Roadway			0	
Agency or Company	WSDOT				Intersection			Wine Country Road	at I-82 SB Ramps
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	section Intersection 2			Analysis Year			2023		
Signalized/Unsignalized	ignalized/Unsignalized Unsignalized								
Input Data					Site Conditions		Base Conditions		
Intersection type (3ST, 4ST, 4SG)					3ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	8,310				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)		1,160			
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	()	0		
Number of signalized or uncontrolled	approaches with a left-t	urn lane (0, 1, 2, 3, 4)			1			0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)					0 0			0	

Present

1.00

Not Present

1.00

Average Annual Crash History (3 or 5-yr average)

Intersection lighting (present/not present)

Calibration Factor, Ci

Intersection crashes	KABC	Fatal and Injury Only	0.4
intersection crashes	PDO	Property Damage Only	0.6

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.56	1.00	0.90	0.50						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections												
(1) (2)		(3)	(4)	(5)	(6)	(7)	(8)					
	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N					
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int					
Clasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)					
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B		(3) (0) (7)					
Total	2.070	0.54	1.000	2.070	0.50	1.00	1.045					
Fatal and Injury (FI)			0.415	0.859	0.50	1.00	0.433					
Property Damage Only (PDO)			0.585	1.211	0.50	1.00	0.611					

	Worksheet 2D Crashes by Severity Level and Collision Type for Rural Two-Lane Two-Way Road Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)					
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C					
Total	1.000	1.045	1.000	0.433	1.000	0.611					
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO					
			SINGLE-VEHICL	.E							
Collision with animal	0.019	0.020	0.008	0.003	0.026	0.016					
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001					
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001					
Overturned	0.013	0.014	0.022	0.010	0.007	0.004					
Ran off road	0.244	0.255	0.240	0.104	0.247	0.151					
Other single-vehicle collision	0.016	0.017	0.011	0.005	0.020	0.012					
Total single-vehicle crashes	0.294	0.307	0.283	0.123	0.302	0.185					
			MULTIPLE-VEHIO	CLE							
Angle collision	0.237	0.248	0.275	0.119	0.210	0.128					
Head-on collision	0.052	0.054	0.081	0.035	0.032	0.020					
Rear-end collision	0.278	0.290	0.260	0.113	0.292	0.178					
Sideswipe collision	0.097	0.101	0.051	0.022	0.131	0.080					
Other multiple-vehicle collision	0.042	0.044	0.050	0.022	0.033	0.020					
Total multiple-vehicle crashes	0.706	0.737	0.717	0.311	0.698	0.427					

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.0						
Fatal and Injury (FI)	0.415	0.4						
Property Damage Only (PDO)	0.585	0.6						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
Total Crashes/yr Fatal and Injury Crashes/yr Property Damage Only Crashes/yr									ishes/yr	
	(KABCO)				(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.0	1.0	0.0	0.4	0.4	0.0	0.6	0.6	0.0	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
I acation Information

General Information		Location Information			
Analyst	James Vorosmarti	Roadway	0		
Agency or Company	WSDOT	Intersection	Wine Country Road at Route 22		
Date Performed	07/17/23	Jurisdiction	WSDOT		
Intersection	Intersection 3	Analysis Year	2023		
Signalized/Unsignalized Unsignalized					
Input Data		Site Conditions		Base Conditions	

Intersection type (3ST, 4ST, 4SG)			4ST				
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 14,700	(veh/day)	9,280				
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 3,500	(veh/day)	3,130				
Intersection skew angle (degrees) [If 4ST, does skew di	ffer for minor legs? Else, No.]	Yes	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	75	0
Number of signalized or uncontrolled approaches with a left-t	mber of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			2			0
Number of signalized or uncontrolled approaches with a right	-turn lane (0, 1, 2, 3, 4)		0			0	
Intersection lighting (present/not present)			Present			Not Present	
Calibration Factor, C <sub>i</sub>			1.00			1.00	

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	0.8	
intersection crasiles	PDO	Property Damage Only	0.6	

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.25	0.52	1.00	0.91	0.59				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int	
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)	
Total	6.241	0.24	1.000	6.241	0.59	1.00	3.679	
Fatal and Injury (FI)			0.431	2.690	0.59	1.00	1.586	
Property Damage Only (PDO)			0.569	3.551	0.59	1.00	2.094	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	3.679	1.000	1.586	1.000	2.094
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)pdo
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.037	0.006	0.010	0.014	0.029
Collision with bicycle	0.001	0.004	0.001	0.002	0.001	0.002
Collision with pedestrian	0.001	0.004	0.001	0.002	0.001	0.002
Overturned	0.005	0.018	0.006	0.010	0.004	0.008
Ran off road	0.122	0.449	0.094	0.149	0.144	0.301
Other single-vehicle collision	0.008	0.029	0.004	0.006	0.010	0.021
Total single-vehicle crashes	0.147	0.541	0.112	0.178	0.174	0.364
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	1.586	0.532	0.844	0.354	0.741
Head-on collision	0.040	0.147	0.060	0.095	0.025	0.052
Rear-end collision	0.242	0.890	0.210	0.333	0.266	0.557
ideswipe collision	0.101	0.372	0.044	0.070	0.144	0.301
Other multiple-vehicle collision	0.039	0.143	0.042	0.067	0.037	0.077
Total multiple-vehicle crashes	0.853	3.139	0.888	1.408	0.826	1.729

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	3.7					
Fatal and Injury (FI)	0.431	1.6					
Property Damage Only (PDO)	0.569	2.1					

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr		Fata	l and Injury Crash	es/yr	Property Damage Only Crashes/yr				
		(KABCO)			(KABC)		(PDO)			
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	3.7	1.8	0.0	1.6	0.8	0.0	2.1	1.0	0.0	
Special Note: When the proje	ect element is not included	in the analysis the results wi	ll all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	ot carried out), the results will	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information		Location information				
Analyst	James Vorosmarti	Roadway	0			
Agency or Company	WSDOT	Intersection	Route 22 at Route 221 and Paterson Ave			
Date Performed	07/17/23	Jurisdiction	WSDOT			
Intersection	Intersection 4	Analysis Year	2023			
Signalized/Unsignalized	Unsignalized					

Input Data			Conditions	Base Conditions
Intersection type (3ST, 4ST, 4SG)			4ST	
AADT <sub>MAX</sub> = 14,700	(veh/day)		6,590	
AADT <sub>MAX</sub> = 3,500	(veh/day)		2,740	
iffer for minor legs? Else, No.]	No	Skew for Leg 1 (All): 0 Skew for Leg 2 (4ST only): 0		0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			2	0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0	0
Intersection lighting (present/not present)			Present	Not Present
			1.00	1.00
	AADT <sub>MAX</sub> = 3,500  iffer for minor legs? Else, No.]  turn lane (0, 1, 2, 3, 4)	AADT <sub>MAX</sub> = 3,500 (veh/day)  iffer for minor legs? Else, No.] No  turn lane (0, 1, 2, 3, 4)	AADT <sub>MAX</sub> = 14,700 (veh/day)  AADT <sub>MAX</sub> = 3,500 (veh/day)  liffer for minor legs? Else, No.]  No Skew for Leg 1 (All):  turn lane (0, 1, 2, 3, 4)  t-turn lane (0, 1, 2, 3, 4)	AADT <sub>MAX</sub> = 14,700 (veh/day) 6,590  AADT <sub>MAX</sub> = 3,500 (veh/day) 2,740  iffer for minor legs? Else, No.] No Skew for Leg 1 (All): 0 Skew for Leg 2 (4ST only): 0  turn lane (0, 1, 2, 3, 4) 2  t-turn lane (0, 1, 2, 3, 4) 0  Present

<b>Average Annual</b>	Crash	History	(3 or	5-yr	average)
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Intersection crashes	KABC	Fatal and Injury Only	0.2
intersection crashes	PDO	Property Damage Only	1.6

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections						
(1)	(2)	(3)	(4)	(5)			
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF			
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>			
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)			
1.00	0.52	1.00	0.91	0.47			

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int	
Crash Severity Lever	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5 (2/TOTAL (4)		Worksheet 2B		(3) (0) (7)	
Total	4.686	0.24	1.000	4.686	0.47	1.00	2.211	
Fatal and Injury (FI)			0.431	2.020	0.47	1.00	0.953	
Property Damage Only (PDO)			0.569	2.666	0.47	1.00	1.258	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8) <sub>FI</sub> from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	2.211	1.000	0.953	1.000	1.258
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.022	0.006	0.006	0.014	0.018
Collision with bicycle	0.001	0.002	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.002	0.001	0.001	0.001	0.001
Overturned	0.005	0.011	0.006	0.006	0.004	0.005
Ran off road	0.122	0.270	0.094	0.090	0.144	0.181
Other single-vehicle collision	0.008	0.018	0.004	0.004	0.010	0.013
Total single-vehicle crashes	0.147	0.325	0.112	0.107	0.174	0.219
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.953	0.532	0.507	0.354	0.445
Head-on collision	0.040	0.088	0.060	0.057	0.025	0.031
Rear-end collision	0.242	0.535	0.210	0.200	0.266	0.335
Sideswipe collision	0.101	0.223	0.044	0.042	0.144	0.181
Other multiple-vehicle collision	0.039	0.086	0.042	0.040	0.037	0.047
Total multiple-vehicle crashes	0.853	1.886	0.888	0.846	0.826	1.039

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections						
(1)	(2)	(3)				
Crash squarity lovel	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)				
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C				
Total	1.000	2.2				
Fatal and Injury (FI)	0.431	1.0				
Property Damage Only (PDO)	0.569	1.3				

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
Total Crashes/yr (KABCO)		Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)				
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	2.2	1.9	0.0	1.0	0.8	0.0	1.3	1.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections					
	Location Information				
	Roadway	0			
	Intersection Route 221 at County Well Road		Well Road		
	Jurisdiction WSDOT				
	Analysis Year	2023			
	Site Conditions		Base Conditions		

AADT <sub>major</sub> (veh/day) AADT <sub>MAX</sub> = 19,500 (veh/day) 3,380		
AADT <sub>minor</sub> (veh/day)		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No Skew for Leg 1 (All): 0 Skew for Leg (4ST only	0	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)		0
Intersection lighting (present/not present)  Not Present		Not Present
Calibration Factor, C <sub>i</sub>		1.00

# Average Annual Crash History (3 or 5-yr average)

James Vorosmarti WSDOT

07/17/23

Intersection 5

Unsignalized

**General Information** 

Agency or Company

Date Performed

Signalized/Unsignalized

Analyst

Intersection

Input Data

Intersection crashes		Property Damage Only	0.0
	KABC	Fatal and Injury Only	0.0

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections						
(1)	(2)	(3)	(4)	(5)			
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF			
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>			
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)			
1.00	1.00	1.00	1.00	1.00			

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int	
Crasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5 (2)TOTAL (4)		Worksheet 2B		(3) (0) (7)	
Total	0.274	0.54	1.000	0.274	1.00	1.00	0.274	
Fatal and Injury (FI)			0.415	0.114	1.00	1.00	0.114	
Property Damage Only (PDO)			0.585	0.160	1.00	1.00	0.160	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.274	1.000	0.114	1.000	0.160
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)pdo
			SINGLE-VEHICL	E		
Collision with animal	0.019	0.005	0.008	0.001	0.026	0.004
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.004	0.022	0.003	0.007	0.001
Ran off road	0.244	0.067	0.240	0.027	0.247	0.040
Other single-vehicle collision	0.016	0.004	0.011	0.001	0.020	0.003
Total single-vehicle crashes	0.294	0.081	0.283	0.032	0.302	0.048
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.065	0.275	0.031	0.210	0.034
Head-on collision	0.052	0.014	0.081	0.009	0.032	0.005
Rear-end collision	0.278	0.076	0.260	0.030	0.292	0.047
Sideswipe collision	0.097	0.027	0.051	0.006	0.131	0.021
Other multiple-vehicle collision	0.042	0.012	0.050	0.006	0.033	0.005
Total multiple-vehicle crashes	0.706	0.194	0.717	0.082	0.698	0.112

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crack coverity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.3					
Fatal and Injury (FI)	0.415	0.1					
Property Damage Only (PDO)	0.585	0.2					

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr (KABCO)		Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.3	0.2	0.0	0.1	0.1	0.0	0.2	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Works	sheet 2A Ge	neral Information and	Input Data for Rural Two-I	_ane Two-V	Vay Roadway Inters	ections	
General Information					<b>Location Information</b>				
Analyst	James Vorosmarti	James Vorosmarti						0	
Agency or Company	WSDOT				Intersection			Route 221 at Cemete	ery Road
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	on Intersection 6				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized								
Input Data					Site Conditions			Base Conditions	
Intersection type (3ST, 4ST, 4SG)					4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	2,870				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	10				
Intersection skew angle (degrees)	[If 4ST, does skew di	iffer for minor legs? Els	se, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolle	ed approaches with a left-t	turn lane (0, 1, 2, 3, 4)				0			0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not p	resent)				Not Present			Not Present	
Calibration Factor, C <sub>i</sub>				1.00			1.00		

NOTES: * AADT: It is important to remember that the AADT(r	(major) = AADT(major approach1) + AAD	OT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Intersection crashes

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

KABC

PDO

Fatal and Injury Only

Property Damage Only

0.0

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int				
Crasii Severity Lever	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Canbration ractor, q	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	0.093	0.24	1.000	0.093	1.00	1.00	0.093				
Fatal and Injury (FI)			0.431	0.040	1.00	1.00	0.040				
Property Damage Only (PDO)			0.569	0.053	1.00	1.00	0.053				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.093	1.000	0.040	1.000	0.053
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)pdo
			SINGLE-VEHIC	.E		
Collision with animal	0.010	0.001	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.000	0.006	0.000	0.004	0.000
Ran off road	0.122	0.011	0.094	0.004	0.144	0.008
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.001
Total single-vehicle crashes	0.147	0.014	0.112	0.004	0.174	0.009
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.040	0.532	0.021	0.354	0.019
Head-on collision	0.040	0.004	0.060	0.002	0.025	0.001
Rear-end collision	0.242	0.022	0.210	0.008	0.266	0.014
Sideswipe collision	0.101	0.009	0.044	0.002	0.144	0.008
Other multiple-vehicle collision	0.039	0.004	0.042	0.002	0.037	0.002
Total multiple-vehicle crashes	0.853	0.079	0.888	0.035	0.826	0.044

	Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections										
(1)	(2)	(3)									
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)									
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C									
Total	1.000	0.1									
Fatal and Injury (FI)	0.431	0.0									
Property Damage Only (PDO)	0.569	0.1									

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr (KABCO)		Fata	l and Injury Crash (KABC)	es/yr	Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.0	0.0	0.0	0.1	0.0	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Works	sheet 2A Ger	neral Information and	Input Data for Rural 1	wo-Lane Two-\	Vay Roadway Inter	sections	
General Information					Location Informati	on			
Analyst	James Vorosmarti				Roadway			0	
Agency or Company	WSDOT				Intersection			Route 221 at Sellard	s Road
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	tion Intersection 7				Analysis Year			2023	
Signalized/Unsignalized	nalized/Unsignalized Unsignalized								
Input Data					Site Conditions			Base Conditions	
Intersection type (3ST, 4ST, 4SG)					4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	3,580				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	760				
Intersection skew angle (degrees)	ction skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			No	Skew for Leg 2 (All): O Skew for Leg 2 (4ST only):		0		
Number of signalized or uncontrolled	approaches with a left-	turn lane (0, 1, 2, 3, 4)			0			0	
Number of signalized or uncontrolled	approaches with a right	-turn lane (0, 1, 2, 3, 4	)				0		0
Intersection lighting (present/not present)					Not Present			Not Present	
Calibration Factor, C <sub>i</sub>					1.00			1.00	
Average Annual Crash History (3 or 5	yr average)								_

NOTES: * AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)	)

Intersection crashes

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

KABC

PDO

Fatal and Injury Only

Property Damage Only

0.6

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N			
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int			
Crash Seventy Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)			
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B					
Total	1.486	0.24	1.000	1.486	1.00	1.00	1.486			
Fatal and Injury (FI)			0.431	0.641	1.00	1.00	0.641			
Property Damage Only (PDO)			0.569	0.846	1.00	1.00	0.846			

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.486	1.000	0.641	1.000	0.846
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.015	0.006	0.004	0.014	0.012
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.007	0.006	0.004	0.004	0.003
Ran off road	0.122	0.181	0.094	0.060	0.144	0.122
Other single-vehicle collision	0.008	0.012	0.004	0.003	0.010	0.008
Total single-vehicle crashes	0.147	0.218	0.112	0.072	0.174	0.147
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.641	0.532	0.341	0.354	0.299
Head-on collision	0.040	0.059	0.060	0.038	0.025	0.021
Rear-end collision	0.242	0.360	0.210	0.135	0.266	0.225
Sideswipe collision	0.101	0.150	0.044	0.028	0.144	0.122
Other multiple-vehicle collision	0.039	0.058	0.042	0.027	0.037	0.031
Total multiple-vehicle crashes	0.853	1.268	0.888	0.569	0.826	0.698

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections						
(1)	(2)	(3)				
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)				
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C				
Total	1.000	1.5				
Fatal and Injury (FI)	0.431	0.6				
Property Damage Only (PDO)	0.569	0.8				

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ashes/yr
		(KABCO)			(KABC)			(PDO)	
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	1.5	2.1	0.6	0.6	0.9	0.3	0.8	1.2	0.3
Special Note: When the proje	pecial Note: When the project element is not included in the analysis the results will all be zeros. In addition if only the analysis only includes determining the predicted average crash frequency (i.e. EB analysis is not carried out), the results will show zero values where EB								

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Worksh	eet 2A Gener	al Information and	Input Data for Rural	Two-Lane Two-V	Vay Roadway Inters	sections	
General Information					Location Information				
Analyst	James Vorosmarti				Roadway			0	
Agency or Company	WSDOT				Intersection Webber Canyon Roa			d at County Well Rd	
Date Performed	07/17/23				Jurisdiction Benton County			Benton County	
Intersection	Intersection 8				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized								
Input Data				Site Conditions				Base Conditions	
Intersection type (3ST, 4ST, 4SG)					3ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	1,090				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)			110		
Intersection skew angle (degrees)	[If 4ST, does skew di	iffer for minor legs? Else,	, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	()	0
Number of signalized or uncontrol	led approaches with a left-	turn lane (0, 1, 2, 3, 4)				0			0
Number of signalized or uncontrol	led approaches with a right	-turn lane (0, 1, 2, 3, 4)			0			0	
Intersection lighting (present/not p	present)					Not Present			Not Present

1.00

1.00

Average Annual Crash History (3 or 5-yr average)

Calibration Factor, C<sub>i</sub>

Intersection crashes	KABC	Fatal and Injury Only	0.0	
intersection classies	PDO	Property Damage Only	0.0	

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections							
(1)	(2)	(3)	(4)	(5)			
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF			
CMF <sub>1i</sub>	CMF 2i	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>			
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)			
1.00	1.00	1.00	1.00	1.00			

		Worksheet 2C I	ntersection Crashe	s for Rural Two-Lane Two-Way R	oadway Intersection	ns		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int	
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B			
Total	0.131	0.54	1.000	0.131	1.00	1.00	0.131	
Fatal and Injury (FI)			0.415	0.054	1.00	1.00	0.054	
Property Damage Only (PDO)			0.585	0.077	1.00	1.00	0.077	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8) <sub>FI</sub> from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.131	1.000	0.054	1.000	0.077
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	E		
Collision with animal	0.019	0.002	0.008	0.000	0.026	0.002
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.002	0.022	0.001	0.007	0.001
Ran off road	0.244	0.032	0.240	0.013	0.247	0.019
Other single-vehicle collision	0.016	0.002	0.011	0.001	0.020	0.002
Total single-vehicle crashes	0.294	0.039	0.283	0.015	0.302	0.023
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.031	0.275	0.015	0.210	0.016
Head-on collision	0.052	0.007	0.081	0.004	0.032	0.002
Rear-end collision	0.278	0.036	0.260	0.014	0.292	0.022
Sideswipe collision	0.097	0.013	0.051	0.003	0.131	0.010
Other multiple-vehicle collision	0.042	0.006	0.050	0.003	0.033	0.003
Total multiple-vehicle crashes	0.706	0.093	0.717	0.039	0.698	0.054

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.1					
Fatal and Injury (FI)	0.415	0.1					
Property Damage Only (PDO)	0.585	0.1					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr (KABCO)		Fata	l and Injury Crash (KABC)	es/yr	Prope	erty Damage Only Cra (PDO)	shes/yr
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information	General Information				Location Information				
Analyst	James Vorosmarti	mes Vorosmarti				Roadway		0	
Agency or Company	WSDOT	WSDOT			Intersection	Intersection Cemetery Ro		avis Road	
Date Performed	07/17/23	07/17/23 J			Jurisdiction Benton County				
Intersection	Intersection 9	Intersection 9			Analysis Year		2023		
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data					Site Conditions			Base Conditions	
Intersection type (3ST, 4ST, 4SG)	_				4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)		910			
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)		20			

0

0

Not Present

0

0

Not Present

1.00

Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4) Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4) Intersection lighting (present/not present)

Calibration Factor, C<sub>1</sub>

Average Annual Crash History (3 or 5-yr average)

Intersection crashes

KABC PDO Property Damage Only

0.0

0.4

Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	1.00	1.00				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Clasii Severity Lever	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Cambration Factor, q	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.071	0.24	1.000	0.071	1.00	1.00	0.071		
Fatal and Injury (FI)			0.431	0.031	1.00	1.00	0.031		
Property Damage Only (PDO)			0.569	0.040	1.00	1.00	0.040		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.071	1.000	0.031	1.000	0.040
		(2)x(3)total		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.001	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.000	0.006	0.000	0.004	0.000
Ran off road	0.122	0.009	0.094	0.003	0.144	0.006
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.000
Total single-vehicle crashes	0.147	0.010	0.112	0.003	0.174	0.007
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.031	0.532	0.016	0.354	0.014
Head-on collision	0.040	0.003	0.060	0.002	0.025	0.001
Rear-end collision	0.242	0.017	0.210	0.006	0.266	0.011
Sideswipe collision	0.101	0.007	0.044	0.001	0.144	0.006
Other multiple-vehicle collision	0.039	0.003	0.042	0.001	0.037	0.001
Total multiple-vehicle crashes	0.853	0.061	0.888	0.027	0.826	0.033

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Constitution laws	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.1						
Fatal and Injury (FI)	0.431	0.0						
Property Damage Only (PDO)	0.569	0.0						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr		Fata	l and Injury Crash	es/yr	Property Damage Only Crashes/yr			
		(KABCO)			(KABC)		(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.0	0.0	0.0	0.0	0.1	0.0
Special Note: When the proje	pecial Note: When the project element is not included in the analysis the results will all be zeros. In addition if only the analysis only includes determining the predicted average crash frequency (i.e. EB analysis is not carried out), the results will show zero values where EB								

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information					Location Information				
Analyst	James Vorosmarti				Roadway	Roadway 0			
Agency or Company	WSDOT				Intersection			S. Plymouth Road at	Locust Grove Road
Date Performed	07/17/23				Jurisdiction			Benton County	
Intersection	Intersection 10				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized	ized							
Input Data					Site Conditions		Base Conditions		
Intersection type (3ST, 4ST, 4SG)					3ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)		-	1,610		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)			150		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				0		0			
Number of signalized or uncontrolled	Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0	

Not Present

1.00

Not Present

1.00

# Average Annual Crash History (3 or 5-yr average)

Intersection lighting (present/not present)

Calibration Factor, C<sub>i</sub>

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.2

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crasii Severity Lever	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.208	0.54	1.000	0.208	1.00	1.00	0.208		
Fatal and Injury (FI)			0.415	0.086	1.00	1.00	0.086		
Property Damage Only (PDO)			0.585	0.122	1.00	1.00	0.122		

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.208	1.000	0.086	1.000	0.122
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHIC	.E		
Collision with animal	0.019	0.004	0.008	0.001	0.026	0.003
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.003	0.022	0.002	0.007	0.001
Ran off road	0.244	0.051	0.240	0.021	0.247	0.030
Other single-vehicle collision	0.016	0.003	0.011	0.001	0.020	0.002
Total single-vehicle crashes	0.294	0.061	0.283	0.024	0.302	0.037
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.049	0.275	0.024	0.210	0.026
Head-on collision	0.052	0.011	0.081	0.007	0.032	0.004
Rear-end collision	0.278	0.058	0.260	0.022	0.292	0.035
Sideswipe collision	0.097	0.020	0.051	0.004	0.131	0.016
Other multiple-vehicle collision	0.042	0.009	0.050	0.004	0.033	0.004
Total multiple-vehicle crashes	0.706	0.147	0.717	0.062	0.698	0.085

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.2						
Fatal and Injury (FI)	0.415	0.1						
Property Damage Only (PDO)	0.585	0.1						

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>				
	Total Crashes/yr			Fata	Fatal and Injury Crashes/yr			Property Damage Only Crashes/yr		
	(KABCO)				(KABC)		(PDO)			
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.2	0.2	0.0	0.1	0.1	0.0	0.1	0.1	0.0	
Special Note: When the proje	ect element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the p	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Works	heet 2A Ger	neral Information and	Input Data for Rural T	wo-Lane Two-	Way Roadway Inter	sections	
General Information					Location Information	on			
Analyst	James Vorosmarti				Roadway			0	
Agency or Company	WSDOT				Intersection			Route 221 at Route	14
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	Intersection 11				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized								
Input Data					Site Conditions			Base Conditions	
Intersection type (3ST, 4ST, 4SG)					4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)			3,410		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)			1,480		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	()	0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)					0			0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)					1			0	
Intersection lighting (present/not present)				Present			Not Present		
Calibration Factor, C <sub>i</sub>					1.00			1.00	
Average Annual Crash History (3 or 5	-yr average)								

NOTES: * AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Intersection crashes

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections						
(1)	(2)	(3)	(4)	(5)		
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF		
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>		
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)		
1.00	1.00	0.86	0.91	0.78		

KABC

PDO

Fatal and Injury Only

Property Damage Only

0.4

		Worksheet 2C I	ntersection Crashe	s for Rural Two-Lane Two-Way R	oadway Intersection	ns		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int	
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B			
Total	2.167	0.24	1.000	2.167	0.78	1.00	1.691	
Fatal and Injury (FI)			0.431	0.934	0.78	1.00	0.729	
Property Damage Only (PDO)			0.569	1.233	0.78	1.00	0.962	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.691	1.000	0.729	1.000	0.962
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.017	0.006	0.004	0.014	0.013
Collision with bicycle	0.001	0.002	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.002	0.001	0.001	0.001	0.001
Overturned	0.005	0.008	0.006	0.004	0.004	0.004
Ran off road	0.122	0.206	0.094	0.069	0.144	0.139
Other single-vehicle collision	0.008	0.014	0.004	0.003	0.010	0.010
Total single-vehicle crashes	0.147	0.249	0.112	0.082	0.174	0.167
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.729	0.532	0.388	0.354	0.341
Head-on collision	0.040	0.068	0.060	0.044	0.025	0.024
Rear-end collision	0.242	0.409	0.210	0.153	0.266	0.256
Sideswipe collision	0.101	0.171	0.044	0.032	0.144	0.139
Other multiple-vehicle collision	0.039	0.066	0.042	0.031	0.037	0.036
Total multiple-vehicle crashes	0.853	1.443	0.888	0.647	0.826	0.795

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections						
(1)	(2)	(3)				
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)				
Crasii severity ievei	(4) from Worksheet 2C	(8) from Worksheet 2C				
Total	1.000	1.7				
Fatal and Injury (FI)	0.431	0.7				
Property Damage Only (PDO)	0.569	1.0				

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fata	l and Injury Crash	es/yr	Property Damage Only Crashes/yr		
				(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	1.7	1.9	0.2	0.7	0.8	0.1	1.0	1.1	0.1
Special Note: When the proje	ect element is not included	in the analysis the results wi	ll all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	ot carried out), the results will	show zero values where EB

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

rksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections	3
Location Information	

General Information		Location Information			
Analyst	James Vorosmarti	Roadway	0		
Agency or Company	WSDOT	Intersection	Route 14 at S. Plymo	outh Rd	
Date Performed	07/17/23	Jurisdiction	WSDOT		
Intersection	Intersection 12	Analysis Year	2023		
Signalized/Unsignalized	Unsignalized		_		

input Data		3	ite Conditions	Base Conditions	
Intersection type (3ST, 4ST, 4SG)			4ST		
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 14,700	(veh/day)		6,390	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 3,500	(veh/day)		1,170	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-	turn lane (0, 1, 2, 3, 4)			2	0
Number of signalized or uncontrolled approaches with a right			0	0	
Intersection lighting (present/not present)			Present	Not Present	
Calibration Factor, C <sub>i</sub>			1.00		1.00

Average Annual Crash History (3 or 5-yr average)
<u>-                                    </u>

Intersection crashes	KABC	Fatal and Injury Only	1.2
intersection crashes	PDO	Property Damage Only	0.8

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	0.52	1.00	0.91	0.47				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(1) (2) (3) (4) (5) (6) (7)								
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Seventy Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B				
Total	2.737	0.24	1.000	2.737	0.47	1.00	1.291		
Fatal and Injury (FI)	Injury (FI)		0.431	1.180	0.47	1.00	0.557		
Property Damage Only (PDO)			0.569	1.558	0.47	1.00	0.735		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.291	1.000	0.557	1.000	0.735
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.013	0.006	0.003	0.014	0.010
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.006	0.006	0.003	0.004	0.003
Ran off road	0.122	0.158	0.094	0.052	0.144	0.106
Other single-vehicle collision	0.008	0.010	0.004	0.002	0.010	0.007
Total single-vehicle crashes	0.147	0.190	0.112	0.062	0.174	0.128
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.557	0.532	0.296	0.354	0.260
Head-on collision	0.040	0.052	0.060	0.033	0.025	0.018
Rear-end collision	0.242	0.313	0.210	0.117	0.266	0.195
Sideswipe collision	0.101	0.130	0.044	0.024	0.144	0.106
Other multiple-vehicle collision	0.039	0.050	0.042	0.023	0.037	0.027
Total multiple-vehicle crashes	0.853	1.102	0.888	0.494	0.826	0.607

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.3						
Fatal and Injury (FI)	0.431	0.6						
Property Damage Only (PDO)	0.569	0.7						

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ashes/yr	
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.3	1.7	0.4	0.6	0.7	0.2	0.7	1.0	0.2	
Special Note: When the proje	ect element is not included	in the analysis the results wi	l all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will :	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Worksheet 2A Gen	eral Information and	Input Data for Rural Two-Lane Two-Way Roadway Inte	ersections	
General Information				Location Information		
Analyst	James Vorosmarti			Roadway	0	
Agency or Company	WSDOT			Intersection	Route 14 at I-82 SB	Ramps
Date Performed	07/17/23			Jurisdiction	WSDOT	
Intersection	Intersection 13			Analysis Year	2023	
Signalized/Unsignalized	Signalized/Unsignalized Unsignalized					
Input Data			Site Conditions		Base Conditions	
Intersection type (3ST, 4ST, 4SG)				4ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> = 14,700	(veh/day)	5,850		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> = 3,500	(veh/day)	580		
Intersection skew angle (degrees)	[If 4ST, does skew di	ffer for minor legs? Else, No.]	No	Skew for Leg 1 (AII): Skew for Leg (4ST only	()	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			0		0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0		0	
Intersection lighting (present/not present)			Not Present		Not Present	
Calibration Factor, C <sub>i</sub>				1.00		1.00

Intersection crashes PDO Property Damage Only

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Average Annual Crash History (3 or 5-yr average)

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	1.00	1.00	1.00	1.00					

KABC

Fatal and Injury Only

0.0

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N			
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int			
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)			
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B					
Total	1.692	0.24	1.000	1.692	1.00	1.00	1.692			
Fatal and Injury (FI)	/ (FI)		0.431	0.729	1.00	1.00	0.729			
Property Damage Only (PDO)			0.569	0.963	1.00	1.00	0.963			

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)	
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C	
Total	1.000	1.692	1.000	0.729	1.000	0.963	
		(2)x(3)TOTAL		(4)x(5)₅ı		(6)x(7)pdo	
			SINGLE-VEHICL	E			
Collision with animal	0.010	0.017	0.006	0.004	0.014	0.013	
Collision with bicycle	0.001	0.002	0.001	0.001	0.001	0.001	
Collision with pedestrian	0.001	0.002	0.001	0.001	0.001	0.001	
Overturned	0.005	0.008	0.006	0.004	0.004	0.004	
Ran off road	0.122	0.206	0.094	0.069	0.144	0.139	
Other single-vehicle collision	0.008	0.014	0.004	0.003	0.010	0.010	
Total single-vehicle crashes	0.147	0.249	0.112	0.082	0.174	0.168	
			MULTIPLE-VEHIO	CLE			
Angle collision	0.431	0.729	0.532	0.388	0.354	0.341	
Head-on collision	0.040	0.068	0.060	0.044	0.025	0.024	
Rear-end collision	0.242	0.409	0.210	0.153	0.266	0.256	
Sideswipe collision	0.101	0.171	0.044	0.032	0.144	0.139	
Other multiple-vehicle collision	0.039	0.066	0.042	0.031	0.037	0.036	
Total multiple-vehicle crashes	0.853	1.443	0.888	0.648	0.826	0.795	

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.7						
Fatal and Injury (FI)	0.431	0.7						
Property Damage Only (PDO)	0.569	1.0						

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	shes/yr	
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.7 1.0 0.0 0.7 0.4 0.0 1.0 0.5 0.0									
Special Note: When the proje	ect element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Intersections
	Location Information

General Information		Location Information	Location Information			
Analyst	James Vorosmarti	Roadway	0			
Agency or Company	WSDOT	Intersection	Route 14 at I-82 NB	Ramps		
Date Performed	07/17/23	Jurisdiction	WSDOT	WSDOT		
Intersection Intersection 14		Analysis Year	2023	2023		
Signalized/Unsignalized Unsignalized						
Input Data		Site Cond	ditions	Base Conditions		

put 2 utu			2000 00110110110			
Intersection type (3ST, 4ST, 4SG)	4ST					
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 14,700	(veh/day)	2,860			
AADT <sub>minor</sub> (veh/day)	$AADT_{MAX} = 3,500$	(veh/day)	1,800			
Intersection skew angle (degrees) [If 4ST, does skew d	iffer for minor legs? Else, No.]	No	Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled approaches with a left-	turn lane (0, 1, 2, 3, 4)			0		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0			0
Intersection lighting (present/not present)			Present			Not Present
Calibration Factor, C			1.00			1.00

Average Annual Crash History (3 or 5-yr average)	

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crasnes	PDO	Property Damage Only	0.0

Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections							
(1)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF			
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>			
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)			
1.00	1.00	1.00	0.91	0.91			

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
		Parameter, k	Distribution	Distribution		Calibration Factor, C <sub>i</sub>	predicted int	
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of		(5)*(5)*(7)	
		10.6.2	5		Worksheet 2B		(5)*(6)*(7)	
Total	2.198	0.24	1.000	2.198	0.91	1.00	1.994	
Fatal and Injury (FI)			0.431	0.947	0.91	1.00	0.859	
Property Damage Only (PDO)			0.569	1.251	0.91	1.00	1.135	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.994	1.000	0.859	1.000	1.135
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.020	0.006	0.005	0.014	0.016
Collision with bicycle	0.001	0.002	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.002	0.001	0.001	0.001	0.001
Overturned	0.005	0.010	0.006	0.005	0.004	0.005
Ran off road	0.122	0.243	0.094	0.081	0.144	0.163
Other single-vehicle collision	0.008	0.016	0.004	0.003	0.010	0.011
Total single-vehicle crashes	0.147	0.293	0.112	0.096	0.174	0.197
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.859	0.532	0.457	0.354	0.402
Head-on collision	0.040	0.080	0.060	0.052	0.025	0.028
Rear-end collision	0.242	0.483	0.210	0.180	0.266	0.302
Sideswipe collision	0.101	0.201	0.044	0.038	0.144	0.163
Other multiple-vehicle collision	0.039	0.078	0.042	0.036	0.037	0.042
Total multiple-vehicle crashes	0.853	1.701	0.888	0.763	0.826	0.937

Worksheet 2E — Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	, , , ,	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	2.0					
Fatal and Injury (FI)	0.431	0.9					
Property Damage Only (PDO)	0.569	1.1					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
		(KABCO)	ABCO) (KABC) (PDO)  ed average frequency   Potential for Improvement   Predicted average crash frequency   Potential for Improvement   Predicted average crash frequency   Pre						
Summary for the project element	Predicted average crash frequency	Expected average crash frequency		average crash	average crash		•		Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	2.0	0.6	0.0	0.9	0.3	0.0	1.1	0.3	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information		Location information	
Analyst	James Vorosmarti	Roadway	0
Agency or Company	WSDOT	Intersection	Coffin Road at I-82 SB Ramps
Date Performed	07/17/23	Jurisdiction	WSDOT
Intersection	Intersection 15	Analysis Year	2023
Signalized/Unsignalized	Unsignalized		

Signalized/Unsignalized U	Jnsignalized								
Input Data						Site C	Conditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)	section type (3ST, 4ST, 4SG)						4ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	400				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)			230		
Intersection skew angle (degrees)	If 4ST, does skew di	fer for minor legs? Els	e, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled appr	roaches with a left-t	urn lane (0, 1, 2, 3, 4)					0		0
Number of signalized or uncontrolled appr	roaches with a right-	turn lane (0, 1, 2, 3, 4)	)				0		0
Intersection lighting (present/not present)	)		•	•		Р	resent		Not Present
Calibration Factor, C <sub>i</sub>			•	•			1.00		1.00

Average Annual Crash History (3 or 5-yr average)			
Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	1.00	1.00	0.91	0.91					

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)					
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N					
Crash Severity Level	N <sub>spf 3ST,</sub> 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, G	predicted int					
Crash Seventy Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10- (2) <sub>TOTAL</sub> * (4)		from (5) of	Calibration Factor, C	/E\*/G\*/7\					
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(Z)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)					
Total	0.192	0.24	1.000	0.192	0.91	1.00	0.175					
Fatal and Injury (FI)			0.431	0.083	0.91	1.00	0.075					
Property Damage Only (PDO)			0.569	0.110	0.91	1.00	0.099					

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.175	1.000	0.075	1.000	0.099
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.002	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.001	0.006	0.000	0.004	0.000
Ran off road	0.122	0.021	0.094	0.007	0.144	0.014
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.001
Total single-vehicle crashes	0.147	0.026	0.112	0.008	0.174	0.017
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.075	0.532	0.040	0.354	0.035
Head-on collision	0.040	0.007	0.060	0.005	0.025	0.002
Rear-end collision	0.242	0.042	0.210	0.016	0.266	0.026
Sideswipe collision	0.101	0.018	0.044	0.003	0.144	0.014
Other multiple-vehicle collision	0.039	0.007	0.042	0.003	0.037	0.004
Total multiple-vehicle crashes	0.853	0.149	0.888	0.067	0.826	0.082

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	, , , , , , , , , , , , , , , , , , , ,	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.2					
Fatal and Injury (FI)	0.431	0.1					
Property Damage Only (PDO)	0.569	0.1					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.2	0.1	0.0	0.1	0.1	0.0	0.1	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		heet 2A Gen	eral Information and	Input Data for Rural T	wo-Lane Two-W	ay Roadway Inters	sections		
General Information					Location Information	on			
Analyst	James Vorosmarti				Roadway			0	
Agency or Company	WSDOT				Intersection			Coffin Road at I-82 N	IB Ramps
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	Intersection 16			Analysis Year			2023		
Signalized/Unsignalized	Signalized/Unsignalized Unsignalized								
Input Data			Site Conditions			Base Conditions			
Intersection type (3ST, 4ST, 4SG)	_				4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	450				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	120				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				0			0		
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not p	resent)			•		Pi	esent		Not Present

1.00

1.00

0.0

KABC Fatal and Injury Only Intersection crashes PDO Property Damage Only 0.0

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Calibration Factor, C<sub>i</sub>

Average Annual Crash History (3 or 5-yr average)

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	0.91	0.91						

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Cambration Factor, C	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)				
Total	0.139	0.24	1.000	0.139	0.91	1.00	0.126				
Fatal and Injury (FI)			0.431	0.060	0.91	1.00	0.054				
Property Damage Only (PDO)			0.569	0.079	0.91	1.00	0.072				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.126	1.000	0.054	1.000	0.072
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.001	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.001	0.006	0.000	0.004	0.000
Ran off road	0.122	0.015	0.094	0.005	0.144	0.010
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.001
Total single-vehicle crashes	0.147	0.019	0.112	0.006	0.174	0.012
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.054	0.532	0.029	0.354	0.025
Head-on collision	0.040	0.005	0.060	0.003	0.025	0.002
Rear-end collision	0.242	0.030	0.210	0.011	0.266	0.019
Sideswipe collision	0.101	0.013	0.044	0.002	0.144	0.010
Other multiple-vehicle collision	0.039	0.005	0.042	0.002	0.037	0.003
Total multiple-vehicle crashes	0.853	0.107	0.888	0.048	0.826	0.059

	Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1) (2) (3)										
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)								
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C								
Total	1.000	0.1								
Fatal and Injury (FI)	0.431	0.1								
Property Damage Only (PDO)	0.569	0.1								

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
		(KABCO)			(KABC)			(PDO)	
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	ted average Expected average	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Inter	sections
	Location Information	

Analyst	James Vorosmarti	Roadway	0
Agency or Company	WSDOT	Intersection	Coffin Road at Bofer Canyon Road
Date Performed	07/17/23	Jurisdiction	Benton County
Intersection	Intersection 17	Analysis Year	2023

Signalized/Unsignalized Unsignalized

Signalized/Unsignalized Unsignalized							
Input Data				Site Conditions			Base Conditions
Intersection type (3ST, 4ST, 4SG)				4ST	-		
AADT <sub>major</sub> (veh/day) AADT <sub>MAX</sub> = 14,700 (veh/day)				390	)		
AADT <sub>minor</sub> (veh/day)	$AADT_{MAX} = 3,500$	(veh/day)	130				
Intersection skew angle (degrees) [If 4ST, does skew	differ for minor legs? Else, No.]	No	Skew for Leg 1 (All):	0 S	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled approaches with a left	-turn lane (0, 1, 2, 3, 4)		0			0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0			0	
Intersection lighting (present/not present)			Not Present			Not Present	
Calibration Factor, C <sub>i</sub>				1.00	)		1.00

Average Annual Crash History (3 or 5-yr average)

**General Information** 

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crasiles	PDO	Property Damage Only	0.0

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, G	predicted int		
Crasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, C	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.134	0.24	1.000	0.134	1.00	1.00	0.134		
Fatal and Injury (FI)			0.431	0.058	1.00	1.00	0.058		
Property Damage Only (PDO)			0.569	0.076	1.00	1.00	0.076		

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.134	1.000	0.058	1.000	0.076
		(2)x(3)total		(4)x(5) <sub>FI</sub>		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.001	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.001	0.006	0.000	0.004	0.000
Ran off road	0.122	0.016	0.094	0.005	0.144	0.011
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.001
Total single-vehicle crashes	0.147	0.020	0.112	0.006	0.174	0.013
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.058	0.532	0.031	0.354	0.027
Head-on collision	0.040	0.005	0.060	0.003	0.025	0.002
Rear-end collision	0.242	0.032	0.210	0.012	0.266	0.020
Sideswipe collision	0.101	0.014	0.044	0.003	0.144	0.011
Other multiple-vehicle collision	0.039	0.005	0.042	0.002	0.037	0.003
Total multiple-vehicle crashes	0.853	0.114	0.888	0.051	0.826	0.063

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.1					
Fatal and Injury (FI)	0.431	0.1					
Property Damage Only (PDO)	0.569	0.1					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr (KABCO)		Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections								
	Location Information							
	Roadway	0						
	Intersection	Bofer Canyon Road at Beck Road						
	Jurisdiction	Benton County						
	Analysis Year	2023						

Intersection	Intersection 18	Intersection 18 A			Analysis Year	2023	
Signalized/Unsignalized	Unsignalized						
Input Data					Site Conditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)					4ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	110		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	40		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No				No	Skew for Leg 1 (All): 0 Skew for (4ST	0	0
Number of signalized or uncontrolle	d approaches with a left-	turn lane (0, 1, 2, 3, 4)			0		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0		0		
Intersection lighting (present/not present)					Not Present		Not Present
Calibration Factor, C <sub>i</sub>					1.00		1.00
A	F						

James Vorosmarti

WSDOT

07/17/23

**General Information** 

Agency or Company

Date Performed

Analyst

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.0

	Worksheet 2B Cras	h Modification Factors for Rural Two-Lane Two-Way	Roadway Intersections	
(1)	(2)	(3)	(4)	(5)
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)
1.00	1.00	1.00	1.00	1.00

		Worksheet 2C I	ntersection Crashe	s for Rural Two-Lane Two-Way R	oadway Intersection	ons		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution		Calibration Factor, C₁	predicted int	
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration ractor, q	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5) (6) (7)	
Total	0.031	0.24	1.000	0.031	1.00	1.00	0.031	
Fatal and Injury (FI)			0.431	0.013	1.00	1.00	0.013	
Property Damage Only (PDO)			0.569	0.017	1.00	1.00	0.017	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.031	1.000	0.013	1.000	0.017
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.000	0.006	0.000	0.014	0.000
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.000	0.006	0.000	0.004	0.000
Ran off road	0.122	0.004	0.094	0.001	0.144	0.003
Other single-vehicle collision	0.008	0.000	0.004	0.000	0.010	0.000
Total single-vehicle crashes	0.147	0.004	0.112	0.001	0.174	0.003
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.013	0.532	0.007	0.354	0.006
Head-on collision	0.040	0.001	0.060	0.001	0.025	0.000
Rear-end collision	0.242	0.007	0.210	0.003	0.266	0.005
Sideswipe collision	0.101	0.003	0.044	0.001	0.144	0.003
Other multiple-vehicle collision	0.039	0.001	0.042	0.001	0.037	0.001
Total multiple-vehicle crashes	0.853	0.026	0.888	0.012	0.826	0.014

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.0					
Fatal and Injury (FI)	0.431	0.0					
Property Damage Only (PDO)	0.569	0.0					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information		Location Information				
Analyst	James Vorosmarti	Roadway	0			
Agency or Company	WSDOT	Intersection	Locust Grove Road at I-82 SB Ramps			
Date Performed	07/17/23	Jurisdiction	WSDOT			
Intersection	Intersection 19	Analysis Year	2023			
Signalized/Unsignalized	Unsignalized					

Input Data			Site Conditions	Base Conditions
Intersection type (3ST, 4ST, 4SG)			4ST	
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 14,700	(veh/day)	2,150	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 3,500	(veh/day)	1,120	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 0 Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			0	0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0	0
Intersection lighting (present/not present)			Present	Not Present
Calibration Factor, C <sub>i</sub>			1.00	1.00

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crasnes	PDO	Property Damage Only	0.0

Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF 2i	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	0.91	0.91				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5) (6) (7)		(7)	(8)			
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N			
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution		Calibration Factor, C₁	predicted int			
Crasii Severity Lever	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration ractor, q	(5)*(6)*(7)			
		10.6.2	5	(2)TOTAL (4)	Worksheet 2B					
Total	1.387	0.24	1.000	1.387	0.91	1.00	1.258			
Fatal and Injury (FI)			0.431	0.598	0.91	1.00	0.542			
Property Damage Only (PDO)			0.569	0.789	0.91	1.00	0.716			

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.258	1.000	0.542	1.000	0.716
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.013	0.006	0.003	0.014	0.010
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.006	0.006	0.003	0.004	0.003
Ran off road	0.122	0.153	0.094	0.051	0.144	0.103
Other single-vehicle collision	0.008	0.010	0.004	0.002	0.010	0.007
Total single-vehicle crashes	0.147	0.185	0.112	0.061	0.174	0.125
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.542	0.532	0.288	0.354	0.253
Head-on collision	0.040	0.050	0.060	0.033	0.025	0.018
Rear-end collision	0.242	0.304	0.210	0.114	0.266	0.190
Sideswipe collision	0.101	0.127	0.044	0.024	0.144	0.103
Other multiple-vehicle collision	0.039	0.049	0.042	0.023	0.037	0.026
Total multiple-vehicle crashes	0.853	1.073	0.888	0.481	0.826	0.591

Worksheet 2E — Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crasii severity ievei	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.3						
Fatal and Injury (FI)	0.431	0.5						
Property Damage Only (PDO)	0.569	0.7						

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>				
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	Property Damage Only Crashes/yr		
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.3	0.5	0.0	0.5	0.2	0.0	0.7	0.3	0.0	
Special Note: When the proje	ect element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

W	neet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections

General Information				Location Information					
Analyst	James Vorosmarti	lames Vorosmarti			Roadway 0		0		
Agency or Company	WSDOT				Intersection			Locust Grove Road a	t I-82 NB Ramps
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	Intersection 20				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data						Site (	Conditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)				4ST					
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)			3,850		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	1,320				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No		Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0			
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				0			0		
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not present)				Present			Not Present		
Calibration Factor, C <sub>i</sub>					1.00				1.00

Intersection crashes	KABC	Fatal and Injury Only	0.2
intersection crashes	PDO	Property Damage Only	0.4

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	1.00	1.00	0.91	0.91					

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, G	predicted int		
Crash Seventy Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, C	(5)*(5)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	2.174	0.24	1.000	2.174	0.91	1.00	1.972		
Fatal and Injury (FI)			0.431	0.937	0.91	1.00	0.850		
Property Damage Only (PDO)			0.569	1.237	0.91	1.00	1.122		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.972	1.000	0.850	1.000	1.122
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)PDO
			SINGLE-VEHIC	.E		
Collision with animal	0.010	0.020	0.006	0.005	0.014	0.016
Collision with bicycle	0.001	0.002	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.002	0.001	0.001	0.001	0.001
Overturned	0.005	0.010	0.006	0.005	0.004	0.004
Ran off road	0.122	0.241	0.094	0.080	0.144	0.162
Other single-vehicle collision	0.008	0.016	0.004	0.003	0.010	0.011
Total single-vehicle crashes	0.147	0.290	0.112	0.095	0.174	0.195
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.850	0.532	0.452	0.354	0.397
Head-on collision	0.040	0.079	0.060	0.051	0.025	0.028
Rear-end collision	0.242	0.477	0.210	0.179	0.266	0.299
Sideswipe collision	0.101	0.199	0.044	0.037	0.144	0.162
Other multiple-vehicle collision	0.039	0.077	0.042	0.036	0.037	0.042
Total multiple-vehicle crashes	0.853	1.683	0.888	0.755	0.826	0.927

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crack coverity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	2.0						
Fatal and Injury (FI)	0.431	0.9						
Property Damage Only (PDO)	0.569	1.1						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
		(KABCO)			(KABC)		(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	2.0	1.0	0.0	0.9	0.4	0.0	1.1	0.6	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information					Location Information		
Analyst	James Vorosmarti				Roadway	0	
Agency or Company	WSDOT				Intersection	Route 397/Locust G	rove Road at Bofer Canyon Rd
Date Performed	07/17/23				Jurisdiction	WSDOT	
Intersection	Intersection 21				Analysis Year	2023	
Signalized/Unsignalized	Unsignalized	Unsignalized					
Input Data			Site Conditions		Base Conditions		
Intersection type (3ST, 4ST, 4SG)					4ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	3,390		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	130		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 OSkew for L	- ()	0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)					0		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0		0		
Intersection lighting (present/not p	resent)				Not Present		Not Present
Calibration Factor, C <sub>i</sub>					1.00		1.00

Intersection crashes	KABC	Fatal and Injury Only	0.2
intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	1.00	1.00				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of		(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.490	0.24	1.000	0.490	1.00	1.00	0.490		
Fatal and Injury (FI)			0.431	0.211	1.00	1.00	0.211		
Property Damage Only (PDO)			0.569	0.279	1.00	1.00	0.279		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.490	1.000	0.211	1.000	0.279
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.005	0.006	0.001	0.014	0.004
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.002	0.006	0.001	0.004	0.001
Ran off road	0.122	0.060	0.094	0.020	0.144	0.040
Other single-vehicle collision	0.008	0.004	0.004	0.001	0.010	0.003
Total single-vehicle crashes	0.147	0.072	0.112	0.024	0.174	0.048
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.211	0.532	0.112	0.354	0.099
Head-on collision	0.040	0.020	0.060	0.013	0.025	0.007
Rear-end collision	0.242	0.119	0.210	0.044	0.266	0.074
Sideswipe collision	0.101	0.049	0.044	0.009	0.144	0.040
Other multiple-vehicle collision	0.039	0.019	0.042	0.009	0.037	0.010
Total multiple-vehicle crashes	0.853	0.418	0.888	0.187	0.826	0.230

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash savarity laval	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.5						
Fatal and Injury (FI)	0.431	0.2						
Property Damage Only (PDO)	0.569	0.3						

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr Fatal (KABCO)		l and Injury Crash (KABC)	es/yr	Property Damage Only Crashes/yr (PDO)				
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.5	0.4	0.0	0.2	0.2	0.0	0.3	0.2	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Land to the form of the

General Information			Location Information						
Analyst	James Vorosmarti				Roadway 0				
Agency or Company	WSDOT				Intersection			Route 397 at S. Olym	npia Street
Date Performed	07/17/23				Jurisdiction			WSDOT	
Intersection	Intersection 22	Intersection 22			Analysis Year			2023	
Signalized/Unsignalized	Unsignalized								
Input Data				Site Conditions			Base Conditions		
Intersection type (3ST, 4ST, 4SG)				3ST					
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	3,960				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)	910				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				1			0		
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not present)				Not Present Not Present				Not Present	

1.00

1.00

### Average Annual Crash History (3 or 5-yr average)

Calibration Factor, C<sub>i</sub>

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	0.56	1.00	1.00	0.56					

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
	N	Overdispersion	Crash Severity	N <sub>spf 3ST, 4ST or 4SG</sub> by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int				
Crasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	1.023	0.54	1.000	1.023	0.56	1.00	0.573				
Fatal and Injury (FI)			0.415	0.425	0.56	1.00	0.238				
Property Damage Only (PDO)			0.585	0.599	0.56	1.00	0.335				

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.573	1.000	0.238	1.000	0.335
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.019	0.011	0.008	0.002	0.026	0.009
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.000
Overturned	0.013	0.007	0.022	0.005	0.007	0.002
Ran off road	0.244	0.140	0.240	0.057	0.247	0.083
Other single-vehicle collision	0.016	0.009	0.011	0.003	0.020	0.007
Total single-vehicle crashes	0.294	0.168	0.283	0.067	0.302	0.101
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.136	0.275	0.065	0.210	0.070
Head-on collision	0.052	0.030	0.081	0.019	0.032	0.011
Rear-end collision	0.278	0.159	0.260	0.062	0.292	0.098
Sideswipe collision	0.097	0.056	0.051	0.012	0.131	0.044
Other multiple-vehicle collision	0.042	0.024	0.050	0.012	0.033	0.011
Total multiple-vehicle crashes	0.706	0.405	0.717	0.170	0.698	0.234

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(2)	(3)							
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.6							
Fatal and Injury (FI)	0.415	0.2							
Property Damage Only (PDO)	0.585	0.3							

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
		(KABCO)	BCO) (KABC)				(PDO)		
Summary for the project element	Predicted average crash frequency	erage crash Expected average crash frequency				Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.6	0.2	0.0	0.2	0.1	0.0	0.3	0.1	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
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General Information			Location Information				
Analyst	James Vorosmarti	James Vorosmarti			-		
Agency or Company	WSDOT			Intersection		Route 397 at S. Nine	Canyon Road
Date Performed	07/17/23			Jurisdiction		WSDOT	
Intersection	Intersection 23			Analysis Year		2023	
Signalized/Unsignalized	Unsignalized						
Input Data				Sit	e Conditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)				4ST			
AADT <sub>major</sub> (veh/day)	veh/day) AADT <sub>MAX</sub> = 14,700 (veh/day)			1,860			
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> = 3,500	(veh/day)		540		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All): Skew for Leg 2 (4ST only):		0		
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				2			0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0
Intersection lighting (present/not present)				Not Present			Not Present
Calibration Factor, C <sub>i</sub>				1.00			1.00

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.4

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	0.52	1.00	1.00	0.52				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int		
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(E\*(G\*(Z\		
		10.6.2	5		Worksheet 2B		(5)*(6)*(7)		
Total	0.815	0.24	1.000	0.815	0.52	1.00	0.424		
Fatal and Injury (FI)			0.431	0.351	0.52	1.00	0.183		
Property Damage Only (PDO)			0.569	0.463	0.52	1.00	0.241		

	Worksheet 2D Crashes by Severity Level and Collision Type for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)			
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)			
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C			
Total	1.000	0.424	1.000	0.183	1.000	0.241			
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)PDO			
			SINGLE-VEHICL	E					
Collision with animal	0.010	0.004	0.006	0.001	0.014	0.003			
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000			
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000			
Overturned	0.005	0.002	0.006	0.001	0.004	0.001			
Ran off road	0.122	0.052	0.094	0.017	0.144	0.035			
Other single-vehicle collision	0.008	0.003	0.004	0.001	0.010	0.002			
Total single-vehicle crashes	0.147	0.062	0.112	0.020	0.174	0.042			
			MULTIPLE-VEHIO	CLE					
Angle collision	0.431	0.183	0.532	0.097	0.354	0.085			
Head-on collision	0.040	0.017	0.060	0.011	0.025	0.006			
Rear-end collision	0.242	0.103	0.210	0.038	0.266	0.064			
Sideswipe collision	0.101	0.043	0.044	0.008	0.144	0.035			
Other multiple-vehicle collision	0.039	0.017	0.042	0.008	0.037	0.009			
Total multiple-vehicle crashes	0.853	0.361	0.888	0.162	0.826	0.199			

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1) (2) (3)								
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.4						
Fatal and Injury (FI)	0.431	0.2						
Property Damage Only (PDO)	0.569	0.2						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
Total Crashes/yr			Fata	l and Injury Crash	es/yr	Property Damage Only Crashes/yr				
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	average crasii		Potential for Improvement			Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.4	0.4	0.0	0.2	0.2	0.0	0.2	0.2	0.0	
Special Note: When the proje	ct element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the p	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Intersections
	Location Information

General Information		Location Information			
Analyst	James Vorosmarti	Roadway	0		
Agency or Company	WSDOT	Intersection	Route 397 at S. Finley Road		
Date Performed	07/17/23	Jurisdiction	WSDOT		
Intersection	Intersection 24	Analysis Year	2023		
Signalized/Unsignalized	Unsignalized				
Input Data		Site Conditions		Base Conditions	

Intersection type (3ST, 4ST, 4SG)			3ST		
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 19,500	(veh/day)		1,100	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 4,300	(veh/day)		240	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (AII):	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			1		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0		0
Intersection lighting (present/not present)			Not Present		Not Present
Calibration Factor, C <sub>i</sub>			1.00		1.00

<b>Average Annual</b>	Crash	History	(3 or	5-yr	average)
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Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.2

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	0.56	1.00	1.00	0.56				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N <sub>spf 3ST, 4ST or 4SG</sub> by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of		(5)*(6)*(7)		
		10.6.2	5		Worksheet 2B		(5) (6) (7)		
Total	0.194	0.54	1.000	0.194	0.56	1.00	0.108		
Fatal and Injury (FI)			0.415	0.080	0.56	1.00	0.045		
Property Damage Only (PDO)			0.585	0.113	0.56	1.00	0.063		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.108	1.000	0.045	1.000	0.063
		(2)x(3)total		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICE	E		
Collision with animal	0.019	0.002	0.008	0.000	0.026	0.002
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.001	0.022	0.001	0.007	0.000
Ran off road	0.244	0.026	0.240	0.011	0.247	0.016
Other single-vehicle collision	0.016	0.002	0.011	0.000	0.020	0.001
Total single-vehicle crashes	0.294	0.032	0.283	0.013	0.302	0.019
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.026	0.275	0.012	0.210	0.013
Head-on collision	0.052	0.006	0.081	0.004	0.032	0.002
Rear-end collision	0.278	0.030	0.260	0.012	0.292	0.019
ideswipe collision	0.097	0.011	0.051	0.002	0.131	0.008
Other multiple-vehicle collision	0.042	0.005	0.050	0.002	0.033	0.002
Total multiple-vehicle crashes	0.706	0.077	0.717	0.032	0.698	0.044

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1) (2) (3)								
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.1						
Fatal and Injury (FI)	0.415	0.0						
Property Damage Only (PDO)	0.585	0.1						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
	(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.1	0.1	0.0	0.0	0.1	0.0	0.1	0.1	0.0
Special Note: When the proje	ect element is not included	in the analysis the results wi	ll all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will :	show zero values where EB

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections								
	Location Information							
	Roadway	0						
	Intersection	Nine Canyon Road at Kirk Road						
	Jurisdiction	Benton County						
	Analysis Year	2023						

Input Data		Site Conditions		Base Conditions	
Intersection type (3ST, 4ST, 4SG)			3ST		
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 19,500	(veh/day)		240	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 4,300	(veh/day)		20	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No			Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-	urn lane (0, 1, 2, 3, 4)		0		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			0		0
Intersection lighting (present/not present)			Not Present		Not Present
Calibration Factor, C <sub>i</sub>				1.00	1.00

James Vorosmarti WSDOT

07/17/23

Intersection 25 Unsignalized

**General Information** 

Agency or Company

Date Performed

Signalized/Unsignalized

Analyst

Intersection

Intersection graphes	KABC	Fatal and Injury Only	0.0
Intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	1.00	1.00				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution		Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Cambration Factor, C	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.017	0.54	1.000	0.017	1.00	1.00	0.017		
Fatal and Injury (FI)			0.415	0.007	1.00	1.00	0.007		
Property Damage Only (PDO)			0.585	0.010	1.00	1.00	0.010		

		Worksheet 2D Crashes b	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.017	1.000	0.007	1.000	0.010
		(2)x(3)total		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	.E		
Collision with animal	0.019	0.000	0.008	0.000	0.026	0.000
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.000	0.022	0.000	0.007	0.000
Ran off road	0.244	0.004	0.240	0.002	0.247	0.002
Other single-vehicle collision	0.016	0.000	0.011	0.000	0.020	0.000
Total single-vehicle crashes	0.294	0.005	0.283	0.002	0.302	0.003
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.004	0.275	0.002	0.210	0.002
Head-on collision	0.052	0.001	0.081	0.001	0.032	0.000
Rear-end collision	0.278	0.005	0.260	0.002	0.292	0.003
Sideswipe collision	0.097	0.002	0.051	0.000	0.131	0.001
Other multiple-vehicle collision	0.042	0.001	0.050	0.000	0.033	0.000
Total multiple-vehicle crashes	0.706	0.012	0.717	0.005	0.698	0.007

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.0						
Fatal and Injury (FI)	0.415	0.0						
Property Damage Only (PDO)	0.585	0.0						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

special route. When the project element is not included in the analysis the results will all de zeros. In addition results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections								
	Location Information							
	Roadway	0						
	Intersection	Nine Canyon Road at Beck Road						
	Jurisdiction Benton County							
	Analysis Year	2023						
_	Site Conditions Base Condit							
·								

Intersection type (3ST, 4ST, 4SG)				3ST	
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 19,500	(veh/day)		260	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> = 4,300	(veh/day)		1	
Intersection skew angle (degrees) [If 4ST, does skew di	No	Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0	
Number of signalized or uncontrolled approaches with a left-			0	0	
Number of signalized or uncontrolled approaches with a right			0	0	
Intersection lighting (present/not present)		No	t Present	Not Present	
Calibration Factor, C <sub>i</sub>				1.00	1.00

James Vorosmarti

WSDOT

07/17/23

Intersection 26

Unsignalized

**General Information** 

Agency or Company

Signalized/Unsignalized

Date Performed

Intersection

Input Data

Analyst

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)				
Total	0.004	0.54	1.000	0.004	1.00	1.00	0.004				
Fatal and Injury (FI)			0.415	0.002	1.00	1.00	0.002				
Property Damage Only (PDO)			0.585	0.002	1.00	1.00	0.002				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.004	1.000	0.002	1.000	0.002
		(2)x(3)total		(4)x(5) <sub>FI</sub>		(6)x(7)pdo
			SINGLE-VEHICL	.E		
Collision with animal	0.019	0.000	0.008	0.000	0.026	0.000
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.000	0.022	0.000	0.007	0.000
Ran off road	0.244	0.001	0.240	0.000	0.247	0.001
Other single-vehicle collision	0.016	0.000	0.011	0.000	0.020	0.000
Total single-vehicle crashes	0.294	0.001	0.283	0.000	0.302	0.001
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.001	0.275	0.000	0.210	0.001
Head-on collision	0.052	0.000	0.081	0.000	0.032	0.000
Rear-end collision	0.278	0.001	0.260	0.000	0.292	0.001
Sideswipe collision	0.097	0.000	0.051	0.000	0.131	0.000
Other multiple-vehicle collision	0.042	0.000	0.050	0.000	0.033	0.000
Total multiple-vehicle crashes	0.706	0.003	0.717	0.001	0.698	0.002

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(2)	(3)							
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.0							
Fatal and Injury (FI)	0.415	0.0							
Property Damage Only (PDO)	0.585	0.0							

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information Lo				Location Information						
Analyst	James Vorosmarti				Roadway	Roadway 0		0		
Agency or Company	WSDOT	WSDOT			Intersection		Nine Canyon Road at	t Coffin Road		
Date Performed	07/17/23	07/17/23 Jr			Jurisdiction Benton County					
Intersection	Intersection 27	Intersection 27			Analysis Year 2023		2023			
Signalized/Unsignalized	Unsignalized									
Input Data					S	Site Conditions		Base Conditions		
Intersection type (3ST, 4ST, 4SG)	_					4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	410			-		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)		100				
•	•	-			Clean feet lee 1	Chau fan Laa 3		<u>.                                      </u>		

Skew for Leg 1 Skew for Leg 2 Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.] No 0 (AII): (4ST only): Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4) 0 0 Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4) 0 0 Intersection lighting (present/not present) Not Present Not Present Calibration Factor, C 1.00 1.00

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	0.0
intersection crashes	PDO	Property Damage Only	0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)				
Total	0.118	0.24	1.000	0.118	1.00	1.00	0.118				
Fatal and Injury (FI)			0.431	0.051	1.00	1.00	0.051				
Property Damage Only (PDO)			0.569	0.067	1.00	1.00	0.067				

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.118	1.000	0.051	1.000	0.067
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO
			SINGLE-VEHIC	E		
Collision with animal	0.010	0.001	0.006	0.000	0.014	0.001
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.001	0.006	0.006 0.000		0.000
Ran off road	0.122	0.014	0.094	0.005	0.144	0.010
Other single-vehicle collision	0.008	0.001	0.004	0.000	0.010	0.001
Total single-vehicle crashes	0.147	0.017	0.112	0.006	0.174	0.012
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.051	0.532	0.027	0.354	0.024
Head-on collision	0.040	0.005	0.060	0.003	0.025	0.002
Rear-end collision	0.242	0.028	0.210	0.011	0.266	0.018
Sideswipe collision	0.101	0.012	0.044	0.002	0.144	0.010
Other multiple-vehicle collision	0.039	0.005	0.042	0.002	0.037	0.002
Total multiple-vehicle crashes	0.853	0.100	0.888	0.045	0.826	0.055

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(3)								
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.1							
Fatal and Injury (FI)	0.431	0.1							
Property Damage Only (PDO)	0.569	0.1							

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
	Total Crashes/yr (KABCO)			Fata	l and Injury Crash (KABC)	es/yr	Prope	Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency Potential fo		
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.1	0.1	0.0	0.1	0.0	0.0	0.1	0.1	0.0	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections							
	Location Information						
	Roadway	0					
Intersection Locust Grove at S. ClodFelter Road							

07/17/23 Date Performed

Date Performed	07/17/23				Jurisdiction Benton County			Benton County	
Intersection	Intersection 28	Intersection 28			Analysis Year 2023				
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data						Site C	onditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)							4ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	1,010				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	190			-	
Intersection skew angle (degrees)	[If 4ST, does skew di	ffer for minor legs? Els	e, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrol	led approaches with a left-t	urn lane (0, 1, 2, 3, 4)			0			0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not present)				Not Present			Not Present		
Calibration Factor, C <sub>i</sub>		•		•	1.00			1.00	

Average Annual Crash History (3 or 5-yr average)

James Vorosmarti WSDOT

**General Information** 

Agency or Company

Analyst

0.2 KABC Fatal and Injury Only Intersection crashes PDO Property Damage Only 0.0

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	1.00	1.00				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int		
Crasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B				
Total	0.299	0.24	1.000	0.299	1.00	1.00	0.299		
Fatal and Injury (FI)			0.431	0.129	1.00	1.00	0.129		
Property Damage Only (PDO)			0.569	0.170	1.00	1.00	0.170		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.299	1.000	0.129	1.000	0.170
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.003	0.006	0.001 0.014		0.002
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.005	0.001	0.006	0.001	0.004	0.001
Ran off road	0.122	0.036	0.094	0.012	0.144	0.024
Other single-vehicle collision	0.008	0.002	0.004	0.001	0.010	0.002
Total single-vehicle crashes	0.147	0.044	0.112	0.014	0.174	0.030
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.129	0.532	0.068	0.354	0.060
Head-on collision	0.040	0.012	0.060	0.008	0.025	0.004
Rear-end collision	0.242	0.072	0.210	0.027	0.266	0.045
Sideswipe collision	0.101	0.030	0.044	0.006	0.144	0.024
Other multiple-vehicle collision	0.039	0.012	0.042	0.005	0.037	0.006
Total multiple-vehicle crashes	0.853	0.255	0.888	0.114	0.826	0.140

Worksheet 2E — Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(2)	(3)							
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.3							
Fatal and Injury (FI)	0.431	0.1							
Property Damage Only (PDO)	0.569	0.2							

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
Total Crashes/yr Fatal and Injury Crashes/yr Property Damage Only Crashes/yr									shes/yr	
	(KABCO)				(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement		
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.3	0.3	0.0	0.1	0.1	0.0	0.2	0.2	0.0	
Special Note: When the proje	ct element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the p	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections							
	Location Information						
Roadway 0							

Webber Canyon Road at Badger Road Agency or Company Date Performed 07/17/23 Jurisdiction Benton County Intersection 2023 Intersection 29 Analysis Year

Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data					Site Conditions			Base Conditions	
Intersection type (3ST, 4ST, 4SG)	Intersection type (3ST, 4ST, 4SG)						3ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	2,210				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)	520				
Intersection skew angle (degrees)	[If 4ST, does skew d	iffer for minor legs? Els	e, No.]	No	Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrol	ed approaches with a left-	turn lane (0, 1, 2, 3, 4)			0			0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0			0		
Intersection lighting (present/not present)				Not Present			Not Present		
Calibration Factor, C <sub>i</sub>					1.00			1.00	

Intersection

Average Annual Crash History (3 or 5-yr average)

James Vorosmarti WSDOT

**General Information** 

Analyst

KABC Fatal and Injury Only 1.0 Intersection crashes PDO Property Damage Only 0.0

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	1.00	1.00						

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1) (2) (3) (4) (5) (6) (7) (8)											
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C₁	predicted int				
Crasii Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, C	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	0.491	0.54	1.000	0.491	1.00	1.00	0.491				
Fatal and Injury (FI)			0.415	0.204	1.00	1.00	0.204				
Property Damage Only (PDO)			0.585	0.287	1.00	1.00	0.287				

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections		
(1)	(2)	(3)	(4)	(5)	(6)	(7)	
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)	
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C	
Total	1.000	0.491	1.000	0.204	1.000	0.287	
		(2)x(3)TOTAL		(4)x(5)₅ı		(6)x(7)pdo	
			SINGLE-VEHICL	E			
Collision with animal	0.019	0.009	0.008	0.002	0.026	0.007	
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000	
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000	
Overturned	0.013	0.006	0.022	0.004	0.007	0.002	
Ran off road	0.244	0.120	0.240	0.049	0.247	0.071	
Other single-vehicle collision	0.016	0.008	0.011	0.002	0.020	0.006	
Total single-vehicle crashes	0.294	0.144	0.283	0.058	0.302	0.087	
			MULTIPLE-VEHIO	CLE			
Angle collision	0.237	0.116	0.275	0.056	0.210	0.060	
Head-on collision	0.052	0.026	0.081	0.016	0.032	0.009	
Rear-end collision	0.278	0.136	0.260	0.053	0.292	0.084	
Sideswipe collision	0.097	0.048	0.051	0.010	0.131	0.038	
Other multiple-vehicle collision	0.042	0.021	0.050	0.010	0.033	0.009	
Total multiple-vehicle crashes	0.706	0.346	0.717	0.146	0.698	0.200	

	Worksheet 2E Summary Results for Rural Two-Lane Two-Way R	oad Intersections				
(1)	(2)	(3)				
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)				
Crasii severity ievei	(4) from Worksheet 2C	(8) from Worksheet 2C				
Total	1.000	0.5				
Fatal and Injury (FI)	0.415	0.2				
Property Damage Only (PDO)	0.585	0.3				

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>					
Total Crashes/yr Fatal and Injury Crashes/yr Property Damage Only Crashes/yr											
		(KABCO)			(KABC)		(PDO)				
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement		
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>			
	0.5 0.8 0.3 0.2 0.3 0.1 0.3 0.5 0.2										
Special Note: When the proje	ct element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB		

special route. When the project element is not included in the analysis the results will all de zeros. In addition results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

ATTACHMENT 1 - HSM PREDICTIVE ANALYSIS

STREET LIGHTING MITIGATION

		Works	heet 2A Ge	neral Information and	d Input Data for Rural Two-Lane Two-Way Roadway Intersections						
General Information	eneral Information					nation					
Analyst	Linde Thatcher				Roadway			0			
Agency or Company	WSDOT				Intersection			Route 221 at Sellard	s Road		
Date Performed	07/21/23				Jurisdiction			WSDOT			
ntersection	Intersection 1				Analysis Year			2023			
Signalized/Unsignalized	Unsignalized										
Input Data					Site Conditions					Base Conditions	
ntersection type (3ST, 4ST, 4SG)					4ST						
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	3,580						
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)			760				
ntersection skew angle (degrees)	[If 4ST, does skew di	iffer for minor legs? Els	e, No.]		Skew for Leg 1 (All)	0	Skew for Leg 2 (4ST only)	()		0	
Number of signalized or uncontrolled	approaches with a left-t	turn lane (0, 1, 2, 3, 4)								0	
Number of signalized or uncontrolled	approaches with a right	-turn lane (0, 1, 2, 3, 4)								0	
ntersection lighting (present/not pres	sent)					Р	resent			Not Present	
Calibration Factor, C <sub>i</sub>					1.00					1.00	
Average Annual Crash History (3 or 5	-yr average)										
Intersection crashes				KABC	Fatal and Injury C	nly	0	.6			
				DDO	Dana a a a b . Dana a a a	Order	1	0			

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF 2i	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	1.00	0.91	0.91						

PDO

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Property Damage Only

1.8

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	1.486	0.24	1.000	1.486	0.91	1.00	1.348				
Fatal and Injury (FI)			0.431	0.641	0.91	1.00	0.581				
Property Damage Only (PDO)			0.569	0.846	0.91	1.00	0.767				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.348	1.000	0.581	1.000	0.767
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.013	0.006	0.003	0.014	0.011
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.007	0.006	0.003	0.004	0.003
Ran off road	0.122	0.165	0.094	0.055	0.144	0.110
Other single-vehicle collision	0.008	0.011	0.004	0.002	0.010	0.008
Total single-vehicle crashes	0.147	0.198	0.112	0.065	0.174	0.133
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.581	0.532	0.309	0.354	0.272
Head-on collision	0.040	0.054	0.060	0.035	0.025	0.019
Rear-end collision	0.242	0.326	0.210	0.122	0.266	0.204
Sideswipe collision	0.101	0.136	0.044	0.026	0.144	0.110
Other multiple-vehicle collision	0.039	0.053	0.042	0.024	0.037	0.028
Total multiple-vehicle crashes	0.853	1.150	0.888	0.516	0.826	0.634

	Worksheet 2E Summary Results for Rural Two-Lane Two-Way R	oad Intersections
(1)	(2)	(3)
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C
Total	1.000	1.3
Fatal and Injury (FI)	0.431	0.6
Property Damage Only (PDO)	0.569	0.8

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>				
		Total Crashes/yr (KABCO)		Fata	l and Injury Crash (KABC)	es/yr	Property Damage Only Crashes/yr (PDO)			
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.3	2.0	0.6	0.6	0.9	0.3	0.8	1.1	0.4	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

		Worksh	eet 2A Ger	neral Information and	nd Input Data for Rural Two-Lane Two-Way Roadway Intersections						
General Information					Location Information						
Analyst	Linde Thatcher				Roadway			0			
Agency or Company	WSDOT				Intersection			Webber Canyon Roa	d at Badger Road		
Date Performed	07/21/23				Jurisdiction			WSDOT			
Intersection	Intersection 4				Analysis Year			2023			
Signalized/Unsignalized	Unsignalized										
Input Data					Site Conditions					Base Conditions	
ntersection type (3ST, 4ST, 4SG)					3ST						
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	2,210					==	
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)			520				
ntersection skew angle (degrees)	[If 4ST, does skew di	ffer for minor legs? Else,	, No.]		Skew for Leg 1	()	Skew for Leg 2 (4ST only)	()		0	
Number of signalized or uncontrolle	d approaches with a left-t	urn lane (0, 1, 2, 3, 4)								0	
Number of signalized or uncontrolle	d approaches with a right	-turn lane (0, 1, 2, 3, 4)								0	
ntersection lighting (present/not pr	esent)				Present					Not Present	
alibration Factor, C <sub>i</sub>					1.00				1.00		
Average Annual Crash History (3 or	5-yr average)		•		-	-	-				
Intersection crashes				KABC	KABC Fatal and Injury Only			.0			
				200	D	0.1	0	0			

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections							
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF 2i	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	1.00	1.00	0.90	0.90				

PDO

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Property Damage Only

0.0

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(1) (2)		(4)	(5)	(6)	(7)	(8)	
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int	
	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
		10.6.2	5	(Z)TOTAL (T)	Worksheet 2B		(3) (0) (7)	
Total	0.491	0.54	1.000	0.491	0.90	1.00	0.442	
Fatal and Injury (FI)			0.415	0.204	0.90	1.00	0.184	
Property Damage Only (PDO)			0.585	0.287	0.90	1.00	0.259	

Worksheet 2D Crashes by Severity Level and Collision Type for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)		
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)		
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C		
Total	1.000	0.442	1.000	0.184	1.000	0.259		
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7) <sub>PDO</sub>		
			SINGLE-VEHIC	E				
Collision with animal	0.019	0.008	0.008	0.001	0.026	0.007		
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000		
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000		
Overturned	0.013	0.006	0.022	0.004	0.007	0.002		
Ran off road	0.244	0.108	0.240	0.044	0.247	0.064		
Other single-vehicle collision	0.016	0.007	0.011	0.002	0.020	0.005		
Total single-vehicle crashes	0.294	0.130	0.283	0.052	0.302	0.078		
			MULTIPLE-VEHIO	CLE				
Angle collision	0.237	0.105	0.275	0.050	0.210	0.054		
Head-on collision	0.052	0.023	0.081	0.015	0.032	0.008		
Rear-end collision	0.278	0.123	0.260	0.048	0.292	0.076		
Sideswipe collision	0.097	0.043	0.051	0.009	0.131	0.034		
Other multiple-vehicle collision	0.042	0.019	0.050	0.009	0.033	0.009		
Total multiple-vehicle crashes	0.706	0.312	0.717	0.132	0.698	0.181		

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.4					
Fatal and Injury (FI)	0.415	0.2					
Property Damage Only (PDO)	0.585	0.3					

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr			Fatal and Injury Crashes/yr			Property Damage Only Crashes/yr		
Summary for the project element	(KABCO)			(KABC)			(PDO)		
	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.4	0.7	0.3	0.2	0.3	0.1	0.3	0.4	0.2
pecial Note: When the project element is not included in the analysis the results will all be zeros. In addition if only the analysis only includes determining the predicted average crash frequency (i.e. EB analysis is not carried out), the results will show zero values where EB									

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

### PROJECT SAFETY PERFORMANCE SUMMARY REPORT

### **General Information**

Project Name Horse Heaven Wind Farm

Project Description Safety Analysis (Lighting Mitigation)

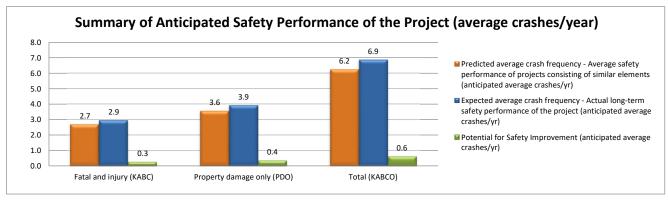
Reference Number 143-67639-23007 Analyst Linde Thatcher Agency/Company WSDOT

Contact Email linde.thatcher@tetratech.com

Contact Phone 508-786-2222
Date Completed 07/21/23

Years of crash data incorporated into the analysis: 5

### PROJECT SUMMARY



		Total Crashes/yr (KABCO)		Fata	and Injury Crash	es/yr	Property	£		
Project Element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	average crash	Potential for Improvement	
	N <sub>predicted</sub> (KABCO)	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	frequency Improvem		
INDIVIDUAL INTERSECTIONS										
Intersection 1	1.3	2.0	0.6	0.6	0.9	0.3	0.8	1.1	0.4	
Intersection 2	2.0	2.0	0.0	0.8	0.9	0.0	1.1	1.1	0.0	
Intersection 3	2.5	2.1	0.0	1.1	0.9	0.0	1.4	1.2	0.0	
Intersection 4	0.4	0.7	0.3	0.2	0.3	0.1	0.3	0.4	0.2	
COMBINED (sum of column)	6.2	6.9	0.6	2.7	2.9	0.3	3.6	3.9	0.4	

### PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Rural 2-Lane Roads

	N predicted(PROJECT)	N expected (PROJECT)	N potential for improvement (PROJECT)	
Crash severity level	Predicted average crash frequency - Average safety performance of projects consisting of similar elements (anticipated average crashes/yr)	Expected average crash frequency - Actual long-term safety performance of the project (anticipated average crashes/yr)	Potential for Safety Improvement (anticipated average crashes/yr)	
Fatal and injury (KABC)	2.7	2.9	0.3	
Property damage only (PDO)	3.6	3.9	0.4	
Total (KABCO)	6.2	6.9	0.6	

HSM1 Extended Spreadsheet for Part C Chapter 10 v.9.1

### **Discussion of Results**

### Given the potential effects of project characteristics on safety performance, results indicate that:

- 1. It is anticipated that the project will, on average, experience 6.9 crashes per year (2.9 fatal and injury crashes per year; and 3.9 property damage only crashes per year).
- 2. A similar project is anticipated, on average, to experience 6.2 crashes per year (2.7 fatal and injury crashes per year; and 3.6 property damage only crashes per year).
- 3. It is anticipated the project has, on average, a potential for safety improvement of 0.6 crashes per year (0.3 fatal and injury crashes per year; and 0.4 property damage only crashes per year).

ATTACHMENT 1 - HSM PREDICTIVE ANALYSIS

RIGHT TURN LANE MITIGATION

	Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections									
General Information					Location Information	า				
Analyst	Linde Thatcher	Linde Thatcher			Roadway			0		
Agency or Company	WSDOT				Intersection			Route 221 at Sellard	Road	
Date Performed	07/21/23	07/21/23			Jurisdiction			WSDOT		
Intersection	ersection Intersection 1			Analysis Year			2023			
Signalized/Unsignalized	alized/Unsignalized Unsignalized									
Input Data					Site Conditions			Base Conditions		
Intersection type (3ST, 4ST, 4SG)				4ST						
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)			3,580			
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)			760			
Intersection skew angle (degrees)	[If 4ST, does skew di	ffer for minor legs? Els	e, No.]		Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	()	0	
Number of signalized or uncontrolled	d approaches with a left-t	urn lane (0, 1, 2, 3, 4)					0		0	
Number of signalized or uncontrolled	d approaches with a right	-turn lane (0, 1, 2, 3, 4)	)				2		0	
Intersection lighting (present/not present)					Not Present			Not Present		
Calibration Factor, C <sub>i</sub>				1.00			1.00			
Average Annual Crash History (3 or	5-yr average)	·								

Intersection crashes	KABC	Fatal and Injury Only	0.6				
intersection crashes	PDO	Property Damage Only	1.8				
NOTES *AADT II ': ' AADT II ': ' AADT II ': ' AADT II '							

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

	Worksheet 2B — Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections										
(1) (2) (3) (4) (5)											
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF							
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>							
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)							
1.00	1.00	0.74	1.00	0.74							

0.6

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)				
Total	1.486	0.24	1.000	1.486	0.74	1.00	1.100				
Fatal and Injury (FI)			0.431	0.641	0.74	1.00	0.474				
Property Damage Only (PDO)			0.569	0.846	0.74	1.00	0.626				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.100	1.000	0.474	1.000	0.626
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.011	0.006	0.003	0.014	0.009
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001
Overturned	0.005	0.005	0.006	0.003	0.004	0.003
Ran off road	0.122	0.134	0.094	0.045	0.144	0.090
Other single-vehicle collision	0.008	0.009	0.004	0.002	0.010	0.006
Total single-vehicle crashes	0.147	0.162	0.112	0.053	0.174	0.109
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.474	0.532	0.252	0.354	0.222
Head-on collision	0.040	0.044	0.060	0.028	0.025	0.016
Rear-end collision	0.242	0.266	0.210	0.100	0.266	0.166
Sideswipe collision	0.101	0.111	0.044	0.021	0.144	0.090
Other multiple-vehicle collision	0.039	0.043	0.042	0.020	0.037	0.023
Total multiple-vehicle crashes	0.853	0.938	0.888	0.421	0.826	0.517

	Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(2)	(3)								
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)								
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C								
Total	1.000	1.1								
Fatal and Injury (FI)	0.431	0.5								
Property Damage Only (PDO)	0.569	0.6								

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Property Damage Only Crashes/yr			
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.1	1.8	0.7	0.5	0.8	0.3	0.6	1.0	0.4	
Special Note: When the proje	ct element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will :	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

	Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections									
General Information					Location Information					
Analyst	Linde Thatcher	Linde Thatcher			Roadway		0			
Agency or Company	WSDOT	WSDOT			Intersection		Route 221 at Route	14		
Date Performed	07/21/23			Jurisdiction		WSDOT				
Intersection	Intersection 2			Analysis Year		2023				
Signalized/Unsignalized	red Unsignalized									
Input Data					Site Conditions			Base Conditions		
Intersection type (3ST, 4ST, 4SG)				4ST						
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	3,410					
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)		1,480				
Intersection skew angle (degrees	) [If 4ST, does skew di	ffer for minor legs? Els	e, No.]		Skew for Leg 1 (All):	Skew for Leg 2 (4ST only)	()	0		
Number of signalized or uncontro	olled approaches with a left-t	urn lane (0, 1, 2, 3, 4)				0		0		
Number of signalized or uncontro	olled approaches with a right-	turn lane (0, 1, 2, 3, 4)				2		0		
Intersection lighting (present/no	Intersection lighting (present/not present)				Present			Not Present		
Calibration Factor, C <sub>i</sub>					1.00			1.00		
Average Annual Crash History (3	or 5-yr average)							_		

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
 (1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					

0.74

KABC

PDO

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

1.00

Intersection crashes

Fatal and Injury Only

Property Damage Only

0.4

1.6

0.67

0.91

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N			
	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int			
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)			
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B					
Total	2.167	0.24	1.000	2.167	0.67	1.00	1.455			
Fatal and Injury (FI)			0.431	0.934	0.67	1.00	0.627			
Property Damage Only (PDO)		-	0.569	1.233	0.67	1.00	0.828			

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.455	1.000	0.627	1.000	0.828
		(2)x(3)total		(4)x(5) <sub>FI</sub>		(6)x(7) <sub>PDO</sub>
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.015	0.006	0.004	0.014	0.012
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.007	0.006	0.004	0.004	0.003
Ran off road	0.122	0.178	0.094	0.059	0.144	0.119
Other single-vehicle collision	0.008	0.012	0.004	0.003	0.010	0.008
Total single-vehicle crashes	0.147	0.214	0.112	0.070	0.174	0.144
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.627	0.532	0.334	0.354	0.293
Head-on collision	0.040	0.058	0.060	0.038	0.025	0.021
Rear-end collision	0.242	0.352	0.210	0.132	0.266	0.220
Sideswipe collision	0.101	0.147	0.044	0.028	0.144	0.119
Other multiple-vehicle collision	0.039	0.057	0.042	0.026	0.037	0.031
Total multiple-vehicle crashes	0.853	1.241	0.888	0.557	0.826	0.684

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	1.5					
Fatal and Injury (FI)	0.431	0.6					
Property Damage Only (PDO)	0.569	0.8					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>				
	Total Crashes/yr (KABCO)			Fata	Fatal and Injury Crashes/yr			Property Damage Only Crashes/yr		
				(KABC)			(PDO)			
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.5	1.8	0.3	0.6	0.8	0.1	0.8	1.0	0.2	
Special Note: When the proje	ect element is not included	in the analysis the results wil	l all be zeros. In addition	if only the analysis only	includes determining the p	oredicted average crash f	requency (i.e. EB analysis is no	t carried out), the results will s	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information		Location Information		
Analyst	Linde Thatcher	Roadway	0	
Agency or Company	WSDOT	Intersection	Route 14 at S. Plymo	outh Rd
Date Performed	07/21/23	Jurisdiction	WSDOT	
Intersection	Intersection 3	Analysis Year	2023	
Signalized/Unsignalized	Unsignalized			
Input Data		Site Conditions		Base Conditions

input Data					Site C	onaitions	Base Conditions
Intersection type (3ST, 4ST, 4SG)						4ST	
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> =	14,700	(veh/day)		(	5,390	
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> =	3,500	(veh/day)		1	1,170	
Intersection skew angle (degrees) [If 4ST, does skew di	iffer for minor legs? Else	e, No.]		Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-	turn lane (0, 1, 2, 3, 4)					2	0
Number of signalized or uncontrolled approaches with a right	-turn lane (0, 1, 2, 3, 4)					2	0
Intersection lighting (present/not present)					P	resent	Not Present
Calibration Factor, C						1.00	1.00

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	1.2
intersection crashes	PDO	Property Damage Only	0.8

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.52	0.74	0.91	0.35						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	2.737	0.24	1.000	2.737	0.35	1.00	0.956				
Fatal and Injury (FI)			0.431	1.180	0.35	1.00	0.412				
Property Damage Only (PDO)			0.569	1.558	0.35	1.00	0.544				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.956	1.000	0.412	1.000	0.544
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.010	0.006	0.002	0.014	0.008
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001
Overturned	0.005	0.005	0.006	0.002	0.004	0.002
Ran off road	0.122	0.117	0.094	0.039	0.144	0.078
Other single-vehicle collision	0.008	0.008	0.004	0.002	0.010	0.005
Total single-vehicle crashes	0.147	0.140	0.112	0.046	0.174	0.095
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.412	0.532	0.219	0.354	0.192
Head-on collision	0.040	0.038	0.060	0.025	0.025	0.014
Rear-end collision	0.242	0.231	0.210	0.086	0.266	0.145
Sideswipe collision	0.101	0.097	0.044	0.018	0.144	0.078
Other multiple-vehicle collision	0.039	0.037	0.042	0.017	0.037	0.020
Total multiple-vehicle crashes	0.853	0.815	0.888	0.366	0.826	0.449

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	1.0					
Fatal and Injury (FI)	0.431	0.4					
Property Damage Only (PDO)	0.569	0.5					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	1.0	1.5	0.6	0.4	0.7	0.2	0.5	0.9	0.3

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

	Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections								
General Information	General Information				Location Information				
Analyst	Linde Thatcher	Linde Thatcher			Roadway		0		
Agency or Company	WSDOT				Intersection		Webber Canyon Roa	d at Badger Road	
Date Performed	07/21/23				Jurisdiction		WSDOT		
Intersection	Intersection 4	Intersection 4			Analysis Year		2023		
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data				Site Conditions			Base Conditions		
Intersection type (3ST, 4ST, 4SG)					3ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	2,210				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)	520				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]					Skew for Leg 1 O Sk	kew for Leg 2 (4ST only):	0	0	
Number of signalized or uncontr	Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			0			0		
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				1			0		
Intersection lighting (present/not present)				Not Present			Not Present		
Calibration Factor, C <sub>i</sub>	•	•		•	1.00			1.00	

Intersection crashes

KABC Fatal and Injury Only
PDO Property Damage Only

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Average Annual Crash History (3 or 5-yr average)

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	1.00	0.86	1.00	0.86						

1.0

0.0

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
•	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)				
	from Equations 10-8, 10-9, or 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	0.491	0.54	1.000	0.491	0.86	1.00	0.422				
Fatal and Injury (FI)			0.415	0.204	0.86	1.00	0.175				
Property Damage Only (PDO)			0.585	0.287	0.86	1.00	0.247				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.422	1.000	0.175	1.000	0.247
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.019	0.008	0.008	0.001	0.026	0.006
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.005	0.022	0.004	0.007	0.002
Ran off road	0.244	0.103	0.240	0.042	0.247	0.061
Other single-vehicle collision	0.016	0.007	0.011	0.002	0.020	0.005
Total single-vehicle crashes	0.294	0.124	0.283	0.050	0.302	0.075
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.100	0.275	0.048	0.210	0.052
Head-on collision	0.052	0.022	0.081	0.014	0.032	0.008
Rear-end collision	0.278	0.117	0.260	0.046	0.292	0.072
Sideswipe collision	0.097	0.041	0.051	0.009	0.131	0.032
Other multiple-vehicle collision	0.042	0.018	0.050	0.009	0.033	0.008
Total multiple-vehicle crashes	0.706	0.298	0.717	0.126	0.698	0.172

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1)	(2)	(3)							
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.4							
Fatal and Injury (FI)	0.415	0.2							
Property Damage Only (PDO)	0.585	0.2							

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.4	0.7	0.3	0.2	0.3	0.1	0.2	0.4	0.2

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

### PROJECT SAFETY PERFORMANCE SUMMARY REPORT

### **General Information**

Project Name Horse Heaven Wind Farm

Project Description Safety Analysis (Right Turn Mitigation)
Reference Number 143-67639-23007

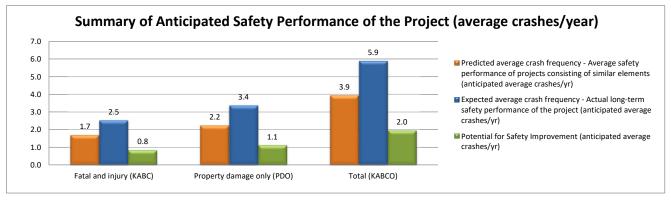
Analyst Linde Thatcher
Agency/Company WSDOT

Contact Email linde.thatcher@tetratech.com

Contact Phone 508-786-2222
Date Completed 07/21/23

Years of crash data incorporated into the analysis: 5

### PROJECT SUMMARY



		Total Crashes/yr (KABCO)		Fata	l and Injury Crash (KABC)	es/yr	Property Damage Only Crashes/yr (PDO)		
Project Element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
INDIVIDUAL INTERSECTIONS									
Intersection 1	1.1	1.8	0.7	0.5	0.8	0.3	0.6	1.0	0.4
Intersection 2	1.5	1.8	0.3	0.6	0.8	0.1	0.8	1.0	0.2
Intersection 3	1.0	1.5	0.6	0.4	0.7	0.2	0.5	0.9	0.3
Intersection 4	0.4	0.7	0.3	0.2	0.3	0.1	0.2	0.4	0.2
COMBINED (sum of column)	3.9	5.9	2.0	1.7	2.5	0.8	2.2	3.4	1.1

### PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Rural 2-Lane Roads

Crash severity level	N predicted(PROJECT)  Predicted average crash frequency - Average safety performance of projects consisting of similar elements (anticipated average crashes/yr)	N expected (PROJECT)  Expected average crash frequency  - Actual long-term safety performance of the project (anticipated average crashes/yr)	Potential for Safety Improvement (anticipated average crashes/yr)	
Fatal and injury (KABC)	1.7	2.5	0.8	
Property damage only (PDO)	2.2	3.4	1.1	
Total (KABCO)	3.9	5.9	2.0	

HSM1 Extended Spreadsheet for Part C Chapter 10 v.9.1

### **Discussion of Results**

### Given the potential effects of project characteristics on safety performance, results indicate that:

- 1. It is anticipated that the project will, on average, experience 5.9 crashes per year (2.5 fatal and injury crashes per year; and 3.4 property damage only crashes per year).
- 2. A similar project is anticipated, on average, to experience 3.9 crashes per year (1.7 fatal and injury crashes per year; and 2.2 property damage only crashes per year).
- 3. It is anticipated the project has, on average, a potential for safety improvement of 2 crashes per year (0.8 fatal and injury crashes per year; and 1.1 property damage only crashes per year).

ATTACHMENT 1 - HSM PREDICTIVE ANALYSIS

LEFT-TURN LANE MITIGATION

	Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections								
General Information					Location Information				
Analyst	Linde Thatcher			Roadway	0				
Agency or Company	WSDOT	WSDOT			Intersection	Route 221 at Sellard	ds Road		
Date Performed	07/21/23				Jurisdiction	WSDOT			
Intersection	Intersection 1	Intersection 1			Analysis Year	2023			
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data					Site Conditions		Base Conditions		
Intersection type (3ST, 4ST, 4SG	)				4ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	3,580				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)	760				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]					Skew for Leg 1 (AII): Skew for (4ST of	- ()	0		
Number of signalized or uncontr	Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			2		0			
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)					0		0		
Intersection lighting (present/not present)				•	Not Present		Not Present		
Calibration Factor, C <sub>i</sub>				•	1.00		1.00		

Intersection crashes

KABC Fatal and Injury Only 0.6
PDO Property Damage Only 1.8

NOTES: \* AADT: It is important to remember that the AADT(major) = AADT(major approach1) + AADT(minor approach2) (refer to p.10-6 in Part C of the HSM)

Average Annual Crash History (3 or 5-yr average)

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF 2i	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.52	1.00	1.00	0.52						

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)				
	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Cambration Factor, C	(5)*(6)*(7)				
	from Equations 10-8, 10-9, or 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B						
Total	1.486	0.24	1.000	1.486	0.52	1.00	0.773				
Fatal and Injury (FI)			0.431	0.641	0.52	1.00	0.333				
Property Damage Only (PDO)			0.569	0.846	0.52	1.00	0.440				

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.773	1.000	0.333	1.000	0.440
		(2)x(3)total		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.008	0.006	0.002	0.014	0.006
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.000
Overturned	0.005	0.004	0.006	0.002	0.004	0.002
Ran off road	0.122	0.094	0.094	0.031	0.144	0.063
Other single-vehicle collision	0.008	0.006	0.004	0.001	0.010	0.004
Total single-vehicle crashes	0.147	0.114	0.112	0.037	0.174	0.077
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.333	0.532	0.177	0.354	0.156
Head-on collision	0.040	0.031	0.060	0.020	0.025	0.011
Rear-end collision	0.242	0.187	0.210	0.070	0.266	0.117
Sideswipe collision	0.101	0.078	0.044	0.015	0.144	0.063
Other multiple-vehicle collision	0.039	0.030	0.042	0.014	0.037	0.016
Total multiple-vehicle crashes	0.853	0.659	0.888	0.296	0.826	0.363

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1) (2) (3)									
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.8							
Fatal and Injury (FI)	0.431	0.3							
Property Damage Only (PDO)	0.569	0.4							

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
	Total Crashes/yr Fatal and Injury Crashes/yr Property Damage Only Crashes/yr									
		(KABCO) (KABC) (PDO)								
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Detential for   Average crash   Average crash   Detential for				Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.8 1.6 0.8 0.3 0.7 0.3 0.4 0.9 0.4									
Special Note: When the proje	ect element is not included	in the analysis the results wi	ll all be zeros. In addition	if only the analysis only	includes determining the	oredicted average crash f	requency (i.e. EB analysis is no	ot carried out), the results will	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Inter	sections				
Location Information						

Analyst Linde Thatcher Roadway WSDOT Route 221 at Route 14 Agency or Company Intersection Date Performed 07/21/23 WSDOT Jurisdiction 2023 Intersection Intersection 2 Analysis Year Signalized/Unsignalized Unsignalized

Input Data			Site C	Conditions	Base Conditions	
Intersection type (3ST, 4ST, 4SG)					4ST	
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> =	14,700	(veh/day)	3	3,410	<del></del>
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> =	3,500	(veh/day)	:	1,480	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]				Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-	urn lane (0, 1, 2, 3, 4)			2		0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)					1	0
Intersection lighting (present/not present)				P	resent	Not Present
Calibration Factor, C <sub>i</sub>					1.00	1.00

Average Annual Crash History (3 or 5-yr average)

General Information

Intersection crashes	KABC	Fatal and Injury Only	0.4
intersection crashes	PDO	Property Damage Only	1.6

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections										
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.52	0.86	0.91	0.41						

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections											
(1)	(1) (2) (3) (4) (5) (6) (7)										
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N				
Crash Severity Level	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int				
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)				
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)				
Total	2.167	0.24	1.000	2.167	0.41	1.00	0.879				
Fatal and Injury (FI)			0.431	0.934	0.41	1.00	0.379				
Property Damage Only (PDO)			0.569	1.233	0.41	1.00	0.500				

	Worksheet 2D Crashes by Severity Level and Collision Type for Rural Two-Lane Two-Way Road Intersections										
(1)	(2)	(3)	(4)	(5)	(6)	(7)					
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)					
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C					
Total	1.000	0.879	1.000	0.379	1.000	0.500					
		(2)x(3)TOTAL		(4)x(5)fl		(6)x(7)PDO					
			SINGLE-VEHICL	E							
Collision with animal	0.010	0.009	0.006	0.002	0.014	0.007					
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001					
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001					
Overturned	0.005	0.004	0.006	0.002	0.004	0.002					
Ran off road	0.122	0.107	0.094	0.036	0.144	0.072					
Other single-vehicle collision	0.008	0.007	0.004	0.002	0.010	0.005					
Total single-vehicle crashes	0.147	0.129	0.112	0.042	0.174	0.087					
			MULTIPLE-VEHIO	CLE							
Angle collision	0.431	0.379	0.532	0.202	0.354	0.177					
Head-on collision	0.040	0.035	0.060	0.023	0.025	0.013					
Rear-end collision	0.242	0.213	0.210	0.080	0.266	0.133					
Sideswipe collision	0.101	0.089	0.044	0.017	0.144	0.072					
Other multiple-vehicle collision	0.039	0.034	0.042	0.016	0.037	0.019					
Total multiple-vehicle crashes	0.853	0.750	0.888	0.337	0.826	0.413					

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections									
(1) (2) (3)									
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)							
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C							
Total	1.000	0.9							
Fatal and Injury (FI)	0.431	0.4							
Property Damage Only (PDO)	0.569	0.5							

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr
		(KABCO) (KABC)						(PDO)	
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Predicted Expected Potential for average crash average crash Potential for Improvement frequency frequency Improvement				Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
0.9 1.5 0.6 0.4 0.6 0.2 0.5 0.8 0.3									
Special Note: When the proje	pecial Note: When the project element is not included in the analysis the results will all be zeros. In addition if only the analysis only includes determining the predicted average crash frequency (i.e. EB analysis is not carried out), the results will show zero values where EB								

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

	sheet 2A Ger	neral Information and	Input Data for Rural Two-Lane Two-Way Roadway Intersections							
General Information					Location Information	1				
Analyst	Linde Thatcher				Roadway			0		
Agency or Company	WSDOT				Intersection			Route 14 at S. Plymo	outh Rd	
Date Performed	07/21/23				Jurisdiction			WSDOT		
Intersection	Intersection 3				Analysis Year			2023		
Signalized/Unsignalized	Unsignalized									
Input Data					Site Conditions				Base Conditions	
Intersection type (3ST, 4ST, 4SG)	_				4ST					
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	14,700	(veh/day)	6,390					
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	3,500	(veh/day)		1,170				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]					Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0	
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)					2			0		
Number of signalized or uncontrolled a	Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)					0 0			0	

Present

1.00

Not Present

1.00

### Average Annual Crash History (3 or 5-yr average)

Intersection lighting (present/not present)

Calibration Factor, C

Intersection crashes	KABC	Fatal and Injury Only	1.2
intersection crashes	PDO	Property Damage Only	0.8

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.52	1.00	0.91	0.47						

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	2.737	0.24	1.000	2.737	0.47	1.00	1.291		
Fatal and Injury (FI)		-	0.431	1.180	0.47	1.00	0.557		
Property Damage Only (PDO)			0.569	1.558	0.47	1.00	0.735		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	1.291	1.000	0.557	1.000	0.735
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7)pdo
			SINGLE-VEHICL	E		
Collision with animal	0.010	0.013	0.006	0.003	0.014	0.010
Collision with bicycle	0.001	0.001	0.001	0.001	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.001	0.001	0.001
Overturned	0.005	0.006	0.006	0.003	0.004	0.003
Ran off road	0.122	0.158	0.094	0.052	0.144	0.106
Other single-vehicle collision	0.008	0.010	0.004	0.002	0.010	0.007
Total single-vehicle crashes	0.147	0.190	0.112	0.062	0.174	0.128
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.557	0.532	0.296	0.354	0.260
Head-on collision	0.040	0.052	0.060	0.033	0.025	0.018
Rear-end collision	0.242	0.313	0.210	0.117	0.266	0.195
Sideswipe collision	0.101	0.130	0.044	0.024	0.144	0.106
Other multiple-vehicle collision	0.039	0.050	0.042	0.023	0.037	0.027
Total multiple-vehicle crashes	0.853	1.102	0.888	0.494	0.826	0.607

Worksheet 2E — Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Control of the Control	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	1.3						
Fatal and Injury (FI)	0.431	0.6						
Property Damage Only (PDO)	0.569	0.7						

	PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>									
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prop	erty Damage Only Cra	ashes/yr	
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	ed Expected average crash frequency Potential for		Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	1.3 1.7 0.4 0.6 0.7 0.2 0.7 1.0 0.2									
Special Note: When the proje	ct element is not included	in the analysis the results wi	l all be zeros. In addition	if only the analysis only	includes determining the	predicted average crash f	requency (i.e. EB analysis is no	ot carried out), the results will	show zero values where EB	

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections							
	Location Information						
	Roadway 0						
	Intersection	Webber Canyon Road at Badger Road					
Jurisdiction WSDOT							

Date Performed	07/21/23				Jurisdiction			WSDOT	
Intersection	Intersection 4				Analysis Year			2023	
Signalized/Unsignalized	Unsignalized								
Input Data						Site (	Conditions		Base Conditions
Intersection type (3ST, 4ST, 4SG)							3ST		
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)			2,210		
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)			520		
Intersection skew angle (degrees)	[If 4ST, does skew d	iffer for minor legs? Els	e, No.]		Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				1		0			
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				0		0			
Intersection lighting (present/not present)					Not Present		Not Present		
Calibration Factor, C <sub>i</sub>					1.00		1.00		

### Average Annual Crash History (3 or 5-yr average)

Linde Thatcher WSDOT

**General Information** 

Agency or Company

Analyst

Intersection crashes	KABC	Fatal and Injury Only	1.0	
intersection crashes	PDO	Property Damage Only	0.0	

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)						
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF						
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>						
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)						
1.00	0.56	1.00	1.00	0.56						

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
	N <sub>sof 3ST, 4ST or 4SG</sub>	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	spf 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	0.491	0.54	1.000	0.491	0.56	1.00	0.275		
Fatal and Injury (FI)			0.415	0.204	0.56	1.00	0.114		
Property Damage Only (PDO)			0.585	0.287	0.56	1.00	0.161		

		Worksheet 2D Crashes by	y Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.275	1.000	0.114	1.000	0.161
		(2)x(3)TOTAL		(4)x(5) <sub>FI</sub>		(6)x(7) <sub>PDO</sub>
			SINGLE-VEHICL	E		
Collision with animal	0.019	0.005	0.008	0.001	0.026	0.004
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.004	0.022	0.003	0.007	0.001
Ran off road	0.244	0.067	0.240	0.027	0.247	0.040
Other single-vehicle collision	0.016	0.004	0.011	0.001	0.020	0.003
Total single-vehicle crashes	0.294	0.081	0.283	0.032	0.302	0.049
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.065	0.275	0.031	0.210	0.034
Head-on collision	0.052	0.014	0.081	0.009	0.032	0.005
Rear-end collision	0.278	0.076	0.260	0.030	0.292	0.047
Sideswipe collision	0.097	0.027	0.051	0.006	0.131	0.021
Other multiple-vehicle collision	0.042	0.012	0.050	0.006	0.033	0.005
Total multiple-vehicle crashes	0.706	0.194	0.717	0.082	0.698	0.112

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1)	(2)	(3)						
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
Crash seventy level	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.3						
Fatal and Injury (FI)	0.415	0.1						
Property Damage Only (PDO)	0.585	0.2						

				PROJECT ELEME	ENT RESULTS SUM	MARY <sup>1</sup>			
		Total Crashes/yr		Fata	al and Injury Crash	es/yr	Prop	erty Damage Only Cra	shes/yr
	(KABCO)				(KABC)		(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.3	0.6	0.3	0.1	0.2	0.1	0.2	0.3	0.2

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

### PROJECT SAFETY PERFORMANCE SUMMARY REPORT

### **General Information**

Project Name Horse Heaven Wind Farm

Project Description Safety Analysis (Left Turn Mitigation)

Reference Number 143-67639-23007 Analyst Linde Thatcher Agency/Company WSDOT

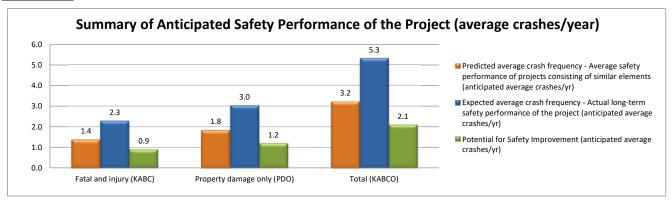
Contact Email linde.thatcher@tetratech.com

Contact Phone 508-786-2222

Date Completed 07/21/23

Years of crash data incorporated into the analysis: 5

### PROJECT SUMMARY



		Total Crashes/yr (KABCO)		Fata	l and Injury Crash (KABC)	es/yr	Property Damage Only Crashes/yr (PDO)			
Project Element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	
	N <sub>predicted</sub> (KABCO)	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
INDIVIDUAL INTERSECTIONS										
Intersection 1	0.8	1.6	0.8	0.3	0.7	0.3	0.4	0.9	0.4	
Intersection 2	0.9	1.5	0.6	0.4	0.6	0.2	0.5	0.8	0.3	
Intersection 3	1.3	1.7	0.4	0.6	0.7	0.2	0.7	1.0	0.2	
Intersection 4	0.3	0.6	0.3	0.1	0.2	0.1	0.2	0.3	0.2	
COMBINED (sum of column)	3.2	5.3	2.1	1.4	2.3	0.9	1.8	3.0	1.2	

### PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Rural 2-Lane Roads

	N predicted(PROJECT)	N expected (PROJECT)	N potential for improvement (PROJECT)	
Crash severity level	performance of projects consisting of similar elements	Expected average crash frequency	Potential for Safety Improvement (anticipated average crashes/yr)	
	(anticipated average crashes/yr)			
Fatal and injury (KABC)	1.4	2.3	0.9	
Property damage only (PDO)	1.8	3.0	1.2	
Total (KABCO)	3.2	5.3	2.1	

HSM1 Extended Spreadsheet for Part C Chapter 10 v.9.1

### **Discussion of Results**

### Given the potential effects of project characteristics on safety performance, results indicate that:

- 1. It is anticipated that the project will, on average, experience 5.3 crashes per year (2.3 fatal and injury crashes per year; and 3 property damage only crashes per year).
- 2. A similar project is anticipated, on average, to experience 3.2 crashes per year (1.4 fatal and injury crashes per year; and 1.8 property damage only crashes per year).
- 3. It is anticipated the project has, on average, a potential for safety improvement of 2.1 crashes per year (0.9 fatal and injury crashes per year; and 1.2 property damage only crashes per year).

ATTACHMENT 1 - HSM PREDICTIVE ANALYSIS

OVERALL MITIGATION

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections							
	Location Information						
	Roadway	0					
	Intersection	Route 221 at Sellards Road					
	Jurisdiction	WSDOT					
	Analysis Year	2023					

0.6

1.8

Input Data				Site Conditions			Base Conditions
Intersection type (3ST, 4ST, 4SG)					4ST		
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> = 14,700	(veh/day)			3,580		
AADT <sub>minor</sub> (veh/day)	AADT <sub>minor</sub> (veh/day) AADT <sub>MAX</sub> = 3,500 (veh/day)				760		
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0	0
Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				2		0	
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			2			0	
Intersection lighting (present/not present)			Present			Not Present	
Calibration Factor, C <sub>i</sub>			1.00				1.00

Average Annual Crash History (3 or 5-yr average)		
	KABC	Fatal and Injury Only

Linde Thatcher WSDOT

07/21/23

Intersection 1

Unsignalized

General Information

Agency or Company
Date Performed

Signalized/Unsignalized

Intersection crashes

Analyst

Intersection

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections							
(1)	(2)	(3)	(4)	(5)			
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF			
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>			
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)			
1.00	0.52	0.74	0.91	0.35			

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	
	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N	
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution		Calibration Factor, C	predicted int	
Crash Seventy Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)	
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	5 (2)TOTAL (4)			(5)*(6)*(7)	
Total	1.486	0.24	1.000	1.486	0.35	1.00	0.519	
Fatal and Injury (FI)			0.431	0.641	0.35	1.00	0.224	
Property Damage Only (PDO)			0.569	0.846	0.35	1.00	0.295	

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.519	1.000	0.224	1.000	0.295
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.005	0.006	0.001	0.014	0.004
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.000
Overturned	0.005	0.003	0.006	0.001	0.004	0.001
Ran off road	0.122	0.063	0.094	0.021	0.144	0.043
Other single-vehicle collision	0.008	0.004	0.004	0.001	0.010	0.003
Total single-vehicle crashes	0.147	0.076	0.112	0.025	0.174	0.051
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.224	0.532	0.119	0.354	0.105
Head-on collision	0.040	0.021	0.060	0.013	0.025	0.007
Rear-end collision	0.242	0.126	0.210	0.047	0.266	0.079
Sideswipe collision	0.101	0.052	0.044	0.010	0.144	0.043
Other multiple-vehicle collision	0.039	0.020	0.042	0.009	0.037	0.011
Total multiple-vehicle crashes	0.853	0.443	0.888	0.199	0.826	0.244

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crash severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.5					
Fatal and Injury (FI)	0.431	0.2					
Property Damage Only (PDO)	0.569	0.3					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)			
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.5	1.2	0.7	0.2	0.5	0.3	0.3	0.7	0.4

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and I	nput Data for Rural Two-Lane Two-Way Roadway Intersections
	Location Information

General Information		Location Information				
Analyst	Linde Thatcher	Roadway	0			
Agency or Company	WSDOT	Intersection	Route 221 at Route 14			
Date Performed	07/21/23	Jurisdiction	WSDOT			
Intersection	Intersection 2	Analysis Year	2023			
Signalized/Unsignalized	Unsignalized					

organization of the second of							
Input Data				Site Conditions			Base Conditions
Intersection type (3ST, 4ST, 4SG)					4ST		
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> =	14,700	(veh/day)	3,410			
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> =	3,500	(veh/day)	1,480			
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]			Skew for Leg 1 (All):	Skew for Leg 2 (4ST only):	0	0	
Number of signalized or uncontrolled approaches with a	Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)			2			0
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)			2			0	
Intersection lighting (present/not present)				Present			Not Present
Calibration Factor, C <sub>i</sub>				1.00			1.00

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	0.4
intersection crashes	PDO	Property Damage Only	1.6

Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)				
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF				
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>				
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)				
1.00	0.52	0.74	0.91	0.35				

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
•	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
Crash Severity Level	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
		10.6.2	5	(2)TOTAL (4)	Worksheet 2B		(5)*(6)*(7)		
Total	2.167	0.24	1.000	2.167	0.35	1.00	0.757		
Fatal and Injury (FI)			0.431	0.934	0.35	1.00	0.326		
Property Damage Only (PDO)			0.569	1.233	0.35	1.00	0.431		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.757	1.000	0.326	1.000	0.431
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)PDO
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.008	0.006	0.002	0.014	0.006
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.000
Overturned	0.005	0.004	0.006	0.002	0.004	0.002
Ran off road	0.122	0.092	0.094	0.031	0.144	0.062
Other single-vehicle collision	0.008	0.006	0.004	0.001	0.010	0.004
Total single-vehicle crashes	0.147	0.111	0.112	0.037	0.174	0.075
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.326	0.532	0.174	0.354	0.152
Head-on collision	0.040	0.030	0.060	0.020	0.025	0.011
Rear-end collision	0.242	0.183	0.210	0.068	0.266	0.115
Sideswipe collision	0.101	0.076	0.044	0.014	0.144	0.062
Other multiple-vehicle collision	0.039	0.030	0.042	0.014	0.037	0.016
Total multiple-vehicle crashes	0.853	0.645	0.888	0.290	0.826	0.356

Worksheet 2E — Summary Results for Rural Two-Lane Two-Way Road Intersections								
(1) (2) (3)								
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)						
	(4) from Worksheet 2C	(8) from Worksheet 2C						
Total	1.000	0.8						
Fatal and Injury (FI)	0.431	0.3						
Property Damage Only (PDO)	0.569	0.4						

PROJECT ELEMENT RESULTS SUMMARY <sup>1</sup>										
		Total Crashes/yr		Fata	l and Injury Crash	es/yr	Prope	erty Damage Only Cra	ishes/yr	
		(KABCO)			(KABC)			(PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for	
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>		
	0.8	1.3	0.6	0.3	0.6	0.3	0.4	0.8	0.3	
Special Note: When the proje	ecial Note: When the project element is not included in the analysis the results will all be zeros. In addition if only the analysis only includes determining the predicted average crash frequency (i.e. EB analysis is not carried out), the results will show zero values where EB									

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

Worksheet 2A General Information and Input Data for Rural Two-Lane Two-Way Roadway Intersections
Location Information

General Information		Location Information				
Analyst	Linde Thatcher	Roadway	0	0		
Agency or Company	WSDOT	Intersection	Route 14 at S. Plymouth Rd			
Date Performed	07/21/23	Jurisdiction	WSDOT			
Intersection	Intersection 3	Analysis Year	2023			
Signalized/Unsignalized	Unsignalized					
Input Data		Site Conditions		Base Conditions		

input Data					Site C	onaitions	Base Conditions
Intersection type (3ST, 4ST, 4SG)						4ST	
AADT <sub>major</sub> (veh/day)	AADT <sub>MAX</sub> =	14,700	(veh/day)	6,390			
AADT <sub>minor</sub> (veh/day)	AADT <sub>MAX</sub> =	3,500	(veh/day)		1	1,170	
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]				Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only):	0
Number of signalized or uncontrolled approaches with a left-		2			0		
Number of signalized or uncontrolled approaches with a right-turn lane (0, 1, 2, 3, 4)				2			0
Intersection lighting (present/not present)					P	resent	Not Present
Calibration Factor, C					1.00	1.00	

Average Annual Crash History (3 or 5-yr average)

Intersection crashes	KABC	Fatal and Injury Only	1.2
intersection crashes	PDO	Property Damage Only	0.8

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	0.52	0.74	0.91	0.35					

Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)		
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N		
	N <sub>spf 3ST, 4ST or 4SG</sub>	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int		
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	Calibration Factor, G	(5)*(6)*(7)		
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B				
Total	2.737	0.24	1.000	2.737	0.35	1.00	0.956		
Fatal and Injury (FI)			0.431	1.180	0.35	1.00	0.412		
Property Damage Only (PDO)			0.569	1.558	0.35	1.00	0.544		

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type <sub>(FI)</sub>	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.956	1.000	0.412	1.000	0.544
		(2)x(3)TOTAL		(4)x(5)FI		(6)x(7)pdo
			SINGLE-VEHICL	.E		
Collision with animal	0.010	0.010	0.006	0.002	0.014	0.008
Collision with bicycle	0.001	0.001	0.001	0.000	0.001	0.001
Collision with pedestrian	0.001	0.001	0.001	0.000	0.001	0.001
Overturned	0.005	0.005	0.006	0.002	0.004	0.002
Ran off road	0.122	0.117	0.094	0.039	0.144	0.078
Other single-vehicle collision	0.008	0.008	0.004	0.002	0.010	0.005
Total single-vehicle crashes	0.147	0.140	0.112	0.046	0.174	0.095
			MULTIPLE-VEHIO	CLE		
Angle collision	0.431	0.412	0.532	0.219	0.354	0.192
Head-on collision	0.040	0.038	0.060	0.025	0.025	0.014
Rear-end collision	0.242	0.231	0.210	0.086	0.266	0.145
Sideswipe collision	0.101	0.097	0.044	0.018	0.144	0.078
Other multiple-vehicle collision	0.039	0.037	0.042	0.017	0.037	0.020
Total multiple-vehicle crashes	0.853	0.815	0.888	0.366	0.826	0.449

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crasii severity level	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	1.0					
Fatal and Injury (FI)	0.431	0.4					
Property Damage Only (PDO)	0.569	0.5					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
Summary for the project element	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	1.0	1.5	0.6	0.4	0.7	0.2	0.5	0.9	0.3

special roue. When the project element is not included in the analysis life results will all de zeros. In adult results are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

	Worksheet 2A General Informati						Nay Roadway Inter	sections	
General Information	General Information				Location Information				
Analyst	Linde Thatcher	Linde Thatcher			Roadway			0	
Agency or Company	WSDOT	WSDOT			Intersection			Webber Canyon Roa	d at Badger Road
Date Performed	07/21/23	07/21/23			Jurisdiction			WSDOT	
Intersection	Intersection 4	Intersection 4			Analysis Year			2023	
Signalized/Unsignalized	Unsignalized	Unsignalized							
Input Data			Site Conditions				Base Conditions		
Intersection type (3ST, 4ST, 4SG)					3ST				
AADT <sub>major</sub> (veh/day)		AADT <sub>MAX</sub> =	19,500	(veh/day)	2,210				
AADT <sub>minor</sub> (veh/day)		AADT <sub>MAX</sub> =	4,300	(veh/day)	520				
Intersection skew angle (degrees) [If 4ST, does skew differ for minor legs? Else, No.]			Skew for Leg 1 (All):	0	Skew for Leg 2 (4ST only)	()	0		
Number of signalized or uncontrolled	Number of signalized or uncontrolled approaches with a left-turn lane (0, 1, 2, 3, 4)				1				0
Number of signalized or uncontrolled	ed approaches with a right	-turn lane (0, 1, 2, 3, 4)	)	•	1				0
Intersection lighting (present/not p	resent)	•		•	Present			Not Present	

1.00

1.00

Average Annual Crash History (3 or 5-yr average)

Calibration Factor, C<sub>i</sub>

Intersection crashes	KABC	Fatal and Injury Only	1.0	
intersection crashes	PDO	Property Damage Only	0.0	

	Worksheet 2B Crash Modification Factors for Rural Two-Lane Two-Way Roadway Intersections								
(1)	(2)	(3)	(4)	(5)					
CMF for Intersection Skew Angle	CMF for Left-Turn Lanes	CMF for Right-Turn Lanes	CMF for Lighting	Combined CMF					
CMF <sub>1i</sub>	CMF <sub>2i</sub>	CMF <sub>3i</sub>	CMF <sub>4i</sub>	CMF <sub>COMB</sub>					
from Equations 10-22 or 10-23	from Table 10-13	from Table 10-14	from Equation 10-24	(1)*(2)*(3)*(4)					
1.00	0.56	0.86	0.90	0.43					

	Worksheet 2C Intersection Crashes for Rural Two-Lane Two-Way Roadway Intersections									
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)			
Crash Severity Level	N	Overdispersion	Crash Severity	N spf 3ST, 4ST or 4SG by Severity	Combined CMFs		Predicted average crash frequency, N			
	N <sub>spf</sub> 3ST, 4ST or 4SG	Parameter, k	Distribution	Distribution	Combined Civirs	Calibration Factor, C	predicted int			
Crash Severity Level	from Equations 10-8, 10-9, or 10-10	from Section	from Table 10-	(2) <sub>TOTAL</sub> * (4)	from (5) of	cambration ractor, q	(5)*(6)*(7)			
	110111 Equations 10-8, 10-9, 01 10-10	10.6.2	5	(2)TOTAL (4)	Worksheet 2B					
Total	0.491	0.54	1.000	0.491	0.43	1.00	0.213			
Fatal and Injury (FI)			0.415	0.204	0.43	1.00	0.088			
Property Damage Only (PDO)			0.585	0.287	0.43	1.00	0.125			

		Worksheet 2D Crashes by	Severity Level and Collision Type	for Rural Two-Lane Two-Way Road	Intersections	
(1)	(2)	(3)	(4)	(5)	(6)	(7)
Collision Type	Proportion of Collision Type(TOTAL)	N predicted int (TOTAL) (crashes/year)	Proportion of Collision Type(FI)	N predicted int (FI) (crashes/year)	Proportion of Collision Type(PDO)	N predicted int (PDO) (crashes/year)
	from Table 10-6	(8)TOTAL from Worksheet 2C	from Table 10-6	(8)FI from Worksheet 2C	from Table 10-6	(8)PDO from Worksheet 2C
Total	1.000	0.213	1.000	0.088	1.000	0.125
		(2)x(3)total		(4)x(5) <sub>FI</sub>		(6)x(7)PDO
			SINGLE-VEHICL	E		
Collision with animal	0.019	0.004	0.008	0.001	0.026	0.003
Collision with bicycle	0.001	0.000	0.001	0.000	0.001	0.000
Collision with pedestrian	0.001	0.000	0.001	0.000	0.001	0.000
Overturned	0.013	0.003	0.022	0.002	0.007	0.001
Ran off road	0.244	0.052	0.240	0.021	0.247	0.031
Other single-vehicle collision	0.016	0.003	0.011	0.001	0.020	0.002
Total single-vehicle crashes	0.294	0.063	0.283	0.025	0.302	0.038
			MULTIPLE-VEHIO	CLE		
Angle collision	0.237	0.050	0.275	0.024	0.210	0.026
Head-on collision	0.052	0.011	0.081	0.007	0.032	0.004
Rear-end collision	0.278	0.059	0.260	0.023	0.292	0.036
Sideswipe collision	0.097	0.021	0.051	0.005	0.131	0.016
Other multiple-vehicle collision	0.042	0.009	0.050	0.004	0.033	0.004
Total multiple-vehicle crashes	0.706	0.150	0.717	0.063	0.698	0.087

Worksheet 2E Summary Results for Rural Two-Lane Two-Way Road Intersections							
(1)	(2)	(3)					
Crash severity level	Crash Severity Distribution (proportion)	Predicted average crash frequency (crashes / year)					
Crasii severity ievei	(4) from Worksheet 2C	(8) from Worksheet 2C					
Total	1.000	0.2					
Fatal and Injury (FI)	0.415	0.1					
Property Damage Only (PDO)	0.585	0.1					

				PROJECT ELEME	NT RESULTS SUM	MARY <sup>1</sup>			
	Total Crashes/yr (KABCO)			Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)		
Summary for the project element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
	0.2	0.5	0.3	0.1	0.2	0.1	0.1	0.3	0.2

special route. When the project element is not included in the analysis the results will all de zeros. In adultiresults are usually displayed.

The values in this table are **rounded** values. For unrounded values, refer to Workdheet 1L.

### PROJECT SAFETY PERFORMANCE SUMMARY REPORT

### **General Information**

Project Name Horse Heaven Wind Farm

Project Description Safety Analysis (Full Mitigation)
Reference Number 143-67639-23007

Analyst
Agency/Company

Linde Thatcher WSDOT

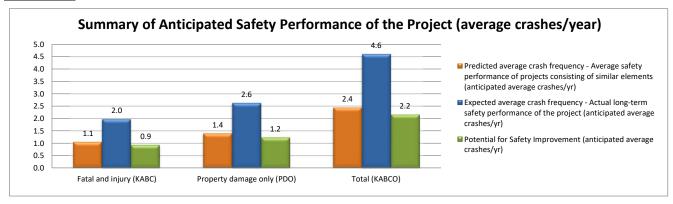
Contact Email linde.thatcher@tetratech.com
Contact Phone 508-786-2222

Contact Phone 508-786-2222

Date Completed 07/21/23

Years of crash data incorporated into the analysis: 5

### PROJECT SUMMARY



	Total Crashes/yr (KABCO)		Fatal and Injury Crashes/yr (KABC)			Property Damage Only Crashes/yr (PDO)			
Project Element	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement	Predicted average crash frequency	Expected average crash frequency	Potential for Improvement
	N <sub>predicted (KABCO)</sub>	N <sub>expected (KABCO)</sub>		N <sub>predicted (KABC)</sub>	N <sub>expected (KABC)</sub>		N <sub>predicted (O)</sub>	N <sub>expected (O)</sub>	
INDIVIDUAL INTERSECTIONS									
Intersection 1	0.5	1.2	0.7	0.2	0.5	0.3	0.3	0.7	0.4
Intersection 2	0.8	1.3	0.6	0.3	0.6	0.3	0.4	0.8	0.3
Intersection 3	1.0	1.5	0.6	0.4	0.7	0.2	0.5	0.9	0.3
Intersection 4	0.2	0.5	0.3	0.1	0.2	0.1	0.1	0.3	0.2
COMBINED (sum of column)	2.4	4.6	2.2	1.1	2.0	0.9	1.4	2.6	1.2

### PROJECT SUMMARY -- Site-Specific EB Method Summary Results for Rural 2-Lane Roads

	N predicted(PROJECT)	N expected (PROJECT)	N potential for improvement (PROJECT)	
Crash severity level	performance of projects consisting of similar elements	Expected average crash frequency	Potential for Safety Improvement (anticipated average crashes/yr)	
	(anticipated average crashes/yr)	(,		
Fatal and injury (KABC)	1.1	2.0	0.9	
Property damage only (PDO)	1.4	2.6	1.2	
Total (KABCO)	2.4	4.6	2.2	

HSM1 Extended Spreadsheet for Part C Chapter 10 v.9.1

### **Discussion of Results**

### Given the potential effects of project characteristics on safety performance, results indicate that:

- 1. It is anticipated that the project will, on average, experience 4.6 crashes per year (2 fatal and injury crashes per year; and 2.6 property damage only crashes per year).
- 2. A similar project is anticipated, on average, to experience 2.4 crashes per year (1.1 fatal and injury crashes per year; and 1.4 property damage only crashes per year).
- 3. It is anticipated the project has, on average, a potential for safety improvement of 2.2 crashes per year (0.9 fatal and injury crashes per year; and 1.2 property damage only crashes per year).

# APPENDIX D: SIGHT DISTANCE CALCULATIONS

### Location: Route 221 at Sellards Road

### **STOPPING SIGHT DISTANCE:**

### STOPPING SIGHT DISTANCE FROM NORTH

Inputs	
--------	--

V=speed, mph V= 72 (85th percentile speed) G=percent of grade G= -1.1 (%) t=brake reaction time t= 2.5 a=deceleration rate, ft/sec<sup>2</sup> a= 11.2

### Calculations

Brake Reaction Distance	1.47Vt	265 feet	
Braking Distance	<u>V<sup>2</sup>/30((a/32.2)+G)</u>	<u>513.0</u> feet	
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	780 feet	

### STOPPING SIGHT DISTANCE FROM SOUTH

Inputs

### Calculations

Brake Reaction Distance	1.47Vt	261 feet	
Braking Distance	<u>V<sup>2</sup>/30((a/32.2)+G)</u>	<u>466.3</u> feet	
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	730 feet	

### Location: Route 14 at Route 221

### **STOPPING SIGHT DISTANCE:**

### STOPPING SIGHT DISTANCE FROM EAST

Inputs
--------

### Calculations

Brake Reaction Distance <u>Braking Distance</u>	1.47Vt <u>V<sup>2</sup>/30((a/32.2)+G)</u>	239 feet <u>410.8</u> feet	
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	650 feet	

### STOPPING SIGHT DISTANCE FROM WEST

Inputs

V=speed, mph V= 65 (Posted Speed Limit)
G=percent of grade G= 0.9 (%)
t=brake reaction time t= 2.5
a=deceleration rate, ft/sec² a= 11.2

### Calculations

Brake Reaction Distance  Braking Distance	1.47Vt <u>V<sup>2</sup>/30((a/32.2)+G)</u>	239 feet <u>394.7</u> feet
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	635 feet

# Location: Route 14 at S. Plymouth Road

### **STOPPING SIGHT DISTANCE:**

### STOPPING SIGHT DISTANCE FROM EAST

1		4_
m	pu	เร

V=speed, mph	V=	55	(Posted Speed Limit)
G=percent of grade	G=	-0.3	(%)
t=brake reaction time	t=	2.5	
a=deceleration rate, ft/sec <sup>2</sup>	a=	11.2	

### Calculations

Brake Reaction Distance <u>Braking Distance</u>	1.47Vt <u>V<sup>2</sup>/30((a/32.2)+G)</u>	202 feet <u>292.4</u> feet	
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	495 feet	

### STOPPING SIGHT DISTANCE FROM WEST

Inputs

V=speed, mph	V=	55	(Posted Speed Limit)
G=percent of grade	G=	0.6	(%)
t=brake reaction time	t=	2.5	
a=deceleration rate, ft/sec <sup>2</sup>	a=	11.2	

### Calculations

Brake Reaction Distance	1.47Vt	202 feet
Braking Distance	<u>V²/30((a/32.2)+G)</u>	285.0 feet
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	490 feet

# Location: Webber Canyon Road at Badger Road

### **STOPPING SIGHT DISTANCE:**

### STOPPING SIGHT DISTANCE FROM NORTH

vu	

V=speed, mph	V=	40	(Posted Speed Limit)
G=percent of grade	G=	-0.5	(%)
t=brake reaction time	t=	2.5	
a=deceleration rate, ft/sec <sup>2</sup>	a=	11.2	

### Calculations

Brake Reaction Distance	1.47Vt	147 feet
Braking Distance	<u>V²/30((a/32.2)+G)</u>	<u>155.6</u> feet
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	305 feet

### STOPPING SIGHT DISTANCE FROM SOUTH

Inputs

V=speed, mph	V=	50	(Posted Speed Limit)
G=percent of grade	G=	-0.3	(%)
t=brake reaction time	t=	2.5	
a=deceleration rate, ft/sec <sup>2</sup>	a=	11.2	

### Calculations

Brake Reaction Distance <u>Braking Distance</u>	1.47Vt <u>V<sup>2</sup>/30((a/32.2)+G)</u>	184 feet <u>241.7</u> feet
Stopping Sight Distance =	1.47Vt + V <sup>2</sup> /30[(a/32.2)+G]	430 feet

#### **APPENDIX E: BACKGROUND TRAFFIC GROWTH RATE**

#### **Background Traffic Growth Rate**Horse Heaven Wind Farm, Benton County Washington

	I-82 Average Annual	
	Daily Traffic Volume <sup>1</sup>	
2018	20971	
2019	20939	
2020	18890	
2021	22093	2018 to 2022 Percent Growth
2022	22011	1.0%

Note 1) AADT is provided by WSDOT using the Traffic Count Database System (TCDS) at Continuous Count Station P09 (North of Coffin Road)

#### **APPENDIX F: PROJECT TRIP DISTANCE CALCULATIONS**

										<u>.</u>	diation Gravity		/N YARD #1 - LC	CAL TRAVEL ROU	TES			1	
								To/From	ı West			To/Fror	n North			To/From South		1	
						Effective													
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
County		State	Population	Time to Site	Zone	Zone	Population			Road								100% CHECK	Total
Benton County	101	WA	4,873	30 mins	100.0%	4873	0.661%					100%						100%	0.7%
Benton County	102.01	WA	5,718	30 mins	100.0%	5718	0.776%					100%						100%	0.8%
Benton County	102.03	WA	4,637	30 mins	100.0%	4637	0.629%					100%						100%	0.6%
Benton County	102.04 103	WA WA	2,830 6,153	30 mins	100.0% 100.0%	2830 6153	0.384% 0.835%					100%						100% 100%	0.4%
Benton County Benton County	103	WA	3,637	30 mins	100.0%	3637	0.833%					100%						100%	0.5%
Benton County	105	WA	3,271	30 mins	100.0%	3271	0.444%					100%						100%	0.4%
Benton County	106	WA	4,930	30 mins	100.0%	4930	0.669%					100%						100%	0.7%
Benton County	107.01	WA	2,021	35 mins	98.3%	1986	0.270%					100%						100%	0.3%
Benton County Benton County	107.03 107.05	WA WA	3,424 5,741	35 mins 35 mins	98.3% 98.3%	3365 5643	0.457% 0.766%					100%						100% 100%	0.5% 0.8%
Benton County	107.07	WA	3,775	30 mins	100.0%	3775	0.512%					100%						100%	0.5%
Benton County	107.08	WA	4,332	30 mins	100.0%	4332	0.588%					100%						100%	0.6%
Benton County	108.07	WA	1,762	25 mins	100.0%	1762	0.239%					100%						100%	0.2%
Benton County Benton County	108.09 108.10	WA WA	6,391 4,861	25 mins 25 mins	100.0% 100.0%	6391 4861	0.867% 0.660%	<b> </b>		<del> </del>	<del>                                     </del>	100%		<del>                                     </del>		<del>                                     </del>		100% 100%	0.9% 0.7%
Benton County	108.10	WA	5,275	20 Mins or less	100.0%	5275	0.860%					100%				+ +		100%	0.7%
Benton County	108.14	WA	5,186	20 Mins or less	100.0%	5186	0.704%			<u> </u>		100%						100%	0.7%
Benton County	108.15	WA	8,567	20 Mins or less	100.0%	8567	1.163%					50%	50%					100%	1.2%
Benton County	108.16	WA	5,589	20 Mins or less	100.0%	5589	0.759%			1		50%	50%			<del>                                     </del>		100%	0.8%
Benton County Benton County	108.17 108.18	WA WA	6,198 3,274	25 mins 25 mins	100.0% 100.0%	6198 3274	0.841% 0.444%					100%						100% 100%	0.8%
Benton County	108.19	WA	3,304	25 mins	100.0%	3304	0.448%					100%						100%	0.4%
Benton County	108.20	WA	3,737	25 mins	100.0%	3737	0.507%					50%	50%					100%	0.5%
Benton County	109.01	WA	6,251	25 mins	100.0%	6251	0.848%					50%	50%					100%	0.8%
Benton County	109.02	WA	5,698	20 Mins or less	100.0%	5698	0.773%						100%	F00/				100%	0.8%
Benton County Benton County	110.01 110.02	WA WA	6,025 4,859	25 mins 20 Mins or less	100.0% 100.0%	6025 4859	0.818% 0.659%						50% 100%	50%				100% 100%	0.8%
Benton County	111	WA	7,879	20 Mins or less	100.0%	7879	1.069%						50%	50%				100%	1.1%
Benton County	112.01	WA	4,267	20 Mins or less	100.0%	4267	0.579%						50%	50%				100%	0.6%
Benton County	112.02	WA	3,323	20 Mins or less	100.0%	3323	0.451%						50%	50%				100%	0.5%
Benton County Benton County	113 114.01	WA WA	5,040 3,580	25 mins 20 Mins or less	100.0% 100.0%	5040 3580	0.684%						50% 50%	50% 50%				100% 100%	0.7% 0.5%
Benton County	114.01	WA	5,415	20 Mins or less	100.0%	5415	0.480%						30%	100%				100%	0.7%
Benton County	115.01	WA	6,443	25 mins	100.0%	6443	0.874%								100%			100%	0.9%
Benton County	115.04	WA	2,866	20 Mins or less	100.0%	2866	0.389%							100%				100%	0.4%
Benton County Benton County	115.05 115.06	WA WA	4,177 7,519	20 Mins or less 20 Mins or less	100.0% 100.0%	4177 7519	0.567% 1.020%						100%	100%				100% 100%	0.6% 1.0%
Benton County	117.01	WA	3,012	40 Mins	96.5%	2906	0.394%					100%		100%				100%	0.4%
Benton County	117.02	WA	5,132	40 Mins	96.5%	4952	0.672%					100%						100%	0.7%
Benton County	118.01	WA	3,655	45 mins	94.6%	3457	0.469%					100%						100%	0.5%
Benton County	118.02	WA	2,665	45 mins	94.6%	2520	0.342%			1		100%				1		100%	0.3%
Benton County Benton County	119 120	WA WA	6,325 0	30 Mins 40 mins	100.0% 96.5%	6325 0	0.858%					100%				+ +		100% 0%	0.9%
Franklin County	201.01	WA	1,828	30 Mins	100.0%	1828	0.248%			Ì			50%	50%				100%	0.2%
Franklin County	201.02	WA	6,609	30 Mins	100.0%	6609	0.897%						100%					100%	0.9%
Franklin County	201.03	WA	3,811	30 Mins	100.0%	3811	0.517%			ļ			75%	25%				100%	0.5%
Franklin County Franklin County	202.01 202.02	WA WA	2,201 4,142	25 Mins 30 Mins	100.0% 100.0%	2201 4142	0.299% 0.562%			1			75% 75%	25% 25%		<del>                                     </del>		100% 100%	0.3%
Franklin County Franklin County	202.02	WA	6,088	30 Mins	100.0%	6088	0.826%			1			100%	23/0		<del>                                     </del>		100%	0.8%
Franklin County	204.01	WA	1,065	25 Mins	100.0%	1065	0.145%						100%					100%	0.1%
Franklin County	204.02	WA	1,101	25 Mins	100.0%	1101	0.149%		·			· · · · · · · · · · · · · · · · · · ·	100%		· · · · · · · · · · · · · · · · · · ·		·	100%	0.1%
Franklin County Franklin County	204.03 204.04	WA	3,611	25 Mins	100.0%	3611	0.490%			1			100% 100%			+ + +		100% 100%	0.5%
Franklin County Franklin County	204.04	WA WA	2,928 5,161	25 Mins 30 Mins	100.0% 100.0%	2928 5161	0.397% 0.700%			1		50%	50%			+		100%	0.4%
Franklin County	205	WA	3,296	30 Mins	100.0%	3296	0.447%					3070	100%					100%	0.4%
Franklin County	205	WA	6,522	30 Mins	100.0%	6522	0.885%						100%					100%	0.9%
Franklin County	206	WA	4,546	35 Mins	98.3%	4468	0.606%			ļ		50%	50%					100%	0.6%
Franklin County Franklin County	206 206	WA WA	9,548 8,729	35 Mins 35 Mins	98.3% 98.3%	9385 8580	1.274% 1.164%			<del> </del>		50% 50%	50% 50%			<del>                                     </del>		100% 100%	1.3%
Franklin County	206	WA	6,719	35 Mins	98.3%	6604	0.896%			1		50%	50%			<del>                                     </del>		100%	0.9%
Franklin County	206	WA	6,601	35 Mins	98.3%	6488	0.881%			<u> </u>		50%	50%					100%	0.9%
Morrow County	9701.01	OR	5,034	45 Mins	94.6%	4761	0.646%									100%		100%	0.6%
Morrow County	9701.02	OR	3,676	30 Mins	100.0%	3676	0.499%									100%		100%	0.5%

												LAYDOW	VN YARD #1 - LO	CAL TRAVEL ROU	TES			
								To/From	West			To/From	m North			To/From South		
						Effective												
	Census		Total	Estimated Drive	% Population in Drive Time	Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road Roue 221		
County		State	Population	Time to Site	Zone	Zone	Population			Nodu							100% CHECK	Total
Morrow County	9702	OR	3,254	0	88.1%	2867	0.389%									100%	100%	0.4%
Umatilla County	9400	OR	3,072	0	88.1%	2706	0.367%						250/			100%	100%	0.4%
Umatilla County Umatilla County	9501 9502.01	OR OR	4,659 3,712	0	76.9% 76.9%	3581 2853	0.486% 0.387%			+			25% 25%			75% 75%	100% 100%	0.5%
Umatilla County	9502.02	OR	4,043	0	76.9%	3108	0.422%						25%			75%	100%	0.4%
Umatilla County	9503	OR	3,371	0	82.9%	2796	0.379%									100%	100%	0.4%
Umatilla County Umatilla County	9504 9505	OR OR	6,038 4,754	0	88.1% 88.1%	5319 4188	0.722% 0.568%			<del> </del>						100%	100% 100%	0.7% 0.6%
Umatilla County	9506.01	OR	2,600	0	88.1%	2290	0.311%			1						100%	100%	0.3%
Umatilla County	9506.02	OR	3,212	0	88.1%	2830	0.384%									100%	100%	0.4%
Umatilla County	9507	OR	2,513	55 Mins	90.4%	2272	0.308%			<u> </u>						100%	100%	0.3%
Umatilla County Umatilla County	9508 9509	OR OR	7,508 5,133	25 Mins 20 Mins or less	100.0% 100.0%	7508 5133	1.019% 0.697%									100%	100% 100%	1.0% 0.7%
Umatilla County	9510	OR	6,224	25 Mins	100.0%	6224	0.845%									100%	100%	0.8%
Umatilla County	9511	OR	6,018	25 Mins	100.0%	6018	0.817%									100%	100%	0.8%
Umatilla County Umatilla County	9512.01 9512.02	OR OR	6,207 4,040	30 Mins 30 Mins	100.0% 100.0%	6207 4040	0.842% 0.548%			+						100%	100% 100%	0.8% 0.5%
Umatilla County	9513	OR	3,989	30 Mins	100.0%	3989	0.541%			1			1			100%	100%	0.5%
Umatilla County	9514	OR	2,416	1:10	82.9%	2004	0.272%									100%	100%	0.3%
Union County	9701 9702	OR OR	3,238 3,376	2:00 1:55	23.1% 36.7%	749 1238	0.102% 0.168%			1						100%	100% 100%	0.1% 0.2%
Union County Union County	9702	OR	2,427	1:55	36.7%	890	0.168%			+				+		100%	100%	0.2%
Union County	9704	OR	2,732	1:50	46.3%	1264	0.172%									100%	100%	0.2%
Union County	9705	OR	3,196	1:35	65.0%	2076	0.282%									100%	100%	0.3%
Union County Union County	9706 9707	OR OR	3,927 3,416	1:50 1:40	46.3% 59.8%	1817 2043	0.247% 0.277%									100%	100% 100%	0.2%
Union County	9708	OR	3,943	1:40	59.8%	2358	0.320%			1						100%	100%	0.3%
Adams County	9501	WA	2,577	1:30	69.4%	1789	0.243%						100%				100%	0.2%
Adams County	9502	WA	1,794	1:20	76.9%	1379	0.187%			-			100%				100%	0.2%
Adams County Adams County	9503.01 9503.02	WA WA	1,790 2,738	1:20 1:20	76.9% 76.9%	1376 2104	0.187% 0.286%					50%	100% 50%				100% 100%	0.2%
Adams County	9503.03	WA	2,555	1:15	80.0%	2045	0.278%					50%	50%				100%	0.3%
Adams County	9504	WA	3,100	1:15	80.0%	2481	0.337%					50%	50%				100%	0.3%
Adams County Columbia County	9505 9602	WA WA	5,799 3,969	1:15 1:30	80.0% 69.4%	4642 2755	0.630% 0.374%					50%	50% 100%				100% 100%	0.6% 0.4%
Franklin County	207	WA	1,277	1:15	80.0%	1022	0.139%			†			100%				100%	0.1%
Franklin County	208.01	WA	3,401	1:00	88.1%	2996	0.407%						100%				100%	0.4%
Franklin County	208.02	WA	3,129	1:00	88.1%	2756	0.374%			<u> </u>			100%				100%	0.4%
Grant County Grant County	101 102	WA WA	3,610 3,382	greater than 2:00 greater than 2:00	0.0%	0	0.000%						100% 100%				100%	0.0%
Grant County	103	WA	5,425	greater than 2:00	0.0%	0	0.000%						100%				100%	0.0%
Grant County	104.01	WA	3,148	greater than 2:00	0.0%	0	0.000%						100%				100%	0.0%
Grant County Grant County	104.02 105	WA WA	5,495 3,270	greater than 2:00 greater than 2:00	0.0%	0	0.000%			+		50%	100% 50%				100%	0.0%
Grant County	106	WA	7,614	greater than 2:00	0.0%	0	0.000%			1		50%	50%				100%	0.0%
Grant County	107	WA	3,186	1:45	53.7%	1712	0.232%			1		50%	50%				100%	0.2%
Grant County Grant County	108 109.01	WA WA	5,398 1,679	1:30 1:30	69.4% 69.4%	3747 1165	0.509% 0.158%			<del>                                     </del>			100% 100%			<del>                                     </del>	100% 100%	0.5% 0.2%
Grant County  Grant County	109.01	WA	5,281	1:30	69.4%	3666	0.158%						100%			<del>                                     </del>	100%	0.2%
Grant County	109.04	WA	6,136	1:30	69.4%	4259	0.578%						100%				100%	0.6%
Grant County	110.01	WA	5,723	1:30	69.4%	3973	0.539%						100%				100%	0.5%
Grant County Grant County	110.02 111.01	WA WA	6,225 4,657	1:30 1:30	69.4% 69.4%	4321 3233	0.586% 0.439%			+			100% 100%			<del>                                     </del>	100% 100%	0.6%
Grant County	111.02	WA	2,891	1:30	69.4%	2007	0.272%			1			100%				100%	0.3%
Grant County	112	WA	7,100	1:45	53.7%	3814	0.518%					50%	50%				100%	0.5%
Grant County Grant County	113 114.01	WA WA	3,367 2,249	1:20 1:20	76.9% 76.9%	2588 1729	0.351% 0.235%			<del> </del>			100% 100%			<del>                                     </del>	100% 100%	0.4%
Grant County  Grant County	114.01	WA	4,871	1:15	80.0%	3899	0.235%			+		100%	100%	+		<del>                                     </del>	100%	0.2%
Grant County	114.04	WA	963	1:30	69.4%	668	0.091%					100%					100%	0.1%
Grant County	114.05	WA	3,019	1:30	69.4%	2096	0.284%			<u> </u>		50%	50%				100%	0.3%
Grant County Kittitas County	114.06 9751.01	WA WA	3,185 2,363	1:30 greater than 2:00	69.4% 0.0%	2211 0	0.300%			+		50% 100%	50%				100% 100%	0.3%
Kittitas County  Kittitas County	9751.01	WA	1,290	greater than 2:00	0.0%	0	0.000%					100%				<del>                                     </del>	100%	0.0%
Kittitas County	9751.03	WA	1,444	greater than 2:00	0.0%	0	0.000%					100%					100%	0.0%

										<u> </u>	diation Gravity i		VN YARD #1 - LC	CAL TRAVEL ROU	TES			1	
								To/Fron	n West				m North			To/From South			
						Effective													
				5.1	-	Population in	0/ . 5 = 1	Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
County	Census Tract	State I	Total Population	Estimated Drive Time to Site	in Drive Time Zone	Drive Time Zone	% of Total Population			Road				, , ,				100% CHECK	Total
Kittitas County		WA	1,644	greater than 2:00	0.0%	0	0.000%					100%						100%	0.0%
Kittitas County		WA	3,364	greater than 2:00	0.0%	0	0.000%					100%						100%	0.0%
Kittitas County	9752.02	WA	1,395	1:45	53.7%	749	0.102%					100%						100%	0.1%
Kittitas County	9752.03	WA	1,098	1:45	53.7%	590	0.080%					100%						100%	0.1%
Kittitas County	9753	WA	5,316	2:00	23.1%	1230	0.167%					100%						100%	0.2%
Kittitas County Kittitas County	9754.02 9754.03	WA WA	4,713 2,921	1:45 1:45	53.7% 53.7%	2532 1569	0.344% 0.213%					100%						100% 100%	0.3% 0.2%
Kittitas County	9754.04	WA	5,145	1:45	53.7%	2764	0.375%					100%						100%	0.4%
Kittitas County	9755	WA	5,956	1:45	53.7%	3200	0.434%					100%						100%	0.4%
Kittitas County	9756	WA	2,790	1:45	53.7%	1499	0.203%					100%						100%	0.2%
Kittitas County	9757	WA	4,708	2:00	23.1%	1089	0.148%	500/				100%						100%	0.1%
Klickitat County Klickitat County	9501.01 9501.02	WA WA	1,538 3,507	1:00 1:20	88.1% 76.9%	1355 2696	0.184% 0.366%	50%				50% 100%						100% 100%	0.2% 0.4%
Klickitat County	9501.03	WA	4,189	1:45	53.7%	2251	0.305%					100%				100%		100%	0.3%
Klickitat County	9502	WA	4,383	greater than 2:00	0.0%	0	0.000%									100%		100%	0.0%
Klickitat County	9503.01	WA	3,465	greater than 2:00	0.0%	0	0.000%									100%		100%	0.0%
Klickitat County	9503.02	WA	5,396	greater than 2:00	0.0%	0	0.000%						40001			100%		100%	0.0%
Walla Walla County Walla Walla County	9200 9201	WA WA	6,176 5,165	1:00 1:00	88.1% 88.1%	5441 4550	0.738% 0.618%			+	<del>                                     </del>		100% 75%	+		25%		100% 100%	0.7% 0.6%
Walla Walla County	9201	WA	4,715	1:15	88.1%	3774	0.512%			+	+		50%	+		50%		100%	0.5%
Walla Walla County	9203.01	WA	3,243	1:15	80.0%	2596	0.352%						50%			50%		100%	0.4%
Walla Walla County	9203.02	WA	5,434	1:15	80.0%	4350	0.590%						50%			50%		100%	0.6%
Walla Walla County	9204	WA	2,640	1:15	80.0%	2113	0.287%						50%			50%		100%	0.3%
Walla Walla County Walla Walla County	9205 9206	WA WA	2,959 6,205	1:15 1:15	80.0% 80.0%	2368 4967	0.321% 0.674%						50% 50%			50%		100% 100%	0.3% 0.7%
Walla Walla County		WA	3,545	1:15	80.0%	2838	0.874%						50%			50%		100%	0.7%
Walla Walla County		WA	4,293	1:15	80.0%	3436	0.466%						50%			50%		100%	0.5%
Walla Walla County	9208.01	WA	4,945	1:15	80.0%	3958	0.537%						50%			50%		100%	0.5%
Walla Walla County		WA	3,223	1:15	80.0%	2580	0.350%						50%			50%		100%	0.4%
Walla Walla County	9209.01 9209.02	WA WA	4,134 5,491	1:15 1:15	80.0% 80.0%	3309 4395	0.449% 0.596%						50% 50%			50%		100% 100%	0.4%
Walla Walla County Yakima County	1	WA	3,072	1:15	80.0%	2459	0.334%					100%	30%			50%		100%	0.8%
Yakima County	2	WA	5,595	1:15	80.0%	4478	0.608%					100%						100%	0.6%
Yakima County	3.01	WA	2,473	1:15	80.0%	1979	0.269%					100%						100%	0.3%
Yakima County	3.02	WA	2,283	1:15	80.0%	1827	0.248%					100%						100%	0.2%
Yakima County	4.01	WA	5,958	1:15	80.0%	4769	0.647%					100%						100%	0.6%
Yakima County Yakima County	4.02 5	WA WA	2,407 4,599	1:15 1:15	80.0% 80.0%	1927 3681	0.261% 0.500%					100%						100% 100%	0.3% 0.5%
Yakima County	6	WA	5,696	1:15	80.0%	4559	0.619%					100%						100%	0.6%
Yakima County	7	WA	7,077	1:15	80.0%	5665	0.769%					100%						100%	0.8%
Yakima County	8	WA	4,484	1:15	80.0%	3589	0.487%					100%						100%	0.5%
Yakima County	9.02	WA	4,507	1:20	76.9%	3464	0.470%					100%				<del>                                     </del>		100% 100%	0.5%
Yakima County Yakima County	9.03 9.04	WA WA	4,008 3,332	1:20 1:20	76.9% 76.9%	3081 2561	0.418% 0.348%			1		100%				<del>                                     </del>		100%	0.4%
Yakima County	10	WA	6,499	1:20	76.9%	4995	0.678%			†		100%						100%	0.7%
Yakima County	11	WA	7,361	1:15	80.0%	5892	0.800%					100%						100%	0.8%
Yakima County	12.01	WA	4,723	1:15	80.0%	3780	0.513%					100%						100%	0.5%
Yakima County	12.02	WA	7,051	1:15	80.0%	5644	0.766%					100%				+ + +		100%	0.8%
Yakima County Yakima County	13 14	WA WA	2,653 4,099	1:10 1:10	82.9% 82.9%	2201 3400	0.299% 0.461%			1		100%				+ +		100% 100%	0.3% 0.5%
Yakima County	15.02	WA	2,658	1:10	82.9%	2205	0.461%			1		100%				<del>                                     </del>		100%	0.3%
Yakima County	15.03	WA	4,558	1:10	82.9%	3781	0.513%					100%						100%	0.5%
Yakima County	15.04	WA	2,894	1:10	82.9%	2401	0.326%					100%			<u></u>			100%	0.3%
Yakima County	16.01	WA	2,537	1:15	80.0%	2031	0.276%					100%						100%	0.3%
Yakima County Yakima County	16.02 17.01	WA WA	8,633 3,654	1:15 1:10	80.0% 82.9%	6910 3031	0.938% 0.411%					100%						100% 100%	0.9%
Yakima County Yakima County	17.01	WA	6,565	1:10	82.9% 82.9%	5446	0.411%			1		100%						100%	0.4%
Yakima County	18.01	WA	4,419	40 Mins	96.5%	4264	0.579%			1		100%						100%	0.6%
Yakima County	18.02	WA	2,933	40 Mins	96.5%	2830	0.384%					100%						100%	0.4%
Yakima County	19.01	WA	3,680	40 Mins	96.5%	3551	0.482%					100%			<u></u>			100%	0.5%
Yakima County	19.02	WA	6,678	40 Mins	96.5%	6443	0.874%			1		100%				1		100%	0.9%
Yakima County Yakima County	20.03 20.04	WA WA	5,057 4,734	45 Mins 45 Mins	94.6% 94.6%	4783 4477	0.649% 0.608%					100%				+ +		100% 100%	0.6% 0.6%
Yakima County Yakima County	20.04	WA	2,544	45 Mins	94.6%	2406	0.808%			+		100%				<del>                                     </del>		100%	0.6%
a country	_0.00		-, •	.515	3/0		2.02.70				1		1	ı				200,0	5.576

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											LAYDOV	VN YARD #1 - L	OCAL TRAVEL ROU	TES					•
							To/Fro	m West			To/Fro	m North				To/From South		1	,
County	Census Tract	Tot State Popul		_		% of Total Population	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221	100% CHECK	Total
Yakima County	20.06	WA 6,9	34 45 Mins	94.6%	6558	0.890%					100%							100%	0.9%
Yakima County	21.01	WA 2,4	68 45 Mins	94.6%	2334	0.317%					100%							100%	0.3%
Yakima County	21.03	WA 2,7	09 45 Mins	94.6%	2562	0.348%					100%							100%	0.3%
Yakima County	21.04	WA 5,0	99 50 Mins	92.6%	4719	0.640%					100%							100%	0.6%
Yakima County	22.01	WA 5,1	53 55 Mins	90.4%	4658	0.632%					100%							100%	0.6%
Yakima County	22.02	WA 2,0	17 1:00	88.1%	1777	0.241%					100%							100%	0.2%
Yakima County	27.01	WA 3,4	66 40 Mins	96.5%	3344	0.454%	100%											100%	0.5%
Yakima County	28.01	WA 5,6	27 1:20	76.9%	4325	0.587%					100%							100%	0.6%
Yakima County	28.03	WA 5,8	09 1:20	76.9%	4465	0.606%					100%							100%	0.6%
Yakima County	28.04	WA 3,6	07 1:20	76.9%	2772	0.376%					100%							100%	0.4%
Yakima County	29	WA 7,1	31 1:30	69.4%	4950	0.672%					100%							100%	0.7%
Yakima County	30.02	WA 4,0	1:30	69.4%	2836	0.385%					100%							100%	0.4%
Yakima County	30.03	WA 1,7	24 greater than 2:	0.0%	0	0.000%					100%							100%	0.0%
Yakima County	30.04	WA 2,6	1:40	59.8%	1581	0.215%					100%							100%	0.2%
Yakima County	31	WA 5,2	97 1:15	80.0%	4240	0.575%					100%							100%	0.6%
Yakima County	32	WA 7,0	12 1:15	80.0%	5613	0.762%					100%							100%	0.8%
Yakima County	34	WA 5,2		80.0%	4185	0.568%					100%							100%	0.6%
Yakima County	9400.01	WA 6,5	1:10	82.9%	5420	0.736%					100%							100%	0.7%
Yakima County	9400.02	WA 4,7		88.1%	4195	0.569%					100%							100%	0.6%
Yakima County	9400.03	WA 3,2		80.0%	2635	0.358%					100%							100%	0.4%
Yakima County	9400.05	WA 4,7		88.1%	4207	0.571%					100%							100%	0.6%
Yakima County	9400.06	WA 4,7		88.1%	4192	0.569%					100%			·				100%	0.6%
Yakima County	9400.07	WA 3,4		82.9%	2861	0.388%					100%							100%	0.4%
Yakima County	9400.08	WA 2,1	49 1:10	82.9%	1783	0.242%					100%							100%	0.2%
		Total 919,	798		736,840	100.00%	0.55%	0.00%	0.00%	0.00%	52.26%	25.63%	4.66%	0.87%	16.03%	0.00%	0.00%	100%	100.00%
						Use	1%	0%	0%	0%	52%	25%	5%	1%	16%	0%	0%	100%	1

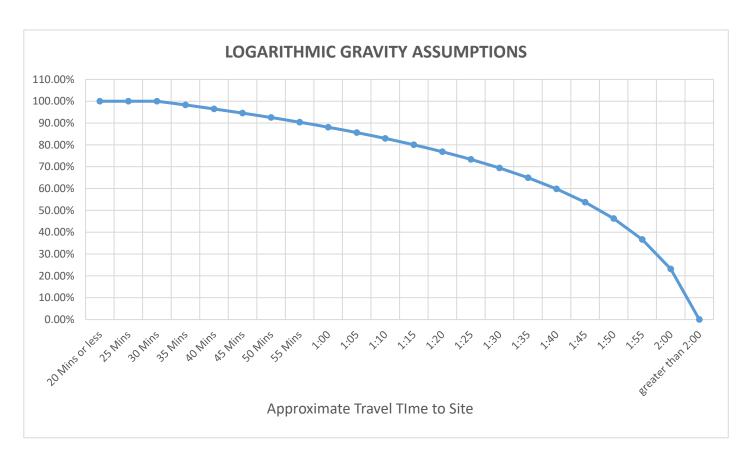
											LAYDOV	VN YARD #2 - LO	OCAL TRAVEL ROU	TES				
							To/From	West			To/Fro	m North			To/From South			
					Effective													
	Census	Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road	Roue 221		
County		State Population	Time to Site	Zone	Zone	Population			Road								100% CHECK	Total
Benton County	101	WA 4,873	30 mins	100.0%	4873	0.660%				100%							100%	0.7%
Benton County	102.01	WA 5,718	30 mins	100.0%	5718	0.775%				100%							100%	0.8%
Benton County	102.03	WA 4,637	30 mins	100.0%	4637	0.629%				100%							100%	0.6%
Benton County	102.04	WA 2,830	30 mins	100.0%	2830	0.384%				100%							100%	0.4%
Benton County Benton County	103 104	WA 6,153 WA 3,637	30 mins 30 mins	100.0% 100.0%	6153 3637	0.834% 0.493%				100% 100%					+ +		100% 100%	0.8%
Benton County	105	WA 3,037	25 mins	100.0%	3271	0.443%				100%							100%	0.4%
Benton County	106	WA 4,930	25 mins	100.0%	4930	0.668%				100%							100%	0.7%
Benton County	107.01	WA 2,021	25 mins	100.0%	2021	0.274%			100%								100%	0.3%
Benton County	107.03	WA 3,424	30 mins	100.0%	3424	0.464%			100%	1000/							100%	0.5%
Benton County Benton County	107.05 107.07	WA 5,741 WA 3,775	30 mins 30 mins	100.0% 100.0%	5741 3775	0.778% 0.512%			33%	100% 34%	33%						100% 100%	0.8%
Benton County	107.07	WA 4,332	30 mins	100.0%	4332	0.512%			33%	34%	33%						100%	0.5%
Benton County	108.07	WA 1,762	25 mins	100.0%	1762	0.239%			33%	34%	33%				<u> </u>		100%	0.2%
Benton County	108.09	WA 6,391	20 Mins or less	100.0%	6391	0.866%				100%	·			·			100%	0.9%
Benton County	108.10	WA 4,861	20 Mins or less	100.0%	4861	0.659%			1	75%	25%	1	1		1		100%	0.7%
Benton County Benton County	108.11 108.14	WA 5,275 WA 5,186	20 Mins or less 20 Mins or less	100.0% 100.0%	5275 5186	0.715% 0.703%				100% 100%							100% 100%	0.7%
Benton County	108.14	WA 8,567	20 Mins or less	100.0%	8567	1.161%				100%					+ +		100%	1.2%
Benton County	108.16	WA 5,589	20 Mins or less	100.0%	5589	0.758%				100%					<u> </u>		100%	0.8%
Benton County	108.17	WA 6,198	20 Mins or less	100.0%	6198	0.840%				75%	25%						100%	0.8%
Benton County	108.18	WA 3,274	25 mins	100.0%	3274	0.444%				75%	25%						100%	0.4%
Benton County Benton County	108.19 108.20	WA 3,304 WA 3,737	20 Mins or less 25 mins	100.0% 100.0%	3304 3737	0.448% 0.507%				75% 75%	25% 25%				+ +		100% 100%	0.4%
Benton County	109.01	WA 6,251	20 Mins or less	100.0%	6251	0.847%				75%	25%						100%	0.8%
Benton County	109.02	WA 5,698	20 Mins or less	100.0%	5698	0.772%				75%	25%						100%	0.8%
Benton County	110.01	WA 6,025	25 mins	100.0%	6025	0.817%				50%		50%					100%	0.8%
Benton County	110.02	WA 4,859	20 Mins or less	100.0%	4859	0.659%				50%		50%	F00/				100%	0.7%
Benton County Benton County	111 112.01	WA 7,879 WA 4,267	20 Mins or less 20 Mins or less	100.0% 100.0%	7879 4267	1.068% 0.578%						50% 50%	50% 50%		+ +		100% 100%	1.1% 0.6%
Benton County	112.02	WA 3,323	20 Mins or less	100.0%	3323	0.450%						50%	50%				100%	0.5%
Benton County	113	WA 5,040	25 mins	100.0%	5040	0.683%						50%	50%				100%	0.7%
Benton County	114.01	WA 3,580	20 Mins or less	100.0%	3580	0.485%						50%	50%				100%	0.5%
Benton County Benton County	114.02 115.01	WA 5,415 WA 6,443	20 Mins or less 25 mins	100.0% 100.0%	5415 6443	0.734% 0.873%						50%	50%	100%			100% 100%	0.7% 0.9%
Benton County	115.01	WA 2,866	20 Mins or less	100.0%	2866	0.388%						50%	50%	10070			100%	0.4%
Benton County	115.05	WA 4,177	20 Mins or less	100.0%	4177	0.566%						100%					100%	0.6%
Benton County	115.06	WA 7,519	20 Mins or less	100.0%	7519	1.019%						50%	50%				100%	1.0%
Benton County	117.01	WA 3,012	30 Mins	100.0%	3012	0.408%		100%									100%	0.4%
Benton County Benton County	117.02 118.01	WA 5,132 WA 3,655	30 Mins 35 mins	100.0% 98.3%	5132 3592	0.696% 0.487%		100% 100%							+ +		100% 100%	0.7% 0.5%
Benton County	118.01	WA 2,665	40 mins	96.5%	2571	0.349%		100%							<del> </del>		100%	0.3%
Benton County	119	WA 6,325	25 mins	100.0%	6325	0.857%			100%								100%	0.9%
Benton County	120	WA 0	40 mins	96.5%	0	0.000%			100%								100%	0.0%
Franklin County	201.01	WA 1,828	30 Mins	100.0%	1828	0.248%						100% 100%			1		100% 100%	0.2%
Franklin County Franklin County	201.02 201.03	WA 6,609 WA 3,811	30 Mins 30 Mins	100.0% 100.0%	6609 3811	0.896% 0.517%						100%	+				100%	0.9% 0.5%
Franklin County	202.01	WA 2,201	30 Mins	100.0%	2201	0.298%						100%					100%	0.3%
Franklin County	202.02	WA 4,142	30 Mins	100.0%	4142	0.561%						100%					100%	0.6%
Franklin County	203	WA 6,088	30 Mins	100.0%	6088	0.825%						100%					100%	0.8%
Franklin County Franklin County	204.01 204.02	WA 1,065 WA 1,101	30 Mins 25 Mins	100.0%	1065	0.144% 0.149%				+		100% 100%	+				100% 100%	0.1%
Franklin County Franklin County	204.02	WA 1,101 WA 3,611	25 Mins	100.0%	1101 3611	0.149%						100%					100%	0.1%
Franklin County	204.04	WA 2,928	25 Mins	100.0%	2928	0.397%						100%	1				100%	0.4%
Franklin County	205.01	WA 5,161	30 Mins	100.0%	5161	0.700%				100%							100%	0.7%
Franklin County	205.03	WA 3,296	30 Mins	100.0%	3296	0.447%			1	50%		50%	1		1		100%	0.4%
Franklin County Franklin County	205.04 206.03	WA 6,522 WA 4,546	30 Mins 30 Mins	100.0%	6522 4546	0.884% 0.616%				50% 100%		50%			1		100% 100%	0.9%
Franklin County	206.05	WA 4,548 WA 9,548	30 Mins	100.0%	9548	1.294%				50%		50%			+ +		100%	1.3%
Franklin County	206.06	WA 8,729	25 mins	100.0%	8729	1.183%				100%							100%	1.2%
Franklin County	206.07	WA 6,719	35 Mins	98.3%	6604	0.895%				50%		50%					100%	0.9%
Franklin County	206.08	WA 6,601	40 Mins	96.5%	6369	0.863%				50%		50%			1000/		100%	0.9%
Morrow County  Morrow County	9701.01 9701.02	OR 5,034 OR 3,676	45 Mins 30 Mins	94.6%	4761 3676	0.645% 0.498%				+		1	+		100%	-	100%	0.6% 0.5%
iviorrow County	9/01.02	UK 3,6/6	3U IVIINS	100.0%	30/6	0.498%						<u> </u>			100%		100%	0.5%

												LAYDOV	VN YARD #2 - LC	OCAL TRAVEL ROUTE	S				]	
								To/Fro	m West			To/Fro	m North				To/From South			
					0/ B	Effective														
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221		1
County		State I	Population	Time to Site	Zone	Zone	Population			Noau									100% CHECK	Total
Morrow County	9702	OR	3,254	1:00	88.1%	2867	0.389%										100%		100%	0.4%
Umatilla County	9400	OR	3,072	1:00	88.1%	2706	0.367%										100%		100%	0.4%
Umatilla County	9501	OR	4,659	1:20	76.9%	3581	0.485%						25%				75%		100%	0.5%
Umatilla County Umatilla County	9502.01 9502.02	OR OR	3,712 4,043	1:20 1:20	76.9% 76.9%	2853 3108	0.387% 0.421%						25% 25%				75% 75%		100% 100%	0.4%
Umatilla County	9503	OR	3,371	1:10	82.9%	2796	0.421%						25/6				100%		100%	0.4%
Umatilla County	9504	OR	6,038	1:00	88.1%	5319	0.721%										100%		100%	0.7%
Umatilla County	9505	OR	4,754	1:00	88.1%	4188	0.568%										100%		100%	0.6%
Umatilla County	9506.01	OR	2,600	1:00	88.1%	2290	0.310%										100%		100%	0.3%
Umatilla County Umatilla County	9506.02 9507	OR OR	3,212 2,513	1:00 55 Mins	88.1% 90.4%	2830 2272	0.384%										100% 100%		100% 100%	0.4%
Umatilla County	9508	OR	7,508	25 Mins	100.0%	7508	1.018%										100%		100%	1.0%
Umatilla County	9509	OR	5,133	20 Mins or less	100.0%	5133	0.696%										100%		100%	0.7%
Umatilla County	9510	OR	6,224	25 Mins	100.0%	6224	0.844%										100%		100%	0.8%
Umatilla County	9511	OR	6,018	25 Mins	100.0%	6018	0.816%										100%		100%	0.8%
Umatilla County	9512.01	OR	6,207	30 Mins	100.0%	6207	0.841%									1	100%		100%	0.8%
Umatilla County Umatilla County	9512.02 9513	OR OR	4,040 3,989	30 Mins 30 Mins	100.0% 100.0%	4040 3989	0.548% 0.541%	<del> </del>			+			+		+	100% 100%		100% 100%	0.5% 0.5%
Umatilla County	9514	OR	2,416	1:10	82.9%	2004	0.341%									1	100%		100%	0.3%
Union County	9701	OR	3,238	2:00	23.1%	749	0.102%										100%		100%	0.1%
Union County	9702	OR	3,376	1:55	36.7%	1238	0.168%										100%		100%	0.2%
Union County	9703	OR	2,427	1:55	36.7%	890	0.121%										100%		100%	0.1%
Union County Union County	9704 9705	OR OR	2,732 3,196	1:50 1:35	46.3% 65.0%	1264 2076	0.171% 0.281%										100% 100%		100% 100%	0.2%
Union County	9706	OR	3,927	1:50	46.3%	1817	0.246%										100%		100%	0.3%
Union County	9707	OR	3,416	1:40	59.8%	2043	0.277%										100%		100%	0.3%
Union County	9708	OR	3,943	1:40	59.8%	2358	0.320%										100%		100%	0.3%
Adams County	9501	WA	2,577	1:30	69.4%	1789	0.242%						100%						100%	0.2%
Adams County Adams County	9502 9503.01	WA WA	1,794 1,790	1:20 1:20	76.9% 76.9%	1379 1376	0.187% 0.186%						100% 100%						100% 100%	0.2% 0.2%
Adams County	9503.01	WA	2,738	1:20	76.9%	2104	0.186%				50%		50%						100%	0.2%
Adams County	9503.03	WA	2,555	1:15	80.0%	2045	0.277%				50%		50%						100%	0.3%
Adams County	9504	WA	3,100	1:15	80.0%	2481	0.336%				50%		50%						100%	0.3%
Adams County	9505	WA	5,799	1:15	80.0%	4642	0.629%				50%		50%						100%	0.6%
Columbia County	9602 207	WA	3,969 1,277	1:30 1:15	69.4% 80.0%	2755 1022	0.373% 0.139%						100% 100%						100% 100%	0.4%
Franklin County Franklin County	208.01	WA	3,401	1:00	88.1%	2996	0.139%						100%						100%	0.1%
Franklin County	208.02	WA	3,129	1:00	88.1%	2756	0.374%						100%						100%	0.4%
Grant County	101	WA	3,610	greater than 2:00	0.0%	0	0.000%						100%						100%	0.0%
Grant County	102	WA	3,382	greater than 2:00	0.0%	0	0.000%						100%						100%	0.0%
Grant County	103	WA	5,425	greater than 2:00	0.0%	0	0.000%						100%						100% 100%	0.0%
Grant County Grant County	104.01 104.02	WA WA	3,148 5,495	greater than 2:00 greater than 2:00	0.0%	0	0.000%						100% 100%						100%	0.0%
Grant County	104.02	WA	3,270	greater than 2:00	0.0%	0	0.000%	1		25%	25%		50%			1	†		100%	0.0%
Grant County	106	WA	7,614	greater than 2:00	0.0%	0	0.000%			25%	25%		50%		_				100%	0.0%
Grant County	107	WA	3,186	1:45	53.7%	1712	0.232%			25%	25%		50%						100%	0.2%
Grant County	108	WA	5,398	1:30	69.4%	3747	0.508%						100%			1			100%	0.5%
Grant County Grant County	109.01 109.03	WA WA	1,679 5,281	1:30 1:30	69.4% 69.4%	1165 3666	0.158% 0.497%	<del> </del>		1	+		100% 100%			+	+		100% 100%	0.2% 0.5%
Grant County	109.04	WA	6,136	1:30	69.4%	4259	0.577%						100%	+		†	†		100%	0.6%
Grant County	110.01	WA	5,723	1:30	69.4%	3973	0.538%						100%						100%	0.5%
Grant County	110.02	WA	6,225	1:30	69.4%	4321	0.586%						100%						100%	0.6%
Grant County	111.01	WA	4,657	1:30	69.4%	3233	0.438%						100%						100%	0.4%
Grant County Grant County	111.02 112	WA WA	2,891 7,100	1:30 1:45	69.4% 53.7%	2007 3814	0.272% 0.517%			25%	25%		100% 50%				-		100% 100%	0.3% 0.5%
Grant County	113	WA	3,367	1:20	76.9%	2588	0.317%			23/0	23/0		100%	+			+		100%	0.3%
Grant County	114.01	WA	2,249	1:20	76.9%	1729	0.234%				<u> </u>		100%						100%	0.2%
Grant County	114.03	WA	4,871	1:15	80.0%	3899	0.528%			50%	50%								100%	0.5%
Grant County	114.04	WA	963	1:30	69.4%	668	0.091%			50%	50%			<u> </u>					100%	0.1%
Grant County	114.05 114.06	WA	3,019 3,185	1:30 1:30	69.4% 69.4%	2096 2211	0.284%			25% 25%	25% 25%		50% 50%	+		+			100% 100%	0.3%
Grant County Kittitas County	9751.01	WA WA	2,363	greater than 2:00	0.0%	0	0.300%		25%	50%	25%		30%			+			100%	0.3%
Kittitas County  Kittitas County	9751.02	WA	1,290	greater than 2:00	0.0%	0	0.000%	1	25%	50%	25%					1			100%	0.0%
Kittitas County	9751.03	WA	1,444	greater than 2:00	0.0%	0	0.000%		25%	50%	25%								100%	0.0%

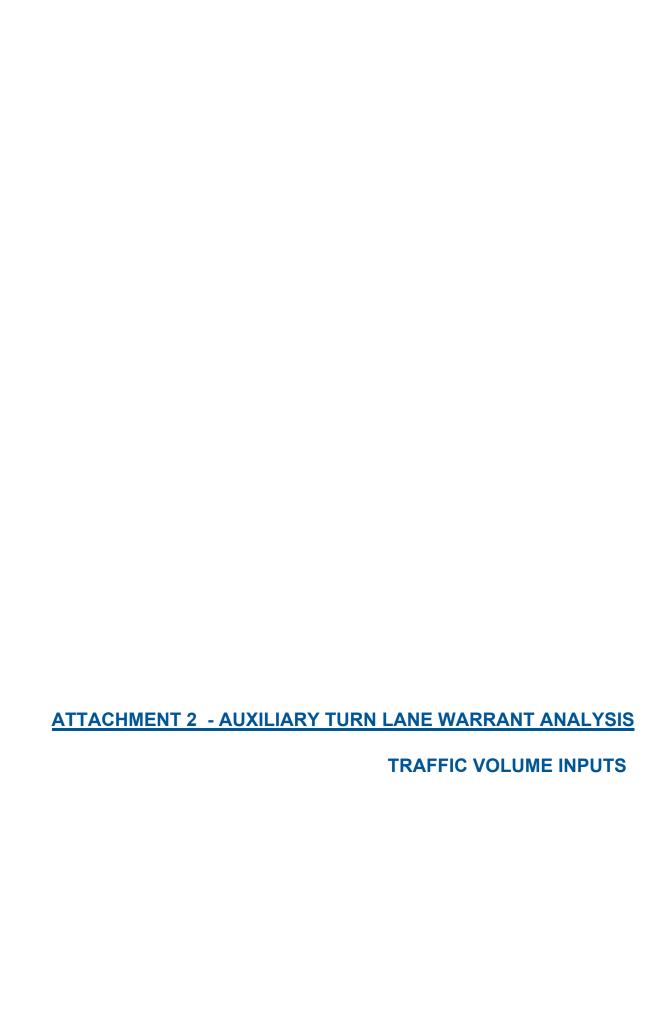
												LAYDOV	VN YARD #2 - LC	CAL TRAVEL ROU	TES		7	
								To/Fron	n West			To/Fro	m North			To/From South	1	
						Effective												
	Census		Total	Estimated Drive	% Population in Drive Time	Population in Drive Time	% of Total	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road S. Plymouth Road Roue 221		
County		State I	opulation	Time to Site	Zone	Zone	Population			Noau							100% CHECK	Total
Kittitas County	9751.04	WA	1,644	greater than 2:00	0.0%	0	0.000%		25%	50%	25%						100%	0.0%
Kittitas County	9752.01	WA	3,364	greater than 2:00	0.0%	0	0.000%		25%	50%	25%						100%	0.0%
Kittitas County	9752.02	WA	1,395	1:45	53.7%	749	0.102%		25%	50%	25%						100%	0.1%
Kittitas County Kittitas County	9752.03 9753	WA WA	1,098 5,316	1:45 2:00	53.7% 23.1%	590 1230	0.080% 0.167%	+	25% 25%	50% 50%	25% 25%						100% 100%	0.1%
Kittitas County	9754.02	WA	4,713	1:45	53.7%	2532	0.343%		25%	50%	25%					<del> </del>	100%	0.2%
Kittitas County	9754.03	WA	2,921	1:45	53.7%	1569	0.213%		25%	50%	25%						100%	0.2%
Kittitas County	9754.04	WA	5,145	1:45	53.7%	2764	0.375%		25%	50%	25%						100%	0.4%
Kittitas County	9755 9756	WA WA	5,956 2,790	1:45 1:45	53.7% 53.7%	3200	0.434%		25% 25%	50% 50%	25% 25%						100% 100%	0.4%
Kittitas County Kittitas County	9757	WA	4,708	2:00	23.1%	1499 1089	0.203% 0.148%		25%	50%	25%						100%	0.2%
Klickitat County	9501.01	WA	1,538	1:00	88.1%	1355	0.184%	50%	50%	1	1000						100%	0.2%
Klickitat County	9501.02	WA	3,507	1:20	76.9%	2696	0.365%		75%	25%							100%	0.4%
Klickitat County	9501.03	WA	4,189	1:45	53.7%	2251	0.305%			+			-			100%	100%	0.3%
Klickitat County Klickitat County	9502 9503.01	WA WA	4,383 3,465	greater than 2:00 greater than 2:00	0.0%	0	0.000%									100%	100% 100%	0.0%
Klickitat County	9503.02	WA	5,396	greater than 2:00	0.0%	0	0.000%			1				+		100%	100%	0.0%
Walla Walla County	9200	WA	6,176	1:00	88.1%	5441	0.737%						100%				100%	0.7%
Walla Walla County	9201	WA	5,165	1:00	88.1%	4550	0.617%						75%			25%	100%	0.6%
Walla Walla County	9202	WA	4,715	1:15	80.0%	3774	0.512%			1			50%			50%	100%	0.5%
Walla Walla County Walla Walla County	9203.01 9203.02	WA WA	3,243 5,434	1:15 1:15	80.0% 80.0%	2596 4350	0.352% 0.590%			1			50% 50%			50%	100% 100%	0.4%
Walla Walla County	9204	WA	2,640	1:15	80.0%	2113	0.286%						50%			50%	100%	0.3%
Walla Walla County	9205	WA	2,959	1:15	80.0%	2368	0.321%						50%			50%	100%	0.3%
Walla Walla County	9206	WA	6,205	1:15	80.0%	4967	0.673%						50%			50%	100%	0.7%
Walla Walla County Walla Walla County	9207.01 9207.02	WA WA	3,545 4,293	1:15 1:15	80.0% 80.0%	2838 3436	0.385% 0.466%						50% 50%			50%	100% 100%	0.4% 0.5%
Walla Walla County	9208.01	WA	4,293	1:15	80.0%	3958	0.400%						50%			50%	100%	0.5%
Walla Walla County	9208.02	WA	3,223	1:15	80.0%	2580	0.350%						50%			50%	100%	0.3%
Walla Walla County	9209.01	WA	4,134	1:15	80.0%	3309	0.449%						50%			50%	100%	0.4%
Walla Walla County	9209.02	WA WA	5,491	1:15	80.0% 80.0%	4395	0.596%		F00/	F.00/			50%			50%	100% 100%	0.6%
Yakima County Yakima County	2	WA	3,072 5,595	1:15 1:15	80.0%	2459 4478	0.333% 0.607%		50% 50%	50% 50%							100%	0.3%
Yakima County	3.01	WA	2,473	1:15	80.0%	1979	0.268%		50%	50%							100%	0.3%
Yakima County	3.02	WA	2,283	1:15	80.0%	1827	0.248%		50%	50%							100%	0.2%
Yakima County	4.01	WA	5,958	1:15	80.0%	4769	0.646%		50%	50%							100%	0.6%
Yakima County Yakima County	4.02 5	WA WA	2,407 4,599	1:15 1:15	80.0% 80.0%	1927 3681	0.261% 0.499%		50% 50%	50% 50%							100% 100%	0.3% 0.5%
Yakima County	6	WA	5,696	1:15	80.0%	4559	0.499%		50%	50%						<del> </del>	100%	0.5%
Yakima County	7	WA	7,077	1:15	80.0%	5665	0.768%		50%	50%							100%	0.8%
Yakima County	8	WA	4,484	1:15	80.0%	3589	0.486%		50%	50%							100%	0.5%
Yakima County	9.02	WA	4,507	1:20	76.9%	3464	0.470%	<del>                                     </del>	50%	50%			<del> </del>	+		<del>                                     </del>	100%	0.5%
Yakima County Yakima County	9.03 9.04	WA WA	4,008 3,332	1:20 1:20	76.9% 76.9%	3081 2561	0.418% 0.347%		50% 50%	50% 50%				+		<del>                                     </del>	100% 100%	0.4%
Yakima County	10	WA	6,499	1:20	76.9%	4995	0.677%		50%	50%				<u>                                     </u>			100%	0.7%
Yakima County	11	WA	7,361	1:15	80.0%	5892	0.799%		50%	50%		·					100%	0.8%
Yakima County	12.01	WA	4,723	1:15	80.0%	3780	0.512%		50%	50%			-				100%	0.5%
Yakima County Yakima County	12.02 13	WA WA	7,051 2,653	1:15 1:10	80.0% 82.9%	5644 2201	0.765% 0.298%	+	50% 50%	50% 50%						<del>                                     </del>	100% 100%	0.8%
Yakima County	14	WA	4,099	1:10	82.9%	3400	0.461%	1	50%	50%			1				100%	0.5%
Yakima County	15.02	WA	2,658	1:10	82.9%	2205	0.299%		50%	50%							100%	0.3%
Yakima County	15.03	WA	4,558	1:10	82.9%	3781	0.512%		50%	50%							100%	0.5%
Yakima County Yakima County	15.04 16.01	WA WA	2,894 2,537	1:10 1:15	82.9% 80.0%	2401 2031	0.325% 0.275%		50% 50%	50% 50%							100% 100%	0.3%
Yakima County	16.01	WA	8,633	1:15	80.0%	6910	0.273%		50%	50%	+			+		<del>                                     </del>	100%	0.5%
Yakima County	17.01	WA	3,654	1:10	82.9%	3031	0.411%		25%	50%	25%			<u> </u>			100%	0.4%
Yakima County	17.02	WA	6,565	1:10	82.9%	5446	0.738%		50%	50%							100%	0.7%
Yakima County	18.01	WA	4,419	40 Mins	96.5%	4264	0.578%		50%	50%						<del>                                     </del>	100%	0.6%
Yakima County Yakima County	18.02 19.01	WA WA	2,933 3,680	40 Mins 40 Mins	96.5% 96.5%	2830 3551	0.384% 0.481%	+	75% 50%	25% 50%						<del>                                     </del>	100% 100%	0.4%
Yakima County	19.02	WA	6,678	40 Mins	96.5%	6443	0.873%	1	50%	50%							100%	0.9%
Yakima County	20.03	WA	5,057	45 Mins	94.6%	4783	0.648%		50%	50%							100%	0.6%
Yakima County	20.04	WA	4,734	45 Mins	94.6%	4477	0.607%		50%	50%							100%	0.6%
Yakima County	20.05	WA	2,544	45 Mins	94.6%	2406	0.326%		50%	50%							100%	0.3%

												LAYDOV	VN YARD #2 - LO	OCAL TRAVEL ROUT	TES			·		
								To/Fro	m West			To/Fro	m North				To/From South			
County	Census Tract	State P	Total opulation	Estimated Drive Time to Site	% Population in Drive Time Zone	Effective Population in Drive Time Zone	% of Total Population	Sellards Road	Route 221	Weber Canyon Road	S. Clodfelter Road	I-82	Route 395	S. Olympia Street	Route 397	Bofer Canyon Road	S. Plymouth Road	Roue 221	100% CHECK	Total
Yakima County	20.06	WA	6,934	45 Mins	94.6%	6558	0.889%		50%	50%									100%	0.9%
Yakima County	21.01		2,468	45 Mins	94.6%	2334	0.316%		50%	50%									100%	0.3%
Yakima County	21.03	WA	2,709	45 Mins	94.6%	2562	0.347%		50%	50%									100%	0.3%
Yakima County	21.04	WA	5,099	50 Mins	92.6%	4719	0.640%		50%	50%									100%	0.6%
Yakima County	22.01	WA	5,153	55 Mins	90.4%	4658	0.631%		50%	50%									100%	0.6%
Yakima County	22.02	WA	2,017	1:00	88.1%	1777	0.241%		50%	50%									100%	0.2%
Yakima County	27.01	WA	3,466	40 Mins	96.5%	3344	0.453%	100%											100%	0.5%
Yakima County	28.01	WA	5,627	1:20	76.9%	4325	0.586%		50%	50%									100%	0.6%
Yakima County	28.03	WA	5,809	1:20	76.9%	4465	0.605%		50%	50%									100%	0.6%
Yakima County	28.04	WA	3,607	1:20	76.9%	2772	0.376%		50%	50%									100%	0.4%
Yakima County	29	WA	7,131	1:30	69.4%	4950	0.671%		50%	50%									100%	0.7%
Yakima County	30.02	WA	4,085	1:30	69.4%	2836	0.384%		50%	50%									100%	0.4%
Yakima County	30.03	WA	1,724	greater than 2:00	0.0%	0	0.000%		50%	50%									100%	0.0%
Yakima County	30.04	WA	2,644	1:40	59.8%	1581	0.214%		50%	50%									100%	0.2%
Yakima County	31	WA	5,297	1:15	80.0%	4240	0.575%		50%	50%									100%	0.6%
Yakima County	32	WA	7,012	1:15	80.0%	5613	0.761%		50%	50%									100%	0.8%
Yakima County	34		5,228	1:15	80.0%	4185	0.567%		50%	50%									100%	0.6%
Yakima County	9400.01	WA	6,534	1:10	82.9%	5420	0.735%		50%	50%									100%	0.7%
Yakima County	9400.02	WA	4,762	1:00	88.1%	4195	0.569%		75%	25%									100%	0.6%
Yakima County	9400.03	WA	3,292	1:15	80.0%	2635	0.357%		75%	25%									100%	0.4%
Yakima County	9400.05	WA	4,776	1:00	88.1%	4207	0.570%		75%	25%									100%	0.6%
Yakima County	9400.06	WA	4,758	1:00	88.1%	4192	0.568%		75%	25%									100%	0.6%
Yakima County	9400.07	WA	3,449	1:10	82.9%	2861	0.388%		75%	25%									100%	0.4%
Yakima County	9400.08		2,149	1:10	82.9%	1783	0.242%	0.55%	75%	25%	24.460/	4.570/	22.500/	2.700/	0.070/	0.000/	16.010/	0.000/	100%	0.2%
		Total	919,798			737,776	100.00%	0.55%	17.54%	17.09%	21.16%	1.57%	22.50%	2.70%	0.87%	0.00%	16.01%	0.00%	100%	100.00%
							Use	1%	18%	17%	21%	1%	22%	3%	1%	0%	16%	0%	100%	I

LOGARITHMIC GRA	VITY ASSUMPTIONS
Travel time to Site (approx.)	Adjustment Factor (0-100%)
20 Mins or less	100.00%
25 Mins	100.00%
30 Mins	100.00%
35 Mins	98.29%
40 Mins	96.48%
45 Mins	94.57%
50 Mins	92.55%
55 Mins	90.40%
1:00	88.09%
1:05	85.62%
1:10	82.95%
1:15	80.04%
1:20	76.86%
1:25	73.35%
1:30	69.41%
1:35	64.96%
1:40	59.81%
1:45	53.72%
1:50	46.28%
1:55	36.67%
2:00	23.14%
greater than 2:00	0.00%



#### **APPENDIX G: AUXILIARY LANE WARRANT ANALYSIS**



#### Auxiliary Lane Warrant Analysis Traffic Volume Inputs

Horse Heaven Wind Farm Benton County, Washington

#### 2023 Existing Conditions - Weekday Morning Peak Hour

	Route 221 at	Sellards Road	Route 221	at Route 14	Route 14 at S.	Plymouth Road		yon Road at r Road
_	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southbound
Left Turning Movement	0	37	12	1	1	11	0	15
Through Movement	23	150	8	95	35	332	29	41
Right Turning Movement	9	0	0	134	1	30	2	0
For Left Turn Warrant:								
Total Design Hour Volume (DHV)	2	19	2:	50	4:	13	8	7
% of Left Turns in Combined Volume	0%	17%	5%	0%	0%	3%	0%	17%
For Right Turn Warrant:								
Right Turning Movement	9	0	0	134	1	30	2	0
Combined Through + Right Turn Volume	32	150	8	229	36	362	31	41

#### 2023 Existing Conditions - Weekday Afternoon Peak Hour

	Route 221 at	Sellards Road	Route 221	at Route 14	Route 14 at S.	Plymouth Road		iyon Road at r Road
_	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southbound
Left Turning Movement	1	39	52	0	9	28	0	53
Through Movement	143	55	64	22	280	98	73	35
Right Turning Movement	42	2	3	52	8	99	8	0
For Left Turn Warrant:					_			
Total Design Hour Volume (DHV)		82		93	57			69
% of Left Turns in Combined Volume	0%	14%	27%	0%	2%	5%	0%	31%
For Right Turn Warrant:								
Right Turning Movement	42	2	3	52	8	99	8	0
Combined Through + Right Turn Volume	185	57	67	74	288	197	81	35

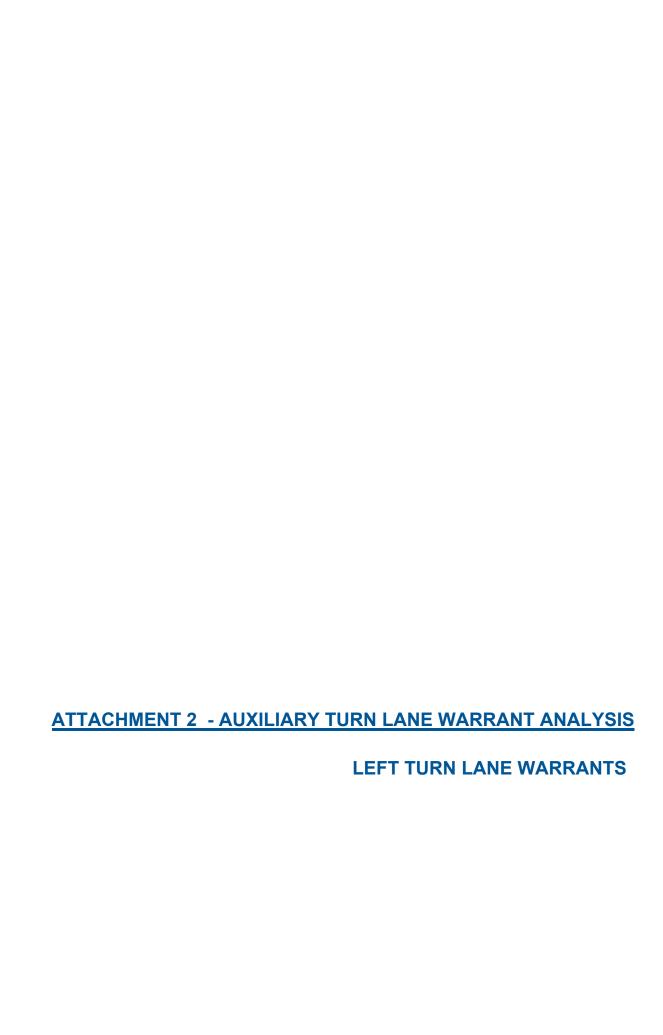
#### 2025 Phase 1 to Laydown Yard 2 - Weekday Morning Peak Hour

	Route 221 at Sellards Road		Route 221 at Route 14		Route 14 at S. Plymouth Road		Webber Canyon Road at Badger Road	
-	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southbound
Left Turning Movement	0	108	12	1	1	11	0	15
Through Movement	23	153	8	97	36	342	32	108
Right Turning Movement	9	0	0	137	1	93	2	0
For Left Turn Warrant:								
Total Design Hour Volume (DHV)	293		255		484		157	
% of Left Turns in Combined Volume	0%	37%	5%	0%	0%	2%	0%	10%
For Right Turn Warrant:								
Right Turning Movement	9	0	0	137	1	93	2	0
Combined Through + Right Turn Volume	32	153	8	234	37	435	34	108

#### 2025 Phase 1 to Laydown Yard 2 - Weekday Afternoon Peak Hour

	Route 221 at Sellards Road		Route 221 at Route 14		Route 14 at S. Plymouth Road		Webber Canyon Road at Badger Road	
-	Northbound	Southbound	Eastbound	Westbound	Eastbound	Westbound	Northbound	Southbound
Left Turning Movement	0	3	53	0	9	29	0	54
Through Movement	284	114	65	22	286	100	140	38
Right Turning Movement	8	0	3	53	8	103	8	0
For Left Turn Warrant:								
Total Design Hour Volume (DHV)	409		196		535		240	
% of Left Turns in Combined Volume	0%	1%	27%	0%	2%	5%	0%	23%
For Right Turn Warrant:								
Right Turning Movement	8	0	3	53	8	103	8	0
Combined Through + Right Turn Volume	292	114	68	75	294	203	148	38
Notes: Speed Limit Along Roadways	65 MPH		65 MPH		55 MPH		50 MPH	

Source: Tetra Tech



# 2023 Existing - Morning Peak Hour Route 14 at S Plymouth Road

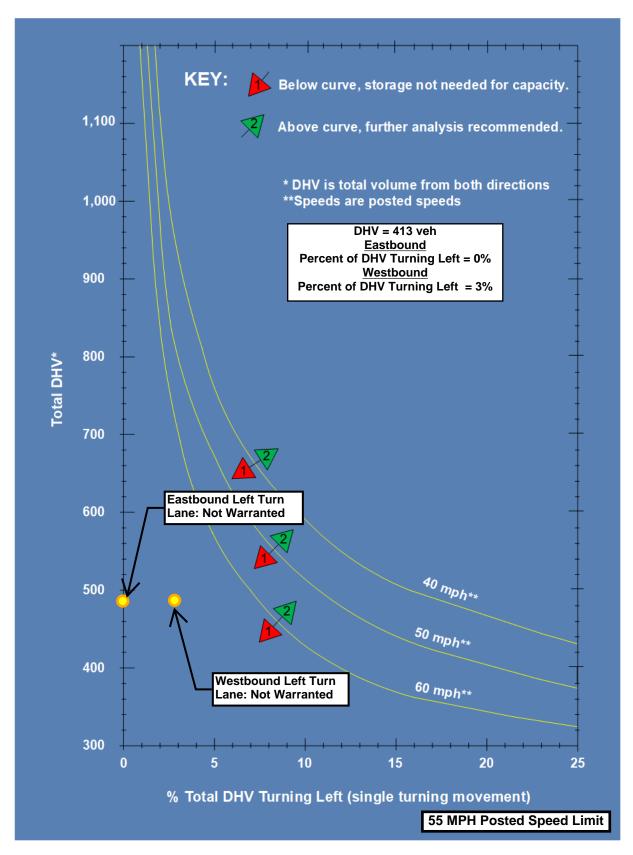


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

# 2023 Existing - Evening Peak Hour Route 14 at S Plymouth Road

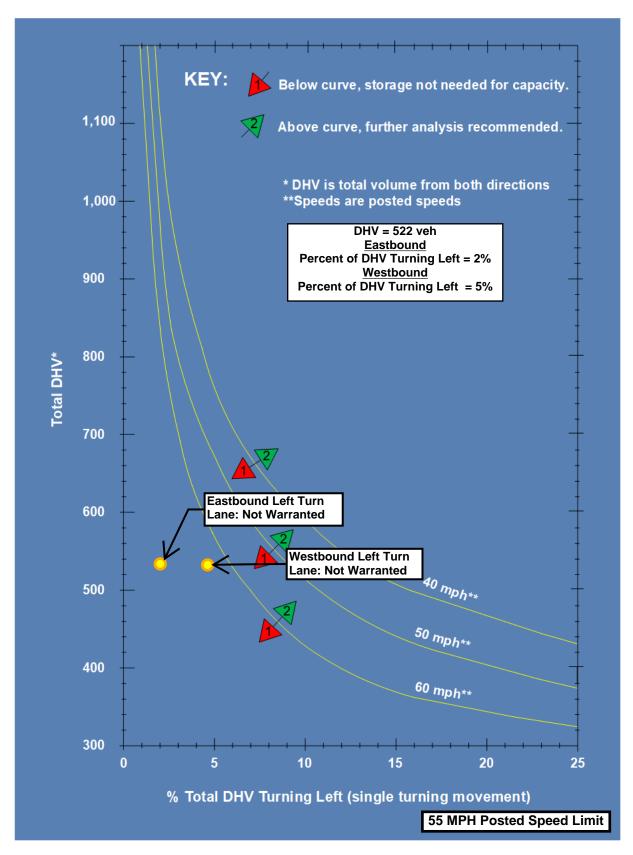
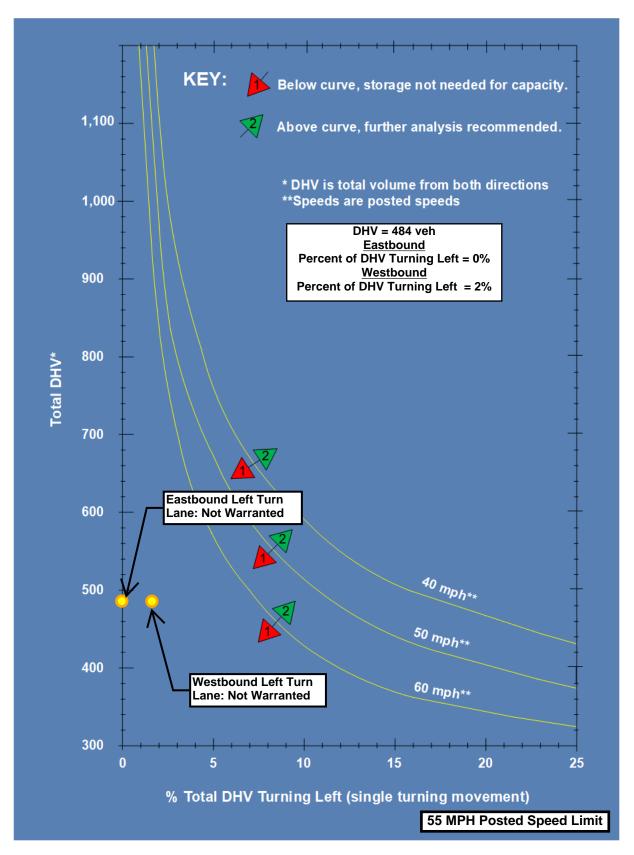
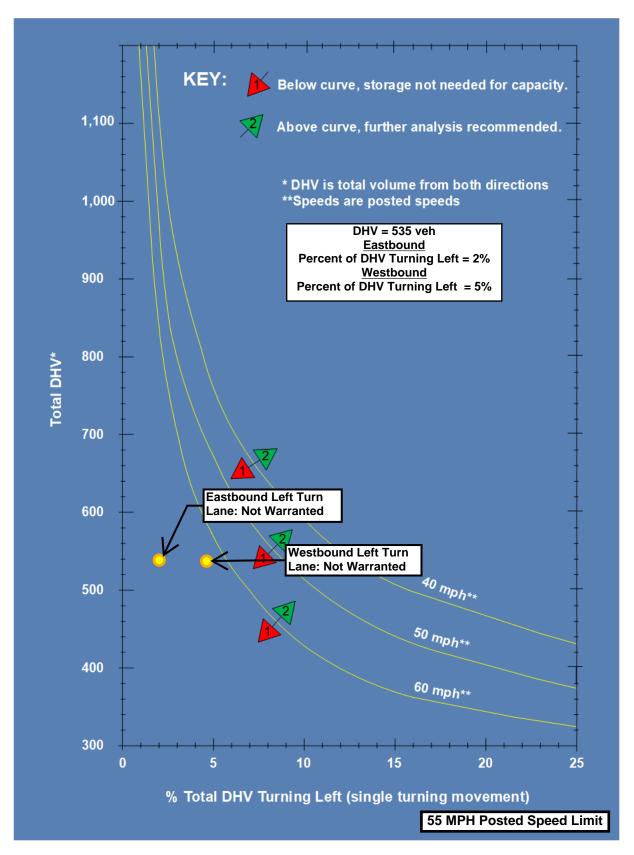


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 14 at S Plymouth Road



## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 14 at S Plymouth Road



#### 2023 Existing - Morning Peak Hour Route 221 at Route 14

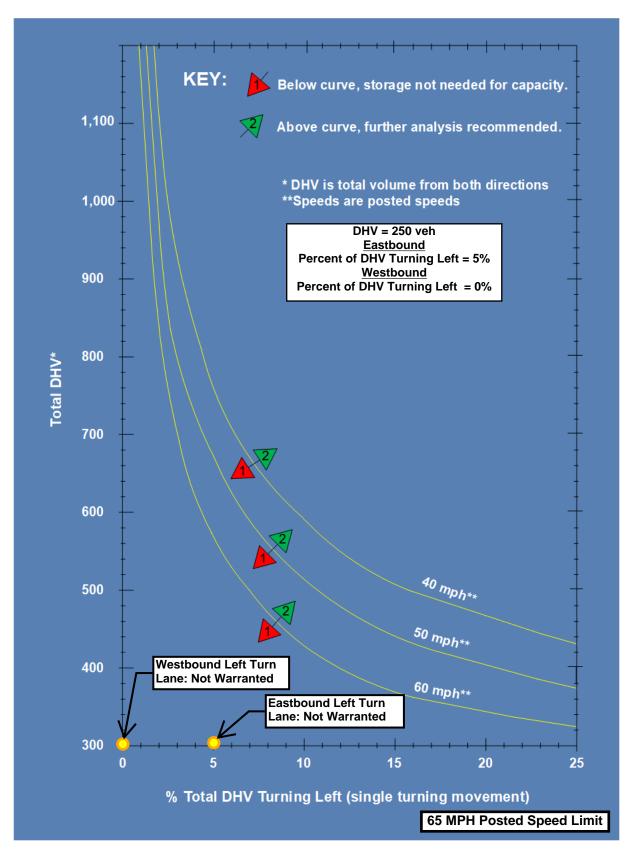


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2023 Existing - Evening Peak Hour Route 221 at Route 14

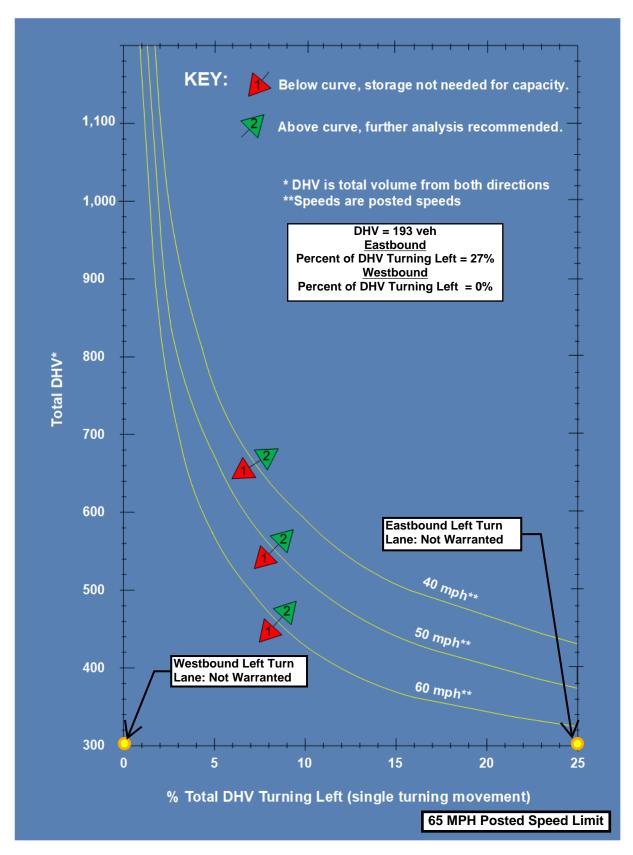
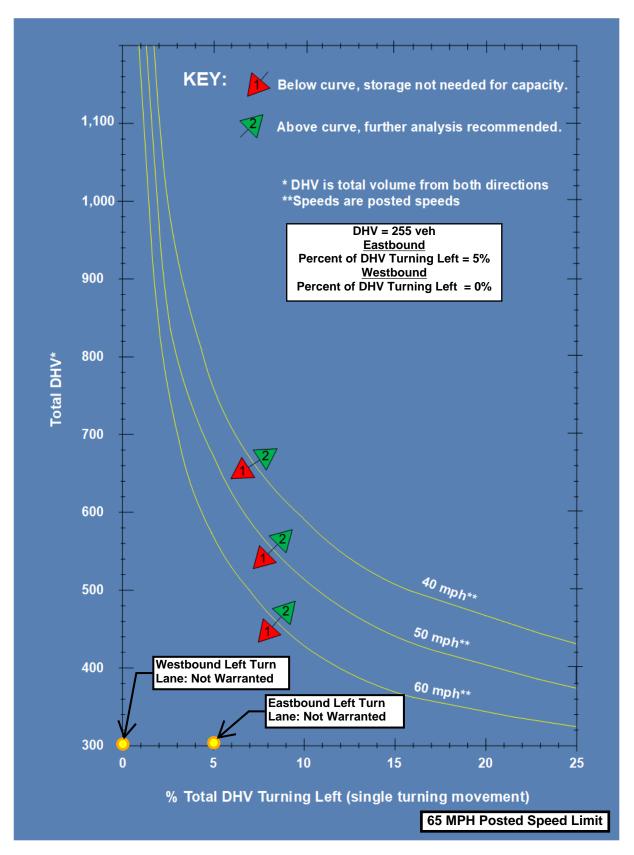
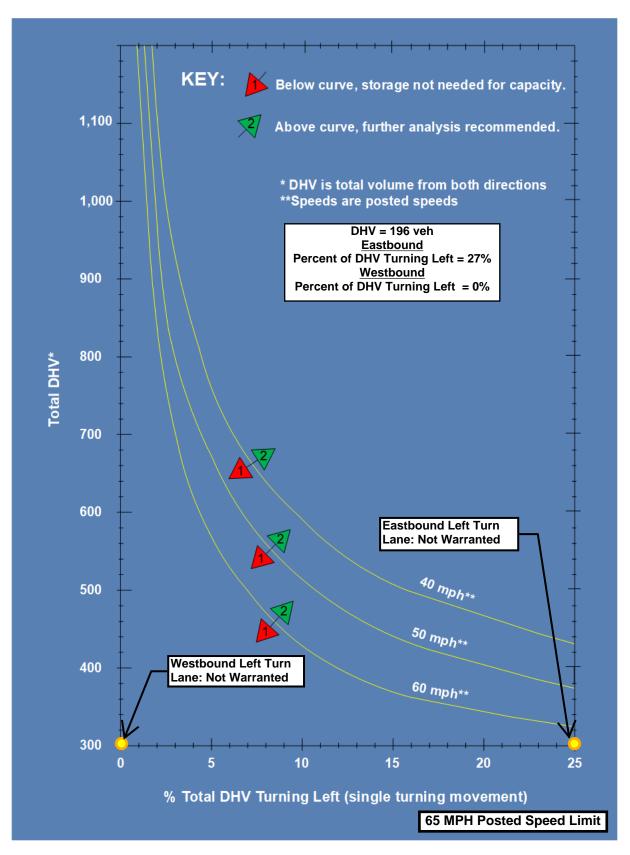


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

#### 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 221 at Route 14



## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 221 at Route 14



## 2023 Existing - Morning Peak Hour Route 221 at Sellards Road

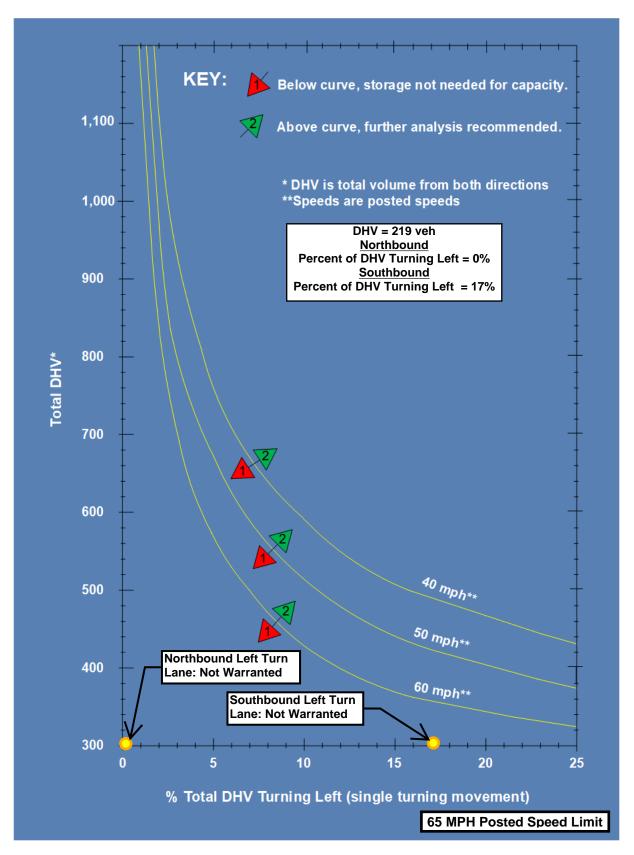


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2023 Existing - Evening Peak Hour Route 221 at Sellards Road

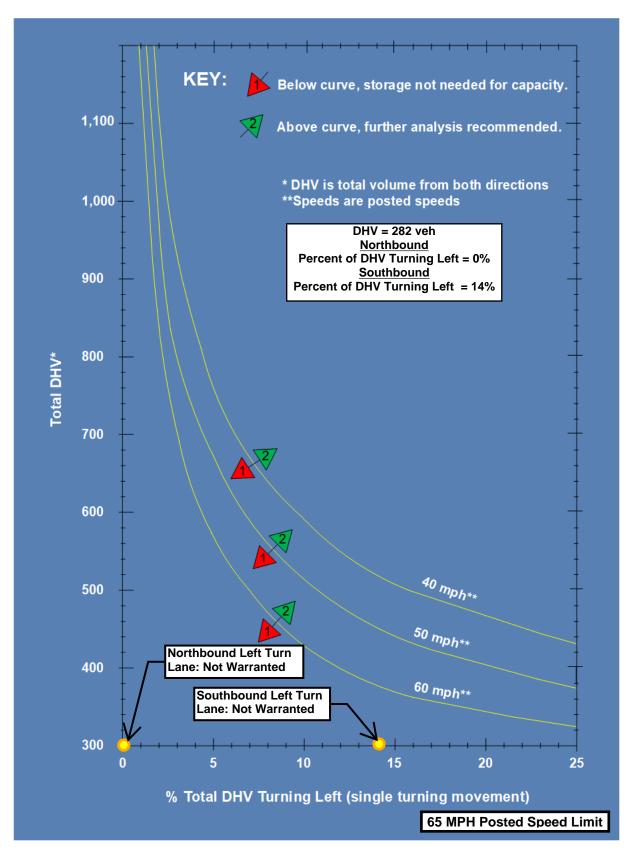


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 221 at Sellards Road

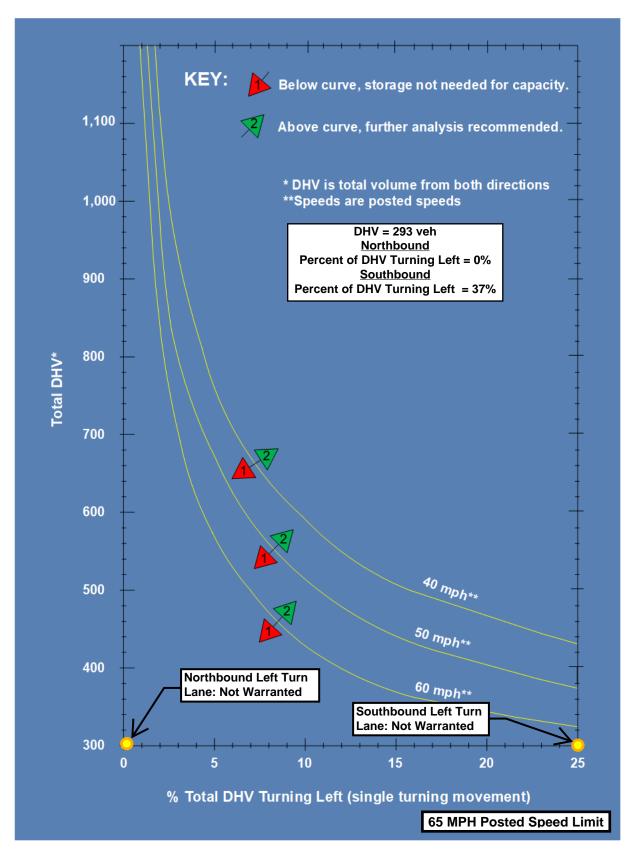
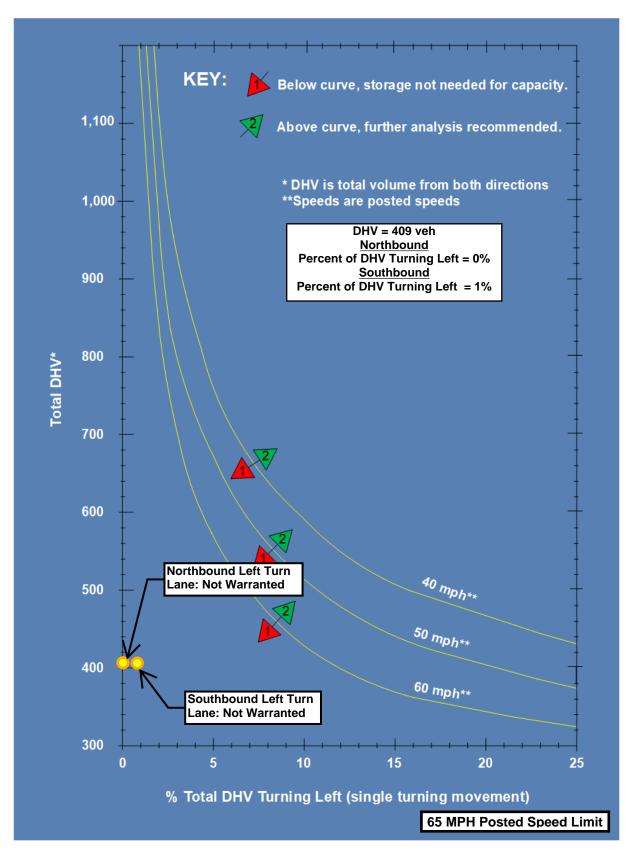


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 221 at Sellards Road



# 2023 Existing - Morning Peak Hour Webber Canyon Road at Badger Road

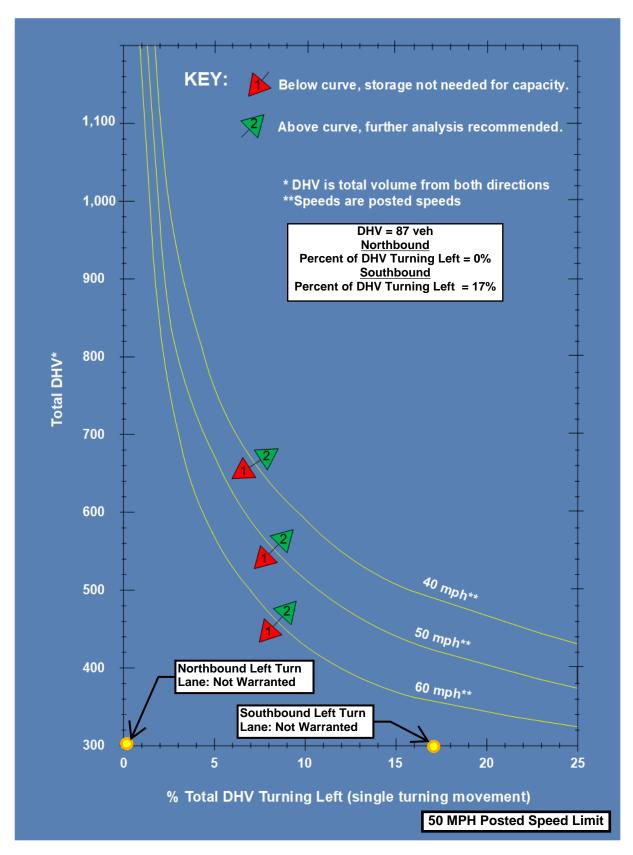


Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

# 2023 Existing - Evening Peak Hour Webber Canyon Road at Badger Road

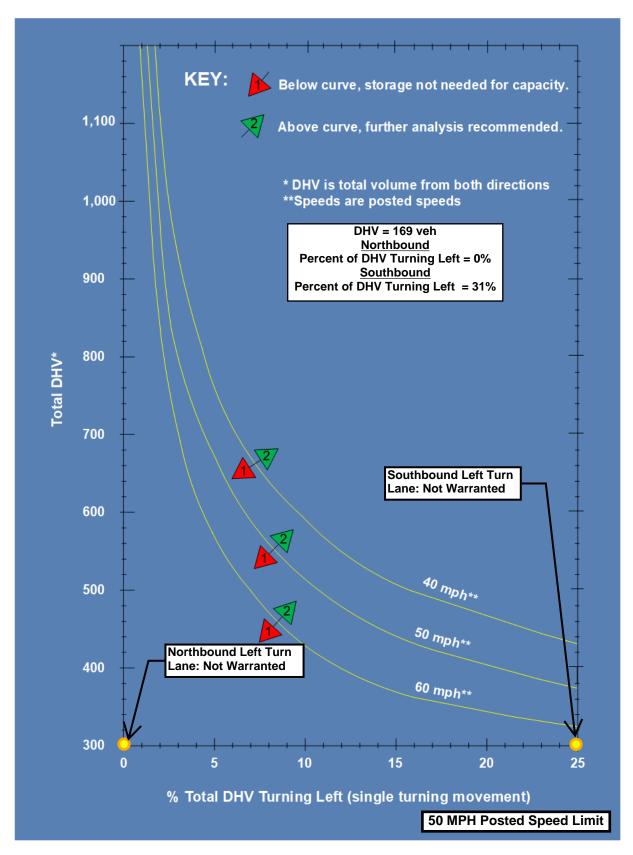
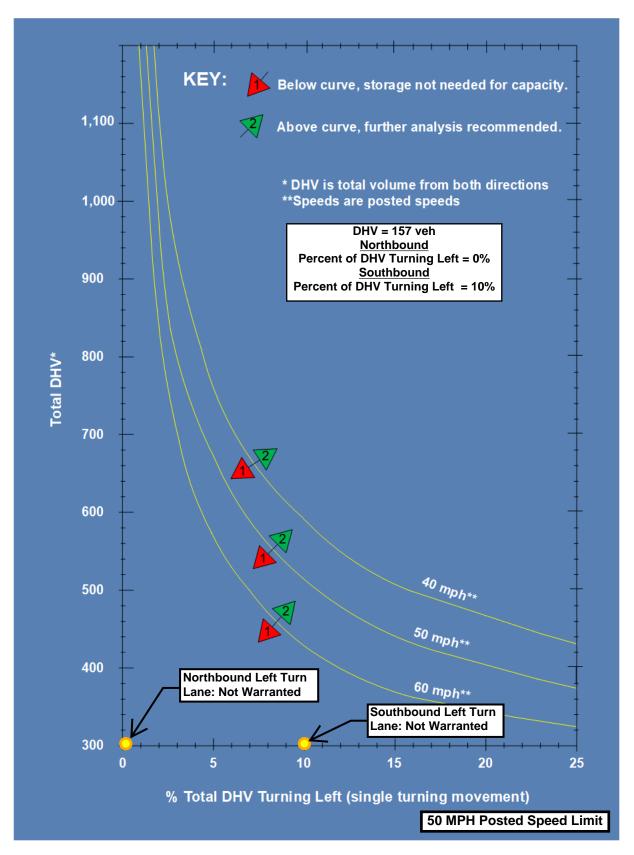
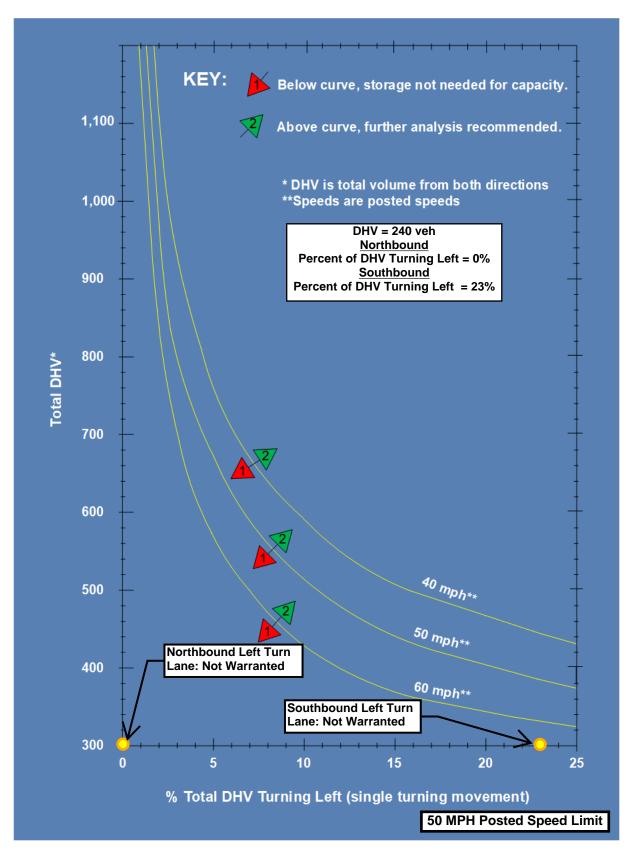


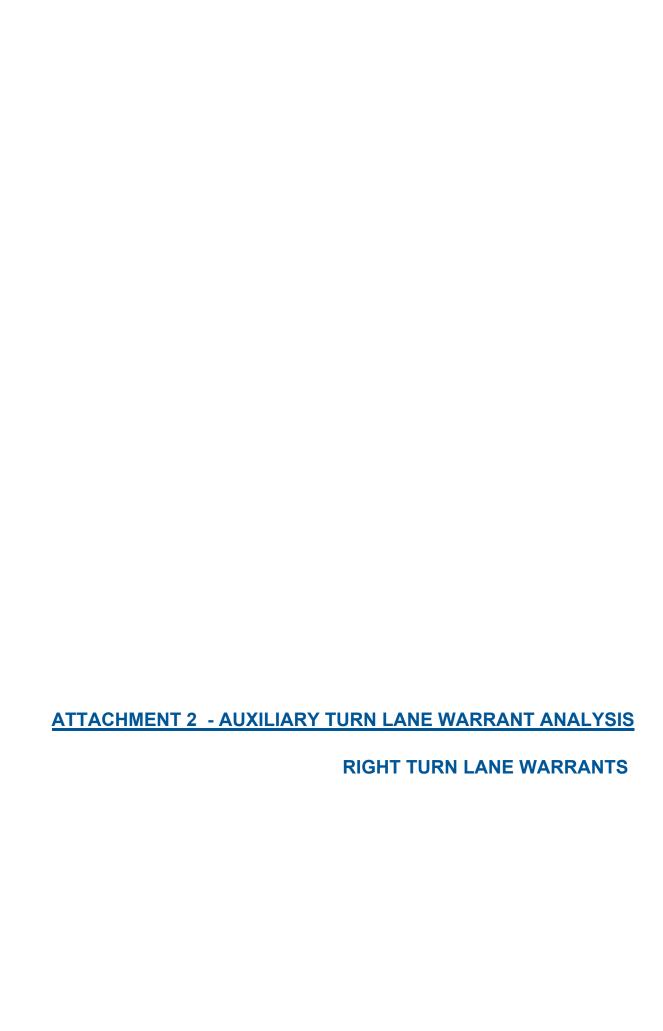
Exhibit 1310-7 Left-Turn Storage Guidelines: Two-Lane, Unsignalized

## 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Webber Canyon Road at Badger Road

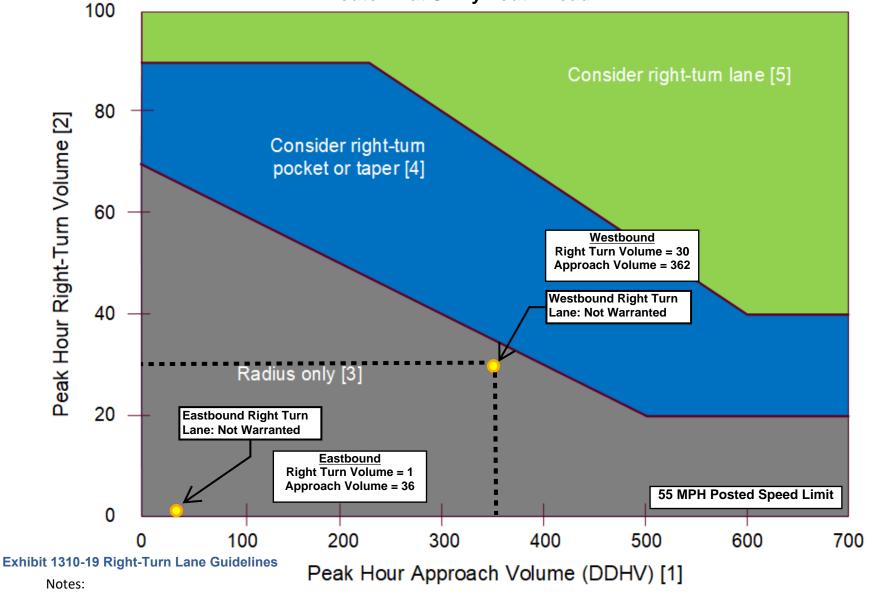


## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Webber Canyon Road at Badger Road





## 2023 Existing - Morning Peak Hour Route 14 at S. Plymouth Road

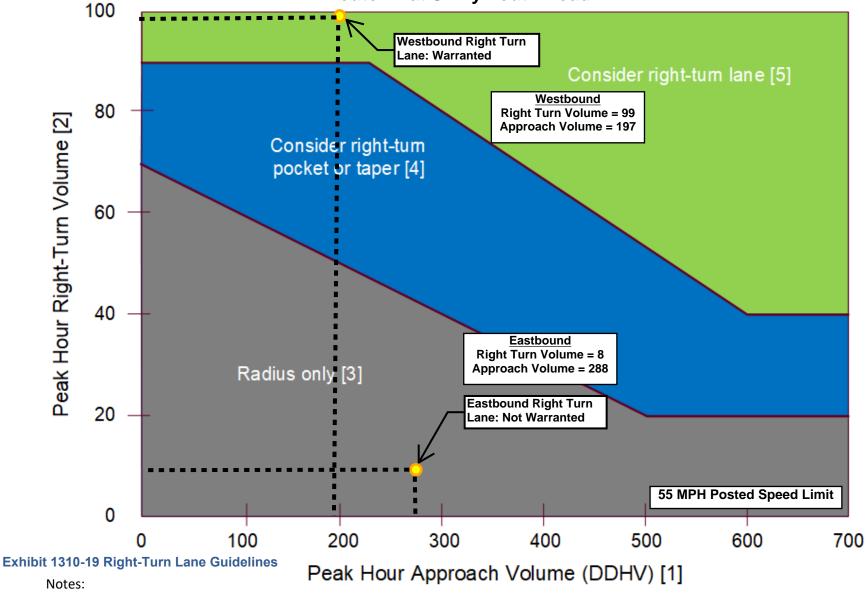


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

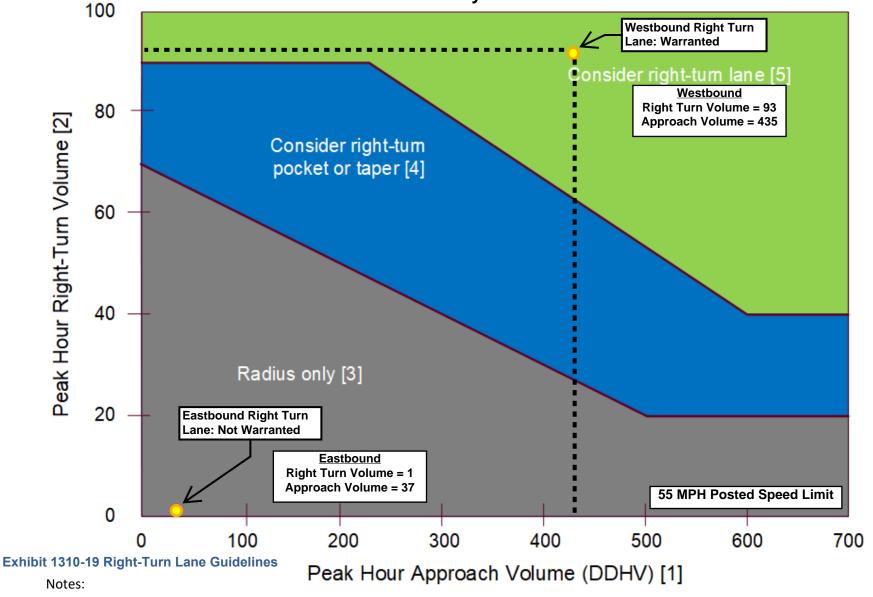
## 2023 Existing - Evening Peak Hour Route 14 at S. Plymouth Road



- [1] For two-lane highways, use the peak hour DDHV (through + right-turn). For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

## 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 14 at S. Plymouth Road

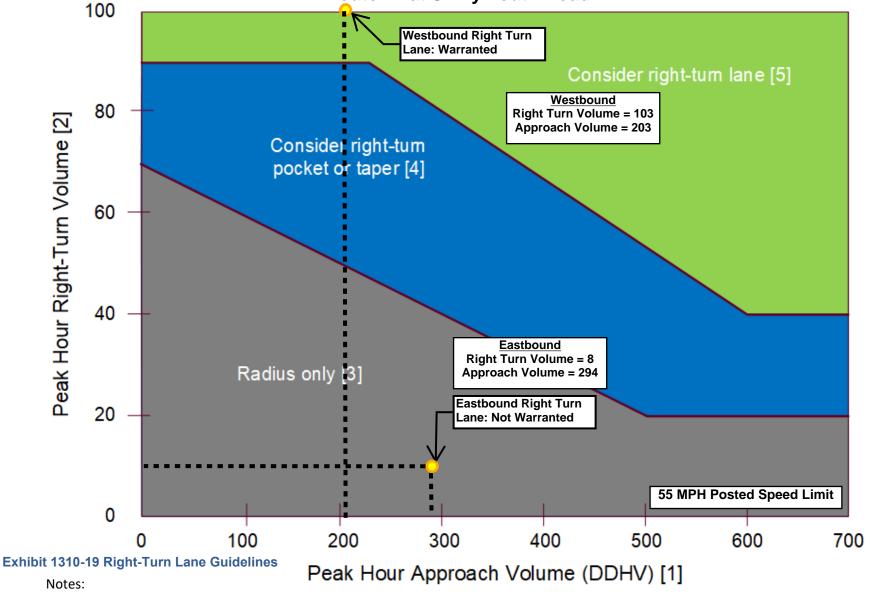


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

# 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 14 at S. Plymouth Road



- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

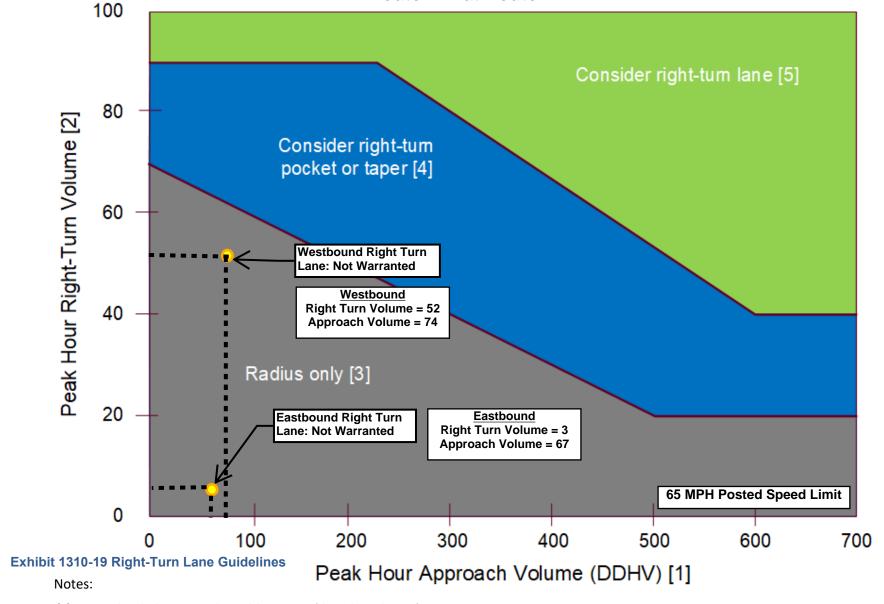
- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

#### 2023 Existing - Morning Peak Hour Route 221 at Route 14 100 Westbound Right Turn Lane: Warranted Consider right-turn lane [5] Westbound 80 Right Turn Volume = 134 Peak Hour Right-Turn Volume [2] Approach Volume = 229 Consider right-tum pocket or taper [4] 60 Eastbound **Right Turn Volume = 0** Approach Volume = 8 40 Eastbound Right Turn Lane: Not Warranted Radius only [3] 20 **65 MPH Posted Speed Limit** 0 100 200 300 400 500 600 700 **Exhibit 1310-19 Right-Turn Lane Guidelines** Peak Hour Approach Volume (DDHV) [1] Notes:

- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).
  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

## 2023 Existing - Evening Peak Hour Route 221 at Route 14



- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

#### 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 221 at Route 14 100 Westbound Right Turn Lane: Warranted Consider right-turn lane [5] Westbound 80 Right Turn Volume = 137 Peak Hour Right-Turn Volume [2] Approach Volume = 234 Consider right-tum pocket or taper [4] 60 Eastbound **Right Turn Volume = 0** Approach Volume = 8 40 Eastbound Right Turn Lane: Not Warranted Radius only [3] 20 **65 MPH Posted Speed Limit** 0 100 200 300 400 500 600 700

Peak Hour Approach Volume (DDHV) [1]

Notes:
[1] For two-lane highways, use the peak hour DDHV (through + right-turn).

For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).

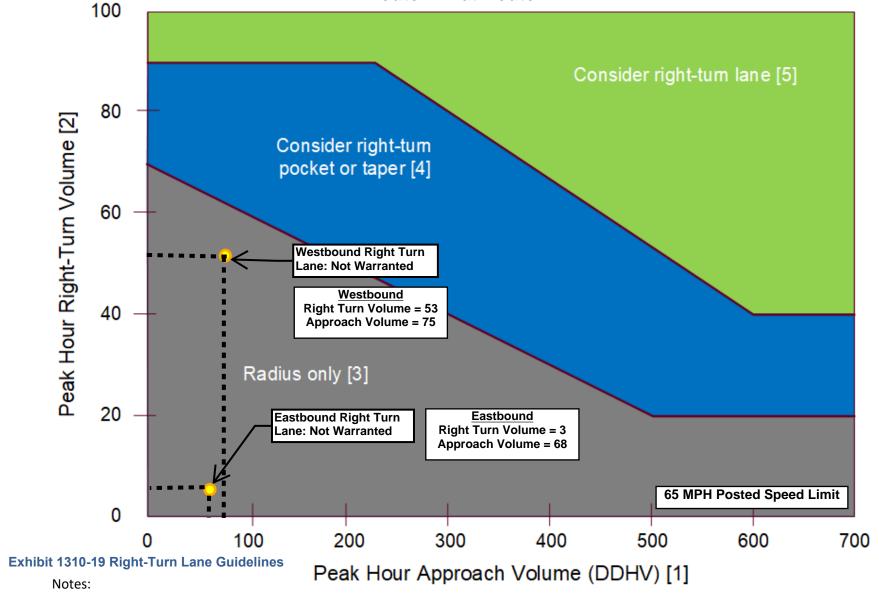
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below

**Exhibit 1310-19 Right-Turn Lane Guidelines** 

- The right-turn volume is greater than 40 VPH
- o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 221 at Route 14

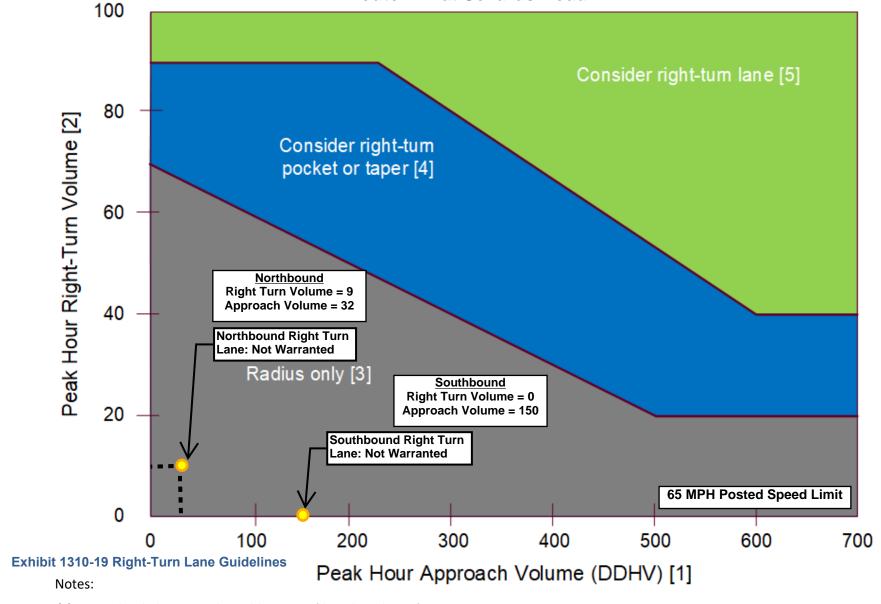


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
- [2] When all three of the following conditions are met, reduce the right-turn DDHV by 20:
  - o The posted speed is 45 mph or below
  - The right-turn volume is greater than 40 VPH
  - o The peak hour approach volume (DDHV) is less than 300 VPH

- [3] For right-turn corner design, see Exhibit 1310-6.
- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

## 2023 Existing - Morning Peak Hour Route 221 at Sellards Road

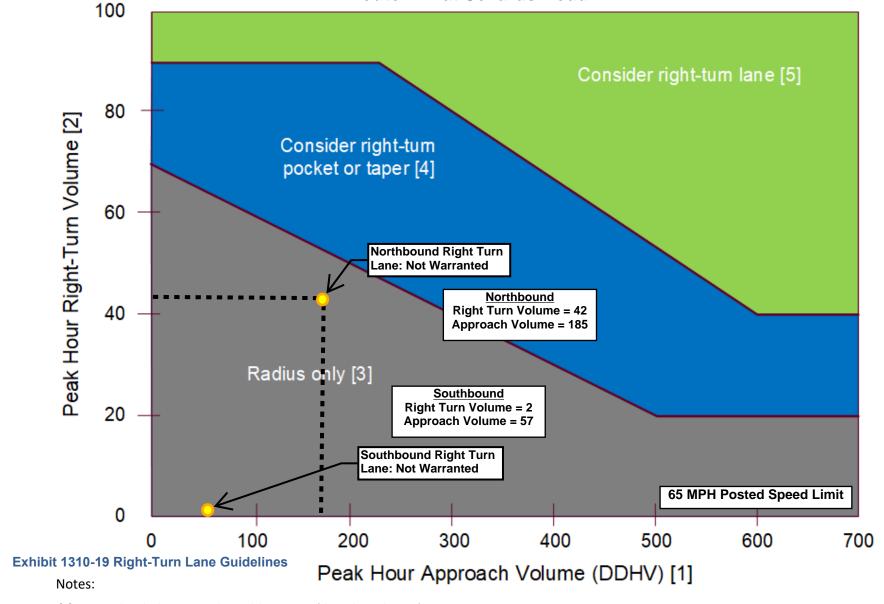


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- [4] For right-turn pocket or taper design, see Exhibit 1310-20.
- [5] For right-turn lane design, see Exhibit 1310-21.

## 2023 Existing - Evening Peak Hour Route 221 at Sellards Road

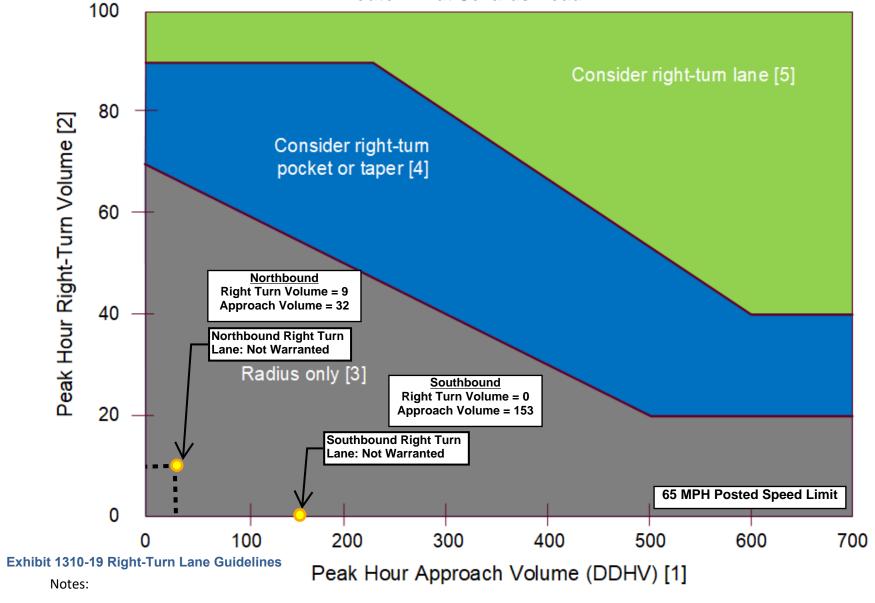


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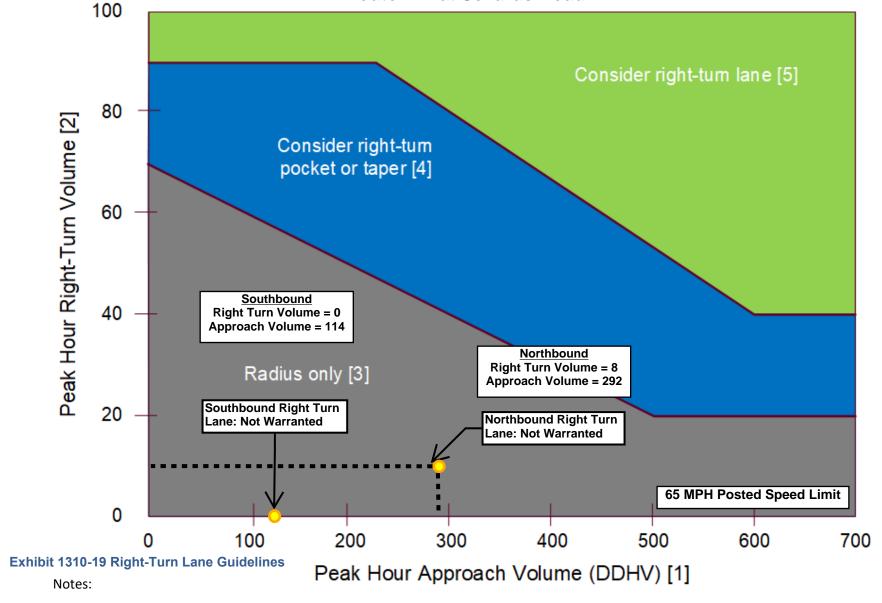
## 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Route 221 at Sellards Road



- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).
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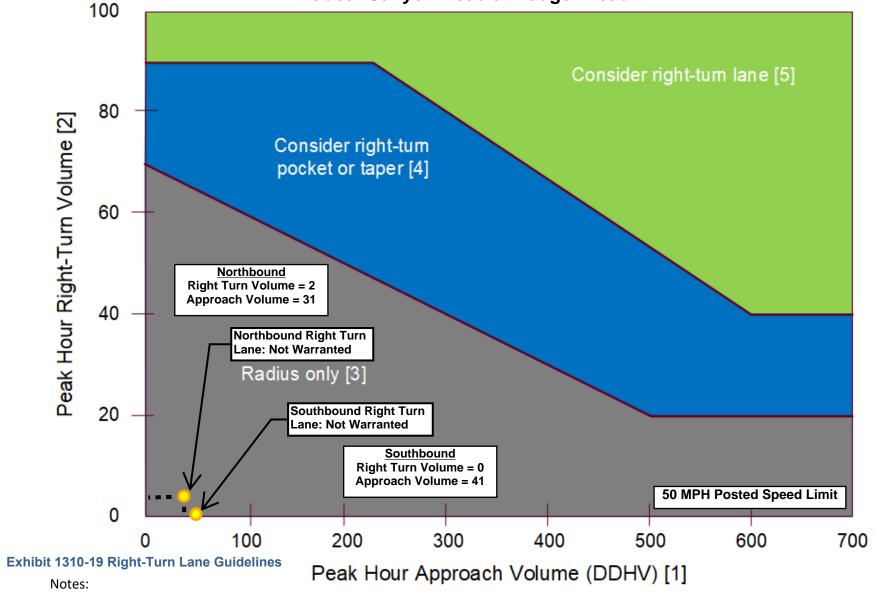
## 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Route 221 at Sellards Road



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# 2023 Existing - Morning Peak Hour Webber Canyon Road at Badger Road

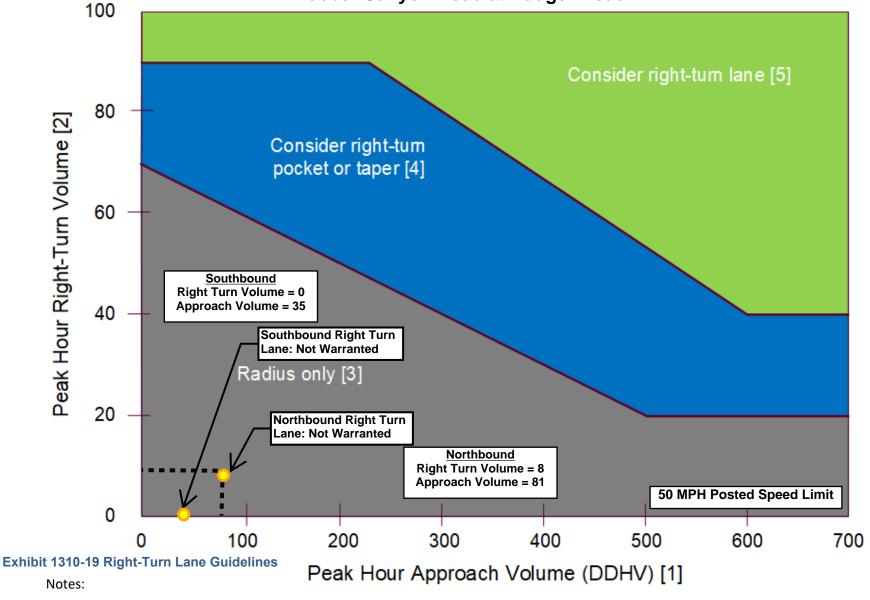


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

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# 2023 Existing - Evening Peak Hour Webber Canyon Road at Badger Road

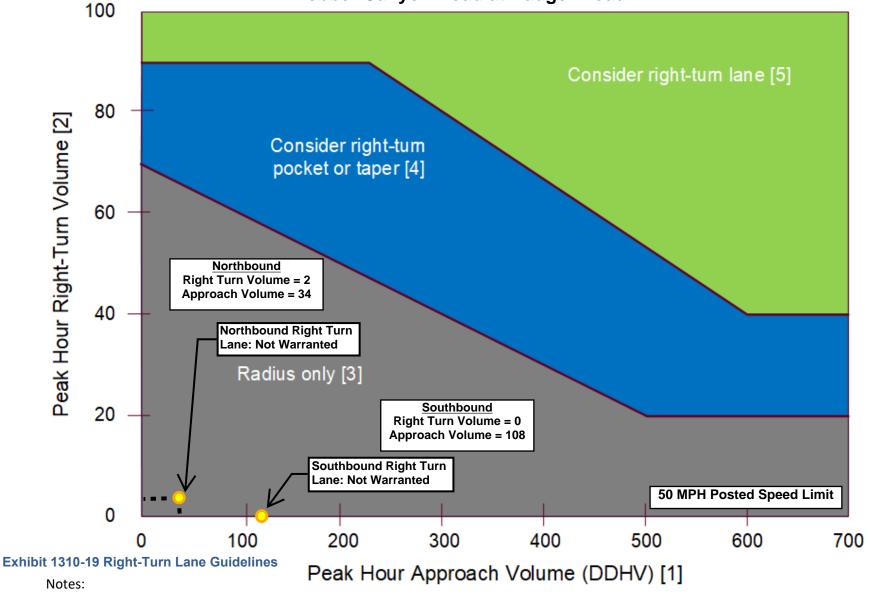


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

  For multilane, highways (posted speed 45 mph or above), use the right-lane peak hour approach volume (through + right-turn).
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# 2025 Phase 1 to Laydown Yard 2 - Morning Peak Hour Webber Canyon Road at Badger Road

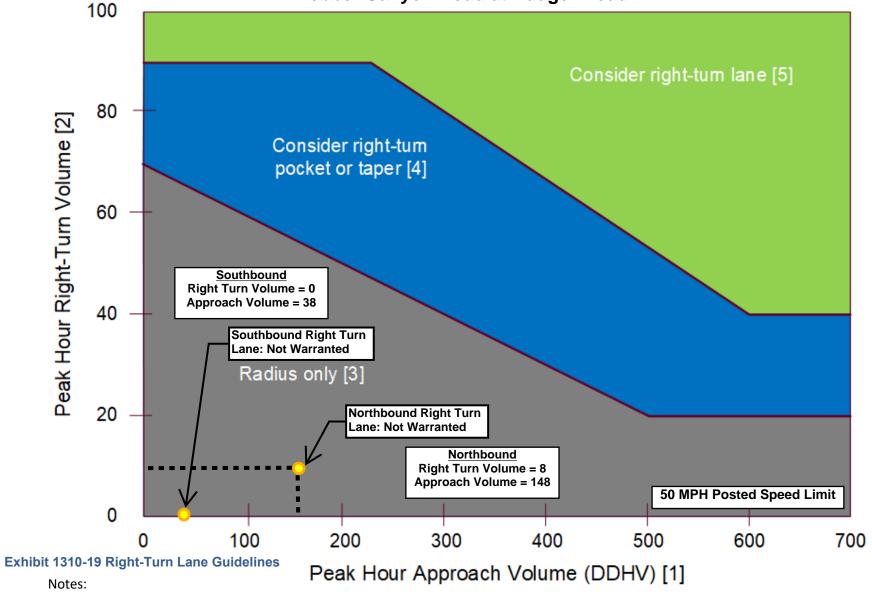


- [1] For two-lane highways, use the peak hour DDHV (through + right-turn).

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# 2025 Phase 1 to Laydown Yard 2 - Evening Peak Hour Webber Canyon Road at Badger Road

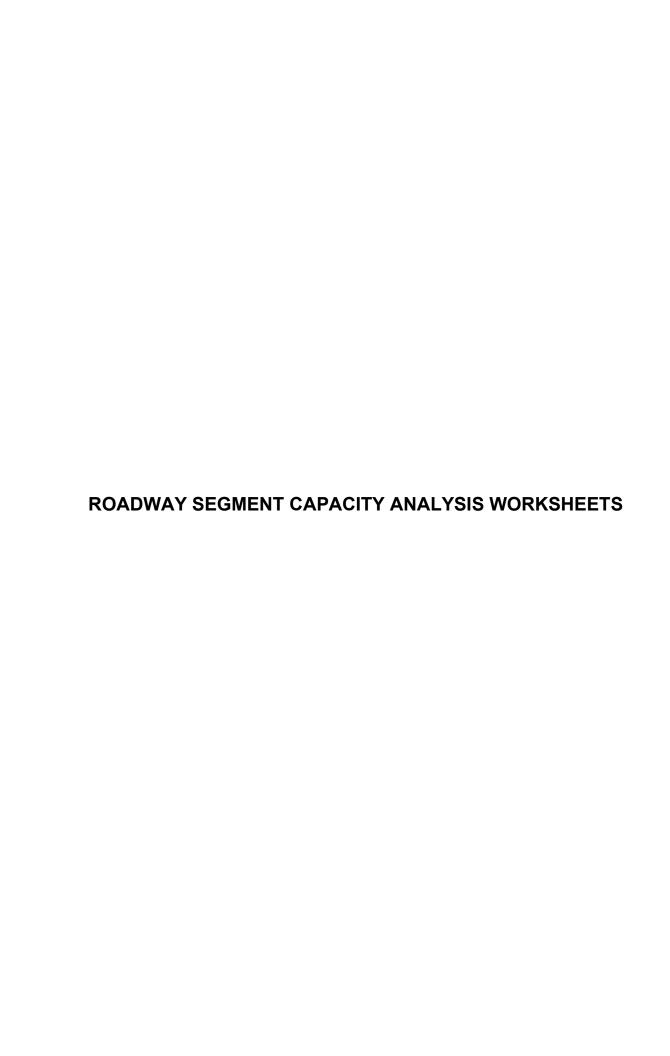


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  - o The posted speed is 45 mph or below
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  - o The peak hour approach volume (DDHV) is less than 300 VPH

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#### APPENDIX H: ROADWAY SEGMENT CAPACITY WORKSHEET





		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	[	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2023
Jurisdicti	ion		1	Time Analyzed		Morning Peak Hour
Project [	Description	Badger Canyon Road Northbound (North of Sellards Road)		Units		U.S. Customary
		S	egme	ent 1		
Vehicl	le Inputs					
Segment	t Type	Passing Zone	L	ength, ft		5280
Lane Wid	dth, ft	11	9	Shoulder Width, f	t	0
Speed Li	imit, mi/h	40	A	Access Point Density, pts/mi		0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4		Opposing Deman	d Flow Rate, veh/h	2
Peak Ho	ur Factor	0.50		Total Trucks, %		0.00
Segment	t Capacity, veh/h	1700	[	Demand/Capacity	/ (D/C)	0.00
Intern	nediate Results					
Segment	t Vertical Class	1	F	ree-Flow Speed,	mi/h	40.8
Speed SI	lope Coefficient (m)	2.38472	5	Speed Power Coefficient (p)		0.65994
PF Slope	e Coefficient (m)	-1.12663	F	PF Power Coefficient (p)		0.77902
In Passin	ng Lane Effective Length?	No	1	Total Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	1-		-	40.8
Vehicl	e Results					
Average	Speed, mi/h	40.8	F	Percent Followers	, %	1.5
Segment	t Travel Time, minutes	1.47	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A						
Facilit	y Results		·			
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	1	0.00			0.0	А
an riaht	© 2023 University of Florida All Rights	Decembed LICC TON I	Highways	Version 2022		Generated: 07/12/2023 10:53:1

		HCS Two-La	ne H	lighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	[	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2023
Jurisdicti	ion		7	Time Analyzed		Afternoon Peak Hour
Project [	Description	Badger Canyon Road Northbound (North of Sellards Road)		Units		U.S. Customary
		S	egme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	L	Length, ft		5280
Lane Wi	dth, ft	11	9	Shoulder Width, f	t	0
Speed Li	imit, mi/h	40	A	Access Point Density, pts/mi		0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	12		Opposing Deman	d Flow Rate, veh/h	1
Peak Ho	ur Factor	0.67		Total Trucks, %		13.00
Segmen	t Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.01
Intern	nediate Results					
Segmen	t Vertical Class	1	F	Free-Flow Speed,	mi/h	40.4
Speed Sl	lope Coefficient (m)	2.35926	9	Speed Power Coefficient (p)		0.66206
PF Slope	e Coefficient (m)	-1.12231	F	PF Power Coefficient (p)		0.78001
In Passin	ng Lane Effective Length?	No	1	Total Segment De	ensity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	ç	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	40.4
Vehicl	le Results					
Average	Speed, mi/h	40.4	F	Percent Followers	, %	3.5
Segmen	t Travel Time, minutes	1.49	F	Follower Density (	(FD), followers/mi/ln	0.0
Vehicle L	Vehicle LOS A					
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	2	0.00			0.0	A
`anıriaht (	© 2023 University of Florida All Rights	Deserved LICCEN I	11° - 1	: Version 2022		Generated: 07/12/2023 10:55:

		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Pate		7/12/2023
Agency		Tetra Tech	А	Analysis Year		2023
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour
Project [	Description	Badger Canyon Road Southbound (North of Sellards Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	shoulder Width, f	t	0
Speed L	imit, mi/h	40	А	Access Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	С	Opposing Demand Flow Rate, veh/h		8
Peak Ho	our Factor	0.25	T	Total Trucks, %		0.00
Segmen	t Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.00
Intern	nediate Results	•				•
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	40.8
Speed S	lope Coefficient (m)	2.39938	S	speed Power Coe	fficient (p)	0.64460
PF Slope	e Coefficient (m)	-1.14011	Р	PF Power Coefficie	ent (p)	0.77498
In Passir	ng Lane Effective Length?	No	T	otal Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	40.8
Vehicl	le Results					·
Average	Speed, mi/h	40.8	Р	ercent Followers,	, %	1.6
Segmen	t Travel Time, minutes	1.47	F	follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	OS A				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A
	@ 2022   Jainerait - of Florido All Diobto			Varsian 2022		Canaratad: 07/12/2022 11:01:2

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	Analysis Year		2023
Jurisdicti	on		Tin	Time Analyzed		Afternoon Peak Hour
Project D	Description	Badger Canyon Road Southbound (North of Sellards Road)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	11	Sh	oulder Width, f	t	0
Speed Li	mit, mi/h	40	Ac	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Opposing D		d Flow Rate, veh/h	32
Peak Hou	ur Factor	0.25	.25 Total 1			0.00
Segment	t Capacity, veh/h	1700	Demand/Capacity (D/C)		0.00	
Interm	nediate Results					
Segment	t Vertical Class	1	Fre	e-Flow Speed,	mi/h	40.8
Speed SI	ope Coefficient (m)	2.42870	Sp	eed Power Coe	fficient (p)	0.61537
PF Slope	Coefficient (m)	-1.16653	PF	Power Coefficie	ent (p)	0.76733
In Passin	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					
Average	Speed, mi/h	40.8	Per	rcent Followers	, %	1.7
Segment	t Travel Time, minutes	1.47	7 Follower Density (FD), followers/mi/ln		0.0	
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	I I		ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

Project I Analyst Agency Jurisdiction Project Desc		James Vorosmarti Tetra Tech Build Ph1 to LY1 Badger				7/12/2023
Agency Jurisdiction		Tetra Tech	Ana			7/12/2022
Jurisdiction				lunia Vana		1/12/2023
		Build Ph1 to LY1 Badger	т:	Analysis Year		2025
Project Des	cription	Build Ph1 to LY1 Badger	I IIM	Time Analyzed		Morning Peak Hour
		Canyon Road Northbou (North of Sellards Road)	nd	Units		U.S. Customary
		Se	gmen	t 1		
Vehicle I	Inputs					
Segment Ty	/pe	Passing Zone	Len	gth, ft		5280
Lane Width,	, ft	11	Sho	ulder Width, ft	i	0
Speed Limit	t, mi/h	40	Acc	ess Point Dens	ity, pts/mi	0.0
Demand	l and Capacity					
Directional	Demand Flow Rate, veh/h	4	Opposing Demand Flow Rate, veh/h		2	
Peak Hour F	Factor	0.50	Total Trucks, %		0.00	
Segment Ca	apacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.00
Interme	diate Results					
Segment Ve	ertical Class	1	Free	e-Flow Speed,	mi/h	40.8
Speed Slope	e Coefficient (m)	2.38472	Spe	ed Power Coet	fficient (p)	0.65994
PF Slope Co	pefficient (m)	-1.12663	PF F	Power Coefficie	ent (p)	0.77902
In Passing L	ane Effective Length?	No	Tota	l Segment De	nsity, veh/mi/ln	0.0
%Improvem	nent to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subsegn	ment Data					
# Segm	nent Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tange	ent	5280	-		-	40.8
Vehicle I	Results					
Average Spe	eed, mi/h	40.8	Perd	cent Followers,	%	1.5
Segment Tra	avel Time, minutes			FD), followers/mi/ln	0.0	
Vehicle LOS		А				
Facility F	Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	1	0.00			0.0	A

Analyst Agency Jurisdiction Project Descr	nformation	James Vorosmarti				
Agency Jurisdiction Project Descr		James Vorosmarti				
Jurisdiction Project Descr			Dat	e		7/12/2023
Project Descr		Tetra Tech	Ana	Analysis Year		2025
			Tim	Time Analyzed		Afternoon Peak Hour
	ription	Build Ph1 to LY1 Badger Canyon Road Northbou (North of Sellards Road)	nd	Units		U.S. Customary
		Se	gmen	t 1		
Vehicle Ir	nputs					
Segment Typ	De .	Passing Zone	Len	gth, ft		5280
Lane Width,	ft	11	Sho	oulder Width, f	t	0
Speed Limit,	mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demand	and Capacity					
Directional D	Demand Flow Rate, veh/h	12	Орі	Opposing Demand Flow Rate, veh/h		1
Peak Hour Fa	actor	0.67	Tota	Total Trucks, %		13.00
Segment Cap	pacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Intermed	liate Results					
Segment Ver	rtical Class	1	Free	e-Flow Speed,	mi/h	40.4
Speed Slope	Coefficient (m)	2.35926	Spe	ed Power Coe	fficient (p)	0.66206
PF Slope Coe	efficient (m)	-1.12231	PF I	Power Coefficie	ent (p)	0.78001
In Passing La	ane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improveme	ent to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subsegm	nent Data					
# Segme	ent Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tanger	nt	5280	-		-	40.4
Vehicle R	Results	<u>'</u>				
Average Spe	ed, mi/h	40.4	Per	cent Followers,	, %	3.5
Segment Tra	vel Time, minutes	1.49 Follower Density (FD), followers/mi/li		FD), followers/mi/ln	0.0	
Vehicle LOS		А				
Facility R	Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	2	0.00			0.0	A

		port	hway Re	ne H	HCS Two-Lar		
						t Information	Projec
	7/12/2023		e	I	James Vorosmarti		Analyst
	2025	Analysis Year		,	Tetra Tech		Agency
k Hour	Morning Peak H	Time Analyzed		1		on	Jurisdictio
ıry	U.S. Customary	Units		ound	Build Ph1 to LY1 Badge Canyon Road Southbo (North of Sellards Road	escription	Project D
			t 1	egm	Se		
						e Inputs	Vehicle
	5280		gth, ft	ı	Passing Zone	Туре	Segment
	0	t	ulder Width, ft	!	11	th, ft	Lane Wid
	0.0	ity, pts/mi	ess Point Dens	,	40	nit, mi/h	Speed Lir
						nd and Capacity	Demar
	8	Opposing Demand Flow Rate, veh/h			4	al Demand Flow Rate, veh/h	Direction
	0.00	Total Trucks, %		-	0.25	ır Factor	Peak Hou
	0.00	Demand/Capacity (D/C)		ı	1700	Capacity, veh/h	Segment
	•					ediate Results	Interm
	40.8	mi/h	e-Flow Speed,	T	1	Vertical Class	Segment
	0.64460	fficient (p)	ed Power Coef		2.39938	ope Coefficient (m)	Speed Slo
	0.77498	ent (p)	ower Coefficie	1	-1.14011	Coefficient (m)	PF Slope
	0.0	nsity, veh/mi/ln	I Segment De	-	No	g Lane Effective Length?	In Passing
	0.0	Speed	provement to	(	0.0	ement to Percent Followers	%Improv
	·					gment Data	Subsec
ed, mi/h	Average Speed	Superelevation, %	t .	Radiu	Length, ft	gment Type	# Seg
	40.8	-		-	5280	gent	1 Tan
						e Results	Vehicle
	1.6	%	ent Followers,	1	40.8	Speed, mi/h	Average S
	0.0	FD), followers/mi/ln			Travel Time, minutes	Segment	
					А	OS	Vehicle LO
				,		/ Results	Facility
;	LOS	Follower Density, followers/		VHD veh-h/p		VMT veh-mi/AP	Т
	A	0.0			0.00	0	1
3	0.0	FD), followers/mi/ln ensity, followers/ mi/ln	Follower De		1.47 A VHD veh-h/p	Speed, mi/h Travel Time, minutes OS  / Results  VMT veh-mi/AP	Average S Segment Vehicle LO Facility T

		TICS TWO Latt	e Hig	ıhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	Analysis Year		2025
Jurisdiction	on		Tim	Time Analyzed		Afternoon Peak Hour
Project D	Description	Build Ph1 to LY1 Badger Canyon Road Southbou (North of Sellards Road)	nd	Units		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Len	gth, ft		5280
Lane Wid	lth, ft	11	Sho	ulder Width, ft	t	0
Speed Lir	mit, mi/h	40	Acc	ess Point Dens	ity, pts/mi	0.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	4	Орр	Opposing Demand Flow Rate, veh/h		32
Peak Hou	ur Factor	0.25	Tota	Total Trucks, %		0.00
Segment	: Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	: Vertical Class	1	Free	e-Flow Speed,	mi/h	40.8
Speed Slo	ope Coefficient (m)	2.42870	Spe	ed Power Coef	fficient (p)	0.61537
PF Slope	Coefficient (m)	-1.16653	PF F	Power Coefficie	ent (p)	0.76733
In Passing	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	40.8
Vehicle	e Results					
Average :	Speed, mi/h	40.8	Perd	cent Followers,	%	1.7
Segment	Travel Time, minutes	1.47	Foll	Follower Density (FD), followers/mi/ln		0.0
Vehicle Lo	OS	А				
Facility	y Results	•				·
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	Analysis Year		2025
Jurisdiction	on		Tir	Time Analyzed		Morning Peak Hour
Project D	escription	Build Ph1 to LY2 Badg Canyon Road Northbo (North of Sellards Roa	ound	Units		U.S. Customary
		S	egme	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ength, ft		5280
Lane Wid	lth, ft	11	Sh	noulder Width, f	t	0
Speed Lir	mit, mi/h	40	Ac	ccess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	4	Oı	Opposing Demand Flow Rate, veh/h		2
Peak Hou	ur Factor	0.50	То	Total Trucks, %		0.00
Segment	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	Vertical Class	1	Fre	ee-Flow Speed,	mi/h	40.8
Speed Slo	ope Coefficient (m)	2.38472	Sp	peed Power Coe	fficient (p)	0.65994
PF Slope	Coefficient (m)	-1.12663	PF	Power Coefficie	ent (p)	0.77902
In Passing	g Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	ement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	gment Data	·				
# Seg	gment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					
Average	Speed, mi/h	40.8	Pe	ercent Followers	, %	1.5
Segment			ollower Density (	(FD), followers/mi/ln	0.0	
Vehicle L	OS	A				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
		<del>-</del>				

		HCS Two-Lan	e Hig	ıhway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	Analysis Year		2025
Jurisdict	tion		Tim	Time Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY2 Badger Canyon Road Northbou (North of Sellards Road)	nd	Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	idth, ft	11	Sho	ulder Width, f	t	0
Speed L	imit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	12	Орр	Opposing Demand Flow Rate, veh/h		1
Peak Ho	our Factor	0.67	Tota	Total Trucks, %		13.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Intern	nediate Results					·
Segmen	nt Vertical Class	1	Free	e-Flow Speed,	mi/h	40.4
Speed S	Slope Coefficient (m)	2.35926	Spe	ed Power Coe	fficient (p)	0.66206
PF Slope	e Coefficient (m)	-1.12231	PF F	Power Coefficie	ent (p)	0.78001
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	40.4
Vehic	le Results					
Average	e Speed, mi/h	40.4	Perd	cent Followers,	, %	3.5
Segmen	nt Travel Time, minutes			(FD), followers/mi/ln	0.0	
Vehicle I	LOS	А				
Facilit	ty Results		,			
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	2	0.00			0.0	A

		HCS Two-Lar	ne Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdiction	on		Tim	Time Analyzed		Morning Peak Hour
Project D	escription	Build Ph1 to LY2 Badge Canyon Road Southbou (North of Sellards Road	und	Units		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Len	gth, ft		5280
Lane Wid	lth, ft	11	Sho	ulder Width, ft		0
Speed Lir	mit, mi/h	40	Acc	ess Point Dens	ity, pts/mi	0.0
Demar	nd and Capacity					·
Direction	al Demand Flow Rate, veh/h	4	Орр	Opposing Demand Flow Rate, veh/h		8
Peak Hou	ır Factor	0.25	Tota	Total Trucks, %		0.00
Segment	Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	Vertical Class	1	Free	e-Flow Speed,	mi/h	40.8
Speed Slo	ope Coefficient (m)	2.39938	Spe	ed Power Coef	fficient (p)	0.64460
PF Slope	Coefficient (m)	-1.14011	PF P	ower Coefficie	ent (p)	0.77498
In Passing	g Lane Effective Length?	No	Tota	ıl Segment Dei	nsity, veh/mi/ln	0.0
%Improv	ement to Percent Followers	0.0	%In	provement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, ft	t	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	40.8
Vehicle	e Results					
Average :	Speed, mi/h	40.8	Perc	ent Followers,	%	1.6
Segment Travel Time, minutes 1.47 Follower Density (FD), followers/mi/ln		0.0				
Vehicle Lo	OS	А				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	А

		HCS Two-La	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	Analysis Year		2025
Jurisdiction	on		Tin	Time Analyzed		Afternoon Peak Hour
Project D	Description	Build Ph1 to LY2 Badge Canyon Road Southbo (North of Sellards Road	und	Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Ler	ngth, ft		5280
Lane Wid	dth, ft	11	Sho	oulder Width, f	t	0
Speed Lir	mit, mi/h	40	Acc	cess Point Dens	sity, pts/mi	0.0
Demar	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	4 Opposin		d Flow Rate, veh/h	32
Peak Hou	ur Factor	0.25	0.25 Total Tr			0.00
Segment	Capacity, veh/h	1700	Demand/Capacity (D/C)		0.00	
Interm	nediate Results					
Segment	: Vertical Class	1	Fre	e-Flow Speed,	mi/h	40.8
Speed Slo	ope Coefficient (m)	2.42870	Spe	Speed Power Coefficient (p)		0.61537
PF Slope	Coefficient (m)	-1.16653	PF	PF Power Coefficient (p)		0.76733
In Passing	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	gment Data					·
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	40.8
Vehicle	e Results					
Average :	Speed, mi/h	40.8	Per	cent Followers	, %	1.7
Segment	Travel Time, minutes	1.47	1.47 Follower Density (FD), followers/m		(FD), followers/mi/ln	0.0
Vehicle LO	OS	А				
Facility	y Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
	Veii-iiii/Ai					

		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2025
Jurisdicti	ion		Tir	me Analyzed		Morning Peak Hour
Project D	Description	No Build Badger Canyon Road Northbound (Nort of Sellards Road)		Units		U.S. Customary
		Se	gme	nt 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wid	dth, ft	11	Sh	noulder Width, f	t	0
Speed Li	mit, mi/h	40	Ac	ccess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		2
Peak Hour Factor		0.50		Total Trucks, %		0.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.00
Intern	nediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		40.8
Speed Slope Coefficient (m)		2.38472		Speed Power Coefficient (p)		0.65994
PF Slope Coefficient (m)		-1.12663		PF Power Coefficient (p)		0.77902
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-	-		40.8
Vehicl	e Results					
Average Speed, mi/h		40.8		Percent Followers, %		1.5
Segment Travel Time, minutes		1.47		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A

		HCS Two-Lar	ne Hig	Jhway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	ion		Tim	e Analyzed		Afternoon Peak Hour
Project [	Description	No Build Badger Canyo Road Northbound (Nor of Sellards Road)		Units		U.S. Customary
		Se	gmen	t 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	Len	Length, ft		5280
Lane Wi	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	imit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	12		Opposing Demand Flow Rate, veh/h		1
Peak Hour Factor		0.67		Total Trucks, %		13.00
Segmen	t Capacity, veh/h	1700 Den		emand/Capacity (D/C)		0.01
Intern	nediate Results					·
Segment Vertical Class		1	Free	Free-Flow Speed, mi/h		40.4
Speed Slope Coefficient (m)		2.35926	Spe	Speed Power Coefficient (p)		0.66206
PF Slope Coefficient (m)		-1.12231	PF I	PF Power Coefficient (p)		0.78001
In Passing Lane Effective Length?		No	Tota	Total Segment Density, veh/mi/ln		0.0
%Improv	vement to Percent Followers 0.0 %Improvement to Speed		Speed	0.0		
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	40.4
Vehicl	le Results					
Average Speed, mi/h		40.4		Percent Followers, %		3.5
Segment Travel Time, minutes		1.49		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		Α				
Facilit	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	2	0.00			0.0	A
Facility Results  T VMT VHD veh-mi/AP veh-h/p				mi/ln		

		HCS Two-Lar	ne Hig	Jhway Re	port				
Projec	ct Information								
Analyst		James Vorosmarti	Dat	e		7/12/2023			
Agency		Tetra Tech	Ana	lysis Year		2025			
Jurisdict	ion		Tim	e Analyzed		Morning Peak Hour			
Project I	Description	No Build Badger Canyon Road Southbound (Nor of Sellards Road)		Units		U.S. Customary			
Segment 1									
Vehic	le Inputs								
Segmen	t Type	Passing Zone	Len	gth, ft		5280			
Lane Wi	dth, ft	11	Sho	ulder Width, f	t	0			
Speed L	imit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0			
Dema	and Capacity								
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		8			
Peak Hour Factor		0.25		Total Trucks, %		0.00			
Segmen	egment Capacity, veh/h 1700		Der	Demand/Capacity (D/C)		0.00			
Intern	nediate Results					<u> </u>			
Segment Vertical Class		1	Fre	Free-Flow Speed, mi/h		40.8			
Speed Slope Coefficient (m)		2.39938	Spe	Speed Power Coefficient (p)		0.64460			
PF Slope Coefficient (m)		-1.14011	PF I	PF Power Coefficient (p)		0.77498			
In Passing Lane Effective Length?		No	Tota	Total Segment Density, veh/mi/ln		0.0			
%Impro	vement to Percent Followers	ement to Percent Followers 0.0 %Improvement to Speed		Speed	0.0				
Subse	egment Data								
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h			
1 Ta	ingent	5280	-		-	40.8			
Vehic	le Results								
Average Speed, mi/h		40.8		Percent Followers, %		1.6			
Segment Travel Time, minutes		1.47		Follower Density (FD), followers/mi/ln		0.0			
Vehicle LOS		А							
Facilit	ty Results								
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	0	0.00			0.0	A			

		HCS Two-Lar	ne H	lighway Re	port			
Projec	ct Information							
Analyst		James Vorosmarti		Date		7/12/2023		
Agency		Tetra Tech	1	Analysis Year		2025		
Jurisdict	ion		7	Time Analyzed		Afternoon Peak Hour		
Project [	Description	No Build Badger Canyo Road Southbound (No of Sellards Road)		Units		U.S. Customary		
Segment 1								
Vehicl	le Inputs							
Segmen	nt Type	Passing Zone		Length, ft		5280		
Lane Wi	dth, ft	11	Shoulder Width, ft		t	0		
Speed L	imit, mi/h	40	1	Access Point Dens	sity, pts/mi	0.0		
Dema	and Capacity					·		
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		32		
Peak Ho	our Factor	0.25		Total Trucks, %		0.00		
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.00		
Intern	nediate Results	•						
Segment Vertical Class		1		Free-Flow Speed, mi/h		40.8		
Speed Slope Coefficient (m)		2.42870		Speed Power Coefficient (p)		0.61537		
PF Slope Coefficient (m)		-1.16653		PF Power Coefficient (p)		0.76733		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ingent	5280	-		-	40.8		
Vehicl	le Results					·		
Average Speed, mi/h		40.8		Percent Followers, %		1.7		
Segment Travel Time, minutes		1.47		Follower Density (FD), followers/mi/ln		0.0		
Vehicle LOS		А						
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS		
1	0	0.00			0.0	A		
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		HCS Two-Lan	ne Hig	Jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Morning Peak Hour
Project D	Description	Build Ph2 to LY2 Badger Canyon Road Northbou (North of Sellards Road)	ınd	ts		U.S. Customary
		Se	gmen	t 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wid	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	oosing Deman	d Flow Rate, veh/h	2
Peak Ho	ur Factor	0.50	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Der	mand/Capacity	/ (D/C)	0.00
Intern	nediate Results					
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.8
Speed SI	lope Coefficient (m)	2.38472	Spe	Speed Power Coefficient (p)		0.65994
PF Slope	Coefficient (m)	-1.12663	PF F	PF Power Coefficient (p)		0.77902
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicl	e Results					<u>'</u>
Average	Speed, mi/h	40.8	Perd	cent Followers,	, %	1.5
Segment Travel Time, minutes 1.47		1.47	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	, , , , , , , , , , , , , , , , , , ,		LOS	
1	1	0.00			0.0	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	Ph2 to LY 2 Badger Canyon Road Northbou (North of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	туре	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	12	Орр	oosing Deman	d Flow Rate, veh/h	1
Peak Hou	ur Factor	0.67	Tota	al Trucks, %		13.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.01
Interm	nediate Results					
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.4
Speed SI	ope Coefficient (m)	2.35926	Spe	Speed Power Coefficient (p)		0.66206
PF Slope	Coefficient (m)	-1.12231	PF F	PF Power Coefficient (p)		0.78001
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%lmprov	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.4
Vehicle	e Results					
Average	Speed, mi/h	40.4	Perd	cent Followers,	, %	3.5
Segment Travel Time, minutes 1.49		1.49	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	2	0.00			0.0	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Badger Canyo Road Southbound (Nort of Sellards Road)		ts		U.S. Customary
		Seg	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	ity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	oosing Deman	d Flow Rate, veh/h	8
Peak Hou	ur Factor	0.25	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.00
Interm	nediate Results					
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	40.8
Speed SI	lope Coefficient (m)	2.39938	Spe	Speed Power Coefficient (p)		0.64460
PF Slope	Coefficient (m)	-1.14011	PF F	PF Power Coefficient (p)		0.77498
In Passin	g Lane Effective Length?	No	Tota	l Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# See	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					
Average	Speed, mi/h	40.8	Pero	cent Followers,	%	1.6
Segment Travel Time, minutes 1.47		1.47	Follo	ower Density (	FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	1 2: 1		LOS	
1	0	0.00			0.0	A

		HCS Two-Lan	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	Ph 2 to LY2 Badger Canyon Road Southbou (North of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орг	Opposing Demand Flow Rate, veh/h		32
Peak Hou	ur Factor	0.25	0.25 Total Trucks, %			0.00
Segment	t Capacity, veh/h	1700	Der	mand/Capacity	(D/C)	0.00
Interm	nediate Results					<u>.</u>
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.8
Speed SI	lope Coefficient (m)	2.42870	Spe	Speed Power Coefficient (p)		0.61537
PF Slope	Coefficient (m)	-1.16653	PF F	PF Power Coefficient (p)		0.76733
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					
Average	Speed, mi/h	40.8	Per	cent Followers,	, %	1.7
Segment Travel Time, minutes 1.47		1.47	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	0	0.00			0.0	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Morning Peak Hour
Project D	Description	No Build - Badger Canyo Road Northbound (Nort of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	posing Demand Flow Rate, veh/h		2
Peak Hou	ur Factor	0.50	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.00
Interm	nediate Results					<u> </u>
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.8
Speed SI	ope Coefficient (m)	2.38472	Spe	Speed Power Coefficient (p)		0.65994
PF Slope	Coefficient (m)	-1.12663	PF F	PF Power Coefficient (p)		0.77902
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					·
Average	Speed, mi/h	40.8	Pero	cent Followers	, %	1.5
Segment Travel Time, minutes 1.47		1.47	Follo	ower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A						
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	1	0.00			0.0	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	No Build - Badger Canyo Road Northbound (Nort of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	12	Орр	oosing Deman	d Flow Rate, veh/h	1
Peak Hou	ur Factor	0.67	Tota	al Trucks, %		13.00
Segment	t Capacity, veh/h	1700	Der	nand/Capacity	(D/C)	0.01
Interm	nediate Results					
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.4
Speed SI	ope Coefficient (m)	2.35926	Spe	Speed Power Coefficient (p)		0.66206
PF Slope	Coefficient (m)	-1.12231	PF F	PF Power Coefficient (p)		0.78001
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.4
Vehicle	e Results					·
Average	Speed, mi/h	40.4	Perd	cent Followers,	, %	3.5
Segment Travel Time, minutes 1.49		1.49	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/In		LOS
1	2	0.00			0.0	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	No Build - Badger Canyo Road Southbound (Nort of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	т Туре	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	pposing Demand Flow Rate, veh/h		8
Peak Hou	ur Factor	0.25	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Der	nand/Capacity	(D/C)	0.00
Interm	nediate Results					
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	40.8
Speed SI	ope Coefficient (m)	2.39938	Spe	Speed Power Coefficient (p)		0.64460
PF Slope	Coefficient (m)	-1.14011	PF F	PF Power Coefficient (p)		0.77498
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%lmprov	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	40.8	Perd	cent Followers,	, %	1.6
3 1		1.47	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	0	0.00			0.0	A

		HCS Two-Lan	ie Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	No Build - Badger Canyo Road Southbound (Nort of Sellards Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	40	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	posing Demand Flow Rate, veh/h		32
Peak Hou	ur Factor	0.25	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.00
Interm	nediate Results					
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		40.8
Speed SI	ope Coefficient (m)	2.42870	Spe	Speed Power Coefficient (p)		0.61537
PF Slope	Coefficient (m)	-1.16653	PF F	PF Power Coefficient (p)		0.76733
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	40.8
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	40.8	Pero	cent Followers,	, %	1.7
Segment Travel Time, minutes 1.47		1.47	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	Follower Density, followers/		LOS	
1	0	0.00			0.0	A



		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	ion		Tin	ne Analyzed		Morning Peak Hour
Project D	Description	Bofer Canyon Road Northbound (North of Coffin Road)	Un	iits		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	16	Sh	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	0	Op	posing Deman	d Flow Rate, veh/h	1
Peak Hou	ur Factor	0.94	Tot	tal Trucks, %		0.00
Segment	t Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.00
Interm	nediate Results	•				
Segment	t Vertical Class	3	Fre	Free-Flow Speed, mi/h		55.2
Speed SI	lope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.71618
PF Slope	Coefficient (m)	-1.05274	PF	PF Power Coefficient (p)		0.81129
In Passin	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data		·			
# See	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average	Speed, mi/h	55.2	Pei	rcent Followers	, %	0.0
Segment Travel Time, minutes 1.09		1.09	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	0	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	ion		Tir	ne Analyzed		Afternoon Peak Hour
Project D	Description	Bofer Canyon Road Northbound (North of Coffin Road)	Un	iits		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	16	Sh	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	8	Op	posing Deman	d Flow Rate, veh/h	0
Peak Hou	ur Factor	0.63	0.63 Total Trucks, %		0.00	
Segment	t Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.00
Interm	nediate Results	•				
Segment	t Vertical Class	3	Fre	ee-Flow Speed,	mi/h	55.2
Speed SI	lope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.73607
PF Slope	Coefficient (m)	-1.04027	PF	PF Power Coefficient (p)		0.81504
In Passin	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data		·			
# See	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average	Speed, mi/h	55.2	Pe	rcent Followers	, %	2.0
Segment Travel Time, minutes 1.09		1.09	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	· · · · · · · · · · · · · · · · · · ·		LOS	
1	1	0.00			0.0	A

		HCS Two-Lar	ne Hi	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Da	nte		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2023
Jurisdict	tion		Tir	ne Analyzed		Morning Peak Hour
Project	Description	Bofer Canyon Road Southbound (North of Coffin Road)		nits		U.S. Customary
		Se	egmei	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ngth, ft		5280
Lane W	idth, ft	16	Sh	oulder Width, f	t	0
Speed L	_imit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	4	Op	Opposing Demand Flow Rate, veh/h		0
Peak Ho	our Factor	0.25	To	tal Trucks, %		0.00
Segmer	nt Capacity, veh/h	1700	De	emand/Capacity	/ (D/C)	0.00
Interr	mediate Results					
Segmer	nt Vertical Class	3		ee-Flow Speed,	mi/h	55.2
Speed S	Slope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.73607
PF Slop	e Coefficient (m)	-1.04027	PF	PF Power Coefficient (p)		0.81504
In Passii	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.0
%lmpro	ovement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.2
Vehic	le Results					<u>'</u>
Average	e Speed, mi/h	55.2	Pe	rcent Followers,	, %	1.1
Segment Travel Time, minutes		1.09	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	0	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port	
Project I	Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	Analysis Year		2023
Jurisdiction			Т	ime Analyzed		Afternoon Peak Hour
Project Des	cription	Bofer Canyon Road Southbound (North of Coffin Road)		Jnits		U.S. Customary
		Se	egme	ent 1		
Vehicle I	Inputs					
Segment Ty	/pe	Passing Zone	L	ength, ft		5280
Lane Width	, ft	16	S	shoulder Width, f	t	0
Speed Limit	t, mi/h	50	А	Access Point Dens	sity, pts/mi	0.0
Demand	l and Capacity					
Directional	Demand Flow Rate, veh/h	0	С	Dpposing Deman	d Flow Rate, veh/h	5
Peak Hour F	Factor	0.94	To	otal Trucks, %		0.00
Segment Ca	apacity, veh/h	1700	D	Demand/Capacity	/ (D/C)	0.00
Interme	diate Results					
Segment Ve	ertical Class	3		ree-Flow Speed,	mi/h	55.2
Speed Slop	e Coefficient (m)	3.11550	S	Speed Power Coefficient (p)		0.69241
PF Slope Co	pefficient (m)	-1.06805		PF Power Coefficient (p)		0.80682
In Passing L	ane Effective Length?	No	To	otal Segment De	0.0	
%Improvem	nent to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subsegn	nent Data					
# Segm	nent Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Tange	ent	5280	-		-	55.2
Vehicle	Results					
Average Sp	eed, mi/h	55.2	Р	Percent Followers	, %	0.0
		1.09	F	follower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS		А				
Facility I	Results		,			
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/In		LOS
1	0	0.00			0.0	A

		HCS Two-Lan	ne Hig	ıhway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY1 Bofer Canyon Road Northbou (North of Coffin Road)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	idth, ft	16	Sho	Shoulder Width, ft		0
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	66	Орг	Opposing Demand Flow Rate, veh/h		3
Peak Ho	our Factor	0.94	Tota	Total Trucks, %		0.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.04
Intern	nediate Results					
Segmen	nt Vertical Class	3	Free	e-Flow Speed,	mi/h	55.2
Speed S	Slope Coefficient (m)	3.11550	Spe	ed Power Coe	fficient (p)	0.70199
PF Slope	e Coefficient (m)	-1.06182	PF I	Power Coefficie	ent (p)	0.80862
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	55.2
Vehic	le Results					
Average	e Speed, mi/h	55.2	Per	cent Followers,	, %	11.1
Segmen	5 1		Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	16	0.00			0.1	A

		HCS Two-Lai	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdiction	on		Tir	me Analyzed		Afternoon Peak Hour
Project D	Description	Build Ph1 to LY1 Bofer Canyon Road Northbo (North of Coffin Road)		Units		U.S. Customary
		Se	egmei	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ngth, ft		5280
Lane Wid	lth, ft	16	Sh	Shoulder Width, ft		0
Speed Lir	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	11	Op	Opposing Demand Flow Rate, veh/h		98
Peak Hou	ur Factor	0.63	To	Total Trucks, %		0.00
Segment	: Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					·
Segment	: Vertical Class	3	Fre	ee-Flow Speed,	mi/h	55.2
Speed Slo	ope Coefficient (m)	3.11550	Sp	eed Power Coe	fficient (p)	0.56920
PF Slope	Coefficient (m)	-1.15650	PF	Power Coefficie	ent (p)	0.78340
In Passing	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	55.2
Vehicle	e Results					
Average :	Speed, mi/h	55.2	Pe	rcent Followers	, %	3.3
Segment	Segment Travel Time, minutes 1.09		Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	OS	А				
Facility	y Results					·
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
	1	· ·				1

		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdicti	on		Tir	ne Analyzed		Morning Peak Hour
Project D	Description	Build Ph1 to LY1 Bofer Canyon Road Southbou (North of Coffin Road)		Units		U.S. Customary
		Se	gmei	nt 1		
Vehicle	e Inputs					
Segment	туре	Passing Zone	Le	ngth, ft		5280
Lane Wic	dth, ft	16	Sh	Shoulder Width, ft		0
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	12	Op	Opposing Demand Flow Rate, veh/h		248
Peak Hou	ur Factor	0.25	To	Total Trucks, %		0.00
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					·
Segment	t Vertical Class	3	Fre	ee-Flow Speed,	mi/h	55.2
Speed SI	ope Coefficient (m)	3.11550	Sp	eed Power Coe	fficient (p)	0.49663
PF Slope	Coefficient (m)	-1.21976	PF	Power Coefficie	ent (p)	0.76846
In Passin	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results	·				·
Average	Speed, mi/h	55.2	Pe	rcent Followers	, %	4.0
Segment	Segment Travel Time, minutes 1.09		Fo	llower Density (	FD), followers/mi/ln	0.0
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A

iption  iption  aputs	James Vorosmarti Tetra Tech  Build Ph1 to LY1 Bofer Canyon Road Southbou (North of Coffin Road)	Tim	alysis Year ne Analyzed		7/12/2023 2025 Afternoon Peak Hour U.S. Customary
puts	Tetra Tech  Build Ph1 to LY1 Bofer Canyon Road Southbou (North of Coffin Road)	Ana Tim Uni	alysis Year ne Analyzed		2025 Afternoon Peak Hour
puts	Build Ph1 to LY1 Bofer Canyon Road Southbou (North of Coffin Road)	Tim Uni	ne Analyzed		Afternoon Peak Hour
puts	Canyon Road Southbou (North of Coffin Road)	und	-		
puts	Canyon Road Southbou (North of Coffin Road)	ınd	its		U.S. Customary
	Se	gmen			
			it 1		
e					
	Passing Zone	Ler	Length, ft		5280
t	16	Sho	Shoulder Width, ft		0
mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
and Capacity					
emand Flow Rate, veh/h	66	Ор	Opposing Demand Flow Rate, veh/h		7
ctor	0.94	Tot	Total Trucks, %		0.00
pacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04
iate Results					
tical Class	3	Fre	e-Flow Speed,	mi/h	55.2
Coefficient (m)	3.11550	Spe	eed Power Coet	fficient (p)	0.68473
fficient (m)	-1.07309	PF	Power Coefficie	ent (p)	0.80537
ne Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.1
nt to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
ent Data					
nt Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
t	5280	-		-	55.2
esults					
ed, mi/h	55.2	Per	cent Followers,	, %	11.3
5 1		Fol	lower Density (	FD), followers/mi/ln	0.1
	А				
esults					
VMT veh-mi/AP	VHD veh-h/p				LOS
	0.00		_		A
n a e c f n t e e e e e e e e e e e e e e e e e e	minimal capacity  mand Capacity  mand Flow Rate, veh/h  tor  acity, veh/h  ate Results  cal Class  Coefficient (m)  ficient (m)  e Effective Length?  at to Percent Followers  ent Data  t Type  esults  d, mi/h  el Time, minutes  esults  vmT	mind Capacity  mand Flow Rate, veh/h  tor 0.94  acity, veh/h 1700  ate Results  cal Class 3  Coefficient (m) -1.07309  e Effective Length? No  at to Percent Followers 0.0  ent Data  t Type Length, ft 5280  esults  d, mi/h 55.2  el Time, minutes 1.09  A  sults  VMT veh-mi/AP  VHD veh-h/p	mind Capacity  mand Flow Rate, veh/h  for 0.94  ate Results  cal Class 3 Free Coefficient (m) -1.07309  e Effective Length? No Tot  at to Percent Followers 0.0  cant Data  t Type Length, ft Radius, 1  sults  d, mi/h 55.2 Per  el Time, minutes 1.09  A  sults  VMT  veh-mi/AP  VHD  veh-h/p	Init/h 50 Access Point Density  Ind Capacity  Imand Flow Rate, veh/h 66 Opposing Deman for 0.94 Total Trucks, % Demand/Capacity  Indicity, veh/h 1700 Demand/Capacity  Indicity Results  Indicit Calcas 3 Free-Flow Speed, Speed Power Coefficient (m) Speed P	Access Point Density, pts/mi  and Capacity  mand Flow Rate, veh/h  tor  0.94  Total Trucks, %  acity, veh/h  1700  Demand/Capacity (D/C)  ate Results  cal Class  3 Free-Flow Speed, mi/h  Speed Power Coefficient (p)  ficient (m)  -1.07309  PF Power Coefficient (p)  et Effective Length?  No  Total Segment Density, veh/mi/ln  at to Percent Followers  t Type  Length, ft  Salous, ft  Superelevation, %  5280  -  -  Sults  d, mi/h  55.2  Percent Followers, %  Follower Density (FD), followers/mi/ln  A  Sults  VMT  veh-mi/AP  VHD  veh-h/p  Follower Density, followers/ mi/ln

Project In Analyst Agency Jurisdiction Project Desc	nformation					
Agency  Jurisdiction						
Jurisdiction		James Vorosmarti	Dat	te		7/12/2023
		Tetra Tech	Ana	alysis Year		2025
Project Desc			Tin	ne Analyzed		Morning Peak Hour
	ription	Build Ph1 to LY2 Bofer Canyon Road Northbou (North of Coffin Road)		Units		U.S. Customary
		Se	gmen	nt 1		
Vehicle I	nputs					
Segment Typ	pe	Passing Zone	Ler	Length, ft		5280
Lane Width,	ft	16	Sho	Shoulder Width, ft		0
Speed Limit,	mi/h	50	Acc	cess Point Dens	sity, pts/mi	0.0
Demand	and Capacity					
Directional D	Demand Flow Rate, veh/h	0	Ор	Opposing Demand Flow Rate, veh/h		1
Peak Hour Fa	actor	0.94	Tot	Total Trucks, %		0.00
Segment Ca	pacity, veh/h	1700	De	Demand/Capacity (D/C)		0.00
Intermed	diate Results					
Segment Ver	rtical Class	3	Fre	e-Flow Speed,	mi/h	55.2
Speed Slope	e Coefficient (m)	3.11550	Spe	eed Power Coe	fficient (p)	0.71618
PF Slope Co	efficient (m)	-1.05274	PF	Power Coefficie	ent (p)	0.81129
In Passing La	ane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.0
%Improvem	ent to Percent Followers	0.0	%Ir	mprovement to	Speed	0.0
Subsegm	nent Data					
# Segme	ent Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tangei	nt	5280	-		-	55.2
Vehicle R	Results					
Average Spe	eed, mi/h	55.2	Per	cent Followers,	, %	0.0
Segment Tra	3 1		Fol	lower Density (	FD), followers/mi/ln	0.0
Vehicle LOS		А				
Facility R	Results		,			
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lai	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdiction	on		Tin	ne Analyzed		Afternoon Peak Hour
Project D	Description	Build Ph1 to LY2 Bofer Canyon Road Northbo (North of Coffin Road)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Lei	ngth, ft		5280
Lane Wid	dth, ft	16	Sh	Shoulder Width, ft		0
Speed Lir	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	8	Op	Opposing Demand Flow Rate, veh/h		0
Peak Hou	ur Factor	0.63	Tot	Total Trucks, %		0.00
Segment	: Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	: Vertical Class	3	Fre	e-Flow Speed,	mi/h	55.2
Speed Slo	ope Coefficient (m)	3.11550	Sp	eed Power Coe	fficient (p)	0.73607
PF Slope	Coefficient (m)	-1.04027	PF	Power Coefficie	ent (p)	0.81504
In Passing	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	55.2
Vehicle	e Results					,
Average	Speed, mi/h	55.2	Per	rcent Followers	, %	2.0
Segment	Segment Travel Time, minutes 1.09		Fo	lower Density (	(FD), followers/mi/ln	0.0
Vehicle Lo	OS	А				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
		<u> </u>				

		HCS Two-Lar	ne Hig	ghway Re	port	
Project	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdictio	on		Tin	ne Analyzed		Morning Peak Hour
Project D	escription	Build Ph1 to LY2 Bofer Canyon Road Southbou (North of Coffin Road)		Units		U.S. Customary
		Se	gmer	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Ler	ngth, ft		5280
Lane Wid	lth, ft	16	Sho	Shoulder Width, ft		0
Speed Lir	mit, mi/h	50	Acc	cess Point Dens	sity, pts/mi	0.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	4	Ор	Opposing Demand Flow Rate, veh/h		0
Peak Hou	ur Factor	0.25	Tot	Total Trucks, %		0.00
Segment	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	Vertical Class	3	Fre	e-Flow Speed,	mi/h	55.2
Speed Slo	ope Coefficient (m)	3.11550	Spe	eed Power Coe	fficient (p)	0.73607
PF Slope	Coefficient (m)	-1.04027	PF	Power Coefficie	ent (p)	0.81504
In Passino	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.0
%Improv	ement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subseç	gment Data					
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	55.2
Vehicle	e Results					
Average S	Speed, mi/h	55.2	Per	cent Followers	, %	1.1
Segment	Segment Travel Time, minutes 1.09		Fol	lower Density (	(FD), followers/mi/ln	0.0
Vehicle LO	OS	А				
Facility	y Results		,			
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
		<u> </u>		+	0.0	A

	HCS Two-Lane Highway Report							
Projec	t Information							
Analyst		James Vorosmarti	Date	е		7/12/2023		
Agency		Tetra Tech	Ana	lysis Year		2025		
Jurisdiction	on		Tim	e Analyzed		Afternoon Peak Hour		
Project D	Pescription	Build Ph1 to LY2 Bofer Canyon Road Southbou (North of Coffin Road)	Unit	:S		U.S. Customary		
		Se	gmen	t 1				
Vehicle	e Inputs							
Segment	Туре	Passing Zone	Len	gth, ft		5280		
Lane Wid	lth, ft	16	Sho	ulder Width, ft	t	0		
Speed Lir	mit, mi/h	50	Acce	ess Point Dens	ity, pts/mi	0.0		
Demai	nd and Capacity							
Direction	al Demand Flow Rate, veh/h	0	Орр	Opposing Demand Flow Rate, veh/h		5		
Peak Hou	ur Factor	0.94	Tota	Total Trucks, %		0.00		
Segment	: Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.00		
Interm	nediate Results					·		
Segment	Vertical Class	3	Free	e-Flow Speed,	mi/h	55.2		
Speed Slo	ope Coefficient (m)	3.11550	Spe	ed Power Coef	fficient (p)	0.69241		
PF Slope	Coefficient (m)	-1.06805	PF F	ower Coefficie	ent (p)	0.80682		
In Passing	g Lane Effective Length?	No	Tota	I Segment De	nsity, veh/mi/ln	0.0		
%Improv	rement to Percent Followers	0.0	%Im	provement to	Speed	0.0		
Subse	gment Data							
# Seg	gment Type	Length, ft	Radius, ft	t	Superelevation, %	Average Speed, mi/h		
1 Tar	ngent	5280	-		-	55.2		
Vehicle	e Results							
Average	Speed, mi/h	55.2	Perc	ent Followers,	%	0.0		
Segment	Travel Time, minutes	1.09	Follower Density (FD), followers/mi/ln		0.0			
Vehicle L	OS	А						
Facility	y Results							
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	0	0.00			0.0	А		

23 g Peak Hour stomary
g Peak Hour
tomary
e Speed, mi/h
LOS
A

		HCS Two-Lar	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	Δ	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	No Build Bofer Canyon Road Northbound (Nor of Coffin Road)		Jnits		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	16	S	Shoulder Width, ft	t	0
Speed L	_imit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					·
Directio	onal Demand Flow Rate, veh/h	8		Opposing Demand Flow Rate, veh/h		0
Peak Ho	our Factor	0.63	Т	Total Trucks, %		0.00
Segmer	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.00
Interr	mediate Results					·
Segmer	nt Vertical Class	3	F	ree-Flow Speed,	mi/h	55.2
Speed S	Slope Coefficient (m)	3.11550	S	Speed Power Coef	fficient (p)	0.73607
PF Slop	e Coefficient (m)	-1.04027	P	PF Power Coefficie	ent (p)	0.81504
In Passii	ng Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	0.0
%lmpro	ovement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.2
Vehic	le Results					
Average	e Speed, mi/h	55.2	P	Percent Followers,	, %	2.0
Segmen	nt Travel Time, minutes	1.09	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	ion		Т	īme Analyzed		Morning Peak Hour
Project [	Description	No Build Bofer Canyon Road Southbound (Nor of Coffin Road)	rth	Jnits		U.S. Customary
		Se	gme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	16	S	Shoulder Width, ft	t	0
Speed L	imit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and Capacity					<u> </u>
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		0
Peak Ho	our Factor	0.25	Т	Total Trucks, %		0.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.00
Intern	nediate Results					·
Segmen	t Vertical Class	3	F	ree-Flow Speed,	mi/h	55.2
Speed S	lope Coefficient (m)	3.11550	S	Speed Power Coef	fficient (p)	0.73607
PF Slope	e Coefficient (m)	-1.04027	P	PF Power Coefficie	ent (p)	0.81504
In Passir	ng Lane Effective Length?	No	Т	Total Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	55.2
Vehicl	le Results					
Average	Speed, mi/h	55.2	P	Percent Followers,	, %	1.1
Segmen	t Travel Time, minutes	1.09	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdict	tion		Т	Time Analyzed		Afternoon Peak Hour
Project	Description	No Build Bofer Canyon Road Southbound (Nor of Coffin Road)	rth	Jnits		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	Ī	ength, ft		5280
Lane Wi	idth, ft	16	S	Shoulder Width, f	t	0
Speed L	_imit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					·
Directio	onal Demand Flow Rate, veh/h	0		Opposing Demand Flow Rate, veh/h		5
Peak Ho	our Factor	0.94	Т	Total Trucks, %		0.00
Segmer	nt Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.00
Interr	mediate Results					·
Segmer	nt Vertical Class	3	F	ree-Flow Speed,	mi/h	55.2
Speed S	Slope Coefficient (m)	3.11550	S	Speed Power Coe	fficient (p)	0.69241
PF Slop	e Coefficient (m)	-1.06805	F	PF Power Coefficie	ent (p)	0.80682
In Passii	ng Lane Effective Length?	No	Т	Total Segment De	nsity, veh/mi/ln	0.0
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.2
Vehic	le Results					
Average	e Speed, mi/h	55.2	F	Percent Followers,	, %	0.0
Segmen	nt Travel Time, minutes	1.09	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A
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		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ne Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Bofer Canyo Road Northbound (Nor of Coffin Road)		Units		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wic	dth, ft	16	Sho	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	0	Орј	Opposing Demand Flow Rate, veh/h		1
Peak Hour Factor		0.94 To		Total Trucks, %		0.00
Segment Capacity, veh/h		1700 E		Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3		Free-Flow Speed, mi/h		55.2
Speed Slope Coefficient (m)		3.11550		Speed Power Coefficient (p)		0.71618
PF Slope Coefficient (m)		-1.05274		PF Power Coefficient (p)		0.81129
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0 %II		%Improvement to Speed		0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					<u>'</u>
Average Speed, mi/h		55.2 F		Percent Followers, %		0.0
Segment Travel Time, minutes		1.09 Fo		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hig	ıhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Bofer Canyo Road Northbound (Nor of Coffin Road)	on Uni	Units		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone		gth, ft		5280
Lane Wic	dth, ft	16	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	8		Opposing Demand Flow Rate, veh/h		0
Peak Hour Factor		0.63		Total Trucks, %		0.00
Segment	t Capacity, veh/h	1700		Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3		Free-Flow Speed, mi/h		55.2
Speed Slope Coefficient (m)		3.11550		Speed Power Coefficient (p)		0.73607
PF Slope Coefficient (m)		-1.04027		PF Power Coefficient (p)		0.81504
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0 %		%Improvement to Speed		0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average Speed, mi/h		55.2 F		Percent Followers, %		2.0
Segment Travel Time, minutes		1.09 Fe		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facility	y Results		·			
Т	VMT veh-mi/p	VHD veh-h/p	l		ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A

		HCS Two-Lar	ne Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	Date		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Bofer Canyo Road Southbound (Nor of Coffin Road)		Units		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wic	dth, ft	16	Sho	ulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	ity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орр	Opposing Demand Flow Rate, veh/h		0
Peak Hour Factor		0.25 T		Total Trucks, %		0.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3	Free	Free-Flow Speed, mi/h		55.2
Speed Slope Coefficient (m)		3.11550	Spe	Speed Power Coefficient (p)		0.73607
PF Slope Coefficient (m)		-1.04027	PF F	PF Power Coefficient (p)		0.81504
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0 %l		%Improvement to Speed		0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average Speed, mi/h		55.2 F		Percent Followers, %		1.1
Segment Travel Time, minutes		1.09 Fe		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	· · · · · · · · · · · · · · · · · · ·		ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	ion		Tim	ne Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Bofer Canyo Road Southbound (Nor of Coffin Road)	on Uni	Units		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wic	dth, ft	16	Sho	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	0	Ор	Opposing Demand Flow Rate, veh/h		5
Peak Hour Factor		0.94 T		Total Trucks, %		0.00
Segment Capacity, veh/h		1700 E		Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3		Free-Flow Speed, mi/h		55.2
Speed Slope Coefficient (m)		3.11550		Speed Power Coefficient (p)		0.69241
PF Slope Coefficient (m)		-1.06805		PF Power Coefficient (p)		0.80682
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0 %I		%Improvement to Speed		0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius, 1	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					<u>'</u>
Average Speed, mi/h		55.2 F		Percent Followers, %		0.0
Segment Travel Time, minutes		1.09 Fe		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ie Analyzed		Morning Peak Hour
Project D	Description	No Build - Bofer Canyor Road Northbound (Nort of Coffin Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	16	Sho	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	0	Орг	posing Deman	d Flow Rate, veh/h	1
Peak Hou	ur Factor	0.94	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Der	mand/Capacity	(D/C)	0.00
Interm	nediate Results					
Segment	t Vertical Class	3	Free	e-Flow Speed,	mi/h	55.2
Speed SI	ope Coefficient (m)	3.11550	Spe	ed Power Coe	fficient (p)	0.71618
PF Slope	Coefficient (m)	-1.05274	PF F	Power Coefficie	ent (p)	0.81129
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average	Speed, mi/h	55.2	Per	cent Followers,	, %	0.0
Segment Travel Time, minutes		1.09	Foll	Follower Density (FD), followers/mi/ln		0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ie Analyzed		Afternoon Peak Hour
Project D	Description	No Build - Bofer Canyor Road Northbound (Nor of Coffin Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	16	Sho	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	8	Орі	posing Deman	d Flow Rate, veh/h	0
Peak Hou	ur Factor	0.63	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3	Free	e-Flow Speed,	mi/h	55.2
Speed SI	ope Coefficient (m)	3.11550	Spe	ed Power Coe	fficient (p)	0.73607
PF Slope	Coefficient (m)	-1.04027	PF I	Power Coefficie	ent (p)	0.81504
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	55.2	Per	cent Followers	, %	2.0
Segment Travel Time, minutes		1.09	Foll	Follower Density (FD), followers/mi/ln		0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	1	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ie Analyzed		Morning Peak Hour
Project D	Description	No Build - Bofer Canyor Road Southbound (Nor of Coffin Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	16	Sho	oulder Width, f	t	0
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Орг	posing Deman	d Flow Rate, veh/h	0
Peak Hou	ur Factor	0.25	Tota	al Trucks, %		0.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.00
Interm	nediate Results					
Segment	t Vertical Class	3	Free	e-Flow Speed,	mi/h	55.2
Speed SI	ope Coefficient (m)	3.11550	Spe	ed Power Coe	fficient (p)	0.73607
PF Slope	Coefficient (m)	-1.04027	PF F	Power Coefficie	ent (p)	0.81504
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.2
Vehicle	e Results					
Average	Speed, mi/h	55.2	Per	cent Followers	, %	1.1
Segment Travel Time, minutes 1		1.09	Foll	Follower Density (FD), followers/mi/ln		0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Di	ate		7/12/2023
Agency		Tetra Tech	Aı	nalysis Year		2026
Jurisdict	ion		Ti	me Analyzed		Afternoon Peak Hour
Project [	Description	No Build - Bofer Canyo Road Southbound (Nor of Coffin Road)		nits		U.S. Customary
		Se	gme	nt 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	dth, ft	16	Sh	noulder Width, ft		0
Speed Li	imit, mi/h	50	Ad	ccess Point Dens	ity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	0	0	pposing Demand	d Flow Rate, veh/h	5
Peak Ho	our Factor	0.94	To	otal Trucks, %		0.00
Segmen	nt Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.00
Intern	nediate Results					
Segmen	nt Vertical Class	3	Fr	ee-Flow Speed,	mi/h	55.2
Speed S	lope Coefficient (m)	3.11550	Sp	peed Power Coef	fficient (p)	0.69241
PF Slope	e Coefficient (m)	-1.06805	PF	F Power Coefficie	ent (p)	0.80682
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	55.2
Vehicl	le Results					
Average	Speed, mi/h	55.2	Pe	ercent Followers,	%	0.0
Segmen	t Travel Time, minutes	1.09	Fo	ollower Density (	FD), followers/mi/ln	0.0
Vehicle l	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A
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	HCS Basic Fr	reeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2023
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors	-		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			·
Demand Volume (V), veh/h	1056	Heavy Vehicle Adjustment Factor (fHV)	0.843
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	666
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.28
Passenger Car Equivalent (ET)	2.86		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.4
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	Jmaes Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2023
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	767	Heavy Vehicle Adjustment Factor (fHV)	0.751
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	543
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	14	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	86	Volume-to-Capacity Ratio (v/c)	0.23
Passenger Car Equivalent (ET)	2.33		
Speed and Density	•		
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (frlc)	-	Density (D), pc/mi/ln	6.9
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		
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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2023
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	236	Heavy Vehicle Adjustment Factor (fHV)	0.659
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	190
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	16	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	84	Volume-to-Capacity Ratio (v/c)	0.08
Passenger Car Equivalent (ET)	2.33		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.4
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		
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	HCS Basic Front	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2023
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors	-		
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1420	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	949
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.40
Passenger Car Equivalent (ET)	2.51		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.2
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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HCS Basic Freeway Report				
Project Information				
Analyst	James Vorosmarti	Date	7/12/2023	
Agency	Tetra Tech	Analysis Year	2025	
Jurisdiction		Time Analyzed	Morning Peak Hour	
Project Description	Build Ph1 to LY1 Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary	
Geometric Data				
Number of Lanes (N), In	2	Terrain Type	Specific Grade	
Segment Length (L), ft	-	Percent Grade, %	2.70	
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00	
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-	
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0	
Right-Side Lateral Clearance, ft	-			
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000	
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000	
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000	
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000	
Demand and Capacity				
Demand Volume (V), veh/h	1077	Heavy Vehicle Adjustment Factor (fHV)	0.843	
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	680	
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400	
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400	
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400	
Passenger Car Equivalent (ET)	2.86	Volume-to-Capacity Ratio (v/c)	0.28	
Speed and Density				
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0	
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.6	
Total Ramp Density Adjustment	-	Level of Service (LOS)	А	
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0			

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HCS Basic Freeway Report					
Project Information					
Analyst	Jmaes Vorosmarti	Date	7/12/2023		
Agency	Tetra Tech	Analysis Year	2025		
Jurisdiction		Time Analyzed	Afternoon Peak Hour		
Project Description	Build Ph1 to LY1 I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary		
Geometric Data					
Number of Lanes (N), In	2	Terrain Type	Specific Grade		
Segment Length (L), ft	-	Percent Grade, %	2.70		
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00		
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-		
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0		
Right-Side Lateral Clearance, ft	-				
Adjustment Factors					
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000		
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000		
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000		
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000		
Demand and Capacity					
Demand Volume (V), veh/h	782	Heavy Vehicle Adjustment Factor (fHV)	0.751		
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	554		
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400		
Single-Unit Trucks (SUT), %	14	Initial Adjusted Capacity (cadj), pc/h/ln	2400		
Tractor-Trailers (TT), %	86	Final Adjusted Capacity (cadj), pc/h/ln	2400		
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.23		
Speed and Density					
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0		
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	7.0		
Total Ramp Density Adjustment	-	Level of Service (LOS)	А		
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0				

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	Build Ph1 to LY1 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	241	Heavy Vehicle Adjustment Factor (fHV)	0.659
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	194
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	16	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	84	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.08
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.5
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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	HCS Basic F	reeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	Build Ph1 to LY1 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1449	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	968
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.51	Volume-to-Capacity Ratio (v/c)	0.40
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.4
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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	HCS Basic Fre	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	Build Ph1 to LY2 Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1077	Heavy Vehicle Adjustment Factor (fHV)	0.843
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	680
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.86	Volume-to-Capacity Ratio (v/c)	0.28
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.6
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		

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	HCS Basic F	reeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	Build Ph1 to LY2 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1449	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	968
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.51	Volume-to-Capacity Ratio (v/c)	0.40
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.4
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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	HCS Basic F	reeway Report	
Project Information			
Analyst	Jmaes Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	Build Ph1 to LY2 I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	782	Heavy Vehicle Adjustment Factor (fHV)	0.751
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	554
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	14	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	86	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.23
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	7.0
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	Build Ph1 to LY2 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	241	Heavy Vehicle Adjustment Factor (fHV)	0.659
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	194
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	16	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	84	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.08
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.5
Total Ramp Density Adjustment	-	Level of Service (LOS)	A
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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	HCS Basic F	reeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	No Build Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1077	Heavy Vehicle Adjustment Factor (fHV)	0.843
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	680
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.86	Volume-to-Capacity Ratio (v/c)	0.28
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.6
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	Jmaes Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	No Build I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	782	Heavy Vehicle Adjustment Factor (fHV)	0.751
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	554
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	14	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	86	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.23
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	7.0
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		
Converight @ 2022 University of Florida, All Dights F		ous Version 2022	Caparatad: 07/20/2022 14:01:20

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	No Build I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	241	Heavy Vehicle Adjustment Factor (fHV)	0.659
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	194
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	16	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	84	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.33	Volume-to-Capacity Ratio (v/c)	0.08
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.5
Total Ramp Density Adjustment	-	Level of Service (LOS)	A
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		
Converight @ 2022 University of Florida, All Bights F		ous Version 2022	Canaratad: 07/20/2022 14:02:

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2025
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	No Build I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Final Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1449	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	968
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Initial Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Final Adjusted Capacity (cadj), pc/h/ln	2400
Passenger Car Equivalent (ET)	2.51	Volume-to-Capacity Ratio (v/c)	0.40
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.4
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		

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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2026
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	PH2 to LY2 Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1088	Heavy Vehicle Adjustment Factor (fHV)	0.843
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	686
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.29
Passenger Car Equivalent (ET)	2.86		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.7
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		

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	HCS Basic Fre	eeway Report	
Project Information			
Analyst	Jmaes Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2026
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	PH2 to LY2 I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	790	Heavy Vehicle Adjustment Factor (fHV)	0.751
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	560
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	14	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	86	Volume-to-Capacity Ratio (v/c)	0.23
Passenger Car Equivalent (ET)	2.33		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	7.1
Total Ramp Density Adjustment	-	Level of Service (LOS)	Α
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0		
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	HCS Basic Fi	reeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2026
Jurisdiction		Time Analyzed	Morning Peak Hour
Project Description	PH2 to LY2 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	243	Heavy Vehicle Adjustment Factor (fHV)	0.659
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	196
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	16	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	84	Volume-to-Capacity Ratio (v/c)	0.08
Passenger Car Equivalent (ET)	2.33		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.5
Total Ramp Density Adjustment	-	Level of Service (LOS)	А
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		
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	HCS Basic Fr	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2026
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	PH2 to LY2 I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1463	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	978
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.41
Passenger Car Equivalent (ET)	2.51		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.6
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		
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HCS Basic Freeway Report					
Project Information					
Analyst	James Vorosmarti	Date	7/12/2023		
Agency	Tetra Tech	Analysis Year	2026		
Jurisdiction		Time Analyzed	Morning Peak Hour		
Project Description	No Build - Interstate 82 Eastbound (North of Coffin Road)	Units	U.S. Customary		
Geometric Data					
Number of Lanes (N), In	2	Terrain Type	Specific Grade		
Segment Length (L), ft	-	Percent Grade, %	2.70		
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00		
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-		
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0		
Right-Side Lateral Clearance, ft	-				
Adjustment Factors					
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000		
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000		
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000		
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000		
Demand and Capacity					
Demand Volume (V), veh/h	1088	Heavy Vehicle Adjustment Factor (fHV)	0.843		
Peak Hour Factor (PHF)	0.94	Flow Rate (v <sub>p</sub> ), pc/h/ln	686		
Total Trucks, %	10.00	Capacity (c), pc/h/ln	2400		
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400		
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.29		
Passenger Car Equivalent (ET)	2.86				
Speed and Density					
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0		
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	8.7		
Total Ramp Density Adjustment	-	Level of Service (LOS)	А		
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0				
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	HCS Basic Fr	eeway Report			
Project Information					
Analyst	Jmaes Vorosmarti	Date	7/12/2023		
Agency	Tetra Tech	Analysis Year	2026		
Jurisdiction		Time Analyzed	Afternoon Peak Hour		
Project Description	No Build I-82 Eastbound (North of Coffin Road)	Units	U.S. Customary		
Geometric Data					
Number of Lanes (N), In	2	Terrain Type	Specific Grade		
Segment Length (L), ft	-	Percent Grade, %	2.70		
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00		
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-		
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	79.0		
Right-Side Lateral Clearance, ft	-				
Adjustment Factors					
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000		
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000		
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000		
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000		
Demand and Capacity					
Demand Volume (V), veh/h	790	Heavy Vehicle Adjustment Factor (fHV)	0.751		
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	560		
Total Trucks, %	25.00	Capacity (c), pc/h/ln	2400		
Single-Unit Trucks (SUT), %	14	Adjusted Capacity (cadj), pc/h/ln	2400		
Tractor-Trailers (TT), %	86	Volume-to-Capacity Ratio (v/c)	0.23		
Passenger Car Equivalent (ET)	2.33				
Speed and Density					
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	79.0		
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	7.1		
Total Ramp Density Adjustment	-	Level of Service (LOS)	А		
Adjusted Free-Flow Speed (FFSadj), mi/h	79.0				
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	HCS Basic Fre	eeway Report			
Project Information					
Analyst	James Vorosmarti	Date	7/12/2023		
Agency	Tetra Tech	Analysis Year	2026		
Jurisdiction		Time Analyzed	Morning Peak Hour		
Project Description	No Build - I-82 Westbound (North of Coffin Road)	Units	U.S. Customary		
Geometric Data					
Number of Lanes (N), In	2	Terrain Type	Specific Grade		
Segment Length (L), ft	-	Percent Grade, %	2.70		
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00		
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-		
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0		
Right-Side Lateral Clearance, ft	-				
Adjustment Factors	Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000		
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000		
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000		
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000		
Demand and Capacity	Demand and Capacity				
Demand Volume (V), veh/h	243	Heavy Vehicle Adjustment Factor (fHV)	0.659		
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	196		
Total Trucks, %	39.00	Capacity (c), pc/h/ln	2400		
Single-Unit Trucks (SUT), %	16	Adjusted Capacity (cadj), pc/h/ln	2400		
Tractor-Trailers (TT), %	84	Volume-to-Capacity Ratio (v/c)	0.08		
Passenger Car Equivalent (ET)	2.33				
Speed and Density					
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	78.0		
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	2.5		
Total Ramp Density Adjustment	-	Level of Service (LOS)	А		
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0				

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	HCS Basic Fre	eeway Report	
Project Information			
Analyst	James Vorosmarti	Date	7/12/2023
Agency	Tetra Tech	Analysis Year	2026
Jurisdiction		Time Analyzed	Afternoon Peak Hour
Project Description	No Build - I-82 Westbound (North of Coffin Road)	Units	U.S. Customary
Geometric Data			
Number of Lanes (N), In	2	Terrain Type	Specific Grade
Segment Length (L), ft	-	Percent Grade, %	2.70
Measured or Base Free-Flow Speed	Measured	Grade Length, mi	1.00
Base Free-Flow Speed (BFFS), mi/h	-	Total Ramp Density (TRD), ramps/mi	-
Lane Width, ft	-	Free-Flow Speed (FFS), mi/h	78.0
Right-Side Lateral Clearance, ft	-		
Adjustment Factors			
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Demand Adjustment Factor (DAF)	1.000
Incident Type	No Incident	Capacity Adjustment Factor (CAF)	1.000
Proportion of CAVs in Traffic Stream	0	Capacity Adj. Factor for CAVs, CAFCAV	1.000
Demand and Capacity			
Demand Volume (V), veh/h	1463	Heavy Vehicle Adjustment Factor (fHV)	0.796
Peak Hour Factor (PHF)	0.94	Flow Rate (vp), pc/h/ln	978
Total Trucks, %	17.00	Capacity (c), pc/h/ln	2400
Single-Unit Trucks (SUT), %	34	Adjusted Capacity (cadj), pc/h/ln	2400
Tractor-Trailers (TT), %	66	Volume-to-Capacity Ratio (v/c)	0.41
Passenger Car Equivalent (ET)	2.51		
Speed and Density			
Lane Width Adjustment (fLW)	-	Average Speed (S), mi/h	77.9
Right-Side Lateral Clearance Adj. (fRLC)	-	Density (D), pc/mi/ln	12.6
Total Ramp Density Adjustment	-	Level of Service (LOS)	В
Adjusted Free-Flow Speed (FFSadj), mi/h	78.0		
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		HCS Two-La	ane Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Tir	me Analyzed		Morning Peak Hour
Project D	Description	Locust Grove Road Eastbound (between Nicosin Road and I-8	2) Ur	nits		U.S. Customary
		S	Segme	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Le	ngth, ft		5280
Lane Wic	dth, ft	12	Sh	oulder Width, f	t	4
Speed Lii	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	20	Op	Opposing Demand Flow Rate, veh/h		60
Peak Hou	ur Factor	0.60	To	tal Trucks, %		25.00
Segment	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					
Segment	: Vertical Class	1	Fre	Free-Flow Speed, mi/h		54.5
Speed Slo	ope Coefficient (m)	3.19384	Sp	Speed Power Coefficient (p)		0.59501
PF Slope	Coefficient (m)	-1.17940	PF	PF Power Coefficient (p)		0.81248
In Passin	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%lmprov	rement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data	·	·			·
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	54.5
Vehicle	e Results					
Average	Speed, mi/h	54.5	Pe	rcent Followers	, %	4.8
Segment	: Travel Time, minutes	1.10	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A						
Facility	y Results					
т	VMT veh-mi/p	VHD veh-h/p	· · · · · · · · · · · · · · · · · · ·		ensity, followers/ mi/ln	LOS
	<u> </u>					

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2023
Jurisdicti	on		Tir	me Analyzed		Afternoon Peak Hour
Project D	Description	Locust Grove Road Eastbound (between Nicosin Road and I-82		nits		U.S. Customary
		S	egme	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Le	ength, ft		5280
Lane Wic	dth, ft	12	Sh	noulder Width, f	t	4
Speed Li	mit, mi/h	50	Ac	ccess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	60	Oı	pposing Deman	d Flow Rate, veh/h	12
Peak Hou	ur Factor	0.85	То	otal Trucks, %		22.00
Segment	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Interm	nediate Results					
Segment	: Vertical Class	1	Fre	ee-Flow Speed,	mi/h	54.6
Speed SI	ope Coefficient (m)	3.15452	Sp	Speed Power Coefficient (p)		0.63822
PF Slope	Coefficient (m)	-1.14094	PF	PF Power Coefficient (p)		0.82360
In Passin	g Lane Effective Length?	No	То	Total Segment Density, veh/mi/ln		0.1
%lmprov	rement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	54.6
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	54.6	Pe	ercent Followers	, %	10.6
Segment	: Travel Time, minutes	1.10	Fo	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A						
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	13	0.00			0.1	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Tir	me Analyzed		Morning Peak Hour
Project D	Description	Locust Grove Road Westbound (between Nicosin Road and I-82	Un	nits		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	12	Sh	oulder Width, f	t	4
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					-
Direction	nal Demand Flow Rate, veh/h	68	Op	pposing Deman	d Flow Rate, veh/h	23
Peak Hou	ur Factor	0.53	To	tal Trucks, %		11.00
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Interm	nediate Results	•				
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	55.0
Speed SI	ope Coefficient (m)	3.18814	Sp	Speed Power Coefficient (p)		0.62444
PF Slope	Coefficient (m)	-1.15331	PF	PF Power Coefficient (p)		0.81945
In Passin	g Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.1
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data		·			·
# See	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.0
Vehicle	e Results					·
Average	Speed, mi/h	55.0	Pe	rcent Followers,	, %	12.0
Segment	t Travel Time, minutes	1.09	Fo	llower Density (	FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l l		ensity, followers/ mi/ln	LOS
1	9	0.00			0.1	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	nte		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2023
Jurisdicti	on		Tir	me Analyzed		Afternoon Peak Hour
Project D	Description	Locust Grove Road Westbound (between Nicosin Road and I-82		nits		U.S. Customary
		S	egmei	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Le	ngth, ft		5280
Lane Wic	dth, ft	12	Sh	oulder Width, f	t	4
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	16	Op	pposing Deman	d Flow Rate, veh/h	81
Peak Hou	ur Factor	0.63	To	tal Trucks, %		10.00
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	55.0
Speed SI	ope Coefficient (m)	3.23389	Sp	Speed Power Coefficient (p)		0.58333
PF Slope	Coefficient (m)	-1.19019	PF	PF Power Coefficient (p)		0.80869
In Passin	g Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					·
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	55.0
Vehicle	e Results					
Average	Speed, mi/h	55.0	Pe	rcent Followers,	, %	4.1
Segment	t Travel Time, minutes	1.09	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	, , , , , , , , , , , , , , , , , , ,		LOS	
1	3	0.00			0.0	A

		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		Т	ime Analyzed		Morning Peak Hour
Project	Description	Build Ph1 to LY1 Locus Grove Road Eastbound (between Nicosin Road and I-82)	d	Units		U.S. Customary
		So	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane W	idth, ft	12	S	Shoulder Width, f	t	4
Speed L	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity					·
Directio	onal Demand Flow Rate, veh/h	27		Dpposing Deman	d Flow Rate, veh/h	62
Peak Ho	our Factor	0.60	Т	otal Trucks, %		25.00
Segmer	nt Capacity, veh/h	1700	Demand/Capacity		/ (D/C)	0.02
Inter	mediate Results					
Segmer	nt Vertical Class	1		ree-Flow Speed,	mi/h	54.5
Speed S	Slope Coefficient (m)	3.19495	S	Speed Power Coe	fficient (p)	0.59400
PF Slop	e Coefficient (m)	-1.18032	P	PF Power Coefficient (p)		0.81222
In Passi	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%Impro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.5
Vehic	le Results					
Average	e Speed, mi/h	54.5	Р	Percent Followers,	, %	6.0
Segmer	nt Travel Time, minutes	1.10	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A		А				
Facili	ty Results		,			
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	4	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2025
Jurisdicti	ion		Т	ime Analyzed		Afternoon Peak Hour
Project [	Description	Build Ph1 to LY1 Locus Grove Road Eastbound (between Nicosin Road and I-82)	d	Inits		U.S. Customary
		Se	egme	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone	L	ength, ft		5280
Lane Wid	dth, ft	12	S	houlder Width, ft	t	4
Speed Li	mit, mi/h	50	А	ccess Point Dens	sity, pts/mi	1.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	61	С	Opposing Demand Flow Rate, veh/h		16
Peak Ho	ur Factor	0.85	To	otal Trucks, %		22.00
Segment	t Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.04
Intern	nediate Results					
Segment	t Vertical Class	1		ree-Flow Speed,	mi/h	54.6
Speed SI	lope Coefficient (m)	3.16103	Speed Power Coefficient (p)		0.63165	
PF Slope	Coefficient (m)	-1.14673		PF Power Coefficient (p)		0.82189
In Passin	g Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.1
%lmpro\	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	gment Data	·				
# Se	gment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	54.6
Vehicl	e Results					
Average	Speed, mi/h	54.6	Р	ercent Followers,	, %	10.9
Segmen	t Travel Time, minutes	1.10	F	ollower Density (	FD), followers/mi/ln	0.1
Vehicle LOS A						
Facilit	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	13	0.00			0.1	А
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	Д	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Morning Peak Hour
Project	Description	Build Ph1 to LY1 Locus Grove Road Westboun (between Nicosin Road and I-82)	ıd	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	4
Speed L	imit, mi/h	50	Δ	Access Point Dens	sity, pts/mi	1.0
Dema	and Capacity	·				<u> </u>
Directio	onal Demand Flow Rate, veh/h	70	C	Opposing Deman	d Flow Rate, veh/h	30
Peak Ho	our Factor	0.53		otal Trucks, %		11.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.04
Interr	mediate Results					
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.19577	S	Speed Power Coe	fficient (p)	0.61699
PF Slope	e Coefficient (m)	-1.15993	Р	PF Power Coefficient (p)		0.81751
In Passii	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.2
%lmpro	vement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.0
Vehic	le Results					·
Average	e Speed, mi/h	55.0	Р	Percent Followers	, %	12.3
Segmen	nt Travel Time, minutes	1.09	F	follower Density (	(FD), followers/mi/ln	0.2
Vehicle LOS A		А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	9	0.00			0.2	А
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	Δ	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY1 Locus Grove Road Westboun (between Nicosin Road and I-82)	ıd	Jnits		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	4
Speed L	Limit, mi/h	50	Δ	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity	·				
Directio	onal Demand Flow Rate, veh/h	22	C	Dpposing Deman	d Flow Rate, veh/h	83
Peak Ho	our Factor	0.63	Т	otal Trucks, %		10.00
Segmer	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.01
Interr	mediate Results					
Segmer	nt Vertical Class	1		ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.23480	S	Speed Power Coe	fficient (p)	0.58252
PF Slop	e Coefficient (m)	-1.19092	Р	PF Power Coefficient (p)		0.80848
In Passii	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data	·				
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.0
Vehic	le Results					
Average	e Speed, mi/h	55.0	Р	Percent Followers,	, %	5.3
Segmen	nt Travel Time, minutes	1.09	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A		А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	4	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Pate		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		Т	ime Analyzed		Morning Peak Hour
Project	Description	Build Ph1 to LY2 Locus Grove Road Eastbound (between Nicosin Road and I-82)	d	Units		U.S. Customary
		So	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane W	idth, ft	12	S	Shoulder Width, f	t	4
Speed L	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	25		Dpposing Deman	d Flow Rate, veh/h	235
Peak Ho	our Factor	0.60	Т	otal Trucks, %		25.00
Segmer	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.01
Inter	mediate Results					
Segmer	nt Vertical Class	1		ree-Flow Speed,	mi/h	54.5
Speed S	Slope Coefficient (m)	3.27246	S	Speed Power Coe	fficient (p)	0.52998
PF Slop	e Coefficient (m)	-1.23895		PF Power Coefficient (p)		0.79513
In Passi	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%Impro	ovement to Percent Followers	0.0	9	6lmprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.5
Vehic	le Results					
Average	e Speed, mi/h	54.5	Р	Percent Followers	, %	6.4
Segmer	nt Travel Time, minutes	1.10	F	follower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A		А				
Facili	ty Results		,			
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ect Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	1	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		T	Time Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY2 Locus Grove Road Eastbound (between Nicosin Road and I-82)	l b	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane W	/idth, ft	12	S	Shoulder Width, f	t	4
Speed I	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity	·				
Directio	onal Demand Flow Rate, veh/h	184		Opposing Deman	d Flow Rate, veh/h	15
Peak Ho	our Factor	0.85	Т	Total Trucks, %		22.00
Segmer	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.11
Inter	mediate Results					
Segmer	nt Vertical Class	1		ree-Flow Speed,	mi/h	54.6
Speed S	Slope Coefficient (m)	3.15950	S	Speed Power Coe	fficient (p)	0.63318
PF Slop	pe Coefficient (m)	-1.14538		PF Power Coefficient (p)		0.82229
In Passi	ing Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.8
%lmprc	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data	·				
# S	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.0
Vehic	cle Results					
Average	e Speed, mi/h	54.0	F	Percent Followers,	, %	24.7
Segmer	nt Travel Time, minutes	1.11	F	Follower Density (	(FD), followers/mi/ln	0.8
Vehicle LOS A		А				
Facili	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	39	0.01			0.8	A
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		HCS Two-Lai	ne H	ighway Re	port	
Proj	ject Information					
Analy	/st	James Vorosmarti		Pate		7/12/2023
Agen	су	Tetra Tech	A	Analysis Year		2025
Jurisc	diction		Т	ime Analyzed		Morning Peak Hour
Proje	ct Description	Build Ph1 to LY2 Locus Grove Road Westboun (between Nicosin Road and I-82)	d	Units		U.S. Customary
		Se	egme	ent 1		
Veh	icle Inputs					
Segm	nent Type	Passing Zone	L	ength, ft		5280
Lane	Width, ft	12	S	Shoulder Width, ft	t	4
Speed	d Limit, mi/h	50	Α	Access Point Dens	ity, pts/mi	1.0
Den	nand and Capacity					
Direc	tional Demand Flow Rate, veh/h	266	C	Opposing Deman	d Flow Rate, veh/h	28
Peak	Hour Factor	0.53	Т	Total Trucks, %		11.00
Segm	nent Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.16
Inte	rmediate Results					
Segm	nent Vertical Class	1	F	ree-Flow Speed,	mi/h	55.0
Speed	d Slope Coefficient (m)	3.19396	S	Speed Power Coet	fficient (p)	0.61875
PF Slo	ope Coefficient (m)	-1.15837		PF Power Coefficient (p)		0.81797
In Pas	ssing Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	1.6
%lmp	provement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Sub	segment Data					
#	Segment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	5280	-		-	53.9
Veh	icle Results					
Avera	age Speed, mi/h	53.9	P	ercent Followers,	, %	32.4
Segm	nent Travel Time, minutes	1.11	F	ollower Density (	FD), followers/mi/ln	1.6
Vehicle LOS A						
Faci	lity Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	35	0.01			1.6	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	Δ	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY2 Locus Grove Road Westboun (between Nicosin Road and I-82)	ıd	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	4
Speed L	Limit, mi/h	50	Δ	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity	·				
Directio	onal Demand Flow Rate, veh/h	21	C	Opposing Deman	d Flow Rate, veh/h	248
Peak Ho	our Factor	0.63	Т	otal Trucks, %		10.00
Segmer	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.01
Interr	mediate Results					
Segmer	nt Vertical Class	1		ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.30374	Speed Power Coefficient (p)		0.52689	
PF Slop	e Coefficient (m)	-1.24182	Р	PF Power Coefficient (p)		0.79354
In Passii	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.0
Vehic	le Results					
Average	e Speed, mi/h	55.0	Р	Percent Followers,	, %	5.6
Segmen	nt Travel Time, minutes	1.09	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle LOS A		А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/In		LOS
1	3	0.00			0.0	А
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		HCS Two-Lar	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	Δ	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Morning Peak Hour
Project I	Description	No Build Locust Grove Road Eastbound (betwe Nicosin Road and I-82)		Jnits		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, ft	t	4
Speed L	imit, mi/h	50	4	Access Point Dens	ity, pts/mi	1.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	20		Dpposing Demand	d Flow Rate, veh/h	62
Peak Ho	our Factor	0.60	Т	otal Trucks, %		25.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.01
Intern	nediate Results					·
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	54.5
Speed S	Slope Coefficient (m)	3.19495	S	Speed Power Coefficient (p)		0.59400
PF Slope	e Coefficient (m)	-1.18032	P	PF Power Coefficie	ent (p)	0.81222
In Passir	ng Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.5
Vehic	le Results					
Average	e Speed, mi/h	54.5	P	Percent Followers,	%	4.8
Segmen	nt Travel Time, minutes	1.10	F	Follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	3	0.00			0.0	A
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		HCS Two-Lan	e Hig	ıhway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Afternoon Peak Hour
Project I	Description	No Build Locust Grove Road Eastbound (betwe Nicosin Road and I-82)	en Uni	ts		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	4
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	1.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	61	Орг	oosing Deman	d Flow Rate, veh/h	12
Peak Ho	our Factor	0.85	Tota	al Trucks, %		22.00
Segmen	nt Capacity, veh/h	1700	Der	nand/Capacity	, (D/C)	0.04
Interr	mediate Results					
Segmen	nt Vertical Class	1	Free	e-Flow Speed,	mi/h	54.6
Speed S	Slope Coefficient (m)	3.15452	Spe	Speed Power Coefficient (p)		0.63822
PF Slope	e Coefficient (m)	-1.14094	PF I	PF Power Coefficient (p)		0.82360
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.6
Vehic	le Results					<u>'</u>
Average	e Speed, mi/h	54.6	Per	cent Followers,	, %	10.8
Segmen	nt Travel Time, minutes	1.10	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle	LOS	А				
Facilit	ty Results					·
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	13	0.00			0.1	A

		HCS Two-La	ne H	lighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		7	Γime Analyzed		Morning Peak Hour
Project	Description	No Build Locust Grove Road Westbound (between Nicosin Road and I-82)		Units		U.S. Customary
		So	egme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	L	_ength, ft		5280
Lane W	idth, ft	12	9	Shoulder Width, f	t	4
Speed L	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity	·				<u> </u>
Directio	onal Demand Flow Rate, veh/h	70		Opposing Deman	d Flow Rate, veh/h	23
Peak Ho	our Factor	0.53	7	Fotal Trucks, %		11.00
Segmer	nt Capacity, veh/h	1700	[	Demand/Capacity	/ (D/C)	0.04
Inter	mediate Results					
Segmer	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.18814	9	Speed Power Coefficient (p)		0.62444
PF Slop	e Coefficient (m)	-1.15331	F	PF Power Coefficie	ent (p)	0.81945
In Passi	ng Lane Effective Length?	No	7	Total Segment De	nsity, veh/mi/ln	0.2
%Impro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.0
Vehic	le Results					
Average	e Speed, mi/h	55.0	F	Percent Followers	, %	12.2
Segmer	nt Travel Time, minutes	1.09	F	ollower Density (	(FD), followers/mi/ln	0.2
Vehicle	Vehicle LOS A					
Facili	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	9	0.00			0.2	A
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т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
Facility	y Results					
Vehicle LOS A		А				
Segment	Travel Time, minutes	1.09	F	Follower Density (	FD), followers/mi/ln	0.0
Average	Speed, mi/h	55.0	F	Percent Followers,	%	4.1
Vehicle	e Results					
1 Tar	ngent	5280	-		-	55.0
# Seg	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
Subse	gment Data					
%Improv	rement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
	g Lane Effective Length?	No	_	Total Segment Density, veh/mi/ln		0.0
<u> </u>	Coefficient (m)	-1.19092	F	PF Power Coefficient (p)		0.80848
Speed Sl	ope Coefficient (m)	3.23480		Speed Power Coefficient (p)		0.58252
Segment	Vertical Class	1		Free-Flow Speed,	mi/h	55.0
Interm	nediate Results					
Segment	Capacity, veh/h	1700	[	Demand/Capacity	(D/C)	0.01
Peak Hour Factor		0.63		Total Trucks, %		10.00
Direction	al Demand Flow Rate, veh/h	16		Opposing Demand	d Flow Rate, veh/h	83
Demai	nd and Capacity					
Speed Lii	mit, mi/h	50	1	Access Point Dens	ity, pts/mi	1.0
Lane Wic		12		Shoulder Width, ft		4
Segment		Passing Zone		Length, ft		5280
	e Inputs					
		Se	egme	ent 1		
Project D	Description	No Build Locust Grove Road Westbound (between Nicosin Road and I-82)		Units		U.S. Customary
Jurisdicti				Time Analyzed		Afternoon Peak Hour
Agency		Tetra Tech	1	Analysis Year		2025
Analyst		James Vorosmarti		Date		7/12/2023
Projec	t Information					
Droice						

		HCS Two-Lan	ie Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Locust Grove Road Eastbound (betwe Nicosin Road and I-82)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	4
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	23	Орр	oosing Deman	d Flow Rate, veh/h	222
Peak Hou	ur Factor	0.60	Tota	Total Trucks, %		25.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.01
Interm	nediate Results					<u> </u>
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	54.5
Speed SI	ope Coefficient (m)	3.26788	Spe	ed Power Coe	fficient (p)	0.53338
PF Slope	Coefficient (m)	-1.23580	PF F	Power Coefficie	ent (p)	0.79608
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	54.5
Vehicle	e Results					
Average	Speed, mi/h	54.5	Perd	cent Followers,	, %	6.0
Segment Travel Time, minutes 1.10		1.10	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	j		LOS	
1	4	0.00			0.0	A

		HCS Two-Lar	ne Hig	hway Re	port			
Projec	t Information							
Analyst		James Vorosmarti	Date	e		7/12/2023		
Agency		Tetra Tech	Ana	lysis Year		2026		
Jurisdiction	on		Tim	e Analyzed		Afternoon Peak Hour		
Project D	Pescription	PH2 to LY2 Locust Grov Road Eastbound (betwe Nicosin Road and I-82)		ts		U.S. Customary		
Segment 1								
Vehicle	e Inputs							
Segment	Туре	Passing Zone	Len	gth, ft		5280		
Lane Wid	lth, ft	12	Sho	ulder Width, ft	t	4		
Speed Lir	mit, mi/h	50	Acc	ess Point Dens	ity, pts/mi	1.0		
Demai	nd and Capacity							
Direction	al Demand Flow Rate, veh/h	175	Орр	oosing Deman	d Flow Rate, veh/h	14		
Peak Hou	ur Factor	0.85	Tota	al Trucks, %		22.00		
Segment	: Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.10		
Interm	nediate Results							
Segment	Vertical Class	1	Free	e-Flow Speed,	mi/h	54.6		
Speed Slo	ope Coefficient (m)	3.15791	Spe	ed Power Coet	fficient (p)	0.63478		
PF Slope	Coefficient (m)	-1.14397	PF F	Power Coefficie	ent (p)	0.82271		
In Passing	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.8		
%Improv	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0		
Subse	gment Data							
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h		
1 Tan	ngent	5280	-		-	54.0		
Vehicle	e Results							
Average	Speed, mi/h	54.0	Perd	cent Followers,	%	23.9		
Segment	Travel Time, minutes	1.11	Foll	ower Density (	FD), followers/mi/ln	0.8		
Vehicle L	OS	А						
Facility	y Results							
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS		
1	37	0.01			0.8	А		

		HCS Two-Lai	ne Hi	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency	,	Tetra Tech	A	nalysis Year		2026
Jurisdic	tion		Ti	me Analyzed		Morning Peak Hour
Project	Description	Ph2 to LY2 Locust Grov Road Westbound (between Nicosin Road and I-82)		nits		U.S. Customary
		Se	egme	nt 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	Le	ength, ft		5280
Lane W	idth, ft	12	Sł	houlder Width, f	t	4
Speed L	Limit, mi/h	50	A	ccess Point Dens	sity, pts/mi	1.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	251	0	Opposing Demand Flow Rate, veh/h		26
Peak Ho	our Factor	0.53	To	otal Trucks, %		11.00
Segmer	nt Capacity, veh/h	1700	D	emand/Capacity	/ (D/C)	0.15
Interr	mediate Results					
Segmer	nt Vertical Class	1	Fr	ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.19209	Sp	Speed Power Coefficient (p)		0.62057
PF Slop	e Coefficient (m)	-1.15675	PI	F Power Coefficie	ent (p)	0.81844
In Passi	ng Lane Effective Length?	No	To	otal Segment De	ensity, veh/mi/ln	1.4
%lmpro	ovement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					·
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	54.0
Vehic	le Results					
Average	e Speed, mi/h	54.0	Pe	ercent Followers	, %	31.1
	nt Travel Time, minutes	1.11	Fo	ollower Density (	(FD), followers/mi/ln	1.4
Vehicle	Vehicle LOS A					
Facili	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	33	0.01			1.4	А
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		HCS Two-Lar	ne Hi	ighway Re	port	
Proje	ect Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency	1	Tetra Tech	А	nalysis Year		2026
Jurisdic	tion		Ti	ime Analyzed		Afternoon Peak Hour
Project	Description	PH2 to LY2 Locust Grov Road Westbound (between Nicosin Road and I-82)		nits		U.S. Customary
		Se	egme	nt 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	Le	ength, ft		5280
Lane W	/idth, ft	12	SI	houlder Width, f	t	4
Speed L	Limit, mi/h	50	А	ccess Point Dens	sity, pts/mi	1.0
Dema	and and Capacity					
Direction	onal Demand Flow Rate, veh/h	19	О	pposing Demand Flow Rate, veh/h		237
Peak Ho	our Factor	0.63	To	otal Trucks, %		10.00
Segmer	nt Capacity, veh/h	1700	D	emand/Capacity	ι (D/C)	0.01
Interi	mediate Results					
Segmer	nt Vertical Class	1	Fı	ree-Flow Speed,	mi/h	55.0
Speed S	Slope Coefficient (m)	3.30004	Sı	Speed Power Coefficient (p)		0.52960
PF Slop	e Coefficient (m)	-1.23932	P	F Power Coefficie	ent (p)	0.79429
In Passi	ing Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0
%lmprc	ovement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					•
# S	egment Type	Length, ft	Radius	., ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	55.0
Vehic	cle Results					
Average	e Speed, mi/h	55.0	Pe	ercent Followers,	, %	5.2
	nt Travel Time, minutes	1.09	Fo	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle	Vehicle LOS A					
Facili	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	3	0.00			0.0	А
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		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	ion		Tim	e Analyzed		Morning Peak Hour
Project D	Description	No Build - Locust Grove Road Eastbound (betwe Nicosin Road and I-82)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	4
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	20	Орр	oosing Deman	d Flow Rate, veh/h	62
Peak Hou	ur Factor	0.60	Tota	al Trucks, %		25.00
Segment	t Capacity, veh/h	1700	Der	nand/Capacity	(D/C)	0.01
Interm	nediate Results					
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	54.5
Speed SI	lope Coefficient (m)	3.19495	Spe	ed Power Coe	fficient (p)	0.59400
PF Slope	Coefficient (m)	-1.18032	PF F	Power Coefficie	ent (p)	0.81222
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	54.5
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	54.5	Perd	cent Followers,	, %	4.8
Segment	t Travel Time, minutes	1.10	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l		ensity, followers/ mi/ln	LOS
1	3	0.00			0.0	A

		HCS Two-Lan	ie Hig	Jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	No Build Locust Grove Road Eastbound (betwe Nicosin Road and I-82)	en Uni	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	4
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	62	Орр	Opposing Demand Flow Rate, veh/h		12
Peak Hou	ur Factor	0.85	5 Total Tru			22.00
Segment	t Capacity, veh/h	1700	Der	mand/Capacity	(D/C)	0.04
Interm	nediate Results					·
Segment	t Vertical Class	1	Free	Free-Flow Speed, mi/h		54.6
Speed SI	ope Coefficient (m)	3.15452	Spe	Speed Power Coefficient (p)		0.63822
PF Slope	Coefficient (m)	-1.14094	PF F	Power Coefficie	ent (p)	0.82360
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%lmprov	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					·
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	54.6
Vehicle	e Results					
Average	Speed, mi/h	54.6	Perd	cent Followers	, %	11.0
Segment	t Travel Time, minutes	1.10	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					·
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	13	0.00			0.1	A

		HCS Two-Lar	ne H	lighway Re	port	
Project Informa	ntion					
Analyst		James Vorosmarti	[	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2026
Jurisdiction			7	Time Analyzed		Morning Peak Hour
Project Description		No Build Locust Grove Road Westbound (between Nicosin Road and I-82)		Jnits		U.S. Customary
		Se	egme	ent 1		
Vehicle Inputs						
Segment Type		Passing Zone	L	Length, ft		5280
Lane Width, ft		12	9	Shoulder Width, ft	:	4
Speed Limit, mi/h		50	A	Access Point Dens	ity, pts/mi	1.0
Demand and Ca	apacity					
Directional Demand F	low Rate, veh/h	70		Opposing Demand Flow Rate, veh/h		23
Peak Hour Factor		0.53		Total Trucks, %		11.00
Segment Capacity, ve	h/h	1700	[	Demand/Capacity	(D/C)	0.04
Intermediate Re	esults					
Segment Vertical Clas	s	1		Free-Flow Speed,	mi/h	55.0
Speed Slope Coefficie	nt (m)	3.18814		Speed Power Coefficient (p)		0.62444
PF Slope Coefficient (ı	m)	-1.15331		PF Power Coefficient (p)		0.81945
In Passing Lane Effect	ive Length?	No	1	Total Segment Density, veh/mi/ln		0.2
%Improvement to Per	cent Followers	0.0	9	%Improvement to Speed		0.0
Subsegment Da	nta					
# Segment Type		Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Tangent		5280	-		-	55.0
Vehicle Results						
Average Speed, mi/h		55.0	F	Percent Followers,	%	12.2
Segment Travel Time,	minutes	1.09	F	Follower Density (	FD), followers/mi/ln	0.2
Vehicle LOS A		А				
Facility Results						
т ,	VMT /eh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	9	0.00			0.2	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2026
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	No Build Locust Grove Road Westbound (between Nicosin Road and I-82)		Jnits		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	houlder Width, f	t	4
Speed L	imit, mi/h	53	А	access Point Dens	sity, pts/mi	1.0
Dema	and Capacity	·				·
Directio	nal Demand Flow Rate, veh/h	16	С	Dpposing Deman	d Flow Rate, veh/h	81
Peak Ho	our Factor	0.63	To	otal Trucks, %		10.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity	/ (D/C)	0.01
Interr	nediate Results					
Segmer	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	58.4
Speed S	Slope Coefficient (m)	3.41925	S	Speed Power Coefficient (p)		0.58333
PF Slope	e Coefficient (m)	-1.17850	Р	F Power Coefficie	ent (p)	0.81900
In Passii	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.0
%lmpro	vement to Percent Followers	0.0	%	6lmprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	58.4
Vehic	le Results					
Average	e Speed, mi/h	58.4	Р	ercent Followers	, %	3.9
Segmen	nt Travel Time, minutes	1.03	F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle	Vehicle LOS A					
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	3	0.00			0.0	А
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		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date		7/12/2023	
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Tir	Time Analyzed		Morning Peak Hour
Project D	Description	Nine Canyon Road Ur Northbound (South of Route 397)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment Type		Passing Zone		Length, ft		5280
Lane Wic	dth, ft	11	Sh	Shoulder Width, ft		3
Speed Li	mit, mi/h	50	Ac	Access Point Density, pts/mi		0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4		Opposing Demand Flow Rate, veh/h		72
Peak Hour Factor		0.25		Total Trucks, %		100.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.00
Interm	nediate Results	•				·
Segment Vertical Class		5		Free-Flow Speed, mi/h		29.2
Speed Slope Coefficient (m)		39.40648		Speed Power Coefficient (p)		0.71045
PF Slope Coefficient (m)		-0.99171		PF Power Coefficient (p)		0.95225
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	29.2
Vehicle	e Results					
Average Speed, mi/h		29.2		Percent Followers, %		0.5
Segment Travel Time, minutes		2.05		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		А	A			
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/In		LOS
1	0	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	i Date		7/12/2023	
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Tin	ne Analyzed		Afternoon Peak Hour
Project D	Description	Nine Canyon Road Ur Northbound (South of Route 397)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment Type		Passing Zone		Length, ft		5280
Lane Wic	dth, ft	11	Sh	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Ac	Access Point Density, pts/mi		0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	24		Opposing Demand Flow Rate, veh/h		17
Peak Hour Factor		0.58		Total Trucks, %		14.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.01
Interm	nediate Results	•				
Segment Vertical Class		5		Free-Flow Speed, mi/h		50.8
Speed Slope Coefficient (m)		9.73165		Speed Power Coefficient (p)		0.99097
PF Slope Coefficient (m)		-1.20761		PF Power Coefficient (p)		0.93895
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	50.8
Vehicle	e Results					
Average Speed, mi/h		50.8		Percent Followers, %		3.6
Segment Travel Time, minutes		1.18		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		А	A			
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	4	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	marti Date		7/12/2023	
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Time Analyzed		Morning Peak Hour	
Project Description		Nine Canyon Road Southbound (South of Route 397)		Units		U.S. Customary
		S	egmer	nt 1		
Vehicle	e Inputs					
Segment Type		Passing Zone		Length, ft		5280
Lane Wic	dth, ft	11	Sh	Shoulder Width, ft		3
Speed Li	mit, mi/h	50		Access Point Density, pts/mi		0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	32	Op	Opposing Demand Flow Rate, veh/h		2
Peak Hour Factor		0.56		Total Trucks, %		22.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.02
Interm	nediate Results					·
Segment Vertical Class		5		Free-Flow Speed, mi/h		48.8
Speed Slope Coefficient (m)		10.43144		Speed Power Coefficient (p)		1.05827
PF Slope Coefficient (m)		-1.14465		PF Power Coefficient (p)		0.96662
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	48.8
Vehicle	e Results					
Average Speed, mi/h		48.8		Percent Followers, %		4.0
Segment Travel Time, minutes		1.23 I		Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	5	0.00			0.0	A

		HCS Two-La	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	on		Tin	ne Analyzed		Afternoon Peak Hour
Project D	Description	Nine Canyon Road Southbound (South of Route 397)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Ler	Length, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Ace	Access Point Density, pts/mi		0.0
Demai	nd and Capacity					
Direction	Directional Demand Flow Rate, veh/h 16 C		Ор	posing Deman	d Flow Rate, veh/h	22
Peak Hou	ur Factor	ctor 0.63		al Trucks, %		30.00
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					
Segment	t Vertical Class	5	Fre	e-Flow Speed,	mi/h	46.8
Speed SI	ope Coefficient (m)	11.00270	Spe	Speed Power Coefficient (p)		0.99232
PF Slope	Coefficient (m)	-1.17557	PF	PF Power Coefficient (p)		0.96202
In Passin	g Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.0
%Improv	vement to Percent Followers	0.0	%lı	mprovement to	Speed	0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	46.8
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	46.8	Per	cent Followers	, %	2.2
Segment	t Travel Time, minutes	1.28	Fol	lower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l l		ensity, followers/ mi/ln	LOS
1	3	0.00			0.0	A

Project					port	
	Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdictio	n		Tim	ie Analyzed		Morning Peak Hour
Project De	escription	Build Ph1 to LY1 Nine Canyon Road Northbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	t 1		
Vehicle	Inputs					
Segment 1	Гуре	Passing Zone	Len	Length, ft		5280
Lane Widt	h, ft	11	Sho	oulder Width, f	t	3
Speed Lim	nit, mi/h	50	Acc	Access Point Density, pts/mi		0.0
Deman	d and Capacity					
Directiona	rectional Demand Flow Rate, veh/h 4 O		Ор	posing Deman	d Flow Rate, veh/h	72
Peak Hour	· Factor	0.25		al Trucks, %		100.00
Segment (	Capacity, veh/h	1700	Demand/Capacity (D/C)		0.00	
Interme	ediate Results					·
Segment \	Vertical Class	5	Fre	e-Flow Speed,	mi/h	29.2
Speed Slo	pe Coefficient (m)	39.40648	Spe	Speed Power Coefficient (p)		0.71045
PF Slope C	Coefficient (m)	-0.99171		PF Power Coefficient (p)		0.95225
In Passing	Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.0
%Improve	ment to Percent Followers	0.0	%Ir	nprovement to	Speed	0.0
Subseg	ment Data					
# Segr	ment Type	Length, ft	Radius, 1	t	Superelevation, %	Average Speed, mi/h
1 Tang	gent	5280	-		-	29.2
Vehicle	Results					
Average S	peed, mi/h	29.2	Per	cent Followers,	%	0.5
Segment 1	Fravel Time, minutes	2.05	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle LO	S	А				
Facility	Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	0	0.00			0.0	A

		HCS Two-Lan	ne Hig	ıhway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY1 Nine Canyon Road Northbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	11	Sho	ulder Width, f	t	3
Speed L	imit, mi/h	50	Acc	Access Point Density, pts/mi		0.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	ow Rate, veh/h 24 Opposing Demand Flow		d Flow Rate, veh/h	17	
Peak Ho	our Factor	0.58	Total Trucks, %			14.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Intern	nediate Results					
Segmen	nt Vertical Class	5	Free	e-Flow Speed,	mi/h	50.8
Speed S	Slope Coefficient (m)	9.73165	Spe	Speed Power Coefficient (p)		0.99097
PF Slope	e Coefficient (m)	-1.20761	PF I	PF Power Coefficient (p)		0.93895
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	50.8
Vehic	le Results					
Average	e Speed, mi/h	50.8	Per	cent Followers,	, %	3.6
Segmen	nt Travel Time, minutes	1.18	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A

		HCS Two-Lan	e Hig	ıhway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY1 Nine Canyon Road Southbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	11	Sho	ulder Width, f	t	3
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	32	Opposing Demand Flow Rate, veh/h		2	
Peak Ho	our Factor	0.56	Total Trucks, %			22.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.02
Intern	nediate Results					
Segmen	nt Vertical Class	5	Free	e-Flow Speed,	mi/h	48.8
Speed S	Slope Coefficient (m)	10.43144	Spe	Speed Power Coefficient (p)		1.05827
PF Slope	e Coefficient (m)	-1.14465	PF F	PF Power Coefficient (p)		0.96662
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	48.8
Vehic	le Results					'
Average	e Speed, mi/h	48.8	Perd	cent Followers	, %	4.0
Segmen	nt Travel Time, minutes	1.23	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					·
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A

		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdictio	on		Tin	ne Analyzed		Afternoon Peak Hour
Project D	escription	Build Ph1 to LY1 Nine Canyon Road Southbou (South of Route 397)		Units		U.S. Customary
		Se	gmer	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Ler	Length, ft		5280
Lane Wid	lth, ft	11	Sho	oulder Width, f	t	3
Speed Lir	mit, mi/h	50	Ace	cess Point Dens	sity, pts/mi	0.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	16	Opposing Demand Flow Rate, veh/h		22	
Peak Hou	ur Factor	0.63	Total Trucks, %			30.00
Segment	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results					
Segment	Vertical Class	5	Fre	e-Flow Speed,	mi/h	46.8
Speed Slo	ope Coefficient (m)	11.00270	Spe	eed Power Coe	fficient (p)	0.99232
PF Slope	Coefficient (m)	-1.17557	PF	PF Power Coefficient (p)		0.96202
In Passino	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.0
%Improv	ement to Percent Followers	0.0	%lı	mprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	46.8
Vehicle	e Results					·
Average S	Speed, mi/h	46.8	Per	cent Followers	, %	2.2
Segment	Travel Time, minutes	1.28	Fol	lower Density (	(FD), followers/mi/ln	0.0
Vehicle L0	OS	Α				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
		<u> </u>		+	0.0	A

		HCS Two-Lan	ne Hig	hway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY2 Nine Canyon Road Northbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	11	Sho	ulder Width, f	t	3
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					·
Direction	onal Demand Flow Rate, veh/h 4 Opposin		oosing Deman	d Flow Rate, veh/h	72	
Peak Ho	our Factor	0.25		al Trucks, %		100.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.00
Intern	nediate Results					·
Segmen	nt Vertical Class	5	Free	e-Flow Speed,	mi/h	29.2
Speed S	Slope Coefficient (m)	39.40648	Spe	Speed Power Coefficient (p)		0.71045
PF Slope	e Coefficient (m)	-0.99171	PF I	PF Power Coefficient (p)		0.95225
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	29.2
Vehic	le Results	·				·
Average	e Speed, mi/h	29.2	Per	cent Followers,	, %	0.5
Segmen	nt Travel Time, minutes	2.05	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	0	0.00			0.0	A

		HCS Two-Lan	ne Hig	ıhway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY2 Nine Canyon Road Northbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	11	Sho	ulder Width, f	t	3
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	nand Flow Rate, veh/h 24 Opposing De		oosing Deman	d Flow Rate, veh/h	17
Peak Ho	our Factor	0.58	Total Trucks, %			14.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Intern	nediate Results					
Segmen	nt Vertical Class	5	Free	e-Flow Speed,	mi/h	50.8
Speed S	Slope Coefficient (m)	9.73165	Spe	Speed Power Coefficient (p)		0.99097
PF Slope	e Coefficient (m)	-1.20761	PF I	PF Power Coefficient (p)		0.93895
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	50.8
Vehic	le Results					·
Average	e Speed, mi/h	50.8	Per	cent Followers	, %	3.6
Segmen	nt Travel Time, minutes	1.18	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p	<b>_</b>		ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A

Project In Analyst Agency Jurisdiction Project Descri  Vehicle In Segment Type	puts	James Vorosmarti Tetra Tech  Build Ph1 to LY2 Nine Canyon Road Southboo (South of Route 397)	Tir	nalysis Year me Analyzed nits		7/12/2023 2025 Morning Peak Hour U.S. Customary
Agency Jurisdiction Project Descri	puts	Tetra Tech  Build Ph1 to LY2 Nine Canyon Road Southbor (South of Route 397)	An Tir Ur	nalysis Year me Analyzed nits		2025 Morning Peak Hour
Jurisdiction Project Descri	puts	Build Ph1 to LY2 Nine Canyon Road Southbou (South of Route 397)	Tir Ur und	me Analyzed		Morning Peak Hour
Project Descri	puts	Canyon Road Southbou (South of Route 397)	und Ur	nits		
Vehicle In	puts	Canyon Road Southbou (South of Route 397)	und			U.S. Customary
	-	Se	egmei	nt 1		
	-					
Segment Type	е					
		Passing Zone	Le	Length, ft		5280
Lane Width, ft	t	11	Sh	oulder Width, ft	t	3
Speed Limit, n	mi/h	50	Ac	cess Point Dens	ity, pts/mi	0.0
Demand a	and Capacity					
Directional De	emand Flow Rate, veh/h	32	Opposing Demand Flow Rate, veh/h		2	
Peak Hour Fac	ctor	0.56	Total Trucks, %			22.00
Segment Capa	acity, veh/h	1700	De	Demand/Capacity (D/C)		0.02
Intermedi	iate Results					·
Segment Verti	ical Class	5	Fre	ee-Flow Speed,	mi/h	48.8
Speed Slope (	Coefficient (m)	10.43144	Sp	Speed Power Coefficient (p)		1.05827
PF Slope Coef	fficient (m)	-1.14465	PF	PF Power Coefficient (p)		0.96662
In Passing Lan	ne Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%Improvemer	nt to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subsegme	ent Data					·
# Segmen	nt Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tangent	t	5280	-		-	48.8
Vehicle Re	esults					
Average Spee	ed, mi/h	48.8	Pe	rcent Followers,	. %	4.0
Segment Trav	rel Time, minutes	1.23	Fo	llower Density (	FD), followers/mi/ln	0.0
Vehicle LOS		Α				
Facility Re	esults					•
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	5	0.00			0.0	A

		HCS Two-Lan	ne Hig	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	:e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdict	ion		Tim	ne Analyzed		Afternoon Peak Hour
Project [	Description	Build Ph1 to LY2 Nine Canyon Road Southbou (South of Route 397)		Units		U.S. Customary
		Se	gmen	nt 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	Len	Length, ft		5280
Lane Wi	dth, ft	11	Sho	oulder Width, f	t	3
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	16	Opposing Demand Flow Rate, veh/h		d Flow Rate, veh/h	22
Peak Ho	our Factor	0.63	Total Trucks, %			30.00
Segmen	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Intern	nediate Results					·
Segmen	nt Vertical Class	5	Fre	e-Flow Speed,	mi/h	46.8
Speed S	lope Coefficient (m)	11.00270	Spe	Speed Power Coefficient (p)		0.99232
PF Slope	e Coefficient (m)	-1.17557	PF	PF Power Coefficient (p)		0.96202
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%lr	nprovement to	Speed	0.0
Subse	egment Data					·
# Se	egment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	46.8
Vehicl	le Results					
Average	Speed, mi/h	46.8	Per	cent Followers,	, %	2.2
Segmen	t Travel Time, minutes	1.28	Foll	lower Density (	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	3	0.00			0.0	A
1		•				A

		HCS Two-Lar	ne H	lighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	- [	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	tion		1	Гime Analyzed		Morning Peak Hour
Project	Description	No Build Nine Canyon Road Northbound (Sou of Route 397)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	idth, ft	11	5	Shoulder Width, f	t	3
Speed L	_imit, mi/h	50	1	Access Point Density, pts/mi		0.0
Dema	and and Capacity					·
Directio	irectional Demand Flow Rate, veh/h 4 Opposing Demand		d Flow Rate, veh/h	72		
Peak Ho	our Factor	0.25	1	Total Trucks, %		100.00
Segmen	nt Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.00
Interr	mediate Results					·
Segmen	nt Vertical Class	5	5 Free-Flow		mi/h	29.2
Speed S	Slope Coefficient (m)	39.40648		Speed Power Coefficient (p)		0.71045
PF Slope	e Coefficient (m)	-0.99171		PF Power Coefficient (p)		0.95225
In Passir	ng Lane Effective Length?	No	1	Total Segment Density, veh/mi/ln		0.0
%Impro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	29.2
Vehic	le Results					
Average	e Speed, mi/h	29.2	F	Percent Followers	, %	0.5
Segmen	nt Travel Time, minutes	2.05	F	Follower Density (	(FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	0	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	ion		Т	ime Analyzed		Afternoon Peak Hour
Project I	Description	No Build Nine Canyon Road Northbound (Sou of Route 397)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	dth, ft	11	S	Shoulder Width, ft	t	3
Speed L	imit, mi/h	50	A	Access Point Density, pts/mi		0.0
Dema	and Capacity					<u> </u>
Directio	nal Demand Flow Rate, veh/h 24 Opposing Demand Flow Rate, veh/h		17			
Peak Ho	our Factor	0.58	Т	Total Trucks, %		14.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.01
Intern	nediate Results					·
Segmen	t Vertical Class	5	F	ree-Flow Speed,	mi/h	50.8
Speed S	lope Coefficient (m)	9.73165		Speed Power Coefficient (p)		0.99097
PF Slope	e Coefficient (m)	-1.20761		PF Power Coefficient (p)		0.93895
In Passir	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	50.8
Vehic	le Results					
Average	Speed, mi/h	50.8	P	Percent Followers,	, %	3.6
Segmen	t Travel Time, minutes	1.18	F	follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour
Project I	Description	No Build Nine Canyon Road Southbound (Sou of Route 397)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	dth, ft	11	S	Shoulder Width, f	t	3
Speed L	imit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	32	Opposing Demand Flow Rate, veh/h		2	
Peak Ho	our Factor	0.56	1	Total Trucks, %		22.00
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.02
Intern	nediate Results					·
Segmen	t Vertical Class	5	F	ree-Flow Speed,	mi/h	48.8
Speed S	lope Coefficient (m)	10.43144		Speed Power Coefficient (p)		1.05827
PF Slope	e Coefficient (m)	-1.14465		PF Power Coefficient (p)		0.96662
In Passir	ng Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	48.8
Vehic	le Results					
Average	Speed, mi/h	48.8	F	Percent Followers,	, %	4.0
Segmen	t Travel Time, minutes	1.23	F	ollower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	Δ	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	No Build Nine Canyon Road Southbound (Sou of Route 397)		Jnits		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	11	S	Shoulder Width, ft	t	3
Speed L	_imit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	16		Opposing Deman	d Flow Rate, veh/h	22
Peak Ho	our Factor	0.63	Т	otal Trucks, %		30.00
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.01
Interr	mediate Results					·
Segmen	nt Vertical Class	5		ree-Flow Speed,	mi/h	46.8
Speed S	Slope Coefficient (m)	11.00270		Speed Power Coefficient (p)		0.99232
PF Slope	e Coefficient (m)	-1.17557	-1.17557 PF		ent (p)	0.96202
In Passii	ng Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	0.0
%lmpro	ovement to Percent Followers	0.0	9	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	46.8
Vehic	le Results					
Average	e Speed, mi/h	46.8	P	Percent Followers,	, %	2.2
Segmen	nt Travel Time, minutes	1.28	F	follower Density (	FD), followers/mi/ln	0.0
Vehicle	le LOS A					
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	3	0.00			0.0	A
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		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ne Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Nine Canyor Road Northbound (Sou of Route 397)		its		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Len	igth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	Ор	posing Deman	d Flow Rate, veh/h	76
Peak Hou	ur Factor	0.25	Total Trucks, %		100.00	
Segment	Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.00
Interm	nediate Results					·
Segment	: Vertical Class	5	Fre	Free-Flow Speed, mi/h		29.2
Speed SI	ope Coefficient (m)	39.44197	Spe	Speed Power Coefficient (p)		0.70647
PF Slope	Coefficient (m)	-0.99451	PF I	PF Power Coefficient (p)		0.95037
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%lmprov	rement to Percent Followers	0.0	%Ir	nprovement to	Speed	0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	29.2
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	29.2	Per	cent Followers	, %	0.5
3 1		2.05	Foll	lower Density (	(FD), followers/mi/ln	0.0
Vehicle L	OS	A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	, , , , , , , , , , , , , , , , , , ,		LOS	
1	0	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	ion		Tim	ie Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Nine Canyon Road Northbound (Sou of Route 397)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	24	Орј	posing Deman	d Flow Rate, veh/h	17
Peak Hou	ur Factor	0.58	Tota	Total Trucks, %		14.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Interm	nediate Results					·
Segment	t Vertical Class	5	Free	e-Flow Speed,	mi/h	50.8
Speed SI	lope Coefficient (m)	9.73165	Spe	Speed Power Coefficient (p)		0.99097
PF Slope	Coefficient (m)	-1.20761	PF I	PF Power Coefficient (p)		0.93895
In Passin	g Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.0
%lmprov	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	50.8
Vehicle	e Results					
Average	Speed, mi/h	50.8	Per	cent Followers	, %	3.6
- '		1.18	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А	A			
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	4	0.00			0.0	A

Project Analyst Agency Jurisdictio	Information	James Vorosmarti				
Agency	n	James Vorosmarti				
	n		Dat	e		7/12/2023
Jurisdictio	n	Tetra Tech	Ana	alysis Year		2026
			Tim	ie Analyzed		Morning Peak Hour
Project De	escription	PH2 to LY2 Nine Canyo Road Southbound (Sou of Route 397)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	Inputs					
Segment 7	Туре	Passing Zone	Len	gth, ft		5280
Lane Widt	th, ft	11	Sho	oulder Width, ft	t	3
Speed Lim	nit, mi/h	50	Acc	ess Point Dens	ity, pts/mi	0.0
Deman	d and Capacity					
Directiona	al Demand Flow Rate, veh/h	34	Орр	posing Deman	d Flow Rate, veh/h	2
Peak Hour	r Factor	0.56	Tota	Total Trucks, %		22.00
Segment (	Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.02
Interm	ediate Results					·
Segment \	Vertical Class	5	Free	e-Flow Speed,	mi/h	48.8
Speed Slo	pe Coefficient (m)	10.43144	Spe	ed Power Coet	fficient (p)	1.05827
PF Slope C	Coefficient (m)	-1.14465	PF F	Power Coefficie	ent (p)	0.96662
In Passing	Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improve	ement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subseg	ment Data					
# Segi	ment Type	Length, ft	Radius, f		Superelevation, %	Average Speed, mi/h
1 Tang	gent	5280	-		-	48.8
Vehicle	Results					
Average S	peed, mi/h	48.8	Perd	cent Followers,	%	4.3
Segment Travel Time, minutes 1.23		1.23	Foll	ower Density (	FD), followers/mi/ln	0.0
Vehicle LC	)S	A				
Facility	Results	•				·
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	5	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ie Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Nine Canyor Road Southbound (Sour of Route 397)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	16	Орг	posing Deman	d Flow Rate, veh/h	22
Peak Hou	ur Factor	0.63	Total Trucks, %		30.00	
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.01
Interm	nediate Results					
Segment	t Vertical Class	5	Free	e-Flow Speed,	mi/h	46.8
Speed SI	ope Coefficient (m)	11.00270	Spe	Speed Power Coefficient (p)		0.99232
PF Slope	Coefficient (m)	-1.17557	-1.17557 PF		ent (p)	0.96202
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# See	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	46.8
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	46.8	Per	cent Followers	, %	2.2
3 1		1.28	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	3	0.00			0.0	A

		HCS Two-Lar	ne Hi	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2026
Jurisdict	ion		Ti	ime Analyzed		Morning Peak Hour
Project [	Description	No Build - Nine Canyor Road Northbound (Sou of Route 397)		nits		U.S. Customary
		Se	gme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	Le	ength, ft		5280
Lane Wi	dth, ft	11	SI	houlder Width, ft	t	3
Speed Li	imit, mi/h	50	А	ccess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	4	0	pposing Deman	d Flow Rate, veh/h	76
Peak Ho	our Factor	0.25	To	otal Trucks, %		100.00
Segmen	t Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.00
Intern	nediate Results					
Segmen	t Vertical Class	5	Fı	ree-Flow Speed,	mi/h	29.2
Speed S	lope Coefficient (m)	39.44197	Sı	Speed Power Coefficient (p)		0.70647
PF Slope	e Coefficient (m)	-0.99451	P	PF Power Coefficient (p)		0.95037
In Passin	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	29.2
Vehicl	le Results					
Average	Speed, mi/h	29.2	Pe	ercent Followers,	, %	0.5
Segmen	t Travel Time, minutes	2.05	Fo	ollower Density (	FD), followers/mi/ln	0.0
Vehicle I	Vehicle LOS A					
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	0	0.00			0.0	A
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		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	ion		Tim	ne Analyzed		Afternoon Peak Hour
Project D	Description	No Build Nine Canyon Road Northbound (Sou of Route 397)	th	its		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Ler	ngth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Acc	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	24	Ор	posing Deman	d Flow Rate, veh/h	17
Peak Hou	ur Factor	0.58	Total Trucks, %			14.00
Segment	t Capacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.01
Interm	nediate Results					·
Segment	t Vertical Class	5	Fre	e-Flow Speed,	mi/h	50.8
Speed SI	lope Coefficient (m)	9.73165	Spe	Speed Power Coefficient (p)		0.99097
PF Slope	Coefficient (m)	-1.20761	PF	PF Power Coefficient (p)		0.93895
In Passin	g Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.0
%Improv	vement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	50.8
Vehicle	e Results					
Average	Speed, mi/h	50.8	Per	cent Followers	, %	3.6
3 1		1.18	Fol	lower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	1		LOS	
1	4	0.00			0.0	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ie Analyzed		Morning Peak Hour
Project D	Description	No Build Nine Canyon Road Southbound (Sou of Route 397)	Uni th	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	т Туре	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	3
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	34	Орј	posing Deman	d Flow Rate, veh/h	2
Peak Hou	ur Factor	0.56	0.56 Total Trucks, %			22.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.02
Interm	nediate Results					
Segment	t Vertical Class	5	Free	e-Flow Speed,	mi/h	48.8
Speed SI	ope Coefficient (m)	10.43144	Spe	Speed Power Coefficient (p)		1.05827
PF Slope	Coefficient (m)	-1.14465	PF I	PF Power Coefficient (p)		0.96662
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.0
%Improv	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# See	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	48.8
Vehicle	e Results					
Average	Speed, mi/h	48.8	Per	cent Followers	, %	4.3
- '		1.23	Foll	ower Density (	(FD), followers/mi/ln	0.0
Vehicle L	OS	A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			LOS	
1	5	0.00			0.0	A

		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Di	ate		7/12/2023
Agency		Tetra Tech	Aı	nalysis Year		2026
Jurisdict	ion		Ti	me Analyzed		Afternoon Peak Hour
Project I	Description	NO Build Nine Canyon Road Southbound (Sou of Route 397)		nits		U.S. Customary
		Se	gme	nt 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	dth, ft	11	Sh	noulder Width, ft	t	3
Speed L	imit, mi/h	50	Ad	ccess Point Dens	ity, pts/mi	0.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	16	О	pposing Demand	d Flow Rate, veh/h	22
Peak Ho	our Factor	0.63	To	otal Trucks, %		30.00
Segmen	t Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.01
Intern	nediate Results					
Segmen	nt Vertical Class	5	Fr	ee-Flow Speed,	mi/h	46.8
Speed S	ilope Coefficient (m)	11.00270	Sp	Speed Power Coefficient (p)		0.99232
PF Slope	e Coefficient (m)	-1.17557	PF	PF Power Coefficient (p)		0.96202
In Passir	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	46.8
Vehicl	le Results					
Average	Speed, mi/h	46.8	Pe	ercent Followers,	. %	2.2
Segment Travel Time, minutes		1.28	Fo	ollower Density (	FD), followers/mi/ln	0.0
Vehicle I	ehicle LOS A					
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	3	0.00			0.0	A
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HCS Summary Plymouth Road

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2023
Jurisdicti	ion		Tin	ne Analyzed		Morning Peak Hour
Project D	Description	Plymouth Road Northbound (North of Route 14)	Un	its		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	12	Sh	oulder Width, f	t	2
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	5.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	24	Op	Opposing Demand Flow Rate, veh/h		59
Peak Hou	ur Factor	0.88	Tot	tal Trucks, %		38.00
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Interm	nediate Results	•				
Segment	t Vertical Class	2	Fre	ee-Flow Speed,	mi/h	51.7
Speed SI	lope Coefficient (m)	3.26494	Sp	Speed Power Coefficient (p)		0.64312
PF Slope	Coefficient (m)	-1.13744	PF	PF Power Coefficient (p)		0.81845
In Passin	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	51.7
Vehicle	e Results					
Average	Speed, mi/h	51.7	Pei	rcent Followers	, %	5.2
		1.16	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '		LOS	
1	5	0.00			0.0	A

		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2023
Jurisdict	ion		Т	ime Analyzed		Afternoon Peak Hour
Project [	Description	Plymouth Road Northbound (North of Route 14)		Jnits		U.S. Customary
		Se	egme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	12	S	houlder Width, f	t	2
Speed Li	imit, mi/h	50	А	access Point Dens	sity, pts/mi	5.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	72	С	Opposing Deman	d Flow Rate, veh/h	79
Peak Ho	our Factor	0.76	To	otal Trucks, %		40.00
Segmen	t Capacity, veh/h	1700	D	Demand/Capacity	(D/C)	0.04
Intern	nediate Results	•				•
Segmen	t Vertical Class	2	F	Free-Flow Speed, mi/h		51.6
Speed S	lope Coefficient (m)	3.32183	S	Speed Power Coefficient (p)		0.62792
PF Slope	e Coefficient (m)	-1.14672	Р	F Power Coefficie	ent (p)	0.81647
In Passin	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.2
%Improv	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	51.6
Vehicl	le Results					·
Average	Speed, mi/h	51.6	Р	ercent Followers,	, %	12.6
Segment Travel Time, minutes		1.16	F	ollower Density (	FD), followers/mi/ln	0.2
Vehicle I	Vehicle LOS A					
Facilit	y Results		,			
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	14	0.00			0.2	A
	2022 Hairranita of Florido All Bioleto			Varsian 2022		Caparatad: 07/12/2022 11:17:2

Project I Analyst Agency Jurisdiction Project Des		HCS Two-La	ne Hig	ghway Re	port	
Agency Jurisdiction	Information					
Jurisdiction		James Vorosmarti	Dat	te		7/12/2023
		Tetra Tech	Ana	alysis Year		2023
Project Des	l		Tim	ne Analyzed		Morning Peak Hour
Troject Des	scription	Plymouth Road Southbound (North of Route 14)	Uni	its		U.S. Customary
		Se	egmen	nt 1		
Vehicle	Inputs					
Segment Ty	уре	Passing Zone	Len	ngth, ft		5280
Lane Width	ı, ft	12	Sho	oulder Width, f	t	2
Speed Limit	t, mi/h	50	Acc	cess Point Dens	sity, pts/mi	5.0
Demand	d and Capacity					
Directional	Demand Flow Rate, veh/h	68	Ор	posing Deman	d Flow Rate, veh/h	28
Peak Hour I	Factor	0.76	Total Trucks, %			29.00
Segment Ca	apacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04
Interme	diate Results					
Segment Ve	ertical Class	2	Fre	e-Flow Speed,	mi/h	52.0
Speed Slop	e Coefficient (m)	3.11550	Spe	eed Power Coe	fficient (p)	0.67746
PF Slope Co	pefficient (m)	-1.12162	PF	Power Coefficie	ent (p)	0.81931
In Passing L	Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.2
%Improven	nent to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subsegr	ment Data					·
# Segm	nent Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Tange	ent	5280	-		-	52.0
Vehicle	Results					
Average Sp	eed, mi/h	52.0	Per	cent Followers,	, %	11.7
- '		1.15	Fol	lower Density (	(FD), followers/mi/ln	0.2
Vehicle LOS	5	A				
Facility	Results		,			
т	VMT veh-mi/p	VHD veh-h/p	· · · · · · · · · · · · · · · · · · ·		LOS	
1	13	0.00			0.2	A

		HCS Two-La	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	Analysis Year		2023
Jurisdicti	on		Tim	ne Analyzed		Afternoon Peak Hour
Project D	Description	Plymouth Road Southbound (North of Route 14)		Units		U.S. Customary
		Se	egmen	nt 1		
Vehicle	e Inputs					
Segment	туре	Passing Zone		Length, ft		5280
Lane Wic	dth, ft	12	Sho	oulder Width, f	t	2
Speed Li	mit, mi/h	50	Acc	cess Point Dens	sity, pts/mi	5.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	72	Ор	Opposing Demand Flow Rate, veh/h		66
Peak Hour Factor		0.83		Total Trucks, %		23.00
Segment	t Capacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04
Interm	nediate Results					·
Segment Vertical Class		2		Free-Flow Speed, mi/h		52.2
Speed Slope Coefficient (m)		3.11550		Speed Power Coefficient (p)		0.64481
PF Slope Coefficient (m)		-1.15171		PF Power Coefficient (p)		0.80746
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.2
%lmprov	rement to Percent Followers	0.0	%lr	%Improvement to Speed		0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	52.2
Vehicle	e Results					<u>'</u>
Average Speed, mi/h		52.2 F		Percent Followers, %		12.9
Segment Travel Time, minutes		1.15 Fc		Follower Density (FD), followers/mi/ln		0.2
Vehicle LOS		А	A			
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	15	0.00			0.2	A

		HCS Two-Lan	e Hig	hway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY1 Plymou Road Northbound (Nort of Route 14)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	5.0
Dema	and and Capacity					·
Directio	nal Demand Flow Rate, veh/h	24	Орр	Opposing Demand Flow Rate, veh/h		60
Peak Hour Factor		0.88	Tota	Total Trucks, %		38.00
Segmen	nt Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.01
Intern	mediate Results					·
Segment Vertical Class		2	Free	Free-Flow Speed, mi/h		51.7
Speed Slope Coefficient (m)		3.26666	Spe	Speed Power Coefficient (p)		0.64228
PF Slope Coefficient (m)		-1.13810	PF F	PF Power Coefficient (p)		0.81825
In Passing Lane Effective Length?		No	Tota	Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0	%In	%Improvement to Speed		0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	dius, ft Superelevation,		Average Speed, mi/h
1 Ta	angent	5280	-	-		51.7
Vehic	le Results	<u>'</u>				·
Average Speed, mi/h		51.7	Perd	Percent Followers, %		5.2
Segment Travel Time, minutes		1.16	Foll	Follower Density (FD), followers/mi/ln		0.0
Vehicle LOS		A				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	5	0.00			0.0	A

<b>Project</b> Analyst Agency	Information						
Agency		James Vorosmarti	Dat	Date		7/12/2023	
		Tetra Tech	Ana	alysis Year		2025	
Jurisdiction	1		Tim	ne Analyzed		Afternoon Peak Hour	
Project Des	scription	Build Ph1 to LY1 Plymou Road Northbound (Nort of Route 14)		Units		U.S. Customary	
		Se	gmen	nt 1			
Vehicle	Inputs						
Segment Ty	уре	Passing Zone	Len	ngth, ft		5280	
Lane Width	ı, ft	12	Sho	oulder Width, f	t	2	
Speed Limi	it, mi/h	50	Acc	cess Point Dens	sity, pts/mi	5.0	
Demano	d and Capacity						
Directional	Demand Flow Rate, veh/h	74		Opposing Demand Flow Rate, veh/h		80	
Peak Hour Factor		0.76		Total Trucks, %		40.00	
Segment C	apacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04	
Interme	ediate Results						
Segment Vertical Class		2		Free-Flow Speed, mi/h		51.6	
Speed Slope Coefficient (m)		3.32356		Speed Power Coefficient (p)		0.62709	
PF Slope Coefficient (m)		-1.14738		PF Power Coefficient (p)		0.81627	
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.2	
%Improven	ment to Percent Followers	0.0	%lr	%Improvement to Speed		0.0	
Subsegr	ment Data						
# Segm	nent Type	Length, ft	Radius, 1	dius, ft Superelevation, 9		Average Speed, mi/h	
1 Tange	ent	5280	-		-	51.6	
Vehicle	Results					·	
Average Speed, mi/h		51.6		Percent Followers, %		12.8	
Segment Travel Time, minutes		1.16		Follower Density (FD), followers/mi/ln		0.2	
Vehicle LOS		A					
Facility	Results						
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS	
1	14	0.00			0.2	A	

		HCS Two-Lan	e Hig	hway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	e Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY1 Plymou Road Southbound (Nort of Route 14)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Length, ft			5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	5.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	70		Opposing Demand Flow Rate, veh/h		28
Peak Hour Factor		0.76		Total Trucks, %		29.00
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.04
Intern	nediate Results					·
Segment Vertical Class		2	Free	Free-Flow Speed, mi/h		52.0
Speed Slope Coefficient (m)		3.11550	Spe	Speed Power Coefficient (p)		0.67746
PF Slope Coefficient (m)		-1.12162	PF F	PF Power Coefficient (p)		0.81931
In Passing Lane Effective Length?		No	Tota	Total Segment Density, veh/mi/ln		0.2
%Impro	vement to Percent Followers	0.0	%In	%Improvement to Speed		0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	dius, ft Superelevation, 9		Average Speed, mi/h
1 Ta	ngent	5280	-	-		52.0
Vehic	le Results	<u>'</u>				·
Average Speed, mi/h		52.0	Perd	Percent Followers, %		11.9
Segment Travel Time, minutes		1.15		Follower Density (FD), followers/mi/ln		0.2
Vehicle LOS		A				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	13	0.00			0.2	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	Time Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY1 Plymou Road Southbound (Nort of Route 14)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	5.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	73 Opposing		posing Demand Flow Rate, veh/h		67
Peak Ho	our Factor	0.83	Total Trucks, %		23.00	
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.04
Intern	nediate Results					<u> </u>
Segmen	nt Vertical Class	2	Free	e-Flow Speed,	mi/h	52.2
Speed S	Slope Coefficient (m)	3.11550	Spe	Speed Power Coefficient (p)		0.64397
PF Slope	e Coefficient (m)	-1.15237	PF F	Power Coefficie	ent (p)	0.80727
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.2
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.2
Vehic	le Results					
Average	e Speed, mi/h	52.2	Perd	cent Followers,	, %	13.1
Segmen	nt Travel Time, minutes	1.15	Foll	ower Density (	(FD), followers/mi/ln	0.2
Vehicle I	LOS	A				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	15	0.00			0.2	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	Time Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY2 Plymou Road Northbound (Nort of Route 14)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	5.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	94		Opposing Demand Flow Rate, veh/h		63
Peak Ho	our Factor	0.88	Tota	al Trucks, %		38.00
Segmen	nt Capacity, veh/h	1700	Demand/Capacity (D/C)		0.06	
Intern	nediate Results					
Segmen	nt Vertical Class	2	Free	e-Flow Speed,	mi/h	51.7
Speed S	Slope Coefficient (m)	3.27006	Spe	Speed Power Coefficient (p)		0.64061
PF Slope	e Coefficient (m)	-1.13939	PF F	Power Coefficie	ent (p)	0.81785
In Passir	ng Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.3
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	51.7
Vehic	le Results					
Average	e Speed, mi/h	51.7	Perd	cent Followers	, %	15.2
Segmen	nt Travel Time, minutes			ower Density (	(FD), followers/mi/ln	0.3
Vehicle I	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	21	0.00			0.3	A

		HCS Two-Lan	e Hig	hway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	Time Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY2 Plymou Road Northbound (Nort of Route 14)		Units		U.S. Customary
		Se	gmen	t 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	Length, ft		5280
Lane Wi	idth, ft	12	Sho	ulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	5.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	nd Flow Rate, veh/h 76		oosing Deman	d Flow Rate, veh/h	162
Peak Ho	our Factor	0.76	5 Total Trucks, %			40.00
Segmen	nt Capacity, veh/h	1700	Demand/Capacity (D/C)		0.04	
Intern	nediate Results					·
Segmen	nt Vertical Class	2	Free	e-Flow Speed,	mi/h	51.6
Speed S	Slope Coefficient (m)	3.41166	Spe	Speed Power Coefficient (p)		0.58716
PF Slope	e Coefficient (m)	-1.18021	PF F	PF Power Coefficient (p)		0.80630
In Passir	ng Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.2
%Impro	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	51.6
Vehic	le Results	<u>'</u>				·
Average	e Speed, mi/h	51.6	Perd	cent Followers	, %	13.8
Segmen	Segment Travel Time, minutes 1.16		Foll	ower Density (	FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					·
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	15	0.00			0.2	A

		HCS Two-Lan	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2025
Jurisdict	ion		Tir	Time Analyzed		Morning Peak Hour
Project [	Description	Build Ph1 to LY2 Plymou Road Southbound (Nort of Route 14)		Units		U.S. Customary
		Se	gme	nt 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	Le	Length, ft		5280
Lane Wi	dth, ft	12	Sh	noulder Width, ft	t	2
Speed Li	imit, mi/h	50	Ad	ccess Point Dens	ity, pts/mi	5.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	72	Oı	Opposing Demand Flow Rate, veh/h		109
Peak Ho	our Factor	0.76	То	Total Trucks, %		29.00
Segmen	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Intern	nediate Results					•
Segmen	t Vertical Class	2	Fr	ee-Flow Speed,	mi/h	52.0
Speed S	lope Coefficient (m)	3.17421	Sp	Speed Power Coefficient (p)		0.61771
PF Slope	e Coefficient (m)	-1.16809	PF	Power Coefficie	ent (p)	0.80520
In Passin	ng Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	0.2
%Improv	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	52.0
Vehicl	le Results					·
Average	Speed, mi/h	52.0	Pe	ercent Followers,	. %	13.2
Segment Travel Time, minutes 1.15		1.15	Fo	llower Density (	FD), followers/mi/ln	0.2
Vehicle I	Vehicle LOS A					
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.2	A
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		HCS Two-Lan	ie Hi	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	Aı	nalysis Year		2025
Jurisdict	tion		Ti	Time Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY2 Plymou Road Southbound (Nort of Route 14)	uth Ui	Units		U.S. Customary
		Se	gme	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	Length, ft		5280
Lane Wi	idth, ft	12	Sł	noulder Width, ft	i	2
Speed L	imit, mi/h	50	A	ccess Point Dens	ity, pts/mi	5.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	al Demand Flow Rate, veh/h 148 Opposing Demand Flow Rate, veh/		d Flow Rate, veh/h	70	
Peak Ho	our Factor	0.83	To	Total Trucks, %		23.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.09
Interr	nediate Results					·
Segmen	nt Vertical Class	2	Fr	ee-Flow Speed,	mi/h	52.2
Speed S	Slope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.64232
PF Slope	e Coefficient (m)	-1.15367	PF	Power Coefficie	ent (p)	0.80688
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.6
%lmpro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	51.7
Vehic	le Results					·
Average	e Speed, mi/h	51.7	Pe	ercent Followers,	%	21.9
Segmen	Segment Travel Time, minutes 1.16		Fo	ollower Density (	FD), followers/mi/ln	0.6
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	31	0.01			0.6	A
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		HCS Two-Lar	ne H	lighwa	ay Re	port		
Projec	ct Information							
Analyst		James Vorosmarti	1	Date			7/12/2023	
Agency		Tetra Tech	,	Analysis Y	ear		2025	
Jurisdict	ion		-	Time Analyzed		Morning Peak Hour		
Project [	Description	No Build Plymouth Roa Northbound (North of Route 14)		Units		U.S. Customary		
		Se	egmo	ent 1				
Vehicl	le Inputs							
Segmen	nt Type	Passing Zone	l	Length, ft		5280		
Lane Wi	dth, ft	12	9	Shoulder '	Width, ft	t	2	
Speed L	imit, mi/h	50	,	Access Po	int Dens	ity, pts/mi	5.0	
Dema	and Capacity						<u> </u>	
Direction	ectional Demand Flow Rate, veh/h 24 Opposing Demand Flow		d Flow Rate, veh/h	60				
Peak Ho	our Factor	0.88	1	Total Trucks, %		38.00		
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity (D/C)		0.01		
Intern	nediate Results	•						
Segmen	t Vertical Class	2	F	Free-Flow	Speed,	mi/h	51.7	
Speed S	lope Coefficient (m)	3.26666	5	Speed Power Coefficient (p)		0.64228		
PF Slope	e Coefficient (m)	-1.13810	ı	PF Power Coefficient (p)			0.81825	
In Passir	ng Lane Effective Length?	No	-	Total Segment Density, veh/mi/ln			0.0	
%Impro	vement to Percent Followers	0.0	Ç.	%Improve	ment to	Speed	0.0	
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	ıs, ft		Superelevation, %	Average Speed, mi/h	
1 Ta	ingent	5280	-			-	51.7	
Vehicl	le Results							
Average	Speed, mi/h	51.7	F	Percent Fo	ollowers,	%	5.2	
Segmen	nt Travel Time, minutes	1.16	ı	Follower [	Density (	FD), followers/mi/ln	0.0	
Vehicle I	/ehicle LOS A							
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p		Foll		ensity, followers/ mi/ln	LOS	
1	5	0.00				0.0	A	
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		HCS Two-Lar	ne H	ligh	way Re	port		
Proje	ct Information							
Analyst		James Vorosmarti	1	Date			7	7/12/2023
Agency	,	Tetra Tech	,	Analys	sis Year		2	2025
Jurisdict	tion		-	Time A	Analyzed		A	Afternoon Peak Hour
Project	Description	No Build Plymouth Roa Northbound (North of Route 14)		Units		l	J.S. Customary	
		Se	egmo	ent	1			
Vehic	le Inputs							
Segmer	nt Type	Passing Zone	l	Length, ft		5	5280	
Lane Wi	idth, ft	12	9	Should	der Width, ft		2	2
Speed L	_imit, mi/h	50	,	Access	Point Dens	ity, pts/mi	5	5.0
Dema	and and Capacity							
Directio	tional Demand Flow Rate, veh/h 74 Opposing Demand Flow Rate		d Flow Rate, veh/h	8	B0			
Peak Ho	our Factor	0.76	-	Total Trucks, %		4	40.00	
Segmer	nt Capacity, veh/h	1700	ı	Demand/Capacity (D/C)		C	0.04	
Interr	mediate Results							
Segmer	nt Vertical Class	2	F	Free-F	low Speed, i	mi/h	5	51.6
Speed S	Slope Coefficient (m)	3.32356	5	Speed Power Coefficient (p)		C	0.62709	
PF Slop	e Coefficient (m)	-1.14738	ı	PF Power Coefficient (p)			C	0.81627
In Passii	ng Lane Effective Length?	No	-	Total Segment Density, veh/mi/ln			C	0.2
%lmpro	ovement to Percent Followers	0.0	Ç.	%Improvement to Speed		C	0.0	
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	us, ft		Superelevation, %	1	Average Speed, mi/h
1 Ta	angent	5280	-			-	į	51.6
Vehic	le Results							
Average	e Speed, mi/h	51.6	ı	Percer	nt Followers,	%	1	12.8
Segment Travel Time, minutes 1.16		ı	Follow	er Density (	FD), followers/mi/ln	C	0.2	
Vehicle	LOS	А						
Facilit	ty Results		·					
т	VMT veh-mi/AP	VHD veh-h/p		1		ensity, followers/ mi/ln		LOS
1	14	0.00				0.2		A
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		HCS Two-Lar	ne H	lighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	[	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	tion		7	Time Analyzed		Morning Peak Hour
Project	Description	No Build Plymouth Roa Southbound (North of Route 14)	ad l	Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	I	Length, ft		5280
Lane Wi	idth, ft	12	9	Shoulder Width, f	t	2
Speed L	imit, mi/h	50	A	Access Point Dens	sity, pts/mi	5.0
Dema	and Capacity					<u> </u>
Directio	rectional Demand Flow Rate, veh/h 70 Opposing Demand		d Flow Rate, veh/h	28		
Peak Ho	our Factor	0.76	7	Total Trucks, %		29.00
Segmen	nt Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.04
Interr	mediate Results	•				
Segmen	nt Vertical Class	2	F	ree-Flow Speed,	mi/h	52.0
Speed S	Slope Coefficient (m)	3.11550	9	Speed Power Coefficient (p)		0.67746
PF Slope	e Coefficient (m)	-1.12162	F	PF Power Coeffici	ent (p)	0.81931
In Passir	ng Lane Effective Length?	No	1	Total Segment De	0.2	
%lmpro	vement to Percent Followers	0.0	9	%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.0
Vehic	le Results					
Average	e Speed, mi/h	52.0	F	Percent Followers	, %	11.9
Segment Travel Time, minutes 1.15		1.15	F	Follower Density	(FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	13	0.00			0.2	A
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		HCS Two-Lar	ne H	lighway Re	eport	
Proje	ct Information					
Analyst		James Vorosmarti	Г	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	tion		7	Time Analyzed		Afternoon Peak Hour
Project	Description	No Build Plymouth Roa Southbound (North of Route 14)		Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	idth, ft	12	9	Shoulder Width, f	ft	2
Speed L	_imit, mi/h	50	A	Access Point Den	sity, pts/mi	5.0
Dema	and and Capacity					<u> </u>
Directio	onal Demand Flow Rate, veh/h	Demand Flow Rate, veh/h 73 Opposing Demand Flow Rate, veh/h		67		
Peak Ho	our Factor	0.83	7	Total Trucks, %		23.00
Segmen	nt Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.04
Interr	mediate Results					
Segmen	nt Vertical Class	2	F	Free-Flow Speed,	mi/h	52.2
Speed S	Slope Coefficient (m)	3.11550	9	Speed Power Coefficient (p)		0.64397
PF Slope	e Coefficient (m)	-1.15237	F	PF Power Coeffici	ent (p)	0.80727
In Passii	ng Lane Effective Length?	No	1	Total Segment De	ensity, veh/mi/ln	0.2
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	o Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.2
Vehic	le Results					
Average	e Speed, mi/h	52.2	F	Percent Followers	5, %	13.1
Segmen	Segment Travel Time, minutes 1.15		F	Follower Density	(FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	15	0.00			0.2	A
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		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	9		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdictio	on		Time	Time Analyzed		Morning Peak Hour
Project D	Description	PH2 to LY2 Plymouth Ro Northbound (North of Route 14)	oad Unit	Units		U.S. Customary
		Seg	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Leng	Length, ft		5280
Lane Wid	lth, ft	12	Sho	ulder Width, f	t	2
Speed Lir	mit, mi/h	50	Acce	ess Point Dens	sity, pts/mi	5.0
Demar	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	90	Орр	Opposing Demand Flow Rate, veh/h		64
Peak Hou	ur Factor	0.88	Tota	I Trucks, %		38.00
Segment	: Capacity, veh/h	1700	Demand/Capacity (D/C)		/ (D/C)	0.05
Interm	nediate Results					
Segment	: Vertical Class	2	Free	-Flow Speed,	mi/h	51.7
Speed Slo	ope Coefficient (m)	3.27174	Spe	Speed Power Coefficient (p)		0.63978
PF Slope	Coefficient (m)	-1.14003	PF P	PF Power Coefficient (p)		0.81765
In Passing	g Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.3
%Improv	rement to Percent Followers	0.0	%lm	provement to	Speed	0.0
Subse	gment Data		·			
# Sec	gment Type	Length, ft	Radius, ft	<u> </u>	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	51.7
Vehicle	e Results					
Average :	Speed, mi/h	51.7	Perc	ent Followers	, %	14.7
Segment	Segment Travel Time, minutes 1.16		Follo	ower Density (	(FD), followers/mi/ln	0.3
Vehicle LO	OS	А				
Facility	y Results					
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	9		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Time	Time Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Plymouth Ro Northbound (North of Route 14)	ad Unit	Units		U.S. Customary
		Seg	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Leng	Length, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	2
Speed Li	mit, mi/h	50	Acce	ess Point Dens	sity, pts/mi	5.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	78	Орр	osing Deman	d Flow Rate, veh/h	157
Peak Hou	ur Factor	0.76	Tota	I Trucks, %		40.00
Segment	t Capacity, veh/h	1700	Demand/Capacity (D/C)		0.05	
Interm	nediate Results					
Segment	t Vertical Class	2	Free	-Flow Speed,	mi/h	51.6
Speed SI	ope Coefficient (m)	3.40678	Spe	Speed Power Coefficient (p)		0.58926
PF Slope	Coefficient (m)	-1.17841	PF P	PF Power Coefficient (p)		0.80684
In Passin	g Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.2
%lmprov	vement to Percent Followers	0.0	%lm	provement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, ft	<u> </u>	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	51.6
Vehicle	e Results					
Average	Speed, mi/h	51.6	Perc	ent Followers,	, %	13.9
Segment Travel Time, minutes 1.16		Follo	ower Density (	FD), followers/mi/ln	0.2	
Vehicle L	.OS	A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	15	0.00			0.2	A

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	9		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdictio	on		Time	Time Analyzed		Morning Peak Hour
Project D	escription	PH2 to LY2 Plymouth Ro Southbound (North of Route 14)	oad Unit	Units		U.S. Customary
		Seg	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Leng	Length, ft		5280
Lane Wid	lth, ft	12	Sho	ulder Width, f	t	2
Speed Lir	mit, mi/h	50	Acce	ess Point Dens	sity, pts/mi	5.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	74	Орр	osing Deman	d Flow Rate, veh/h	104
Peak Hou	ur Factor	0.76	Tota	l Trucks, %		29.00
Segment	Capacity, veh/h	1700	Demand/Capacity (D/C)		0.04	
Interm	nediate Results					·
Segment	Vertical Class	2	Free	-Flow Speed,	mi/h	52.0
Speed Slo	ope Coefficient (m)	3.16824	Spe	Speed Power Coefficient (p)		0.62046
PF Slope	Coefficient (m)	-1.16585	PF P	PF Power Coefficient (p)		0.80587
In Passino	g Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.2
%Improv	ement to Percent Followers	0.0	%lm	provement to	Speed	0.0
Subse	gment Data					<u> </u>
# Seg	gment Type	Length, ft	Radius, ft	:	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	52.0
Vehicle	e Results					
Average S	Speed, mi/h	52.0	Perc	ent Followers,	, %	13.3
Segment	Segment Travel Time, minutes 1.15		Follo	ower Density (	FD), followers/mi/ln	0.2
Vehicle LO	OS	A				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l , , , , , , , , , , , , , , , , , , ,		ensity, followers/ mi/ln	LOS
		<u> </u>				

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		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	<u> </u>		7/12/2023
Agency		Tetra Tech	Ana	ysis Year		2026
Jurisdiction	on		Time	e Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Plymouth Ro Southbound (North of Route 14)	oad Unit	Units		U.S. Customary
		Seg	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Leng	gth, ft		5280
Lane Wid	Ith, ft	12	Sho	ulder Width, f	t	2
Speed Lir	mit, mi/h	50	Acce	ess Point Dens	sity, pts/mi	5.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	143	Орр	osing Deman	d Flow Rate, veh/h	71
Peak Hou	ur Factor	0.83	Total <sup>-</sup>			23.00
Segment	: Capacity, veh/h	1700	Dem	Demand/Capacity (D/C)		0.08
Interm	nediate Results					
Segment	: Vertical Class	2	Free	-Flow Speed,	mi/h	52.2
Speed Slo	ope Coefficient (m)	3.11550	Spe	Speed Power Coefficient (p)		0.64151
PF Slope	Coefficient (m)	-1.15431	PF P	PF Power Coefficient (p)		0.80668
In Passing	g Lane Effective Length?	No	Tota	l Segment De	nsity, veh/mi/ln	0.6
%Improv	rement to Percent Followers	0.0	%lm	provement to	Speed	0.0
Subse	gment Data		·			<u> </u>
# Sec	gment Type	Length, ft	Radius, ft		Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	51.8
Vehicle	e Results					
Average :	Speed, mi/h	51.8	Perc	ent Followers,	, %	21.4
Segment	Travel Time, minutes	1.16	Follo	ower Density (	(FD), followers/mi/ln	0.6
Vehicle LO	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	l		ensity, followers/ mi/ln	LOS

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		HCS Two-Lan	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2026
Jurisdict	tion		Tir	me Analyzed		Morning Peak Hour
Project [	Description	No Build - Plymouth Ro Northbound (North of Route 14)	ad Ui	Units		U.S. Customary
		Se	gme	nt 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	idth, ft	12	Sh	noulder Width, ft	t	2
Speed L	imit, mi/h	50	Ad	ccess Point Dens	ity, pts/mi	5.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	25	O	Opposing Demand Flow Rate, veh/h		61
Peak Ho	our Factor	0.88	To	otal Trucks, %		38.00
Segmen	nt Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.01
Intern	nediate Results					
Segmen	nt Vertical Class	2	Fr	ee-Flow Speed,	mi/h	51.7
Speed S	Slope Coefficient (m)	3.26837	Sp	Speed Power Coefficient (p)		0.64144
PF Slope	e Coefficient (m)	-1.13875	PF	PF Power Coefficient (p)		0.81804
In Passir	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.0
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	51.7
Vehicl	le Results					
Average	e Speed, mi/h	51.7	Pe	ercent Followers,	%	5.4
Segmen	nt Travel Time, minutes	1.16	Fc	ollower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	6	0.00			0.0	A
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		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	Aı	nalysis Year		2026
Jurisdict	tion		Ti	me Analyzed		Afternoon Peak Hour
Project I	Description	No Build - Plymouth Ro Northbound (North of Route 14)	oad U	nits		U.S. Customary
		Se	gme	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	idth, ft	12	Sł	noulder Width, ft	t	2
Speed L	_imit, mi/h	50	A	ccess Point Dens	ity, pts/mi	5.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	75	О	Opposing Demand Flow Rate, veh/h		82
Peak Ho	our Factor	0.76	To	Total Trucks, %		40.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.04
Intern	mediate Results					•
Segmen	nt Vertical Class	2	Fr	ee-Flow Speed,	mi/h	51.6
Speed S	Slope Coefficient (m)	3.32527	Sp	Speed Power Coefficient (p)		0.62627
PF Slope	e Coefficient (m)	-1.14802	PF	F Power Coefficie	ent (p)	0.81607
In Passir	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.2
%Impro	ovement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	51.6
Vehic	le Results					·
Average	e Speed, mi/h	51.6	Pe	ercent Followers,	. %	12.9
Segmen	nt Travel Time, minutes	1.16	Fo	ollower Density (	FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.2	A
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		HCS Two-Lan	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2026
Jurisdict	ion		Tir	me Analyzed		Morning Peak Hour
Project [	Description	No Build - Plymouth Ro Southbound (North of Route 14)	ad Ur	nits		U.S. Customary
		Se	gmei	nt 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	dth, ft	12	Sh	noulder Width, ft	t	2
Speed Li	imit, mi/h	50	Ac	ccess Point Dens	ity, pts/mi	5.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	71	Or	Opposing Demand Flow Rate, veh/h		29
Peak Ho	our Factor	0.76	То	otal Trucks, %		29.00
Segmen	nt Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Intern	nediate Results					
Segmen	nt Vertical Class	2	Fre	ee-Flow Speed,	mi/h	52.0
Speed S	lope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.67595
PF Slope	e Coefficient (m)	-1.12275	PF	PF Power Coefficient (p)		0.81897
In Passin	ng Lane Effective Length?	No	То	Total Segment Density, veh/mi/ln		0.2
%Improv	vement to Percent Followers	0.0	%I	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	52.0
Vehicl	le Results					
Average	Speed, mi/h	52.0	Pe	ercent Followers,	%	12.1
Segmen	t Travel Time, minutes	1.15	Fo	ollower Density (	FD), followers/mi/ln	0.2
Vehicle L	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.2	A
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		HCS Two-Lan	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2026
Jurisdict	tion		Tir	me Analyzed		Afternoon Peak Hour
Project I	Description	No Build - Plymouth Ro Southbound (North of Route 14)	ad Ur	nits		U.S. Customary
		Se	gme	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	idth, ft	12	Sh	noulder Width, ft	t	2
Speed L	imit, mi/h	50	Ad	ccess Point Dens	ity, pts/mi	5.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	75	Oı	Opposing Demand Flow Rate, veh/h		69
Peak Ho	our Factor	0.83	То	Total Trucks, %		23.00
Segmen	nt Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Intern	nediate Results					•
Segmen	nt Vertical Class	2	Fr	ee-Flow Speed,	mi/h	52.2
Speed S	Slope Coefficient (m)	3.11550	Sp	Speed Power Coefficient (p)		0.64314
PF Slope	e Coefficient (m)	-1.15302	PF	Power Coefficie	ent (p)	0.80707
In Passir	ng Lane Effective Length?	No	То	Total Segment Density, veh/mi/ln		0.2
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.2
Vehic	le Results					·
Average	e Speed, mi/h	52.2	Pe	ercent Followers,	%	13.2
Segmen	nt Travel Time, minutes	1.15	Fo	llower Density (	FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	16	0.00			0.2	A
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		HCS Two-Lai	ne H	lighway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Ī	Date		7/12/2023
Agency		Tetra Tech	,	Analysis Year		2023
Jurisdicti	ion		-	Time Analyzed		Morning Peak Hour
Project D	Description	Route 221 Northbound (South of Sellards Road		Units		U.S. Customary
		Se	egm	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone	I	Length, ft		5280
Lane Wid	dth, ft	11	9	Shoulder Width, ft	i	4
Speed Li	mit, mi/h	65	,	Access Point Dens	ity, pts/mi	1.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	34	(	Opposing Demand	d Flow Rate, veh/h	191
Peak Ho	ur Factor	0.94	0.94			41.00
Segment	t Capacity, veh/h	1700	ı	Demand/Capacity	(D/C)	0.02
Intern	nediate Results					·
Segment	t Vertical Class	1	ı	Free-Flow Speed,	mi/h	70.5
Speed SI	lope Coefficient (m)	4.12241		Speed Power Coef	fficient (p)	0.54167
PF Slope	Coefficient (m)	-1.14311	1	PF Power Coefficie	ent (p)	0.84594
In Passin	g Lane Effective Length?	No	-	Total Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	(	%Improvement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 Taı	ngent	5280	-		-	70.5
Vehicl	e Results					
Average	Speed, mi/h	70.5	I	Percent Followers,	%	6.3
	t Travel Time, minutes	0.85	1	Follower Density (	FD), followers/mi/ln	0.0
Vehicle L	Vehicle LOS A					
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	8	0.00			0.0	A
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		HCS Two-La	ne F	Highway Re	port	
Projec	t Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech		Analysis Year		2023
Jurisdicti	ion			Time Analyzed		Afternoon Peak Hour
Project D	Description	Route 221 Northbound (South of Sellards Road	-	Units		U.S. Customary
		Se	egm	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wid	dth, ft	11		Shoulder Width, f	t	4
Speed Li	mit, mi/h	65		Access Point Dens	sity, pts/mi	1.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	213	213		d Flow Rate, veh/h	89
Peak Ho	ur Factor	0.94		Total Trucks, %		26.00
Segment	t Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.13
Intern	nediate Results	·				
Segment	t Vertical Class	1		Free-Flow Speed,	mi/h	71.0
Speed SI	lope Coefficient (m)	4.10403		Speed Power Coe	fficient (p)	0.57916
PF Slope	Coefficient (m)	-1.11305		PF Power Coefficie	ent (p)	0.85626
In Passin	g Lane Effective Length?	No		Total Segment De	ensity, veh/mi/ln	0.8
%lmprov	vement to Percent Followers	0.0		%Improvement to Speed		0.0
Subse	gment Data	•				
# Se	gment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	69.8
Vehicl	e Results					
Average	Speed, mi/h	69.8		Percent Followers	, %	25.6
Segment	t Travel Time, minutes	0.86		Follower Density (	(FD), followers/mi/ln	0.8
Vehicle L	.OS	А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	50	0.01			0.8	A
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		HCS Two-Lai	ne F	Highway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech		Analysis Year		2023
Jurisdic	tion			Time Analyzed		Morning Peak Hour
Project	Description	Route 221 Southbound (South of Sellards Road		Units		U.S. Customary
		Se	egm	ent 1		·
Vehic	le Inputs					
Segmer	nt Type	Passing Zone		Length, ft		5280
Lane W	idth, ft	11		Shoulder Width, f	t	4
Speed L	Limit, mi/h	65		Access Point Dens	sity, pts/mi	1.0
Dema	and and Capacity					
Direction	onal Demand Flow Rate, veh/h 191		Opposing Deman	d Flow Rate, veh/h	34	
Peak Ho	our Factor	0.94		Total Trucks, %		8.00
Segmer	nt Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.11
Interi	mediate Results					
Segmer	nt Vertical Class	1		Free-Flow Speed,	mi/h	71.6
Speed S	Slope Coefficient (m)	4.09901		Speed Power Coe	fficient (p)	0.61359
PF Slop	e Coefficient (m)	-1.08409		PF Power Coefficie	ent (p)	0.86540
In Passi	ng Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.6
%lmprc	ovement to Percent Followers	0.0		%Improvement to	Speed	0.0
Subse	egment Data					
# S	egment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	70.6
Vehic	le Results		<u> </u>			
Average	e Speed, mi/h	70.6		Percent Followers,	, %	22.8
Segmen	nt Travel Time, minutes	0.85		Follower Density (	(FD), followers/mi/ln	0.6
Vehicle LOS A						
Facili	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	45	0.01			0.6	A
onvright	© 2023 University of Florida. All Rights	Reserved HCS TIM H	lighway	ys Version 2022		Generated: 07/12/2023 13:06::

		HCS Two-La	ne F	Highway Re	port	
Projec	t Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech		Analysis Year		2023
Jurisdicti	ion			Time Analyzed		Afternoon Peak Hour
Project D	Description	Route 221 Southbound (South of Sellards Roa		Units		U.S. Customary
		Se	egm	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wid	dth, ft	11		Shoulder Width, f	t	4
Speed Li	mit, mi/h	65		Access Point Dens	sity, pts/mi	1.0
Dema	nd and Capacity					
Directional Demand Flow Rate, veh/h		89		Opposing Demand Flow Rate, veh/h		213
Peak Hour Factor		0.94		Total Trucks, %		39.00
Segment	t Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.05
Intern	nediate Results					·
Segment	t Vertical Class	1		Free-Flow Speed,	mi/h	70.6
Speed SI	ope Coefficient (m)	4.13378		Speed Power Coe	fficient (p)	0.53574
PF Slope	Coefficient (m)	-1.14716		PF Power Coefficient (p)		0.84398
In Passin	g Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.2
%lmprov	vement to Percent Followers	0.0		%Improvement to Speed		0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	70.6
Vehicl	e Results					
Average	Speed, mi/h	70.6		Percent Followers	, %	13.9
Segment	t Travel Time, minutes	0.85		Follower Density (	(FD), followers/mi/ln	0.2
Vehicle LOS		А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	21	0.00			0.2	А
`opyriaht @	2023 University of Florida All Rights	Decembed LICCTON I	Liabura	rs Version 2022		Generated: 07/12/2023 13:08:

		HCS Two-Lan	ne Hi	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2025
Jurisdict	tion		Ti	ime Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY1 Route 221 Northbound (South Sellards Road)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	idth, ft	11	SI	houlder Width, ft	t	4
Speed L	imit, mi/h	65	А	ccess Point Dens	ity, pts/mi	1.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	35	0	pposing Demand	d Flow Rate, veh/h	196
Peak Ho	our Factor	0.94	To	Total Trucks, %		41.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.02
Intern	nediate Results					·
Segmen	nt Vertical Class	1	Fı	ree-Flow Speed,	mi/h	70.5
Speed S	Slope Coefficient (m)	4.12400		Speed Power Coefficient (p)		0.54045
PF Slope	e Coefficient (m)	-1.14400	P	PF Power Coefficient (p)		0.84556
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	70.5
Vehic	le Results					
Average	e Speed, mi/h	70.5	Pe	ercent Followers,	%	6.5
Segmen	nt Travel Time, minutes	0.85	Fo	ollower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	8	0.00			0.0	A
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		HCS Two-Lar	ne Hi	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2025
Jurisdict	ion		Т	ime Analyzed		Afternoon Peak Hour
Project [	Description	Build Ph1 to LY1 Route 221 Northbound (South Sellards Road)		Units		U.S. Customary
		Se	gme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	houlder Width, ft	t	4
Speed Li	imit, mi/h	65	А	ccess Point Dens	ity, pts/mi	1.0
Dema	nd and Capacity					·
Direction	nal Demand Flow Rate, veh/h	217	С	pposing Demand	d Flow Rate, veh/h	91
Peak Ho	our Factor	0.94	To	Total Trucks, %		26.00
Segmen	t Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.13
Intern	nediate Results					·
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	71.0
Speed S	lope Coefficient (m)	4.10519	S	peed Power Coef	fficient (p)	0.57815
PF Slope	e Coefficient (m)	-1.11383	Р	PF Power Coefficient (p)		0.85596
In Passin	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.8
%Improv	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	69.8
Vehicl	le Results					
Average	Speed, mi/h	69.8	P	ercent Followers,	%	26.0
Segmen	t Travel Time, minutes	0.86	F	ollower Density (	FD), followers/mi/ln	0.8
Vehicle I	LOS	А				
Facilit	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	51	0.01			0.8	A
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		HCS Two-Lan	ne Hi	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2025
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour
Project [	Description	Build Ph1 to LY1 Route 221 Southbound (South Sellards Road)		Units		U.S. Customary
		Se	gme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	houlder Width, ft	t	4
Speed L	imit, mi/h	65	А	ccess Point Dens	sity, pts/mi	1.0
Dema	and Capacity					<u> </u>
Direction	nal Demand Flow Rate, veh/h	196	С	Opposing Demand	d Flow Rate, veh/h	35
Peak Ho	our Factor	0.94	To	Total Trucks, %		8.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.12
Intern	nediate Results					
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	71.6
Speed S	lope Coefficient (m)	4.09995	S	peed Power Coe	fficient (p)	0.61269
PF Slope	e Coefficient (m)	-1.08480	Р	F Power Coefficie	ent (p)	0.86513
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.6
%Impro	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	70.6
Vehicl	le Results					
Average	Speed, mi/h	70.6	P	ercent Followers,	, %	23.2
Segmen	t Travel Time, minutes	0.85	F	ollower Density (	FD), followers/mi/ln	0.6
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	46	0.01			0.6	A
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		HCS Two-Lan	ne Hi	ighway Re	port			
Proje	ct Information							
Analyst		James Vorosmarti	D	Date		7/12/2023		
Agency		Tetra Tech	А	nalysis Year		2025		
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour		
Project I	Description	Build Ph1 to LY1 Route 221 Southbound (South Sellards Road)		Units		U.S. Customary		
Segment 1								
Vehicle Inputs								
Segmen	nt Type	Passing Zone		ength, ft		5280		
Lane Wi	idth, ft	11	S	houlder Width, ft	t	4		
Speed L	imit, mi/h	65	А	ccess Point Dens	ity, pts/mi	1.0		
Dema	and Capacity					<u> </u>		
Directio	nal Demand Flow Rate, veh/h	91		Opposing Demand Flow Rate, veh/h		217		
Peak Ho	our Factor	0.94		Total Trucks, %		39.00		
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.05		
Intern	nediate Results					·		
Segment Vertical Class		1		Free-Flow Speed, mi/h		70.6		
Speed Slope Coefficient (m)		4.13529		Speed Power Coefficient (p)		0.53460		
PF Slope Coefficient (m)		-1.14798		PF Power Coefficient (p)		0.84363		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.2		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	angent	5280	-		-	70.6		
Vehic	le Results							
Average Speed, mi/h		70.6		Percent Followers, %		14.2		
Segment Travel Time, minutes		0.85		Follower Density (FD), followers/mi/ln		0.2		
Vehicle LOS		A						
Facilit	ty Results							
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	22	0.00			0.2	A		
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		HCS Two-Lan	ne Hi	ighway Re	port			
Projec	ct Information							
Analyst		James Vorosmarti	D	Date		7/12/2023		
Agency		Tetra Tech	А	nalysis Year		2025		
Jurisdict	ion		Ti	ime Analyzed		Morning Peak Hour		
Project [	Description	Build Ph1 to LY2 Route 221 Northbound (South Sellards Road)		Units		U.S. Customary		
Segment 1								
Vehicle Inputs								
Segmen	nt Type	Passing Zone		ength, ft		5280		
Lane Wi	dth, ft	11	SI	houlder Width, ft	t	4		
Speed Li	imit, mi/h	65	А	ccess Point Dens	ity, pts/mi	1.0		
Dema	and Capacity					·		
Direction	nal Demand Flow Rate, veh/h	35		Opposing Demand Flow Rate, veh/h		196		
Peak Ho	our Factor	0.94		Total Trucks, %		41.00		
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.02		
Intern	nediate Results					·		
Segment Vertical Class		1		Free-Flow Speed, mi/h		70.5		
Speed Slope Coefficient (m)		4.12400		Speed Power Coefficient (p)		0.54045		
PF Slope Coefficient (m)		-1.14400		PF Power Coefficient (p)		0.84556		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ingent	5280	-		-	70.5		
Vehicl	le Results							
Average Speed, mi/h		70.5		Percent Followers, %		6.5		
Segment Travel Time, minutes		0.85		Follower Density (FD), followers/mi/ln		0.0		
Vehicle LOS		А						
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS		
1	8	0.00			0.0	A		
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		HCS Two-Lan	e Hig	hway Re	port				
Projec	t Information								
Analyst		James Vorosmarti	Date	e		7/12/2023			
Agency		Tetra Tech	Ana	lysis Year		2025			
Jurisdicti	ion		Tim	e Analyzed		Afternoon Peak Hour			
Project D	Description	Build Ph1 to LY2 Route 221 Northbound (South Sellards Road)		Units		U.S. Customary			
	Segment 1								
Vehicle	e Inputs								
Segment	t Type	Passing Zone	Len	Length, ft		5280			
Lane Wic	dth, ft	11	Sho	ulder Width, f	t	4			
Speed Li	mit, mi/h	65	Acc	ess Point Dens	sity, pts/mi	1.0			
Demai	nd and Capacity					-			
Direction	nal Demand Flow Rate, veh/h	217		Opposing Demand Flow Rate, veh/h		91			
Peak Hour Factor		0.94		Total Trucks, %		26.00			
Segment	t Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.13			
Interm	nediate Results								
Segment Vertical Class		1	Free	Free-Flow Speed, mi/h		71.0			
Speed Slope Coefficient (m)		4.10519	Spe	Speed Power Coefficient (p)		0.57815			
PF Slope Coefficient (m)		-1.11383	PF F	PF Power Coefficient (p)		0.85596			
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.8			
%Improv	vement to Percent Followers	0.0	%In	%Improvement to Speed		0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h			
1 Tar	ngent	5280	-		-	69.8			
Vehicle	e Results								
Average Speed, mi/h		69.8		Percent Followers, %		26.0			
Segment Travel Time, minutes		0.86 F		Follower Density (FD), followers/mi/ln		0.8			
Vehicle LOS		A							
Facility	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/In		LOS			
1	51	0.01			0.8	A			

		HCS Two-Lan	ne Hi	ighway Re	port			
Projec	ct Information							
Analyst		James Vorosmarti	D	Date		7/12/2023		
Agency		Tetra Tech	А	nalysis Year		2025		
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour		
Project [	Description	Build Ph1 to LY2 Route 221 Southbound (South Sellards Road)		Units		U.S. Customary		
Segment 1								
Vehicle Inputs								
Segmen	nt Type	Passing Zone		ength, ft		5280		
Lane Wi	dth, ft	11	S	houlder Width, ft	t	4		
Speed L	imit, mi/h	65	А	ccess Point Dens	ity, pts/mi	1.0		
Dema	and Capacity					·		
Direction	nal Demand Flow Rate, veh/h	196		Opposing Demand Flow Rate, veh/h		35		
Peak Ho	our Factor	0.94		Total Trucks, %		8.00		
Segmen	nt Capacity, veh/h	1700		Demand/Capacity (D/C)		0.12		
Intern	nediate Results					·		
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		71.6		
Speed Slope Coefficient (m)		4.09995		Speed Power Coefficient (p)		0.61269		
PF Slope Coefficient (m)		-1.08480		PF Power Coefficient (p)		0.86513		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.6		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ingent	5280	-		-	70.6		
Vehicl	le Results							
Average Speed, mi/h		70.6		Percent Followers, %		23.2		
Segment Travel Time, minutes		0.85		Follower Density (FD), followers/mi/ln		0.6		
Vehicle LOS		А						
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	46	0.01			0.6	A		
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		HCS Two-Lan	ne Hi	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project I	Description	Build Ph1 to LY2 Route 221 Southbound (South Sellards Road)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	11	S	houlder Width, ft	t	4
Speed L	imit, mi/h	65	А	ccess Point Dens	sity, pts/mi	1.0
Dema	and Capacity					<u> </u>
Directio	nal Demand Flow Rate, veh/h	emand Flow Rate, veh/h 91		Opposing Demand Flow Rate, veh/h		217
Peak Ho	our Factor	0.94	Total Trucks, %			39.00
Segmen	nt Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.05
Intern	nediate Results					·
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	70.6
Speed S	Slope Coefficient (m)	4.13529	S	peed Power Coe	fficient (p)	0.53460
PF Slope	e Coefficient (m)	-1.14798	Р	F Power Coefficie	ent (p)	0.84363
In Passir	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.2
%Impro	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	70.6
Vehic	le Results					
Average	e Speed, mi/h	70.6	P	ercent Followers,	, %	14.2
Segmen	nt Travel Time, minutes	0.85	F	ollower Density (	FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	22	0.00			0.2	A
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		HCS Two-La	ne Hi	ighway Re	port	
Project	t Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	А	Analysis Year		2025
Jurisdictio	on		Ti	ime Analyzed		Morning Peak Hour
Project D	escription	No Build Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		Se	egme	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ength, ft		5280
Lane Wid	th, ft	11	SI	houlder Width, f	t	4
Speed Lin	nit, mi/h	65	А	ccess Point Dens	sity, pts/mi	1.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	35	0	Opposing Demand Flow Rate, veh/h		196
Peak Hou	ır Factor	0.94	To	otal Trucks, %		41.00
Segment	Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.02
Interm	ediate Results					·
Segment	Vertical Class	1	Fr	ree-Flow Speed,	mi/h	70.5
Speed Slo	ope Coefficient (m)	4.12400	Sı	Speed Power Coefficient (p)		0.54045
PF Slope	Coefficient (m)	-1.14400	PI	PF Power Coefficient (p)		0.84556
In Passing	g Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.0
%Improve	ement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subseg	gment Data					
# Seg	gment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Tan	gent	5280	-		-	70.5
Vehicle	Results					
Average S	Speed, mi/h	70.5	Pe	ercent Followers	, %	6.5
Segment	Travel Time, minutes	0.85	Fo	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle LO	OS	А				
Facility	/ Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
	8	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	Analysis Year		2025
Jurisdict	tion		Tir	me Analyzed		Afternoon Peak Hour
Project	Description	No Build Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		Se	egme	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ngth, ft		5280
Lane Wi	idth, ft	11	Sh	oulder Width, f	t	4
Speed L	imit, mi/h	65	Ac	ccess Point Dens	sity, pts/mi	1.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	217		Opposing Demand Flow Rate, veh/h		91
Peak Ho	our Factor	0.94	То	tal Trucks, %		26.00
Segmen	nt Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.13
Interr	nediate Results					
Segmen	nt Vertical Class	1	Fre	ee-Flow Speed,	mi/h	71.0
Speed S	Slope Coefficient (m)	4.10519	Sp	Speed Power Coefficient (p)		0.57815
PF Slope	e Coefficient (m)	-1.11383	PF	PF Power Coefficient (p)		0.85596
In Passir	ng Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	0.8
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	69.8
Vehic	le Results					·
Average	e Speed, mi/h	69.8	Pe	rcent Followers	, %	26.0
Segmen	Segment Travel Time, minutes 0.86		Fo	llower Density (	(FD), followers/mi/ln	0.8
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	51	0.01			0.8	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	Analysis Year		2025
Jurisdicti	ion		Tir	me Analyzed		Morning Peak Hour
Project D	Description	No Build Route 221 Southbound (South of Sellards Road)		Units		U.S. Customary
		Se	egmei	nt 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone	Le	ngth, ft		5280
Lane Wid	dth, ft	11	Sh	oulder Width, f	t	4
Speed Li	imit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	196		Opposing Demand Flow Rate, veh/h		35
Peak Ho	ur Factor	0.94	To	tal Trucks, %		8.00
Segment	t Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.12
Intern	nediate Results					
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	71.6
Speed SI	lope Coefficient (m)	4.09995	Sp	Speed Power Coefficient (p)		0.61269
PF Slope	e Coefficient (m)	-1.08480	PF	PF Power Coefficient (p)		0.86513
In Passin	ng Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.6
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	70.6
Vehicl	e Results					
Average	Speed, mi/h	70.6	Pe	rcent Followers,	, %	23.2
Segment	t Travel Time, minutes			llower Density (	(FD), followers/mi/ln	0.6
Vehicle L	_OS	А				
Facilit	y Results		•			
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	46	0.01			0.6	A
Т	VMT veh-mi/AP	veh-h/p			mi/ln	

		HCS Two-La	ne H	ligl	nway Re	port		
Proje	ct Information							
Analyst		James Vorosmarti	- I	Date				7/12/2023
Agency		Tetra Tech	,	Analy	ysis Year			2025
Jurisdict	tion		-	Time	Analyzed			Afternoon Peak Hour
Project I	Description	No Build Route 221 Southbound (South of Sellards Road)		Units			U.S. Customary	
		Se	egm	ent	: 1			
Vehic	le Inputs							
Segmen	nt Type	Passing Zone	Ti	Leng	th, ft		П	5280
Lane Wi	idth, ft	11		Shou	lder Width, ft			4
Speed L	imit, mi/h	65	,	Acce	ss Point Dens	ity, pts/mi		1.0
Dema	and Capacity							
Directio	nal Demand Flow Rate, veh/h	Demand Flow Rate, veh/h 91		Opposing Demand Flow Rate, veh/h		П	217	
Peak Ho	our Factor	0.94	Total Trucks, %			$\neg$	39.00	
Segmen	nt Capacity, veh/h	1700	ı	Demand/Capacity (D/C)			0.05	
Intern	nediate Results							
Segmen	nt Vertical Class	1	Ti	Free-	Flow Speed,	mi/h	П	70.6
Speed S	Slope Coefficient (m)	4.13529		Speed Power Coefficient (p)			0.53460	
PF Slope	e Coefficient (m)	-1.14798	1	PF Pc	ower Coefficie	ent (p)		0.84363
In Passir	ng Lane Effective Length?	No	-	Total	Segment De	nsity, veh/mi/ln		0.2
%Impro	vement to Percent Followers	0.0	(	%lm <sub>l</sub>	orovement to	Speed		0.0
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	us, ft		Superelevation, %		Average Speed, mi/h
1 Ta	angent	5280	-			-		70.6
Vehic	le Results							
Average	e Speed, mi/h	70.6	1	Perce	ent Followers,	%		14.2
Segmen	nt Travel Time, minutes	0.85	ı	Follo	wer Density (	FD), followers/mi/ln		0.2
Vehicle	LOS	А						
Facilit	ty Results							
т	VMT veh-mi/AP	VHD veh-h/p				ensity, followers/ mi/ln		LOS
1	22	0.00				0.2		A
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		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2026
Jurisdicti	on		Tin	ne Analyzed		Morning Peak Hour
Project D	Description	PH2 to LY2 Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Ler	ngth, ft		5280
Lane Wic	dth, ft	11	Sh	oulder Width, f	t	4
Speed Li	mit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	35		Opposing Demand Flow Rate, veh/h		197
Peak Hou	ur Factor	0.94	Tot	tal Trucks, %		41.00
Segment	t Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.02
Interm	nediate Results	•				<u>.</u>
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	70.5
Speed SI	ope Coefficient (m)	4.12439	Sp	Speed Power Coefficient (p)		0.54015
PF Slope	Coefficient (m)	-1.14422	PF	PF Power Coefficient (p)		0.84547
In Passin	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%lı	mprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	70.5
Vehicle	e Results					
Average	Speed, mi/h	70.5	Per	rcent Followers	, %	6.5
Segment	t Travel Time, minutes	0.85	Fol	llower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	8	0.00			0.0	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	alysis Year		2026
Jurisdicti	on		Tir	me Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		S	egmei	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ngth, ft		5280
Lane Wic	lth, ft	11	Sh	oulder Width, f	t	4
Speed Lii	mit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	219		Opposing Demand Flow Rate, veh/h		93
Peak Hou	ur Factor	0.94	To	tal Trucks, %		26.00
Segment	: Capacity, veh/h	1700	De	emand/Capacity	/ (D/C)	0.13
Interm	nediate Results					
Segment	: Vertical Class	1	Fre	ee-Flow Speed,	mi/h	71.0
Speed Slo	ope Coefficient (m)	4.10577	Sp	Speed Power Coefficient (p)		0.57765
PF Slope	Coefficient (m)	-1.11422	PF	PF Power Coefficient (p)		0.85581
In Passin	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.8
%lmprov	rement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data					·
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	69.8
Vehicle	e Results					
Average	Speed, mi/h	69.8	Pe	rcent Followers	, %	26.2
Segment	Travel Time, minutes	0.86	Fo	llower Density (	(FD), followers/mi/ln	0.8
Vehicle L	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p	I		ensity, followers/ mi/ln	LOS
1	52	0.01			0.8	A

		HCS Two-La	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ne Analyzed		Morning Peak Hour
Project D	Description	PH2 to LY2 Route 221 Southbound (South of Sellards Road)		Units		U.S. Customary
		S	egmen	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Ler	ngth, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	4
Speed Lii	mit, mi/h	65	Acc	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	197	Ор	Opposing Demand Flow Rate, veh/h		35
Peak Hou	ur Factor	0.94	Tot	al Trucks, %		8.00
Segment	Capacity, veh/h	1700	1700 Demand/Cap		/ (D/C)	0.12
Interm	nediate Results	•				·
Segment	: Vertical Class	1	Fre	e-Flow Speed,	mi/h	71.6
Speed Slo	ope Coefficient (m)	4.09995	Spe	Speed Power Coefficient (p)		0.61269
PF Slope	Coefficient (m)	-1.08480	PF	PF Power Coefficient (p)		0.86513
In Passin	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.7
%lmprov	rement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	70.6
Vehicle	e Results					
Average	Speed, mi/h	70.6	Per	cent Followers	, %	23.3
Segment	Travel Time, minutes 0.85		Fol	lower Density (	(FD), followers/mi/ln	0.7
Vehicle L	OS	А				
Facility	y Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
	46	0.01		<del>                                     </del>	0.7	A

		HCS Two-La	ne Hi	ghway Re	port	
Project	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2026
Jurisdictio	on		Tin	ne Analyzed		Afternoon Peak Hour
Project D	escription	PH2 to LY2 Route 221 Southbound (South of Sellards Road)		Units		U.S. Customary
		Sc	egmer	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Lei	ngth, ft		5280
Lane Wid	lth, ft	11	Sh	oulder Width, f	t	4
Speed Lir	nit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	93	Ор	Opposing Demand Flow Rate, veh/h		219
Peak Hou	ır Factor	0.94	Tot	tal Trucks, %		39.00
Segment	Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.05
Interm	ediate Results	•				
Segment	Vertical Class	1	Fre	ee-Flow Speed,	mi/h	70.6
Speed Slo	ope Coefficient (m)	4.13603	Sp	Speed Power Coefficient (p)		0.53404
PF Slope	Coefficient (m)	-1.14838	PF	PF Power Coefficient (p)		0.84346
In Passing	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.2
%lmprov	ement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subseg	gment Data		·			·
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	70.6
Vehicle	e Results					
Average S	Speed, mi/h	70.6	Pei	rcent Followers,	, %	14.3
Segment	Travel Time, minutes	0.85	Fo	llower Density (	(FD), followers/mi/ln	0.2
Vehicle L0	OS	А				
Facility	/ Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	22	0.00			0.2	A

		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2026
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour
Project [	Description	No Build Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	houlder Width, ft	t	4
Speed Li	imit, mi/h	65	Α	ccess Point Dens	sity, pts/mi	1.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	te, veh/h 35		Opposing Demand Flow Rate, veh/h		197
Peak Ho	our Factor	0.94	T	otal Trucks, %		41.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.02
Intern	nediate Results	•				
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	70.5
Speed S	lope Coefficient (m)	4.12439	S	Speed Power Coefficient (p)		0.54015
PF Slope	e Coefficient (m)	-1.14422	Р	PF Power Coefficient (p)		0.84547
In Passin	ng Lane Effective Length?	No	T	otal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	70.5
Vehicl	le Results					
Average	Speed, mi/h	70.5	Р	ercent Followers,	, %	6.5
Segmen	t Travel Time, minutes	0.85	F	ollower Density (	FD), followers/mi/ln	0.0
Vehicle l	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	8	0.00			0.0	A
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		HCS Two-La	ane Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	alysis Year		2026
Jurisdictio	on		Tir	me Analyzed		Afternoon Peak Hour
Project D	escription	No Build - Route 221 Northbound (South of Sellards Road)		Units		U.S. Customary
		S	Segmei	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ngth, ft		5280
Lane Wid	Ith, ft	11	Sh	oulder Width, f	t	4
Speed Lir	mit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	219	Op	Opposing Demand Flow Rate, veh/h		93
Peak Hou	ur Factor	0.94	To	tal Trucks, %		26.00
Segment	Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.13
Interm	nediate Results					
Segment	Vertical Class	1	Fre	ee-Flow Speed,	mi/h	71.0
Speed Slo	ope Coefficient (m)	4.10577	Sp	Speed Power Coefficient (p)		0.57765
PF Slope	Coefficient (m)	-1.11422	PF	PF Power Coefficient (p)		0.85581
In Passing	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.8
%Improv	ement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data	•	·			
# Sec	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	69.8
Vehicle	e Results					
Average S	Speed, mi/h	69.8	Pe	rcent Followers	, %	26.2
Segment	: Travel Time, minutes 0.86		Fo	llower Density (	(FD), followers/mi/ln	0.8
Vehicle LO	OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
	i	·				

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	Analysis Year		2026
Jurisdicti	on		Tin	ne Analyzed		Morning Peak Hour
Project D	Description	No Build - Route 221 Southbound (South o Sellards Road)		Units		U.S. Customary
		S	egmer	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Ler	ngth, ft		5280
Lane Wid	Ith, ft	11	Sh	oulder Width, f	t	4
Speed Lir	mit, mi/h	65	Ac	cess Point Dens	sity, pts/mi	1.0
Demai	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	197	Ор	Opposing Demand Flow Rate, veh/h		35
Peak Hou	ur Factor	0.94	Tot	tal Trucks, %		8.00
Segment	: Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.12
Interm	nediate Results					
Segment	: Vertical Class	1	Fre	e-Flow Speed,	mi/h	71.6
Speed Slo	ope Coefficient (m)	4.09995	Sp	Speed Power Coefficient (p)		0.61269
PF Slope	Coefficient (m)	-1.08480	PF	PF Power Coefficient (p)		0.86513
In Passing	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.7
%Improv	rement to Percent Followers	0.0	%lı	mprovement to	Speed	0.0
Subse	gment Data	·				
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	1-		-	70.6
Vehicle	e Results					
Average	Speed, mi/h	70.6	Pei	rcent Followers	, %	23.3
Segment	Travel Time, minutes	0.85	Fol	llower Density (	(FD), followers/mi/ln	0.7
Vehicle L	OS	А				
Facility	y Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
		- I		1		

		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	Δ	Analysis Year		2026
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour
Project I	Description	No Build Route 221 Southbound (South of Sellards Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	idth, ft	11	S	Shoulder Width, f	t	4
Speed L	imit, mi/h	65	Δ	Access Point Dens	sity, pts/mi	1.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	93	C	Opposing Demand Flow Rate, veh/h		219
Peak Ho	our Factor	0.94	Т	otal Trucks, %		39.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.05
Intern	nediate Results	•				
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	70.6
Speed S	Slope Coefficient (m)	4.13603	S	Speed Power Coe	fficient (p)	0.53404
PF Slope	e Coefficient (m)	-1.14838	Р	PF Power Coefficie	ent (p)	0.84346
In Passir	ng Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	0.2
%Impro	vement to Percent Followers	0.0	9/	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	70.6
Vehic	le Results					
Average	e Speed, mi/h	70.6	Р	Percent Followers	, %	14.3
Segment Travel Time, minutes 0.85		F	follower Density (	(FD), followers/mi/ln	0.2	
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	22	0.00			0.2	A
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		HCS Two-La	ne Hi	ghway Re	port		
Projec	t Information						
Analyst		James Vorosmarti	Da	ate		7/12/2023	
Agency		Tetra Tech	An	nalysis Year		2023	
Jurisdicti	ion		Tir	me Analyzed		Morning Peak Hour	
Project D	Description	Route 397 Eastbound (West of Nine Canyon Road)	Ur	Units		U.S. Customary	
Segment 1							
Vehicle	e Inputs						
Segment	t Type	Passing Zone	Le	Length, ft		5280	
Lane Wic	dth, ft	12	Sh	oulder Width, f	t	6	
Speed Li	mit, mi/h	60	Ac	ccess Point Dens	sity, pts/mi	0.0	
Demai	nd and Capacity						
Direction	nal Demand Flow Rate, veh/h	56	Op	Opposing Demand Flow Rate, veh/h		64	
Peak Hou	ur Factor	0.75	To	Total Trucks, %		19.00	
Segment	t Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.03	
Interm	nediate Results	•					
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	67.8	
Speed SI	lope Coefficient (m)	3.91462	Sp	eed Power Coe	fficient (p)	0.59261	
PF Slope	Coefficient (m)	-1.12326	PF	Power Coefficie	ent (p)	0.84993	
In Passin	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.1	
%Improv	vement to Percent Followers	0.0	%I	Improvement to	Speed	0.0	
Subse	gment Data						
# See	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h	
1 Tar	ngent	5280	-		-	67.8	
Vehicle	e Results						
Average	Speed, mi/h	67.8	Pe	rcent Followers	, %	9.2	
Segment	Segment Travel Time, minutes 0.89		Fo	llower Density (	FD), followers/mi/ln	0.1	
Vehicle L	.OS	А					
Facility	y Results						
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS	
1	11	0.00			0.1	A	

		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	D	Pate		7/12/2023
Agency		Tetra Tech	А	nalysis Year		2023
Jurisdict	ion		Т	ime Analyzed		Afternoon Peak Hour
Project [	Description	Route 397 Eastbound (West of Nine Canyon Road)	U	Units		U.S. Customary
		Sc	egme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	L	Length, ft		5280
Lane Wi	dth, ft	12	S	houlder Width, ft	t	6
Speed Li	imit, mi/h	60	А	ccess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	56	С	Opposing Demand Flow Rate, veh/h		44
Peak Ho	our Factor	0.82	To	otal Trucks, %		30.00
Segmen	t Capacity, veh/h	1700	D	Demand/Capacity (D/C)		0.03
Intern	nediate Results	•				
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	67.4
Speed SI	lope Coefficient (m)	3.88052	S	peed Power Coef	fficient (p)	0.60579
PF Slope	e Coefficient (m)	-1.11357	Р	F Power Coefficie	ent (p)	0.85439
In Passin	ng Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	67.4
Vehicl	le Results					
Average	Speed, mi/h	67.4 Percent Followers, %		9.1		
Segment Travel Time, minutes 0.89		0.89	F	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle L	LOS	А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	12	0.00			0.1	A
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		HCS Two-La	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	С	Date		7/12/2023
Agency		Tetra Tech	А	Analysis Year		2023
Jurisdict	tion		Т	ime Analyzed		Morning Peak Hour
Project I	Description	Route 397 Westbound (West of Nine Canyon Road)		Units		U.S. Customary
		Sc	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	6
Speed L	imit, mi/h	60	А	Access Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	60	С	Opposing Demand Flow Rate, veh/h		53
Peak Ho	our Factor	0.80	T	otal Trucks, %		27.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.04
Intern	nediate Results	•				
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	67.5
Speed S	Slope Coefficient (m)	3.89236	S	Speed Power Coe	fficient (p)	0.59979
PF Slope	e Coefficient (m)	-1.11818	Р	PF Power Coefficie	ent (p)	0.85248
In Passir	ng Lane Effective Length?	No	T	otal Segment De	nsity, veh/mi/ln	0.1
%Impro	vement to Percent Followers	0.0	%	6Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	67.5
Vehic	le Results					
Average	verage Speed, mi/h 67.5 Percent Followers, %		, %	9.7		
Segment Travel Time, minutes 0.89		0.89	F	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	12	0.00			0.1	A
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		HCS Two-La	ne H	ighway Re	eport	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2023
Jurisdict	tion		Т	Time Analyzed		
Project I	Description	Route 397 Westbound (West of Nine Canyon Road)		Units		U.S. Customary
		Sc	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	Length, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	6
Speed L	imit, mi/h	60	A	Access Point Den	sity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	52		Opposing Demand Flow Rate, veh/h		67
Peak Ho	our Factor	0.69	Т	Total Trucks, %		22.00
Segmen	nt Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.03
Intern	nediate Results					
Segmen	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	67.7
Speed S	Slope Coefficient (m)	3.91091	S	Speed Power Coe	efficient (p)	0.59106
PF Slope	e Coefficient (m)	-1.12480	F	PF Power Coeffici	ent (p)	0.84966
In Passir	ng Lane Effective Length?	No	Т	Total Segment De	ensity, veh/mi/ln	0.1
%lmpro	vement to Percent Followers	0.0	9	%Improvement to	o Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	67.7
Vehic	le Results					
Average	e Speed, mi/h	67.7	F	Percent Followers	5, %	8.7
Segment Travel Time, minutes 0.89		F	Follower Density	(FD), followers/mi/ln	0.1	
Vehicle I	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	9	0.00			0.1	A
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		HCS Two-Lai	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdicti	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	Build Ph1 to LY1 Route 397 Eastbound (West o Nine Canyon Road)		ts		U.S. Customary
Segment 1						
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	Length, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, ft	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	57	Орј	Opposing Demand Flow Rate, veh/h		71
Peak Hou	ur Factor	0.75	Tota	Total Trucks, %		19.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03
Interm	nediate Results	<u>'</u>				
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	67.8
Speed SI	ope Coefficient (m)	3.91883	Spe	ed Power Coet	fficient (p)	0.58880
PF Slope	Coefficient (m)	-1.12634	PF I	Power Coefficie	ent (p)	0.84883
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	it	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.8
Vehicle	e Results					
Average	Speed, mi/h	67.8	Per	cent Followers,	, %	9.5
Segment	t Travel Time, minutes	0.89	Foll	Follower Density (FD), followers/mi/ln		0.1
Vehicle L	.OS	А				
Facility	y Results					·
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	11	0.00			0.1	A

		HCS Two-La	ne H	ligh	nway Re	port		
Proje	ct Information							
Analyst		James Vorosmarti	1	Date				7/12/2023
Agency		Tetra Tech	1	Analy	rsis Year			2025
Jurisdict	tion		1	Time	Analyzed			Afternoon Peak Hour
Project I	Description	Build Ph1 to LY1 Route 397 Eastbound (West on Nine Canyon Road)		Units		U.S. Customary		
		Se	egme	ent	1			
Vehic	le Inputs							
Segmen	nt Type	Passing Zone	L	Leng	th, ft			5280
Lane Wi	idth, ft	12	9	Shou	lder Width, ft	t		6
Speed L	imit, mi/h	60	1	Acces	ss Point Dens	ity, pts/mi		0.0
Dema	and Capacity							
Directio	nal Demand Flow Rate, veh/h	62	(	Opposing Demand Flow Rate, veh/h		45		
Peak Ho	our Factor	0.82	-	Total Trucks, %		30.00		
Segmen	nt Capacity, veh/h	1700	1	Demand/Capacity (D/C)			0.04	
Intern	nediate Results							
Segmen	nt Vertical Class	1	F	Free-	Flow Speed,	mi/h		67.4
Speed S	Slope Coefficient (m)	3.88147	9	Spee	d Power Coef	fficient (p)		0.60490
PF Slope	e Coefficient (m)	-1.11429	F	PF Po	wer Coefficie	ent (p)		0.85413
In Passir	ng Lane Effective Length?	No	1	Total	Segment De	nsity, veh/mi/ln		0.1
%Impro	vement to Percent Followers	0.0	ç	%lmp	provement to	Speed		0.0
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	us, ft		Superelevation, %		Average Speed, mi/h
1 Ta	angent	5280	-			-		67.4
Vehic	le Results							
Average	erage Speed, mi/h 67.4 Percent Followers, %			9.9				
Segment Travel Time, minutes 0.89		0.89	F	Follo	wer Density (	FD), followers/mi/ln	1	0.1
Vehicle	LOS	А						
Facilit	ty Results							
т	VMT veh-mi/AP	VHD veh-h/p				ensity, followers/ mi/ln		LOS
1	13	0.00				0.1		A
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HCS Two-Lane Highway Report						
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdiction	on		Tim	e Analyzed		Morning Peak Hour
Project D	escription	Build Ph1 to LY1 Route 397 Westbound (West Nine Canyon Road)		ts .		U.S. Customary
		Se	egmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Len	Length, ft		5280
Lane Wid	lth, ft	12	Sho	ulder Width, ft	<u> </u>	6
Speed Lir	mit, mi/h	60	Acc	ess Point Dens	ity, pts/mi	0.0
Demar	nd and Capacity					·
Direction	al Demand Flow Rate, veh/h	66	Орр	Opposing Demand Flow Rate, veh/h		54
Peak Hou	ır Factor	0.80	Tota	Total Trucks, %		27.00
Segment	Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.04
Interm	nediate Results	<u>'</u>				
Segment	Vertical Class	1	Free	e-Flow Speed,	mi/h	67.5
Speed Slo	ope Coefficient (m)	3.89325	Spe	ed Power Coef	fficient (p)	0.59897
PF Slope	Coefficient (m)	-1.11885	PF F	Power Coefficie	ent (p)	0.85224
In Passing	g Lane Effective Length?	No	Tota	ıl Segment Dei	nsity, veh/mi/ln	0.1
%Improv	ement to Percent Followers	0.0	%lm	nprovement to	Speed	0.0
Subse	gment Data					·
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	67.5
Vehicle	e Results					
Average Speed, mi/h 67.5		Pero	ent Followers,	%	10.5	
Segment Travel Time, minutes 0.89		Follo	ower Density (	FD), followers/mi/ln	0.1	
Vehicle L	OS	A				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	13	0.00			0.1	А

<b>Project</b> Analyst	Information					
Analyst						
•		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdictio	n		Tim	ne Analyzed		
Project De	scription	Build Ph1 to LY1 Route 397 Westbound (West o Nine Canyon Road)		Units		U.S. Customary
		Se	gmen	nt 1		
Vehicle	Inputs					
Segment T	Гуре	Passing Zone	Ler	Length, ft		5280
Lane Widtl	h, ft	12	Sho	oulder Width, f	t	6
Speed Lim	it, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Deman	d and Capacity					
Directiona	l Demand Flow Rate, veh/h	54	Ор	Opposing Demand Flow Rate, veh/h		74
Peak Hour	Factor	0.69	Tot	Total Trucks, %		22.00
Segment C	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.03
Interme	ediate Results					
Segment \	/ertical Class	1	Fre	e-Flow Speed,	mi/h	67.7
Speed Slop	pe Coefficient (m)	3.91539	Spe	eed Power Coe	fficient (p)	0.58702
PF Slope C	Coefficient (m)	-1.12805	PF	Power Coefficie	ent (p)	0.84850
In Passing	Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.1
%Improve	ment to Percent Followers	0.0	%Ir	mprovement to	Speed	0.0
Subseg	ment Data					
# Segr	ment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tang	gent	5280	-		-	67.7
Vehicle	Results					
Average S <sub>l</sub>	peed, mi/h	67.7	Per	cent Followers,	, %	9.0
Segment Travel Time, minutes 0.89		Fol	lower Density (	FD), followers/mi/ln	0.1	
Vehicle LO	S	А	·			
Facility	Results		,			
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	9	0.00			0.1	A

<b>Project</b> Analyst Agency	Information					
Agency		James Vorosmarti	Da	ate		7/12/2023
		Tetra Tech	Ar	Analysis Year		2025
Jurisdiction	1		Tir	me Analyzed		Morning Peak Hour
Project Des	scription	Build Ph1 to LY2 Route 397 Eastbound (West o Nine Canyon Road)		Units		U.S. Customary
		Se	egmei	nt 1		
Vehicle	Inputs					
Segment T	ype	Passing Zone	Le	Length, ft		5280
Lane Width	n, ft	12	Sh	noulder Width, f	t	6
Speed Limi	it, mi/h	60	Ac	ccess Point Dens	ity, pts/mi	0.0
Deman	d and Capacity					
Directional	Demand Flow Rate, veh/h	57	Or	Opposing Demand Flow Rate, veh/h		71
Peak Hour	Factor	0.75	То	Total Trucks, %		19.00
Segment C	Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.03
Interme	ediate Results					
Segment V	/ertical Class	1	Fre	ee-Flow Speed,	mi/h	67.8
Speed Slop	pe Coefficient (m)	3.91883	Sp	peed Power Coe	fficient (p)	0.58880
PF Slope C	oefficient (m)	-1.12634	PF	Power Coefficie	ent (p)	0.84883
In Passing	Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	0.1
%Improver	ment to Percent Followers	0.0	%I	Improvement to	Speed	0.0
Subsegi	ment Data					
# Segn	nent Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Tang	ent	5280	-		-	67.8
Vehicle	Results	<u>'</u>				
Average Sp	peed, mi/h	67.8 Percent Followers, %		9.5		
Segment Travel Time, minutes 0.89		Fo	ollower Density (	FD), followers/mi/ln	0.1	
Vehicle LOS	S	А				
Facility	Results		-			
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	11	0.00			0.1	A

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13
ge Speed, mi/h
LOS
A

		HCS Two-Lar	ne Hig	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	Analysis Year		2025
Jurisdict	tion		Tin	ne Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY2 Route 397 Westbound (West Nine Canyon Road)		Units		U.S. Customary
		Se	gmer	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Ler	Length, ft		5280
Lane Wi	idth, ft	12	Sho	oulder Width, f	t	6
Speed L	imit, mi/h	60	Acc	cess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	66	Ор	Opposing Demand Flow Rate, veh/h		54
Peak Ho	our Factor	0.80	Tot	Total Trucks, %		27.00
Segmen	nt Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	e-Flow Speed,	mi/h	67.5
Speed S	Slope Coefficient (m)	3.89325	Spe	eed Power Coe	fficient (p)	0.59897
PF Slope	e Coefficient (m)	-1.11885	PF	Power Coefficie	ent (p)	0.85224
In Passir	ng Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.1
%Impro	vement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	67.5
Vehic	le Results					'
Average	e Speed, mi/h	67.5 Percent Followers, %		10.5		
Segmen	Segment Travel Time, minutes 0.89		Fol	lower Density (	(FD), followers/mi/ln	0.1
Vehicle	LOS	A	7. "			
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	13	0.00			0.1	A

		HCS Two-Lar	ne H	lighway Re	port			
Projec	ct Information							
Analyst		James Vorosmarti	Г	Date		7/12/2023		
Agency		Tetra Tech	A	Analysis Year		2025		
Jurisdict	tion		7	Гime Analyzed				
Project I	Description	Build Ph1 to LY2 Route 397 Westbound (West Nine Canyon Road)		Units		U.S. Customary		
Segment 1								
Vehicle Inputs								
Segmen	nt Type	Passing Zone		Length, ft		5280		
Lane Wi	idth, ft	12	9	Shoulder Width, f	t	6		
Speed L	imit, mi/h	60	A	Access Point Dens	sity, pts/mi	0.0		
Dema	and Capacity					·		
Directio	nal Demand Flow Rate, veh/h	54		Opposing Demand Flow Rate, veh/h		74		
Peak Ho	our Factor	0.69		Total Trucks, %		22.00		
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.03		
Intern	nediate Results	•				·		
Segment Vertical Class		1		Free-Flow Speed, mi/h		67.7		
Speed Slope Coefficient (m)		3.91539		Speed Power Coefficient (p)		0.58702		
PF Slope Coefficient (m)		-1.12805		PF Power Coefficient (p)		0.84850		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.1		
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h		
1 Ta	angent	5280	-		-	67.7		
Vehic	le Results							
Average Speed, mi/h		67.7		Percent Followers, %		9.0		
Segment Travel Time, minutes		0.89		Follower Density (FD), followers/mi/ln		0.1		
Vehicle LOS		А						
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	9	0.00			0.1	A		
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<b>Project</b> Analyst Agency	Information							
Agency		James Vorosmarti	Dat	te		7/12/2023		
		Tetra Tech	Ana	alysis Year		2025		
Jurisdiction	1		Tim	ne Analyzed		Morning Peak Hour		
Project Des	scription	No Build Route 397 Eastbound (West of Nin Canyon Road)		Units		U.S. Customary		
Segment 1								
Vehicle	Inputs							
Segment T	ype	Passing Zone	Len	ngth, ft		5280		
Lane Width	n, ft	12	Sho	oulder Width, ft	t	6		
Speed Limi	it, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0		
Deman	d and Capacity							
Directional	Demand Flow Rate, veh/h	57		Opposing Demand Flow Rate, veh/h		65		
Peak Hour Factor				Total Trucks, %		19.00		
Segment C	apacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03		
Interme	ediate Results					·		
Segment Vertical Class		1		Free-Flow Speed, mi/h		67.8		
Speed Slope Coefficient (m)		3.91547	Spe	Speed Power Coefficient (p)		0.59183		
PF Slope Coefficient (m)		-1.12389		PF Power Coefficient (p)		0.84970		
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.1		
%Improver	ment to Percent Followers	s 0.0 %Improvement to Speed		0.0				
Subsegi	ment Data							
# Segn	nent Type	Length, ft	Radius, 1	ft	Superelevation, %	Average Speed, mi/h		
1 Tang	ent	5280	-		-	67.8		
Vehicle	Results					·		
Average Speed, mi/h		67.8		Percent Followers, %		9.4		
Segment Travel Time, minutes		0.89		Follower Density (FD), followers/mi/ln		0.1		
Vehicle LOS		А						
Facility	Results							
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS		
1	11	0.00			0.1	A		

		HCS Two-Lar	ne Hig	ghway Re	port			
Proje	ct Information							
Analyst		James Vorosmarti	Dat	Date		7/12/2023		
Agency		Tetra Tech	Ana	Analysis Year		2025		
Jurisdict	tion		Tim	ne Analyzed		Afternoon Peak Hour		
Project I	Description	No Build Route 397 Eastbound (West of Nin Canyon Road)		Units		U.S. Customary		
Segment 1								
Vehic	le Inputs							
Segmen	ment Type Passing Zone Length, ft			5280				
Lane Wi	idth, ft	12	Sho	oulder Width, f	t	6		
Speed L	imit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0		
Dema	and Capacity							
Directio	nal Demand Flow Rate, veh/h	57		Opposing Demand Flow Rate, veh/h		45		
Peak Ho	our Factor			Total Trucks, %		30.00		
Segmen	nt Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03		
Intern	nediate Results							
Segment Vertical Class		1	Fre	Free-Flow Speed, mi/h		67.4		
Speed Slope Coefficient (m)		3.88147	Spe	Speed Power Coefficient (p)		0.60490		
PF Slope Coefficient (m)		-1.11429	PF	PF Power Coefficient (p)		0.85413		
In Passing Lane Effective Length?		No	Tota	Total Segment Density, veh/mi/ln		0.1		
%Impro	vement to Percent Followers	0.0 %In		%Improvement to Speed		0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h		
1 Ta	angent	5280	-		-	67.4		
Vehic	le Results							
Average Speed, mi/h		67.4		Percent Followers, %		9.2		
Segment Travel Time, minutes		0.89		Follower Density (FD), followers/mi/ln		0.1		
Vehicle LOS		А						
Facilit	ty Results							
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS		
1	12	0.00			0.1	A		

		HCS Two-Lar	ne Hig	hway Re	port				
Project	t Information								
Analyst		James Vorosmarti	Date	e		7/12/2023			
Agency		Tetra Tech	Ana	lysis Year		2025			
Jurisdictio	on		Tim	e Analyzed		Morning Peak Hour			
Project De	escription	No Build Route 397 Westbound (West of Ni Canyon Road)		Units		U.S. Customary			
	Segment 1								
Vehicle Inputs									
Segment	Туре	Passing Zone	Len	gth, ft		5280			
Lane Wid	th, ft	12	Sho	ulder Width, ft	t	6			
Speed Lin	nit, mi/h	60	Acce	ess Point Dens	ity, pts/mi	0.0			
Deman	nd and Capacity								
Direction	al Demand Flow Rate, veh/h	61		Opposing Demand Flow Rate, veh/h		54			
Peak Hou	ır Factor	0.80		Total Trucks, %		27.00			
Segment	Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.04			
Interm	ediate Results		-			·			
Segment Vertical Class		1		Free-Flow Speed, mi/h		67.5			
Speed Slope Coefficient (m)		3.89325		Speed Power Coefficient (p)		0.59897			
PF Slope Coefficient (m)		-1.11885		PF Power Coefficient (p)		0.85224			
In Passing Lane Effective Length?		No ·		Total Segment Density, veh/mi/ln		0.1			
%Improve	ement to Percent Followers	0.0	%lm	%Improvement to Speed		0.0			
Subseg	gment Data								
# Seg	gment Type	Length, ft	Radius, ft	t	Superelevation, %	Average Speed, mi/h			
1 Tan	gent	5280	-		-	67.5			
Vehicle	e Results								
Average S	Speed, mi/h	67.5 Pe		Percent Followers, %		9.8			
Segment Travel Time, minutes		0.89 F		Follower Density (FD), followers/mi/ln		0.1			
Vehicle LOS		A							
Facility	/ Results								
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	12	0.00		0.1		А			

		HCS Two-Lan	e Hig	hway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2025
Jurisdict	tion		Tim	e Analyzed		
Project [	Description	No Build Route 397 Westbound (West of Nir Canyon Road)	ne Uni	ts		U.S. Customary
		Se	gmen	t 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	dth, ft	12	Sho	ulder Width, f	t	6
Speed Li	imit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	54	Орр	oosing Deman	d Flow Rate, veh/h	68
Peak Ho	our Factor	0.69	Tota	al Trucks, %		22.00
Segmen	nt Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.03
Intern	mediate Results					
Segmen	nt Vertical Class	1	Free	e-Flow Speed,	mi/h	67.7
Speed S	lope Coefficient (m)	3.91183	Spe	ed Power Coe	fficient (p)	0.59023
PF Slope	e Coefficient (m)	-1.12547	PF F	Power Coefficie	ent (p)	0.84943
In Passin	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	egment Data					<u> </u>
# Se	egment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	67.7
Vehicl	le Results					·
Average	Speed, mi/h	67.7	Perd	cent Followers,	, %	8.9
Segmen	nt Travel Time, minutes	0.89	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p	1 2: 1		LOS	
1	9	0.00			0.1	A
1		·				A

		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ne Analyzed		Morning Peak Hour
Project D	Description	Ph2 to LY2 Route 397 Eastbound (West of Nir Canyon Road)	Uni	its		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	oulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	57	Ор	posing Deman	d Flow Rate, veh/h	71
Peak Hou	ur Factor	0.75	Total Trucks, %		19.00	
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03
Interm	nediate Results					
Segment	t Vertical Class	1	Fre	e-Flow Speed,	mi/h	67.8
Speed SI	ope Coefficient (m)	3.91883	Spe	eed Power Coe	fficient (p)	0.58880
PF Slope	Coefficient (m)	-1.12634	PF I	Power Coefficie	ent (p)	0.84883
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data		·			
# See	gment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.8
Vehicle	e Results					<u>'</u>
Average	Speed, mi/h	67.8	Per	cent Followers	, %	9.5
Segment	t Travel Time, minutes	0.89	Foll	lower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	11	0.00			0.1	A

		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	ion		Tim	ne Analyzed		Afternoon Peak Hour
Project D	Description	PH2 to LY2 Route 397 Eastbound (West of Nir Canyon Road)	Uni	its		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	ngth, ft		5280
Lane Wic	dth, ft	12	Sho	oulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	62	Ор	posing Deman	d Flow Rate, veh/h	45
Peak Hou	ur Factor	0.82	Tot	Total Trucks, %		30.00
Segment	t Capacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04
Interm	nediate Results					
Segment	t Vertical Class	1	Fre	e-Flow Speed,	mi/h	67.4
Speed SI	lope Coefficient (m)	3.88147	Spe	eed Power Coe	fficient (p)	0.60490
PF Slope	Coefficient (m)	-1.11429	PF	Power Coefficie	ent (p)	0.85413
In Passin	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, 1	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.4
Vehicle	e Results					
Average	Speed, mi/h	67.4	Per	cent Followers	, %	9.9
Segment	t Travel Time, minutes	0.89	Foll	lower Density (	FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	13	0.00			0.1	A

		HCS Two-Lar	ne Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdiction	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	PH2 to LY2 Route 397 Westbound (West of Ni Canyon Road)	ne Unit	ts .		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Len	gth, ft		5280
Lane Wid	Ith, ft	12	Sho	ulder Width, f	t	6
Speed Lir	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demar	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	66	Орр	oosing Deman	d Flow Rate, veh/h	54
Peak Hou	ur Factor	0.80	Total Trucks, %		27.00	
Segment	: Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.04
Interm	nediate Results					·
Segment	: Vertical Class	1	Free	e-Flow Speed,	mi/h	67.5
Speed Slo	ope Coefficient (m)	3.89325	Spe	ed Power Coe	fficient (p)	0.59897
PF Slope	Coefficient (m)	-1.11885	PF F	Power Coefficie	ent (p)	0.85224
In Passing	g Lane Effective Length?	No	Tota	l Segment De	nsity, veh/mi/ln	0.1
%Improv	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					<u> </u>
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	67.5
Vehicle	e Results					
Average :	Speed, mi/h	67.5	Pero	cent Followers,	, %	10.5
Segment	Travel Time, minutes	0.89	Follo	ower Density (	(FD), followers/mi/ln	0.1
Vehicle Lo	OS	А				
Facility	y Results		,			
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
	ven-m/p	7 C 1./ P				

		HCS Two-Lan	e Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		
Project D	Description	PH2 to LY2 Route 397 Westbound (West of Nii Canyon Road)	ne Uni	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	54	Орр	oosing Deman	d Flow Rate, veh/h	74
Peak Hou	ur Factor	0.69	Tota	Total Trucks, %		22.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03
Interm	nediate Results					
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	67.7
Speed SI	ope Coefficient (m)	3.91539	Spe	ed Power Coe	fficient (p)	0.58702
PF Slope	Coefficient (m)	-1.12805	PF F	Power Coefficie	ent (p)	0.84850
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# See	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.7
Vehicle	e Results					
Average	Speed, mi/h	67.7	Perd	cent Followers,	, %	9.0
Segment	t Travel Time, minutes	0.89	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	9	0.00			0.1	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Morning Peak Hour
Project D	Description	No Build Route 397 Eastbound (West of Nir Canyon Road)	Uni	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	туре	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	57	Орр	oosing Deman	d Flow Rate, veh/h	65
Peak Hou	ur Factor	0.75	Total Trucks, %			19.00
Segment	t Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.03
Interm	nediate Results	•				·
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	67.8
Speed SI	ope Coefficient (m)	3.91547	Spe	ed Power Coe	fficient (p)	0.59183
PF Slope	Coefficient (m)	-1.12389	PF F	Power Coefficie	ent (p)	0.84970
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%lmprov	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.8
Vehicle	e Results					
Average	Speed, mi/h	67.8	Perd	cent Followers	, %	9.4
Segment	t Travel Time, minutes	0.89	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle L	OS	А				
Facility	y Results		·			
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
	11	0.00			0.1	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	llysis Year		2026
Jurisdicti	on		Tim	e Analyzed		Afternoon Peak Hour
Project D	Description	No Build - Route 397 Eastbound (West of Nir Canyon Road)	Unit	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	57	Орр	oosing Deman	d Flow Rate, veh/h	45
Peak Hou	ur Factor	0.82	Total Trucks, %		30.00	
Segment	t Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.03
Interm	nediate Results					·
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	67.4
Speed SI	ope Coefficient (m)	3.88147	Spe	ed Power Coe	fficient (p)	0.60490
PF Slope	Coefficient (m)	-1.11429	PF F	Power Coefficie	ent (p)	0.85413
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%lmprov	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.4
Vehicle	e Results					
Average	Speed, mi/h	67.4	Perd	cent Followers	, %	9.2
Segment	t Travel Time, minutes	0.89	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results		·			
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	12	0.00			0.1	A

		HCS Two-Lar	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ate		7/12/2023
Agency		Tetra Tech	Ar	nalysis Year		2026
Jurisdict	ion		Tir	me Analyzed		Morning Peak Hour
Project I	Description	No Build Route 397 Westbound (West of Ni Canyon Road)		nits		U.S. Customary
		Se	gme	nt 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	Le	ength, ft		5280
Lane Wi	dth, ft	12	Sh	noulder Width, ft	t	6
Speed L	imit, mi/h	60	Ac	ccess Point Dens	ity, pts/mi	0.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	61	Oı	pposing Demand	d Flow Rate, veh/h	54
Peak Ho	our Factor	0.80	То	otal Trucks, %		27.00
Segmen	nt Capacity, veh/h	1700	De	Demand/Capacity (D/C)		0.04
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	ee-Flow Speed,	mi/h	67.5
Speed S	lope Coefficient (m)	3.89325	Sp	peed Power Coef	fficient (p)	0.59897
PF Slope	e Coefficient (m)	-1.11885	PF	Power Coefficie	ent (p)	0.85224
In Passir	ng Lane Effective Length?	No	То	otal Segment De	nsity, veh/mi/ln	0.1
%Impro	vement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	67.5
Vehicl	le Results					
Average	Speed, mi/h	67.5	Pe	ercent Followers,	%	9.8
Segmen	t Travel Time, minutes	0.89	Fo	ollower Density (	FD), followers/mi/ln	0.1
Vehicle I	Vehicle LOS A					
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	12	0.00			0.1	Α
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		HCS Two-Lan	ie Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	lysis Year		2026
Jurisdicti	on		Tim	e Analyzed		
Project D	Description	No Build - Route 397 Westbound (West of Nir Canyon Road)	ne Unit	ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	ulder Width, f	t	6
Speed Li	mit, mi/h	60	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	54	Орр	oosing Deman	d Flow Rate, veh/h	68
Peak Hou	ur Factor	0.69	Total Trucks, %		22.00	
Segment	t Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.03
Interm	nediate Results					
Segment	t Vertical Class	1	Free	e-Flow Speed,	mi/h	67.7
Speed SI	ope Coefficient (m)	3.91183	Spe	ed Power Coe	fficient (p)	0.59023
PF Slope	Coefficient (m)	-1.12547	PF F	Power Coefficie	ent (p)	0.84943
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data		·			
# See	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	67.7
Vehicle	e Results					
Average	Speed, mi/h	67.7	Perd	cent Followers,	, %	8.9
Segment	t Travel Time, minutes	0.89	Foll	ower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	9	0.00			0.1	A

HCS Summary Sellards Road

		HCS Two-Lar	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	te		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2023
Jurisdicti	on		Tim	ne Analyzed		Morning Peak Hour
Project D	Description	Sellards Road Eastboun (between Route 221 an Tyack Road)	-	its		U.S. Customary
		Se	gmen	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Ler	ngth, ft		5280
Lane Wic	dth, ft	12	Sho	oulder Width, f	t	2
Speed Li	mit, mi/h	50	Acc	cess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	40	Ор	posing Deman	d Flow Rate, veh/h	57
Peak Hou	ur Factor	0.70	Tot	Total Trucks, %		29.00
Segment	t Capacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.02
Interm	nediate Results					·
Segment	t Vertical Class	1	Fre	e-Flow Speed,	mi/h	53.2
Speed SI	ope Coefficient (m)	3.12236	Spe	eed Power Coe	fficient (p)	0.59679
PF Slope	Coefficient (m)	-1.18055	PF	Power Coefficie	ent (p)	0.80954
In Passin	g Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.1
%Improv	vement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0
Subse	gment Data		·			·
# See	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	53.2
Vehicle	e Results					·
Average	Speed, mi/h	53.2	Per	cent Followers	, %	8.3
Segment	t Travel Time, minutes	1.13	Fol	lower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А				
Facility	y Results					·
Т	VMT veh-mi/p	VHD veh-h/p	Follower Density, followers/		LOS	
1	7	0.00			0.1	A

		HCS Two-Lar	ne Hig	jhway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2023
Jurisdicti	on		Tim	ie Analyzed		Afternoon Peak Hour
Project D	Description	Sellards Road Eastboun (Between Route 221 an Tyack Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Len	gth, ft		5280
Lane Wic	dth, ft	12	Sho	oulder Width, f	t	2
Speed Li	mit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	0.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	64	Орј	posing Deman	d Flow Rate, veh/h	48
Peak Hou	ur Factor	0.86	Tota	Total Trucks, %		27.00
Segment	Capacity, veh/h	1700	Der	Demand/Capacity (D/C)		0.04
Interm	nediate Results	•				
Segment	: Vertical Class	1	Free	e-Flow Speed,	mi/h	53.3
Speed SI	ope Coefficient (m)	3.11918	Spe	ed Power Coe	fficient (p)	0.60308
PF Slope	Coefficient (m)	-1.17491	PF I	Power Coefficie	ent (p)	0.81107
In Passin	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1
%lmprov	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0
Subse	gment Data		·			
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	53.3
Vehicle	e Results					
Average	Speed, mi/h	53.3	Per	cent Followers	, %	11.9
Segment	: Travel Time, minutes	1.13	Foll	ower Density (	FD), followers/mi/ln	0.1
Vehicle L	OS	А				
Facility	y Results					·
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS
1	14	0.00			0.1	A

		HCS Two-Lar	ne Hig	hway Re	port				
Projec	t Information								
Analyst		James Vorosmarti	Date	e		7/12/2023			
Agency		Tetra Tech	Ana	lysis Year		2023			
Jurisdiction	on		Tim	e Analyzed		Morning Peak Hour			
Project D	Pescription	Sellards Road Westbou (between Route 221 an Tyack Road)		ts		U.S. Customary			
	Segment 1								
Vehicle	e Inputs								
Segment	Туре	Passing Zone	Len	gth, ft		5280			
Lane Wid	lth, ft	12	Sho	ulder Width, ft	t	2			
Speed Lir	mit, mi/h	50	Acc	ess Point Dens	ity, pts/mi	0.0			
Demai	nd and Capacity								
Direction	al Demand Flow Rate, veh/h	44	Орр	oosing Deman	d Flow Rate, veh/h	31			
Peak Hou	ur Factor	0.91	Tota	al Trucks, %		40.00			
Segment	: Capacity, veh/h	1700	Den	Demand/Capacity (D/C)		0.03			
Interm	nediate Results		·						
Segment	Vertical Class	1	Free	e-Flow Speed,	mi/h	52.9			
Speed Slo	ope Coefficient (m)	3.08165	Spe	ed Power Coef	fficient (p)	0.61646			
PF Slope	Coefficient (m)	-1.16240	PF F	Power Coefficie	ent (p)	0.81516			
In Passing	g Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.1			
%Improv	rement to Percent Followers	0.0	%In	nprovement to	Speed	0.0			
Subse	gment Data								
# Seg	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h			
1 Tar	ngent	5280	-		-	52.9			
Vehicle	e Results								
Average	Speed, mi/h	52.9	Pero	cent Followers,	%	8.7			
Segment	Travel Time, minutes	1.13	Follo	ower Density (	FD), followers/mi/ln	0.1			
Vehicle L	OS	А							
Facility	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/		LOS			
1	10	0.00			0.1	А			

		HCS Two-Lar	ne Hig	hway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Date	e		7/12/2023
Agency		Tetra Tech	Ana	Analysis Year		2023
Jurisdicti	ion		Tim	e Analyzed		Afternoon Peak Hour
Project [	Description	Sellards Road Westbour (between Route 221 and Tyack Road)		ts		U.S. Customary
		Se	gmen	t 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone	Len	gth, ft		5280
Lane Wid	dth, ft	12	Sho	ulder Width, ft	t	2
Speed Li	imit, mi/h	50	Acc	ess Point Dens	ity, pts/mi	0.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	56	Орр	pposing Demand Flow Rate, veh/h		75
Peak Ho	ur Factor	0.73	Tota	al Trucks, %		20.00
Segment	t Capacity, veh/h	1700	Den	nand/Capacity	(D/C)	0.03
Intern	nediate Results					·
Segment	t Vertical Class	1	Free-Flow Speed, mi/h		53.5	
Speed SI	lope Coefficient (m)	3.15022	Spe	Speed Power Coefficient (p)		0.58625
PF Slope	e Coefficient (m)	-1.19041	PF F	PF Power Coefficient (p)		0.80637
In Passin	ng Lane Effective Length?	No	Tota	Total Segment Density, veh/mi/ln		0.1
%lmprov	vement to Percent Followers	0.0	%Im	nprovement to	Speed	0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radius, f	t	Superelevation, %	Average Speed, mi/h
1 Tai	ngent	5280	-		-	53.5
Vehicl	e Results					<u> </u>
Average	Speed, mi/h	53.5	Perd	cent Followers,	%	11.0
Segment	t Travel Time, minutes	1.12	Foll	ower Density (	FD), followers/mi/ln	0.1
Vehicle LOS A						
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	10	0.00			0.1	A
Т	VMT veh-mi/p	veh-h/p			mi/ln	

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т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
Facility	y Results					
Vehicle LOS		А				
Segment	t Travel Time, minutes	1.13	Fo	ollower Density (	FD), followers/mi/ln	0.1
Average	Speed, mi/h	53.2	Pe	ercent Followers,	%	9.5
Vehicle	e Results					
1 Tar	ngent	5280	-		-	53.2
	gment Type	3	Radius	s, ft	Superelevation, %	Average Speed, mi/h
Subse	gment Data					
%Improv	vement to Percent Followers	0.0	%	%Improvement to Speed		0.0
,	g Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.1
•	Coefficient (m)	-1.18136		PF Power Coefficient (p)		0.80930
•	ope Coefficient (m)	3.12333		Speed Power Coefficient (p)		0.59589
Segment	t Vertical Class	1		ree-Flow Speed,	mi/h	53.2
Interm	nediate Results					
Segment	t Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.03
Peak Hour Factor		0.70	To	otal Trucks, %		29.00
Directional Demand Flow Rate, veh/h		47		pposing Demand	d Flow Rate, veh/h	59
Demai	nd and Capacity					
Speed Lii	mit, mi/h	50	A	ccess Point Dens	ity, pts/mi	0.0
Lane Wic		12	Sł	houlder Width, ft		2
Segment		Passing Zone		ength, ft		5280
Vehicle	e Inputs					
		Se	gme	nt 1		
Project D	Pescription	Build Ph1 to LY1 Sellards Road Eastbound (betwe Route 221 and Tyack Road)		nits		U.S. Customary
Jurisdicti	on		Ti	ime Analyzed		Morning Peak Hour
Agency		Tetra Tech	Aı	nalysis Year		2025
Analyst		James Vorosmarti	D	ate		7/12/2023
Projec	t Information					

		HCS Two-Lan	e Hi	ghway Re	port	
Proje	ect Information					
Analys	t	James Vorosmarti	Da	ate		7/12/2023
Agenc	у	Tetra Tech	Ar	nalysis Year		2025
Jurisdi	ction		Tir	me Analyzed		Afternoon Peak Hour
Project	t Description	Build Ph1 to LY1 Sellards Road Eastbound (Betwee Route 221 and Tyack Road)		Units		U.S. Customary
		Seg	gmei	nt 1		
Vehi	cle Inputs					
Segme	ent Type	Passing Zone	Le	ngth, ft		5280
Lane V	Vidth, ft	12	Sh	oulder Width, ft	t	2
Speed	Limit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Dem	and and Capacity					·
Directi	ional Demand Flow Rate, veh/h	65	Or	oposing Deman	d Flow Rate, veh/h	53
Peak H	Hour Factor	0.86	То	tal Trucks, %		27.00
Segme	ent Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.04
Inter	rmediate Results					
Segme	ent Vertical Class	1		ee-Flow Speed,	mi/h	53.3
Speed	Slope Coefficient (m)	3.12342	Sp	Speed Power Coefficient (p)		0.59914
PF Slo	pe Coefficient (m)	-1.17848	PF	PF Power Coefficient (p)		0.81005
In Pass	sing Lane Effective Length?	No	То	Total Segment Density, veh/mi/ln		0.1
%lmpr	rovement to Percent Followers	0.0	%I	Improvement to	Speed	0.0
Subs	segment Data					
# !	Segment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1	Tangent	5280	-		-	53.3
Vehi	cle Results					
Averag	ge Speed, mi/h	53.3	Pe	rcent Followers,	, %	12.1
Segme	ent Travel Time, minutes	1.13	Fo	llower Density (	FD), followers/mi/ln	0.1
Vehicle LOS A		А				
Facil	ity Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	14	0.00			0.1	A
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Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/In	LOS
Facility	y Results					
Vehicle LOS A		А				
Segment	Travel Time, minutes	1.13	ı	Follower Density (	FD), followers/mi/ln	0.1
Average	Speed, mi/h	52.9	F	Percent Followers,	%	8.9
Vehicle	e Results					
1 Tar	ngent	5280	-		-	52.9
	gment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
Subse	gment Data					
%Improv	ement to Percent Followers	0.0		%Improvement to Speed		0.0
•	g Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.1
<u> </u>	Coefficient (m)	-1.16665	- 1	PF Power Coefficient (p)		0.81394
Speed Slo	ope Coefficient (m)	3.08657		Speed Power Coefficient (p)		0.61173
Segment	Vertical Class	1		Free-Flow Speed,	mi/h	52.9
Interm	nediate Results					
Segment	Capacity, veh/h	1700	I	Demand/Capacity	(D/C)	0.03
Peak Hour Factor		0.91		Total Trucks, %		40.00
Directional Demand Flow Rate, veh/h		45		Opposing Demand	d Flow Rate, veh/h	36
Demai	nd and Capacity					
Speed Lir	mit, mi/h	50	/	Access Point Dens	ity, pts/mi	0.0
Lane Wid		12		Shoulder Width, ft		2
Segment		Passing Zone		Length, ft		5280
Vehicle	e Inputs					
		Se	egme	ent 1		
Project D	escription	Build Ph1 to LY1 Sellard Road Westbound (between Route 221 an Tyack Road)		Units		U.S. Customary
Jurisdiction				Time Analyzed		Morning Peak Hour
Agency		Tetra Tech	,	Analysis Year		2025
Analyst		James Vorosmarti	[	Date		7/12/2023
Projec	t Information					

		HCS Two-Lar	ne Hi	ighway Re	port	
Proje	ect Information					
Analyst	t	James Vorosmarti	D	ate		7/12/2023
Agency	l .	Tetra Tech	А	nalysis Year		2025
Jurisdic	ction		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY1 Sellard Road Westbound (between Route 221 ar Tyack Road)		Units		U.S. Customary
		Se	gme	ent 1		
Vehic	cle Inputs					
Segme	nt Type	Passing Zone	L	ength, ft		5280
Lane W	/idth, ft	12	S	houlder Width, f	t	2
Speed I	Limit, mi/h	50	А	ccess Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					
Direction	onal Demand Flow Rate, veh/h	63	С	pposing Deman	d Flow Rate, veh/h	77
Peak H	our Factor	0.73	To	otal Trucks, %		20.00
Segme	nt Capacity, veh/h	1700	D	emand/Capacity	/ (D/C)	0.04
Inter	mediate Results					
Segme	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	53.5
Speed :	Slope Coefficient (m)	3.15103	S	Speed Power Coefficient (p)		0.58553
PF Slop	pe Coefficient (m)	-1.19107	Р	PF Power Coefficient (p)		0.80618
In Passi	ing Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.1
%lmpro	ovement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subs	egment Data					
# S	Segment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	53.5
Vehic	cle Results		<u>'</u>			
Average	e Speed, mi/h	53.5	P	ercent Followers,	, %	12.0
Segme	nt Travel Time, minutes	1.12	F	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A		А		,,,,,		
Facili	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	12	0.00			0.1	A
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		HCS Two-Lan	e Hi	ghway Re	port	
Proje	ect Information					
Analys	t	James Vorosmarti	Da	ite		7/12/2023
Agency	у	Tetra Tech	An	alysis Year		2025
Jurisdio	ction		Tir	ne Analyzed		Morning Peak Hour
Project	t Description	Build Ph1 to LY2 Sellards Road Eastbound (between Route 221 and Tyack Road)		Units		U.S. Customary
		Seg	gmei	nt 1		
Vehic	cle Inputs					
Segme	ent Type	Passing Zone	Le	ngth, ft		5280
Lane W	Vidth, ft	12	Sh	oulder Width, f	t	2
Speed	Limit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	0.0
Dem	and and Capacity					
Directi	onal Demand Flow Rate, veh/h	147	Op	pposing Deman	d Flow Rate, veh/h	63
Peak H	lour Factor	0.70	To	tal Trucks, %		29.00
Segme	ent Capacity, veh/h	1700	De	emand/Capacity	/ (D/C)	0.09
Inter	mediate Results					
Segme	ent Vertical Class	1	Fre	ee-Flow Speed,	mi/h	53.2
Speed	Slope Coefficient (m)	3.12618	Sp	Speed Power Coefficient (p)		0.59329
PF Slop	pe Coefficient (m)	-1.18373	PF	PF Power Coefficient (p)		0.80863
In Pass	ing Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.6
%lmpr	ovement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subs	egment Data					
# 5	Segment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 7	Tangent	5280	-		-	52.7
Vehic	cle Results					
Averag	ge Speed, mi/h	52.7	Pe	rcent Followers,	, %	22.2
Segme	ent Travel Time, minutes	1.14	Fo	llower Density (	(FD), followers/mi/ln	0.6
Vehicle LOS A		А				
Facili	ity Results					
Т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	26	0.00			0.6	А
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		HCS Two-Lan	e Hi	ghway Re	port	
Proje	ect Information					
Analyst	t	James Vorosmarti	Da	ate		7/12/2023
Agency	/	Tetra Tech	Ar	nalysis Year		2025
Jurisdic	ction		Tir	me Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY2 Sellards Road Eastbound (Betwe Route 221 and Tyack Road)		Units		U.S. Customary
		Se	gme	nt 1		
Vehic	cle Inputs					
Segme	nt Type	Passing Zone	Le	ngth, ft		5280
Lane W	/idth, ft	12	Sh	oulder Width, f	t	2
Speed	Limit, mi/h	50	Ad	ccess Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					<u> </u>
Direction	onal Demand Flow Rate, veh/h	69	Oı	pposing Deman	d Flow Rate, veh/h	135
Peak H	our Factor	0.86	То	tal Trucks, %		27.00
Segme	nt Capacity, veh/h	1700	De	emand/Capacity	' (D/C)	0.04
Inter	mediate Results					
Segme	nt Vertical Class	1		ee-Flow Speed,	mi/h	53.3
Speed	Slope Coefficient (m)	3.16800	Sp	Speed Power Coefficient (p)		0.56011
PF Slop	pe Coefficient (m)	-1.21428		PF Power Coefficient (p)		0.79985
In Passi	ing Lane Effective Length?	No	То	Total Segment Density, veh/mi/ln		0.2
%lmpro	ovement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subs	egment Data					
# S	Segment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 T	angent	5280	-		-	53.3
Vehic	cle Results					
Averag	e Speed, mi/h	53.3	Pe	ercent Followers,	, %	13.3
Segme	nt Travel Time, minutes	1.13	Fc	ollower Density (	(FD), followers/mi/ln	0.2
Vehicle LOS A		А				
Facili	ity Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/		LOS
1	15	0.00			0.2	A
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		HCS Two-Lai	ne H	ighway Re	port	
Proje	ect Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	1	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		Т	ime Analyzed		Morning Peak Hour
Project	Description	Build Ph1 to LY2 Sellard Road Westbound (between Route 221 ar Tyack Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segme	nt Type	Passing Zone	L	ength, ft		5280
Lane W	/idth, ft	12	S	Shoulder Width, f	t	2
Speed I	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity	·				
Direction	onal Demand Flow Rate, veh/h	48		Opposing Deman	d Flow Rate, veh/h	113
Peak Ho	our Factor	0.91	Т	otal Trucks, %		40.00
Segme	nt Capacity, veh/h	1700		Demand/Capacity	(D/C)	0.03
Inter	mediate Results					
Segme	nt Vertical Class	1		ree-Flow Speed,	mi/h	52.9
Speed S	Slope Coefficient (m)	3.13443	S	Speed Power Coefficient (p)		0.56857
PF Slop	pe Coefficient (m)	-1.20607	F	PF Power Coefficient (p)		0.80268
In Passi	ing Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.1
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# S	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.9
Vehic	cle Results					
Average	e Speed, mi/h	52.9	F	Percent Followers,	, %	10.1
Segme	nt Travel Time, minutes	1.13	F	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A		А				
Facili	ty Results		·			
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	11	0.00			0.1	A
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		HCS Two-Lai	ne H	ighway Re	port	
Proje	ect Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY2 Sellard Road Westbound (between Route 221 ar Tyack Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segme	nt Type	Passing Zone	L	ength, ft		5280
Lane W	/idth, ft	12	S	Shoulder Width, f	t	2
Speed I	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity	·				
Directio	onal Demand Flow Rate, veh/h	159	C	Dpposing Deman	d Flow Rate, veh/h	81
Peak Ho	our Factor	0.73	Т	otal Trucks, %		20.00
Segme	nt Capacity, veh/h	1700	[	Demand/Capacity	/ (D/C)	0.09
Inter	mediate Results					
Segme	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	53.5
Speed S	Slope Coefficient (m)	3.15343	S	Speed Power Coefficient (p)		0.58339
PF Slop	pe Coefficient (m)	-1.19303	P	PF Power Coefficient (p)		0.80563
In Passi	ing Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.7
%lmpro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# S	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.9
Vehic	cle Results					
Average	e Speed, mi/h	52.9	P	Percent Followers	, %	23.7
Segme	nt Travel Time, minutes	1.13	F	Follower Density (	(FD), followers/mi/ln	0.7
Vehicle LOS A		А				
Facili	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	29	0.01			0.7	А
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		HCS Two-La	ne H	ighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	С	Pate		7/12/2023
Agency		Tetra Tech	А	Analysis Year		2025
Jurisdict	tion		Т	ime Analyzed		Morning Peak Hour
Project	Description	No Build Sellards Road Eastbound (between Route 221 and Tyack Road)	d U	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	12	S	Shoulder Width, f	t	2
Speed L	imit, mi/h	50	Δ	Access Point Dens	sity, pts/mi	0.0
Dema	and Capacity	·				·
Directio	nal Demand Flow Rate, veh/h	41	C	Dpposing Deman	d Flow Rate, veh/h	59
Peak Ho	our Factor	0.70	T	otal Trucks, %		29.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacity	(D/C)	0.02
Interr	nediate Results					
Segmen	nt Vertical Class	1		ree-Flow Speed,	mi/h	53.2
Speed S	Slope Coefficient (m)	3.12333	S	Speed Power Coefficient (p)		0.59589
PF Slope	e Coefficient (m)	-1.18136	Р	PF Power Coefficient (p)		0.80930
In Passir	ng Lane Effective Length?	No	T	Total Segment Density, veh/mi/ln		0.1
%lmpro	vement to Percent Followers	0.0	9/	6lmprovement to	Speed	0.0
Subse	egment Data	·				
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	53.2
Vehic	le Results					
Average	e Speed, mi/h	53.2	Р	Percent Followers	, %	8.6
Segmen	nt Travel Time, minutes	1.13	F	follower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A		А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	7	0.00			0.1	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ect Information					
Analyst	t	James Vorosmarti		Date		7/12/2023
Agency	у	Tetra Tech	A	Analysis Year		2025
Jurisdio	ction		Т	ime Analyzed		Afternoon Peak Hour
Project	t Description	No Build Sellards Road Eastbound (Between Route 221 and Tyack Road)	i L	Units		U.S. Customary
		Se	egme	ent 1		
Vehic	cle Inputs					
Segme	ent Type	Passing Zone	L	ength, ft		5280
Lane W	Vidth, ft	12	S	Shoulder Width, f	t	2
Speed	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dem	and and Capacity	·				
Direction	onal Demand Flow Rate, veh/h	65		Dpposing Deman	d Flow Rate, veh/h	49
Peak H	lour Factor	0.86	Т	otal Trucks, %		27.00
Segme	ent Capacity, veh/h	1700		Demand/Capacity	/ (D/C)	0.04
Inter	mediate Results					
Segme	ent Vertical Class	1		ree-Flow Speed,	mi/h	53.3
Speed	Slope Coefficient (m)	3.12005	S	Speed Power Coefficient (p)		0.60227
PF Slop	pe Coefficient (m)	-1.17564	P	PF Power Coefficient (p)		0.81086
In Pass	ing Lane Effective Length?	No	Т	Total Segment Density, veh/mi/ln		0.1
%lmpr	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subs	egment Data					
# 5	Segment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 T	[angent	5280	-		-	53.3
Vehic	cle Results					
Averag	ge Speed, mi/h	53.3	Р	Percent Followers	, %	12.0
Segme	ent Travel Time, minutes	1.13	F	Follower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A		А				
Facili	ity Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.1	A
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		HCS Two-La	ne H	ighway Re	port	
Proj	ect Information					
Analys	st	James Vorosmarti	[	Date		7/12/2023
Agenc	Ey	Tetra Tech	A	Analysis Year		2025
Jurisdi	iction		7	Time Analyzed		Morning Peak Hour
Projec	t Description	No Build Sellards Road Westbound (between Route 221 and Tyack Road)	d (	Units		U.S. Customary
		Se	egme	ent 1		
Vehi	cle Inputs					
Segme	ent Type	Passing Zone	L	ength, ft		5280
Lane V	Nidth, ft	12	9	Shoulder Width, f	t	2
Speed	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dem	and and Capacity	·				
Direct	ional Demand Flow Rate, veh/h	45		Opposing Deman	d Flow Rate, veh/h	32
Peak H	Hour Factor	0.91	7	Total Trucks, %		40.00
Segme	ent Capacity, veh/h	1700	[	Demand/Capacity (D/C)		0.03
Inte	rmediate Results					
Segme	ent Vertical Class	1		ree-Flow Speed,	mi/h	52.9
Speed	Slope Coefficient (m)	3.08266	9	Speed Power Coefficient (p)		0.61548
PF Slo	pe Coefficient (m)	-1.16328		PF Power Coefficient (p)		0.81491
In Pass	sing Lane Effective Length?	No	7	Total Segment Density, veh/mi/ln		0.1
%lmpı	rovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subs	segment Data	·				
#	Segment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1	Tangent	5280	-		-	52.9
Vehi	cle Results		<u>'                                    </u>			
Averag	ge Speed, mi/h	52.9	F	Percent Followers,	, %	8.9
Segme	ent Travel Time, minutes	1.13	F	Follower Density (	(FD), followers/mi/ln	0.1
Vehicle LOS A		А				
Facil	ity Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	10	0.00			0.1	A
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		HCS Two-La	ne H	ighway Re	port	
Proje	ect Information					
Analyst	 :	James Vorosmarti		Date		7/12/2023
Agency	l .	Tetra Tech	A	Analysis Year		2025
Jurisdic	tion		Т	ime Analyzed		Afternoon Peak Hour
Project	Description	No Build Sellards Road Westbound (between Route 221 and Tyack Road)	d L	Units		U.S. Customary
		S	egme	ent 1		
Vehic	le Inputs					
Segme	nt Type	Passing Zone	L	ength, ft		5280
Lane W	/idth, ft	12	S	Shoulder Width, f	t	2
Speed I	Limit, mi/h	50	A	Access Point Dens	sity, pts/mi	0.0
Dema	and and Capacity					·
Directio	onal Demand Flow Rate, veh/h	58		Opposing Demand Flow Rate, veh/h		77
Peak Hour Factor		0.73		Total Trucks, %		20.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.03
Inter	mediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		53.5
Speed Slope Coefficient (m)		3.15103		Speed Power Coefficient (p)		0.58553
PF Slop	pe Coefficient (m)	-1.19107		PF Power Coefficient (p)		0.80618
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.1
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0
Subse	egment Data					
# S	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	53.5
Vehic	cle Results		<u> </u>			
Average	e Speed, mi/h	53.5		Percent Followers, %		11.2
Segment Travel Time, minutes		1.12		Follower Density (FD), followers/mi/ln		0.1
Vehicle LOS		А				
Facili	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS
1	11	0.00			0.1	A
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		HCS Two-Lar	ne Hi	ghway Re	port				
Proje	ct Information								
Analyst		James Vorosmarti	D	Date		7/12/2023			
Agency	,	Tetra Tech	Aı	nalysis Year		2026			
Jurisdic	tion		Ti	me Analyzed		Morning Peak Hour			
Project	Description	PH2 to LY2 Sellards Roa Eastbound (between Route 221 and Tyack Road)	ad U	Units		U.S. Customary			
	Segment 1								
Vehic	le Inputs								
Segmer	nt Type	Passing Zone	Le	ength, ft		5280			
Lane W	idth, ft	12	Sł	houlder Width, f	t	2			
Speed L	Limit, mi/h	50	A	ccess Point Dens	sity, pts/mi	0.0			
Dema	and and Capacity								
Directio	onal Demand Flow Rate, veh/h	139	О	Opposing Demand Flow Rate, veh/h		61			
Peak Ho	our Factor	0.70		Total Trucks, %		29.00			
Segment Capacity, veh/h		1700	D	Demand/Capacity (D/C)		0.08			
Inter	mediate Results								
Segmer	nt Vertical Class	1		Free-Flow Speed, mi/h		53.2			
Speed Slope Coefficient (m)		3.12524	Sp	Speed Power Coefficient (p)		0.59414			
PF Slop	e Coefficient (m)	-1.18295	PF	PF Power Coefficient (p)		0.80885			
In Passing Lane Effective Length?		No	To	Total Segment Density, veh/mi/ln		0.6			
%Improvement to Percent Followers		0.0	%	%Improvement to Speed		0.0			
Subse	egment Data								
# Se	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	52.8			
Vehic	le Results								
Average	e Speed, mi/h	52.8		Percent Followers, %		21.3			
Segment Travel Time, minutes		1.14	Fo	Follower Density (FD), followers/mi/ln		0.6			
Vehicle LOS		А							
Facili	ty Results								
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	24	0.00			0.6	А			
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		HCS Two-Lar	ne Hi	ighway Re	port				
Proje	ct Information								
Analyst		James Vorosmarti	D	Date		7/12/2023			
Agency		Tetra Tech	А	nalysis Year		2026			
Jurisdict	tion		Ti	ime Analyzed		Afternoon Peak Hour			
Project	Description	PH2 to LY2 Sellards Roa Eastbound (Between Route 221 and Tyack Road)	ad U	Units		U.S. Customary			
	Segment 1								
Vehic	le Inputs								
Segmen	nt Type	Passing Zone	Le	ength, ft		5280			
Lane Wi	idth, ft	12	SI	houlder Width, f	t	2			
Speed L	_imit, mi/h	50	А	ccess Point Dens	sity, pts/mi	0.0			
Dema	and and Capacity								
Directio	onal Demand Flow Rate, veh/h	69	0	Opposing Demand Flow Rate, veh/h		128			
Peak Ho	our Factor	0.86		Total Trucks, %		27.00			
Segment Capacity, veh/h		1700	D	Demand/Capacity (D/C)		0.04			
Interr	mediate Results								
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		53.3			
Speed Slope Coefficient (m)		3.16484	Sı	Speed Power Coefficient (p)		0.56272			
PF Slope	e Coefficient (m)	-1.21186		PF Power Coefficient (p)		0.80054			
In Passing Lane Effective Length?		No	To	Total Segment Density, veh/mi/ln		0.2			
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0			
Subse	egment Data					·			
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	53.3			
Vehic	le Results								
Average Speed, mi/h		53.3		Percent Followers, %		13.2			
Segment Travel Time, minutes		1.13		Follower Density (FD), followers/mi/ln		0.2			
Vehicle LOS		А							
Facilit	ty Results								
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	15	0.00			0.2	А			
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		HCS Two-Lar	ne Hi	ighway Re	port				
Proje	ct Information								
Analyst		James Vorosmarti	D	Date		7/12/2023			
Agency	,	Tetra Tech	А	nalysis Year		2026			
Jurisdic	tion		Ti	ime Analyzed		Morning Peak Hour			
Project	Description	PH2 to LY2 Sellards Roa Westbound (between Route 221 and Tyack Road)	ad U	Units		U.S. Customary			
	Segment 1								
Vehic	le Inputs								
Segmer	nt Type	Passing Zone	Le	ength, ft		5280			
Lane W	idth, ft	12	SI	houlder Width, f	t	2			
Speed L	Limit, mi/h	50	А	ccess Point Dens	sity, pts/mi	0.0			
Dema	and and Capacity	·							
Directio	onal Demand Flow Rate, veh/h	47	0	Opposing Demand Flow Rate, veh/h		107			
Peak Ho	our Factor	0.91		Total Trucks, %		40.00			
Segment Capacity, veh/h		1700	D	Demand/Capacity (D/C)		0.03			
Inter	mediate Results								
Segmer	nt Vertical Class	1		Free-Flow Speed, mi/h		52.9			
Speed Slope Coefficient (m)		3.13117		Speed Power Coefficient (p)		0.57134			
PF Slop	e Coefficient (m)	-1.20350		PF Power Coefficient (p)		0.80341			
In Passing Lane Effective Length?		No	To	Total Segment Density, veh/mi/ln		0.1			
%Improvement to Percent Followers		0.0	%	%Improvement to Speed		0.0			
Subse	egment Data	·				·			
# S	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	52.9			
Vehic	le Results								
Average	e Speed, mi/h	52.9		Percent Followers, %		9.8			
Segment Travel Time, minutes		1.13		Follower Density (FD), followers/mi/ln		0.1			
Vehicle LOS		А							
Facili	ty Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	11	0.00			0.1	A			
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	HCS Two-Lane Highway Report								
Proje	ect Information								
Analyst		James Vorosmarti	D	ate		7/12/2023			
Agency	1	Tetra Tech	A	nalysis Year		2026			
Jurisdic	tion		Ti	ime Analyzed		Afternoon Peak Hour			
Project	Description	PH2 to LY2 Sellards Roa Westbound (between Route 221 and Tyack Road)	(between		U.S. Customary				
		Se	gme	nt 1					
Vehic	le Inputs								
Segmer	nt Type	Passing Zone	Le	Length, ft		5280			
Lane W	/idth, ft	12	Sł	Shoulder Width, ft		2			
Speed I	Limit, mi/h	50	A	Access Point Density, pts/mi		0.0			
Dema	and and Capacity	·							
Directio	onal Demand Flow Rate, veh/h	151	0	pposing Deman	d Flow Rate, veh/h	81			
Peak Ho	our Factor	0.73	To	otal Trucks, %		20.00			
Segmer	nt Capacity, veh/h	1700	D	emand/Capacity	/ (D/C)	0.09			
Interi	mediate Results								
Segmer	nt Vertical Class	1	Fr	ree-Flow Speed,	mi/h	53.5			
Speed S	Slope Coefficient (m)	3.15343	Sp	peed Power Coe	fficient (p)	0.58339			
PF Slop	e Coefficient (m)	-1.19303		PF Power Coefficient (p)		0.80563			
In Passi	ing Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.7			
%lmprc	ovement to Percent Followers	0.0	%	%Improvement to Speed		0.0			
Subse	egment Data	·							
# S	egment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	53.0			
Vehic	cle Results								
Average	e Speed, mi/h	53.0	Pe	ercent Followers,	, %	22.9			
	nt Travel Time, minutes	1.13	Fo	ollower Density (	(FD), followers/mi/ln	0.7			
Vehicle	LOS	А							
Facili	ty Results								
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	28	0.01			0.7	А			
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ion	James Vorosmarti  Tetra Tech  No Build - Sellards Roa Eastbound (between Route 221 and Tyack Road)  See  Passing Zone	Tin	alysis Year ne Analyzed its		7/12/2023 2026 Morning Peak Hour U.S. Customary
	Tetra Tech  No Build - Sellards Roa Eastbound (between Route 221 and Tyack Road)  Se	An Tin d Un	alysis Year ne Analyzed its		2026 Morning Peak Hour
	No Build - Sellards Roa Eastbound (between Route 221 and Tyack Road)	Tin d Un	ne Analyzed its		Morning Peak Hour
	Eastbound (between Route 221 and Tyack Road)	d Un	its		-
	Eastbound (between Route 221 and Tyack Road)				U.S. Customary
		gmer	nt 1		
	Passing Zone				
	Passing Zone	-			
		Ler	Length, ft		5280
	12	Sho	oulder Width, ft	t	2
	50	Ace	Access Point Density, pts/mi		0.0
acity					
w Rate, veh/h	41	Ор	posing Deman	d Flow Rate, veh/h	59
	0.70	Tot	Total Trucks, %		29.00
h	1700	De	mand/Capacity	(D/C)	0.02
sults	·				·
	1	Fre	e-Flow Speed,	mi/h	53.2
: (m)	3.12333	Spe	eed Power Coef	fficient (p)	0.59589
	-1.18136	PF	PF Power Coefficient (p)		0.80930
e Length?	No	Total Segment Density, veh/mi/ln		0.1	
ent Followers	0.0	%lı	nprovement to	Speed	0.0
a	•				
	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
	5280	-		-	53.2
	53.2	Per	cent Followers,	. %	8.6
Average Speed, mi/h Segment Travel Time, minutes		Fol	lower Density (	FD), followers/mi/ln	0.1
	А				
	VHD veh-h/p				LOS
	0.00			0.1	A
	VMT h-mi/p	53.2 sinutes 1.13 A  VMT h-mi/p VHD veh-h/p 7 0.00	5280 -  5280 -  53.2 Per  inutes 1.13 Fol  A  VMT h-mi/p VHD veh-h/p  7 0.00	53.2 Percent Followers, inutes 1.13 Follower Density (  A  VMT VHD veh-h/p  7 0.00	53.2 Percent Followers, %  inutes 1.13 Follower Density (FD), followers/mi/ln  A  VMT h-mi/p Veh-h/p Follower Density, followers/  7 0.00 0.1

	HCS Two-Lane Highway Report								
Proje	ect Information								
Analyst		James Vorosmarti	С	Pate		7/12/2023			
Agency	1	Tetra Tech	Α	nalysis Year		2026			
Jurisdic	tion		Т	ime Analyzed		Afternoon Peak Hour			
Project	Description	No Build Sellards Road Eastbound (Between Route 221 and Tyack Road)	d U	Jnits		U.S. Customary			
		Se	egme	ent 1					
Vehic	le Inputs								
Segmer	nt Type	Passing Zone	g Zone Length, ft		5280				
Lane W	/idth, ft	12	S	houlder Width, f	t	2			
Speed l	Limit, mi/h	50	Access Point Density, pts/mi		0.0				
Dema	and and Capacity								
Directio	onal Demand Flow Rate, veh/h	66	C	Opposing Deman	d Flow Rate, veh/h	49			
Peak Ho	our Factor	0.86	T	otal Trucks, %		27.00			
Segmer	nt Capacity, veh/h	1700	0	Demand/Capacity	/ (D/C)	0.04			
Interi	mediate Results								
Segmer	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	53.3			
Speed S	Slope Coefficient (m)	3.12005	S	peed Power Coe	fficient (p)	0.60227			
PF Slop	e Coefficient (m)	-1.17564		PF Power Coefficient (p)		0.81086			
In Passi	ing Lane Effective Length?	No	T	Total Segment Density, veh/mi/ln		0.2			
%lmprc	ovement to Percent Followers	0.0	9/	%Improvement to Speed		0.0			
Subse	egment Data	•							
# S	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	53.3			
Vehic	cle Results								
Average	e Speed, mi/h	53.3	Р	ercent Followers	, %	12.2			
	nt Travel Time, minutes	1.13	F	ollower Density (	(FD), followers/mi/ln	0.2			
Vehicle	Vehicle LOS A								
Facili	ty Results								
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	14	0.00			0.2	А			
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	HCS Two-Lane Highway Report								
Proje	ect Information								
Analyst	t	James Vorosmarti	С	Pate		7/12/2023			
Agency	/	Tetra Tech	Α	Analysis Year		2026			
Jurisdic	ction		Т	ime Analyzed		Morning Peak Hour			
Project	Description	No Build Sellards Road Westbound (between Route 221 and Tyack Road)	tbound (between te 221 and Tyack		U.S. Customary				
		Se	egme	ent 1					
Vehic	cle Inputs								
Segme	nt Type	Passing Zone	Length, ft		5280				
Lane W	/idth, ft	12	S	shoulder Width, f	t	2			
Speed I	Limit, mi/h	50	Access Point Density, pts/mi		0.0				
Dema	and and Capacity	·				·			
Directio	onal Demand Flow Rate, veh/h	45	С	Dpposing Deman	d Flow Rate, veh/h	32			
Peak Ho	our Factor	0.91	T	otal Trucks, %		40.00			
Segme	nt Capacity, veh/h	1700	С	Demand/Capacity	/ (D/C)	0.03			
Inter	mediate Results								
Segmei	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	52.9			
Speed S	Slope Coefficient (m)	3.08266	S	peed Power Coe	fficient (p)	0.61548			
PF Slop	pe Coefficient (m)	-1.16328		PF Power Coefficient (p)		0.81491			
In Passi	ing Lane Effective Length?	No	T	Total Segment Density, veh/mi/ln		0.1			
%lmpro	ovement to Percent Followers	0.0	9/	%Improvement to Speed		0.0			
Subse	egment Data	·				·			
# S	Segment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	52.9			
Vehic	cle Results								
Average	e Speed, mi/h	52.9	Р	Percent Followers	, %	8.9			
	nt Travel Time, minutes	1.13	F	ollower Density (	(FD), followers/mi/ln	0.1			
Vehicle	Vehicle LOS A								
Facili	ty Results								
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	10	0.00			0.1	А			
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	HCS Two-Lane Highway Report								
Proje	ct Information								
Analyst		James Vorosmarti	D	ate		7/12/2023			
Agency		Tetra Tech	А	Analysis Year		2026			
Jurisdict	tion		Т	ime Analyzed		Afternoon Peak Hour			
Project	Description	No Build Sellards Road Westbound (between Route 221 and Tyack Road)	oound (between 221 and Tyack		U.S. Customary				
		Se	egme	ent 1					
Vehic	le Inputs								
Segmer	nt Type	Passing Zone	Length, ft		5280				
Lane W	idth, ft	12	S	houlder Width, f	t	2			
Speed L	_imit, mi/h	50	Access Point Density, pts/mi		0.0				
Dema	and and Capacity								
Directio	onal Demand Flow Rate, veh/h	58	С	pposing Deman	d Flow Rate, veh/h	78			
Peak Ho	our Factor	0.73	0.73 Total Truck			20.00			
Segmer	nt Capacity, veh/h	1700	D	emand/Capacity	/ (D/C)	0.03			
Interr	mediate Results	<u>'</u>							
Segmer	nt Vertical Class	1	F	ree-Flow Speed,	mi/h	53.5			
Speed S	Slope Coefficient (m)	3.15184	S	peed Power Coe	fficient (p)	0.58481			
PF Slope	e Coefficient (m)	-1.19173		PF Power Coefficient (p)		0.80599			
In Passii	ng Lane Effective Length?	No	To	Total Segment Density, veh/mi/ln		0.1			
%lmpro	ovement to Percent Followers	0.0	%	%Improvement to Speed		0.0			
Subse	egment Data								
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	53.5			
Vehic	le Results								
Average	e Speed, mi/h	53.5	P	ercent Followers,	, %	11.2			
	nt Travel Time, minutes	1.12	F	ollower Density (	(FD), followers/mi/ln	0.1			
Vehicle	Vehicle LOS A								
Facilit	ty Results								
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	11	0.00			0.1	А			
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HCS Summary Travis Road

	HCS Two-Lane Highway Report								
Proj	ject Information								
Analy	vst	James Vorosmarti		Date		7/12/2023			
Agen	су	Tetra Tech		Analysis Year		2023			
Jurisc	diction			Time Analyzed		Morning Peak Hour			
Proje	ct Description	Travis Road Northboun (North of Sellards Road		Units		U.S. Customary			
		Se	egm	ent 1					
Veh	icle Inputs								
Segm	nent Type	Passing Zone		Length, ft		5280			
Lane	Width, ft	11		Shoulder Width, f	t	2			
Speed	d Limit, mi/h	50		Access Point Dens	sity, pts/mi	2.0			
Den	nand and Capacity								
Direc	tional Demand Flow Rate, veh/h	24		Opposing Deman	d Flow Rate, veh/h	65			
Peak	c Hour Factor 0.54 Total Trucks, %			38.00					
Segment Capacity, veh/h 1700		1700		Demand/Capacity	/ (D/C)	0.01			
Inte	rmediate Results					<u> </u>			
Segm	nent Vertical Class	1		Free-Flow Speed,	mi/h	51.8			
Speed	d Slope Coefficient (m)	3.05159		Speed Power Coe	fficient (p)	0.59213			
PF Slo	ope Coefficient (m)	-1.18671		PF Power Coefficie	ent (p)	0.80524			
In Pas	ssing Lane Effective Length?	No		Total Segment De	nsity, veh/mi/ln	0.0			
%lmp	provement to Percent Followers	0.0	0.0 %Improvement		Speed	0.0			
Sub	segment Data								
#	Segment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h			
1	Tangent	5280	-		-	51.8			
Veh	icle Results								
Avera	age Speed, mi/h	51.8		Percent Followers	, %	5.7			
Segm	nent Travel Time, minutes	1.16		Follower Density (	(FD), followers/mi/ln	0.0			
Vehic	le LOS	А							
Faci	lity Results								
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS			
1	3	0.00			0.0	A			
opyrio	aht © 2023 University of Florida. All Rights	Posoniod LCS TIM L	liahway	vs Version 2022		Generated: 07/12/2023 10:42			

		HCS Two-Lai	ne H	lighway Re	port	
Proje	ect Information					
Analys	t	James Vorosmarti	ı	Date		7/12/2023
Agency	у	Tetra Tech	,	Analysis Year		2023
Jurisdi	ction		-	Time Analyzed		Afternoon Peak Hour
Project	t Description	Travis Road Northbour (North of Sellards Road				U.S. Customary
		Se	egmo	ent 1		<u> </u>
Vehi	cle Inputs					
Segme	ent Type	Passing Zone	Ι	Length, ft		5280
Lane V	Vidth, ft	11	9	Shoulder Width, f	t	2
Speed	Limit, mi/h	50	/	Access Point Density, pts/mi		2.0
Dem	and and Capacity					
Directi	onal Demand Flow Rate, veh/h	72	(	Opposing Deman	d Flow Rate, veh/h	26
Peak H	lour Factor	0.76	Total Trucks, %		20.00	
Segme	ent Capacity, veh/h	1700	ı	Demand/Capacity (D/C)		0.04
Inter	mediate Results					·
Segme	ent Vertical Class	1	F	Free-Flow Speed,	mi/h	52.4
Speed	Slope Coefficient (m)	3.05380	9	Speed Power Coe	fficient (p)	0.62067
PF Slop	oe Coefficient (m)	-1.16162	ı	PF Power Coefficie	ent (p)	0.81179
In Pass	ing Lane Effective Length?	No	1	Total Segment De	nsity, veh/mi/ln	0.2
%lmpr	ovement to Percent Followers	0.0	Ç	%Improvement to Speed		0.0
Subs	egment Data	•				<u> </u>
# 9	Segment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h
1 7	Tangent	5280	-		-	52.4
Vehic	cle Results					
Averag	ge Speed, mi/h	52.4	F	Percent Followers,	, %	12.9
Segme	ent Travel Time, minutes	1.14	F	Follower Density (	(FD), followers/mi/ln	0.2
Vehicle	e LOS	A				
Facil	ity Results					·
т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.2	A
	t @ 2022 University of Florida, All Biobts			Norsian 2022		Concreted: 07/12/2022 10:42:1

		HCS Two-La	ne ŀ	Highway Re	port	
Projec	t Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech		Analysis Year		2023
Jurisdicti	ion			Time Analyzed		Morning Peak Hour
Project D	Description	Travis Road Southbour (North of Sellards Road				U.S. Customary
		Sc	egm	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wid	dth, ft	11		Shoulder Width, f	t	2
Speed Li	mit, mi/h	50		Access Point Density, pts/mi		2.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	60		Opposing Deman	d Flow Rate, veh/h	22
Peak Ho	ur Factor	0.58		Total Trucks, %		3.00
Segment Capacity, veh/h		1700		Demand/Capacity	/ (D/C)	0.04
Intern	nediate Results	•				
Segment	t Vertical Class	1		Free-Flow Speed,	mi/h	53.0
Speed SI	ope Coefficient (m)	3.08038		Speed Power Coe	fficient (p)	0.62469
PF Slope	Coefficient (m)	-1.15869		PF Power Coefficie	ent (p)	0.81203
In Passin	g Lane Effective Length?	No	Total Segment Density, veh/mi/ln		0.1	
%Improv	vement to Percent Followers	0.0		%Improvement to Speed		0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	53.0
Vehicl	e Results				-	
Average	Speed, mi/h	53.0		Percent Followers	, %	11.2
Segment	t Travel Time, minutes	1.13		Follower Density (	(FD), followers/mi/ln	0.1
Vehicle L	.OS	А		,, ,,		
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	9	0.00			0.1	A
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		HCS Two-La	ne ŀ	Highway Re	port	
Projec	t Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech		Analysis Year		2023
Jurisdicti	on			Time Analyzed		Afternoon Peak Hour
Project D	Description	Travis Road Southbour (North of Sellards Road		Units		U.S. Customary
		Se	egm	ent 1		
Vehicl	e Inputs					
Segment	t Type	Passing Zone		Length, ft		5280
Lane Wid	dth, ft	11		Shoulder Width, f	t	2
Speed Li	mit, mi/h	50		Access Point Density, pts/mi		2.0
Dema	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	24		Opposing Deman	d Flow Rate, veh/h	66
Peak Ho	ur Factor	0.83		Total Trucks, %		10.00
Segment Capacity, veh/h		1700		Demand/Capacity	' (D/C)	0.01
Intern	nediate Results					·
Segment	t Vertical Class	1		Free-Flow Speed,	mi/h	52.8
Speed SI	ope Coefficient (m)	3.10305		Speed Power Coe	fficient (p)	0.59129
PF Slope	Coefficient (m)	-1.18865		PF Power Coefficie	ent (p)	0.80374
In Passin	g Lane Effective Length?	No		Total Segment Density, veh/mi/ln		0.0
%lmprov	vement to Percent Followers	0.0		%Improvement to Speed		0.0
Subse	gment Data					
# Se	gment Type	Length, ft	Radi	us, ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	52.8
Vehicl	e Results					
Average	Speed, mi/h	52.8		Percent Followers	, %	5.8
Segment	t Travel Time, minutes	1.14		Follower Density (	(FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A
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		HCS Two-Lar	ne H	ighway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2025
Jurisdict	ion		Т	ime Analyzed		Morning Peak Hour
Project [	Description	Build Ph1 to LY1 Travis Road Northbound (Nor of Sellards Road)		Units		U.S. Customary
		Se	egme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	Shoulder Width, ft	t	2
Speed L	imit, mi/h	50	A	Access Point Density, pts/mi		2.0
Dema	and Capacity					<u> </u>
Directio	nal Demand Flow Rate, veh/h	24	C	Opposing Deman	d Flow Rate, veh/h	67
Peak Ho	our Factor	0.54	Т	otal Trucks, %		38.00
Segmen	t Capacity, veh/h	1700	С	Demand/Capacity (D/C)		0.01
Intern	nediate Results					<u> </u>
Segmen	t Vertical Class	1	F	ree-Flow Speed,	mi/h	51.8
Speed S	lope Coefficient (m)	3.05277	S	Speed Power Coef	fficient (p)	0.59106
PF Slope	e Coefficient (m)	-1.18769	P	PF Power Coefficient (p)		0.80496
In Passir	ng Lane Effective Length?	No	Т	otal Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	9	%Improvement to Speed		0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	51.8
Vehicl	le Results					
Average	Speed, mi/h	51.8	P	Percent Followers,	, %	5.7
Segmen	t Travel Time, minutes	1.16	F	follower Density (	FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	3	0.00			0.0	A
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	HCS Two-Lane Highway Report							
Projec	ct Information							
Analyst		James Vorosmarti	Dat	te		7/12/2023		
Agency		Tetra Tech	Ana	alysis Year		2025		
Jurisdict	tion		Tim	Time Analyzed		Afternoon Peak Hour		
Project I	Description	Build Ph1 to LY1 Travis Road Northbound (Nor of Sellards Road)		Units		U.S. Customary		
		Se	gmen	nt 1				
Vehic	le Inputs							
Segmen	t Type	Passing Zone	Length, ft		5280			
Lane Wi	dth, ft	11	Sho	oulder Width, f	t	2		
Speed L	imit, mi/h	50	Acc	Access Point Density, pts/mi		2.0		
Dema	and Capacity					·		
Direction	nal Demand Flow Rate, veh/h	74	Ор	posing Deman	d Flow Rate, veh/h	26		
Peak Ho	our Factor	0.76	Tot	al Trucks, %		20.00		
Segmen	t Capacity, veh/h	1700	Dei	Demand/Capacity (D/C)		0.04		
Intern	nediate Results							
Segmen	nt Vertical Class	1	Fre	e-Flow Speed,	mi/h	52.4		
Speed S	lope Coefficient (m)	3.05380	Spe	Speed Power Coefficient (p)		0.62067		
PF Slope	e Coefficient (m)	-1.16162	PF	PF Power Coefficient (p)		0.81179		
In Passir	ng Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.2		
%Impro	vement to Percent Followers	0.0	%lr	mprovement to	Speed	0.0		
Subse	egment Data							
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h		
1 Ta	ingent	5280	-		-	52.4		
Vehic	le Results					·		
Average	Speed, mi/h	52.4	Per	cent Followers	, %	13.0		
Segmen	Segment Travel Time, minutes 1.14		Fol	lower Density (	(FD), followers/mi/ln	0.2		
Vehicle I	LOS	А						
Facilit	ty Results							
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS		
1	14	0.00			0.2	A		
	17	0.00			J.L	^		

HCS Two-Lane Highway Report							
Project	t Information						
Analyst		James Vorosmarti	Da	te		7/12/2023	
Agency		Tetra Tech	An	alysis Year		2025	
Jurisdictio	on		Tin	Time Analyzed		Morning Peak Hour	
Project De	escription	Build Ph1 to LY1 Travis Road Southbound (Nor of Sellards Road)		Units		U.S. Customary	
		Se	gmer	nt 1			
Vehicle	e Inputs						
Segment	Туре	Passing Zone	Length, ft		5280		
Lane Wid	lth, ft	11	Sh	oulder Width, f	t	2	
Speed Lin	mit, mi/h	50	Ac	Access Point Density, pts/mi		2.0	
Deman	nd and Capacity						
Direction	al Demand Flow Rate, veh/h	62	Ор	posing Deman	d Flow Rate, veh/h	22	
Peak Hou	ır Factor	0.58	Tot	al Trucks, %		3.00	
Segment	Capacity, veh/h	1700	Demand/Capacity (D/C)		0.04		
Interm	nediate Results						
Segment	Vertical Class	1	Fre	e-Flow Speed,	mi/h	53.0	
Speed Slo	ope Coefficient (m)	3.08038	Sp	Speed Power Coefficient (p)		0.62469	
PF Slope	Coefficient (m)	-1.15869	PF	PF Power Coefficient (p)		0.81203	
In Passing	g Lane Effective Length?	No	Tot	Total Segment Density, veh/mi/ln		0.1	
%Improve	ement to Percent Followers	0.0	%I	mprovement to	Speed	0.0	
Subseg	gment Data						
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h	
1 Tan	ngent	5280	-		-	53.0	
Vehicle	e Results						
Average S	Speed, mi/h	53.0	Pei	cent Followers	, %	11.4	
Segment	Travel Time, minutes	1.13	Fol	lower Density (	FD), followers/mi/ln	0.1	
Vehicle LC	OS	А					
Facility	y Results						
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS	
1	9	0.00			0.1	A	

		HCS Two-Lar	ne H	lighway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	1	Date		7/12/2023
Agency	,	Tetra Tech	A	Analysis Year		2025
Jurisdict	tion		1	Гime Analyzed		Afternoon Peak Hour
Project	Description	Build Ph1 to LY1 Travis Road Southbound (Nor of Sellards Road)		Jnits		U.S. Customary
		Se	gme	ent 1		
Vehic	le Inputs					
Segmer	nt Type	Passing Zone	Τι	ength, ft		5280
Lane Wi	idth, ft	11	9	Shoulder Width, f	t	2
Speed L	_imit, mi/h	50	1	Access Point Dens	sity, pts/mi	2.0
Dema	and and Capacity					·
Directio	onal Demand Flow Rate, veh/h	24		Opposing Deman	d Flow Rate, veh/h	67
Peak Ho	our Factor	0.83	7	Total Trucks, %		10.00
Segmer	nt Capacity, veh/h	1700		Demand/Capacity	' (D/C)	0.01
Interr	mediate Results					·
Segmer	nt Vertical Class	1	F	Free-Flow Speed, mi/h		52.8
Speed S	Slope Coefficient (m)	3.10382		Speed Power Coefficient (p)		0.59060
PF Slop	e Coefficient (m)	-1.18929	F	PF Power Coefficie	ent (p)	0.80356
In Passii	ng Lane Effective Length?	No	1	Total Segment De	nsity, veh/mi/ln	0.0
%Impro	ovement to Percent Followers	0.0	9	%Improvement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.8
Vehic	le Results					
Average	e Speed, mi/h	52.8	F	Percent Followers,	, %	5.8
Segmer	Segment Travel Time, minutes 1.14		F	ollower Density (	(FD), followers/mi/ln	0.0
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A
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		HCS Two-Lar	ne Hig	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Dat	:e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdict	tion		Tim	ne Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY2 Travis Road Northbound (Nor of Sellards Road)	Uni th	ts		U.S. Customary
		Se	gmen	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	idth, ft	11	Sho	oulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	2.0
Dema	and Capacity					·
Directio	nal Demand Flow Rate, veh/h	28	Ор	posing Deman	d Flow Rate, veh/h	189
Peak Ho	our Factor	0.54	Tot	al Trucks, %		38.00
Segmen	nt Capacity, veh/h	1700	Der	mand/Capacity	(D/C)	0.02
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	Free-Flow Speed, mi/h		51.8
Speed S	Slope Coefficient (m)	3.11060	Spe	eed Power Coe	fficient (p)	0.54243
PF Slope	e Coefficient (m)	-1.23310	PF	Power Coefficie	ent (p)	0.79216
In Passir	ng Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%lr	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, 1	ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	51.8
Vehic	le Results					<u>'</u>
Average	e Speed, mi/h	51.8	Per	cent Followers,	, %	7.0
Segment Travel Time, minutes 1.16		Fol	lower Density (	(FD), followers/mi/ln	0.0	
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A

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Project				ghway Re	P	
_	Information					
Analyst		James Vorosmarti	Dat	:e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdictio	n		Tim	ne Analyzed		Afternoon Peak Hour
Project De	escription	Build Ph1 to LY2 Travis Road Northbound (Nor of Sellards Road)	Uni	ts		U.S. Customary
		Se	gmen	nt 1		
Vehicle	Inputs					
Segment 1	Гуре	Passing Zone	Ler	gth, ft		5280
Lane Widt	h, ft	11	Sho	oulder Width, f	t	2
Speed Lim	nit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	2.0
Deman	d and Capacity					
Directiona	I Demand Flow Rate, veh/h	161	Ор	posing Deman	d Flow Rate, veh/h	29
Peak Hour	Factor	0.76	Tot	al Trucks, %		20.00
Segment (	Capacity, veh/h	1700	Dei	mand/Capacity	(D/C)	0.09
Interme	ediate Results					·
Segment \	Vertical Class	1	Free-Flow Speed, mi/h		52.4	
Speed Slo	pe Coefficient (m)	3.05639	Spe	eed Power Coe	fficient (p)	0.61814
PF Slope C	Coefficient (m)	-1.16389	PF	Power Coefficie	ent (p)	0.81114
In Passing	Lane Effective Length?	No	Tot	al Segment De	nsity, veh/mi/ln	0.7
%Improve	ment to Percent Followers	0.0	%Ir	nprovement to	Speed	0.0
Subseg	ment Data					
# Segr	ment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tang	gent	5280	-		-	51.9
Vehicle	Results					·
Average S	peed, mi/h	51.9	Per	cent Followers,	, %	23.2
Segment Travel Time, minutes 1.16		Fol	lower Density (	(FD), followers/mi/ln	0.7	
Vehicle LO	)S	А				
Facility	Results		·			
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	31	0.01			0.7	A

		HCS Two-Lar	ne Hig	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Dat	:e		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2025
Jurisdict	tion		Tim	ne Analyzed		Morning Peak Hour
Project I	Description	Build Ph1 to LY2 Travis Road Southbound (Nor of Sellards Road)		Units		U.S. Customary
		Se	gmen	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Len	gth, ft		5280
Lane Wi	idth, ft	11	Sho	oulder Width, f	t	2
Speed L	imit, mi/h	50	Acc	ess Point Dens	sity, pts/mi	2.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	176	Ор	posing Deman	d Flow Rate, veh/h	26
Peak Ho	our Factor	0.58	Tota	al Trucks, %		3.00
Segmen	nt Capacity, veh/h	1700	Der	mand/Capacity	/ (D/C)	0.10
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	Free-Flow Speed, mi/h		53.0
Speed S	Slope Coefficient (m)	3.08402	Spe	eed Power Coe	fficient (p)	0.62112
PF Slope	e Coefficient (m)	-1.16189	PF I	Power Coefficie	ent (p)	0.81111
In Passir	ng Lane Effective Length?	No	Tota	al Segment De	nsity, veh/mi/ln	0.8
%Impro	vement to Percent Followers	0.0	%lr	nprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius, f	ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.4
Vehic	le Results					<u>'</u>
Average	e Speed, mi/h	52.4	Per	cent Followers	, %	24.7
Segment Travel Time, minutes 1.15		1.15	Foll	lower Density (	(FD), followers/mi/ln	0.8
Vehicle	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	26	0.01			0.8	A

		HCS Two-Lar	ne Hig	ghway Re	port	
Project	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdictio	on		Tir	ne Analyzed		Afternoon Peak Hour
Project Do	escription	Build Ph1 to LY2 Travis Road Southbound (Nor of Sellards Road)		Units		U.S. Customary
		Se	gmer	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Lei	ngth, ft		5280
Lane Wid	th, ft	11	Sh	oulder Width, f	t	2
Speed Lin	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	2.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	27	Op	posing Deman	d Flow Rate, veh/h	147
Peak Hou	ır Factor	0.83	To	tal Trucks, %		10.00
Segment	Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.02
Interm	ediate Results					
Segment	Vertical Class	1	Fre	Free-Flow Speed, mi/h		52.8
Speed Slo	ope Coefficient (m)	3.14435	Sp	eed Power Coe	fficient (p)	0.55578
PF Slope	Coefficient (m)	-1.22152	PF	Power Coefficie	ent (p)	0.79447
In Passing	g Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.0
%Improve	ement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subseç	gment Data					
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tan	gent	5280	-		-	52.8
Vehicle	Results					
Average S	Speed, mi/h	52.8	Pe	rcent Followers	, %	6.6
Segment Travel Time, minutes 1.14		1.14	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle LC	OS	А				
Facility	/ Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
		0.00		_	0.0	A

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		HCS Two-La	ne Hi	ghway Re	port	
Proje	ct Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	An	alysis Year		2025
Jurisdict	tion		Tir	me Analyzed		Afternoon Peak Hour
Project I	Description	No Build Travis Road Northbound (North of Sellards Road)		nits		U.S. Customary
		Se	egmei	nt 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	Le	ngth, ft		5280
Lane Wi	idth, ft	11	Sh	oulder Width, f	t	2
Speed L	imit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	2.0
Dema	and Capacity					
Directio	nal Demand Flow Rate, veh/h	74	Op	pposing Deman	d Flow Rate, veh/h	26
Peak Ho	our Factor	0.76	To	tal Trucks, %		20.00
Segmen	nt Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.04
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	Free-Flow Speed, mi/h		52.4
Speed S	Slope Coefficient (m)	3.05380	Sp	eed Power Coe	fficient (p)	0.62067
PF Slope	e Coefficient (m)	-1.16162	PF	Power Coefficie	ent (p)	0.81179
In Passir	ng Lane Effective Length?	No	To	tal Segment De	nsity, veh/mi/ln	0.2
%Impro	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.4
Vehic	le Results					'
Average	e Speed, mi/h	52.4	Pe	rcent Followers	, %	13.0
Segment Travel Time, minutes 1.14		1.14	Fo	llower Density (	(FD), followers/mi/ln	0.2
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	14	0.00			0.2	A

		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	D	ate		7/12/2023
Agency		Tetra Tech	Aı	nalysis Year		2025
Jurisdictio	on		Ti	me Analyzed		Morning Peak Hour
Project D	escription	No Build Travis Road Southbound (North o Sellards Road)		nits		U.S. Customary
		S	egme	nt 1		
Vehicle	e Inputs					
Segment	Туре	Passing Zone	Le	ength, ft		5280
Lane Wid	lth, ft	11	Sł	houlder Width, f	t	2
Speed Lir	mit, mi/h	50	A	ccess Point Dens	sity, pts/mi	2.0
Demar	nd and Capacity					
Direction	al Demand Flow Rate, veh/h	62	О	pposing Deman	d Flow Rate, veh/h	22
Peak Hou	ur Factor	0.58	To	otal Trucks, %		3.00
Segment	Capacity, veh/h	1700	D	emand/Capacity	(D/C)	0.04
Interm	nediate Results					
Segment	Vertical Class	1		Free-Flow Speed, mi/h		53.0
Speed Sk	ope Coefficient (m)	3.08038	Sp	peed Power Coe	fficient (p)	0.62469
PF Slope	Coefficient (m)	-1.15869	PF	F Power Coefficie	ent (p)	0.81203
In Passino	g Lane Effective Length?	No	To	otal Segment De	nsity, veh/mi/ln	0.1
%Improv	ement to Percent Followers	0.0	%	Improvement to	Speed	0.0
Subse	gment Data					
# Seg	gment Type	Length, ft	Radius	, ft	Superelevation, %	Average Speed, mi/h
1 Tan	ngent	5280	-		-	53.0
Vehicle	e Results					
Average S	Speed, mi/h	53.0	Pe	ercent Followers	, %	11.4
Segment Travel Time, minutes 1.13		1.13	Fo	ollower Density (	(FD), followers/mi/ln	0.1
Vehicle L0	OS	А				
Facility	y Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
	I	1				

		HCS Two-La	ne Hi	ghway Re	port	
Projec	ct Information					
Analyst		James Vorosmarti	Da	ite		7/12/2023
Agency		Tetra Tech	Ar	alysis Year		2025
Jurisdict	tion		Tir	me Analyzed		Afternoon Peak Hour
Project [	Description	No Build Travis Road Southbound (North of Sellards Road)		nits		U.S. Customary
		Se	egmei	nt 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	Le	ngth, ft		5280
Lane Wi	dth, ft	11	Sh	oulder Width, f	t	2
Speed L	imit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	2.0
Dema	and Capacity					
Direction	nal Demand Flow Rate, veh/h	24	Op	pposing Deman	d Flow Rate, veh/h	67
Peak Ho	our Factor	0.83	То	tal Trucks, %		10.00
Segmen	t Capacity, veh/h	1700	De	emand/Capacity	(D/C)	0.01
Intern	nediate Results					
Segmen	nt Vertical Class	1	Fre	Free-Flow Speed, mi/h		52.8
Speed S	lope Coefficient (m)	3.10382	Sp	eed Power Coe	fficient (p)	0.59060
PF Slope	e Coefficient (m)	-1.18929	PF	F Power Coefficient (p)		0.80356
In Passir	ng Lane Effective Length?	No	То	tal Segment De	nsity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	52.8
Vehicl	le Results					'
Average	Speed, mi/h	52.8	Pe	rcent Followers	, %	5.8
Segment Travel Time, minutes 1.14		1.14	Fo	llower Density (	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/AP	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A

		HCS Two-La	ne H	ighway Re	eport	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2026
Jurisdict	ion		Т	Time Analyzed		Morning Peak Hour
Project [	Description	Ph2 to LY2 Travis Road Northbound (North of Sellards Road)		Jnits		U.S. Customary
		S	egme	ent 1		
Vehicl	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	Shoulder Width, f	t	2
Speed L	imit, mi/h	50	A	Access Point Den	sity, pts/mi	2.0
Dema	and Capacity					·
Direction	nal Demand Flow Rate, veh/h	28		Dpposing Deman	nd Flow Rate, veh/h	176
Peak Ho	our Factor	0.54	Т	Total Trucks, %		38.00
Segmen	t Capacity, veh/h	1700		Demand/Capacity	y (D/C)	0.02
Intern	nediate Results	•				·
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		51.8
Speed S	ilope Coefficient (m)	3.10563	3.10563		efficient (p)	0.54632
PF Slope	e Coefficient (m)	-1.22942	F	PF Power Coefficient (p)		0.79320
In Passir	ng Lane Effective Length?	No	Т	Total Segment De	ensity, veh/mi/ln	0.0
%Impro	vement to Percent Followers	0.0	9	%Improvement to	o Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ingent	5280	-		-	51.8
Vehicl	le Results					·
Average	e Speed, mi/h	51.8	F	Percent Followers	5, %	6.9
3 1		1.16	F	Follower Density	(FD), followers/mi/ln	0.0
Vehicle I	LOS	А				
Facilit	ty Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	4	0.00			0.0	A
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		HCS Two-La	ne H	ighway Re	eport	
Projec	ct Information					
Analyst		James Vorosmarti		Date		7/12/2023
Agency		Tetra Tech	Δ	Analysis Year		2026
Jurisdict	ion		Т	ime Analyzed		Afternoon Peak Hour
Project [	Description	PH2 to LY2 Travis Road Northbound (North of Sellards Road)		Jnits		U.S. Customary
		Sc	egme	ent 1		
Vehicl	le Inputs					
Segmen	t Type	Passing Zone	L	ength, ft		5280
Lane Wi	dth, ft	11	S	Shoulder Width, f	t	2
Speed Li	imit, mi/h	50	A	Access Point Den	sity, pts/mi	2.0
Dema	nd and Capacity					·
Direction	nal Demand Flow Rate, veh/h	153		Dpposing Deman	nd Flow Rate, veh/h	30
Peak Ho	our Factor	0.76	Т	otal Trucks, %		20.00
Segmen	t Capacity, veh/h	1700		Demand/Capacity	y (D/C)	0.09
Intern	nediate Results	•				
Segmen	t Vertical Class	1	Free-Flow Speed, mi/h		52.4	
Speed S	lope Coefficient (m)	3.05765	S	Speed Power Coefficient (p)		0.61692
PF Slope	e Coefficient (m)	-1.16498	Р	PF Power Coeffici	ent (p)	0.81083
In Passir	ng Lane Effective Length?	No	Т	otal Segment De	ensity, veh/mi/ln	0.7
%Improv	vement to Percent Followers	0.0	9	%Improvement to	o Speed	0.0
Subse	gment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	ngent	5280	-		-	51.9
Vehicl	le Results					
Average	Speed, mi/h	51.9	P	Percent Followers	5, %	22.4
Segmen	t Travel Time, minutes	1.16	F	Follower Density	(FD), followers/mi/ln	0.7
Vehicle l	LOS	А				
Facilit	y Results					
Т	VMT veh-mi/p	VHD veh-h/p		Follower D	ensity, followers/ mi/ln	LOS
1	29	0.01			0.7	A
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		HCS Two-La	ne H	ighway Re	eport	
Proje	ct Information					
Analyst		James Vorosmarti	Г	Date		7/12/2023
Agency		Tetra Tech	A	Analysis Year		2026
Jurisdict	tion		Т	Time Analyzed		Morning Peak Hour
Project	Description	Ph2 to LY2 Travis Road Southbound (North of Sellards Road)		Jnits		U.S. Customary
		S	egme	ent 1		
Vehic	le Inputs					
Segmen	nt Type	Passing Zone	L	ength, ft		5280
Lane Wi	idth, ft	11	5	Shoulder Width,	ft	2
Speed L	_imit, mi/h	50	A	Access Point Den	ısity, pts/mi	2.0
Dema	and and Capacity					
Directio	onal Demand Flow Rate, veh/h	164		Opposing Demar	nd Flow Rate, veh/h	26
Peak Ho	our Factor	0.58	Т	Total Trucks, %		3.00
Segmen	nt Capacity, veh/h	1700	С	Demand/Capacit	y (D/C)	0.10
Interr	mediate Results	•				
Segmen	nt Vertical Class	1	Free-Flow Speed, mi/h		53.0	
Speed S	Slope Coefficient (m)	3.08402	5	Speed Power Coefficient (p)		0.62112
PF Slope	e Coefficient (m)	-1.16189	F	PF Power Coeffic	ient (p)	0.81111
In Passir	ng Lane Effective Length?	No	Т	Total Segment De	ensity, veh/mi/ln	0.7
%lmpro	ovement to Percent Followers	0.0	9	%Improvement t	o Speed	0.0
Subse	egment Data					
# Se	egment Type	Length, ft	Radiu	s, ft	Superelevation, %	Average Speed, mi/h
1 Ta	angent	5280	-		-	52.4
Vehic	le Results		•			
Average	e Speed, mi/h	52.4	F	Percent Followers	s, %	23.5
Segment Travel Time, minutes 1.14		1.14	F	Follower Density	(FD), followers/mi/ln	0.7
Vehicle	LOS	А				
Facilit	ty Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower D	Pensity, followers/ mi/ln	LOS
1	24	0.00			0.7	A
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		HCS Two-La	ne H	lighway R	leport				
Projec	ct Information								
Analyst		James Vorosmarti	Г	Date		7/12/2023			
Agency		Tetra Tech	A	Analysis Year		2026			
Jurisdict	tion		1	Time Analyzed		Afternoon Peak Hour			
Project I	Description	Ph2 to LY2 Travis Road Southbound (North of Sellards Road)		Units		U.S. Customary			
	Segment 1								
Vehic	le Inputs								
Segmen	nt Type	Passing Zone		Length, ft		5280			
Lane Wi	idth, ft	11	9	Shoulder Width	, ft	2			
Speed L	imit, mi/h	50	A	Access Point De	nsity, pts/mi	2.0			
Dema	and Capacity								
Directio	nal Demand Flow Rate, veh/h	28		Opposing Demand Flow Rate, veh/h		140			
Peak Hour Factor		0.83		Total Trucks, %		10.00			
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.02			
Intern	nediate Results	•							
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		52.8			
Speed Slope Coefficient (m)		3.14122		Speed Power Coefficient (p)		0.55833			
PF Slope Coefficient (m)		-1.21913		PF Power Coefficient (p)		0.79515			
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0			
%Impro	vement to Percent Followers	0.0		%Improvement to Speed		0.0			
Subse	egment Data								
# Se	egment Type	Length, ft	Radiu	ıs, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	52.8			
Vehic	le Results								
Average Speed, mi/h		52.8		Percent Followers, %		6.8			
Segment Travel Time, minutes		1.14		Follower Density (FD), followers/mi/ln		0.0			
Vehicle LOS		A							
Facilit	ty Results		·						
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	6	0.00			0.0	A			
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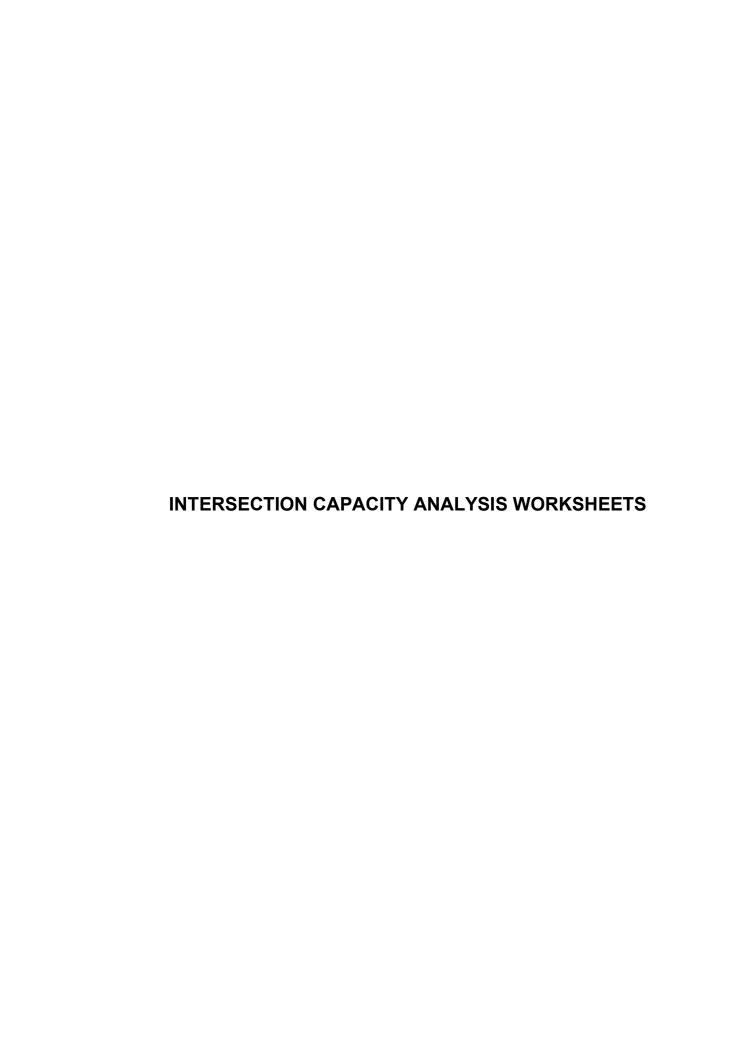
		HCS Two-La	ne Hi	ghway Re	port				
Projec	t Information								
Analyst		James Vorosmarti	Da	Date		7/12/2023			
Agency		Tetra Tech	An	alysis Year		2026			
Jurisdicti	on		Tir	me Analyzed		Morning Peak Hour			
Project D	Description	No Build Travis Road Northbound (North of Sellards Road)		Units		U.S. Customary			
	Segment 1								
Vehicle	e Inputs								
Segment	t Type	Passing Zone	Le	Length, ft		5280			
Lane Wic	dth, ft	11	Sh	oulder Width, f	t	2			
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	2.0			
Demai	nd and Capacity								
Direction	nal Demand Flow Rate, veh/h	24	Op	Opposing Demand Flow Rate, veh/h		67			
Peak Hour Factor		0.54		Total Trucks, %		38.00			
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.01			
Interm	nediate Results	•				·			
Segment	t Vertical Class	1		Free-Flow Speed, mi/h		51.8			
Speed Slope Coefficient (m)		3.05277		Speed Power Coefficient (p)		0.59106			
PF Slope Coefficient (m)		-1.18769		PF Power Coefficient (p)		0.80496			
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.0			
%Improvement to Percent Followers		0.0 %		%Improvement to Speed		0.0			
Subse	gment Data								
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h			
1 Tar	ngent	5280	-		-	51.8			
Vehicle	e Results								
Average Speed, mi/h		51.8		Percent Followers, %		5.7			
Segment Travel Time, minutes		1.16		Follower Density (FD), followers/mi/ln		0.0			
Vehicle LOS		А							
Facility	y Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	3	0.00			0.0	A			

		HCS Two-La	ne Hig	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Dat	Date		7/12/2023
Agency		Tetra Tech	Ana	alysis Year		2026
Jurisdicti	on		Tim	ne Analyzed		Afternoon Peak Hour
Project D	Description	No Build Travis Road Northbound (North of Sellards Road)		Units		U.S. Customary
		S	egmen	nt 1		
Vehicle	e Inputs					
Segment	: Туре	Passing Zone	Ler	Length, ft		5280
Lane Wic	dth, ft	11	Sho	oulder Width, f	t	2
Speed Lii	mit, mi/h	50	Acc	cess Point Dens	sity, pts/mi	2.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	75	Ор	Opposing Demand Flow Rate, veh/h		28
Peak Hour Factor		0.76		Total Trucks, %		20.00
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.04
Interm	nediate Results					
Segment Vertical Class		1		Free-Flow Speed, mi/h		52.4
Speed Slope Coefficient (m)		3.05511		Speed Power Coefficient (p)		0.61939
PF Slope Coefficient (m)		-1.16277		PF Power Coefficient (p)		0.81146
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.2
%Improvement to Percent Followers		0.0 %		%Improvement to Speed		0.0
Subse	gment Data	•	·			
# Seg	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	52.4
Vehicle	e Results					
Average Speed, mi/h		52.4		Percent Followers, %		13.2
Segment Travel Time, minutes		1.14 F		Follower Density (FD), followers/mi/ln		0.2
Vehicle LOS		A				
Facility	y Results					
т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/In		LOS
	14	0.00		<del>                                     </del>	0.2	A

		HCS Two-La	ne H	ighway Re	port				
Projec	ct Information								
Analyst		James Vorosmarti		Date		7/12/2023			
Agency		Tetra Tech	Δ	Analysis Year		2026			
Jurisdict	tion		Т	ime Analyzed		Morning Peak Hour			
Project I	Description	No Build Travis Road Southbound (North of Sellards Road)		Units		U.S. Customary			
	Segment 1								
Vehicle Inputs									
Segmen	nt Type	Passing Zone		Length, ft		5280			
Lane Wi	idth, ft	11	S	shoulder Width, ft	t	2			
Speed L	imit, mi/h	50	Δ	Access Point Dens	sity, pts/mi	2.0			
Dema	and Capacity								
Directio	nal Demand Flow Rate, veh/h	62		Opposing Demand Flow Rate, veh/h		22			
Peak Ho	our Factor	0.58		Total Trucks, %		3.00			
Segment Capacity, veh/h		1700		Demand/Capacity (D/C)		0.04			
Intern	nediate Results	•							
Segmen	nt Vertical Class	1		Free-Flow Speed, mi/h		53.0			
Speed Slope Coefficient (m)		3.08038		Speed Power Coefficient (p)		0.62469			
PF Slope Coefficient (m)		-1.15869		PF Power Coefficient (p)		0.81203			
In Passing Lane Effective Length?		No		Total Segment Density, veh/mi/ln		0.1			
%Improvement to Percent Followers		0.0		%Improvement to Speed		0.0			
Subse	egment Data								
# Se	egment Type	Length, ft	Radius	s, ft	Superelevation, %	Average Speed, mi/h			
1 Ta	angent	5280	-		-	53.0			
Vehic	le Results								
Average Speed, mi/h		53.0		Percent Followers, %		11.4			
Segment Travel Time, minutes		1.13		Follower Density (FD), followers/mi/ln		0.1			
Vehicle LOS		А							
Facilit	ty Results								
Т	VMT veh-mi/p	VHD veh-h/p		Follower Density, followers/ mi/ln		LOS			
1	9	0.00			0.1	A			
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		HCS Two-La	ne Hi	ghway Re	port	
Projec	t Information					
Analyst		James Vorosmarti	Da	te		7/12/2023
Agency		Tetra Tech	An	alysis Year		2026
Jurisdicti	ion		Tin	ne Analyzed		Afternoon Peak Hour
Project D	Description	No Build Travis Road Southbound (North of Sellards Road)	Un	iits		U.S. Customary
		Se	egmer	nt 1		
Vehicle	e Inputs					
Segment	t Type	Passing Zone	Lei	ngth, ft		5280
Lane Wic	dth, ft	11	Sh	oulder Width, f	t	2
Speed Li	mit, mi/h	50	Ac	cess Point Dens	sity, pts/mi	2.0
Demai	nd and Capacity					
Direction	nal Demand Flow Rate, veh/h	25	Op	posing Deman	d Flow Rate, veh/h	69
Peak Hou	ur Factor	0.83	Tot	tal Trucks, %		10.00
Segment	t Capacity, veh/h	1700	De	mand/Capacity	(D/C)	0.01
Interm	nediate Results	•				
Segment	t Vertical Class	1	Fre	ee-Flow Speed,	mi/h	52.8
Speed SI	lope Coefficient (m)	3.10457	Sp	eed Power Coe	fficient (p)	0.58991
PF Slope	Coefficient (m)	-1.18991	PF	Power Coefficie	ent (p)	0.80338
In Passin	g Lane Effective Length?	No	Tot	tal Segment De	nsity, veh/mi/ln	0.0
%Improv	vement to Percent Followers	0.0	%I	mprovement to	Speed	0.0
Subse	gment Data		·			
# Se	gment Type	Length, ft	Radius,	ft	Superelevation, %	Average Speed, mi/h
1 Tar	ngent	5280	-		-	52.8
Vehicle	e Results					
Average	Speed, mi/h	52.8	Pei	rcent Followers	, %	6.0
Segment	t Travel Time, minutes	1.14	Fo	llower Density (	FD), followers/mi/ln	0.0
Vehicle L	.OS	А				
Facility	y Results					
Т	VMT veh-mi/p	VHD veh-h/p			ensity, followers/ mi/ln	LOS
1	5	0.00			0.0	A

## APPENDIX I: UNSIGNALIZED CAPACITY ANALYSIS WORKSHEETS



Intersection						
Int Delay, s/veh	4.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	LOIN	*************************************	↑	₩.	אפאר
Traffic Vol, veh/h	90	101	6	22	134	8
Future Vol, veh/h	90	101	6	22	134	8
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		Stop -	None
Storage Length	<u>-</u>	-	130	-	0	-
Veh in Median Storag		_	-	0	0	_
Grade, %	0, # 0	<u> </u>	_	0	0	<u>-</u>
Peak Hour Factor	77	77	77	77	77	77
	8	18	0	18	8	0
Heavy Vehicles, %		131			174	10
Mvmt Flow	117	131	8	29	1/4	10
Major/Minor	Major1	N	Major2	ľ	Minor1	
Conflicting Flow All	0	0	248	0	228	183
Stage 1	-	-	_	-	183	_
Stage 2	-	-	-	-	45	-
Critical Hdwy	-	-	4.1	-	6.48	6.2
Critical Hdwy Stg 1	_	_	-	_	5.48	_
Critical Hdwy Stg 2	_	_	-	_	5.48	_
Follow-up Hdwy	_	_	2.2	_	3.572	3.3
Pot Cap-1 Maneuver	_	_	1330	_	747	865
Stage 1	_	_	-	_	834	-
Stage 2	_	_	_	_	962	_
Platoon blocked, %	_	_		_	302	
Mov Cap-1 Maneuver			1330	_	743	865
Mov Cap-1 Maneuver		_	1330	_	743	- 003
Stage 1	-	_	-		834	
	-	-	-	-		
Stage 2	-	-	-	-	956	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.7		11.4	
HCM LOS					В	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		749	-		1330	-
HCM Lane V/C Ratio		0.246	-	-	0.006	-
HCM Control Delay (s	5)	11.4	-	-	7.7	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh	۱)	1	-	-	0	-

Intersection						
Int Delay, s/veh	2.9					
		EDD	\\/DI	WDT	NIDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>\$</b>	400	_ ኝ	454	<b>Y</b>	
Traffic Vol, veh/h	141	183	5	151	82	50
Future Vol, veh/h	141	183	5	151	82	50
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	13	9	0	10	31	4
Mvmt Flow	157	203	6	168	91	56
Major/Minor	oio-1		Ania-O		Mine -1	
	ajor1		Major2		Minor1	050
Conflicting Flow All	0	0	360	0	439	259
Stage 1	-	-	-	-	259	-
Stage 2	-	-	-	-	180	-
Critical Hdwy	-	-	4.1	-	6.71	6.24
Critical Hdwy Stg 1	-	-	-	-	5.71	-
Critical Hdwy Stg 2	-	-	-	-	5.71	-
Follow-up Hdwy	-	-	2.2	-	3.779	3.336
Pot Cap-1 Maneuver	-	-	1210	-	525	775
Stage 1	-	-	-	-	721	-
Stage 2	-	-	-	-	786	-
Platoon blocked, %	_	_		_		
Mov Cap-1 Maneuver	_	-	1210	_	522	775
Mov Cap-2 Maneuver	_	_	-	_	522	-
Stage 1	_	_	_	_	721	_
Stage 2	_	<u>_</u>	_	_	782	_
Glage Z	_		_		102	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		13	
HCM LOS					В	
NA' 1 /NA - ' NA (		UDL .4	CDT	EDD	\A/DI	WDT
Minor Lane/Major Mvmt		VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		596	-		1210	-
HCM Lane V/C Ratio		0.246	-	-	0.005	-
HCM Control Delay (s)		13	-	-	8	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		1	-	-	0	-

Intersection												
Int Delay, s/veh	5.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	₽		ሻ	- ↑			4	7		4	02.1
Traffic Vol, veh/h	1	131	30	111	118	4	20	1	192	1	1	0
Future Vol, veh/h	1	131	30	111	118	4	20	1	192	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	147	34	125	133	4	22	1	216	1	1	0
Major/Minor Major/Minor	ajor1		1	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	137	0	0	181	0	0	552	553	164	660	568	135
Stage 1	-	-	-	-	-	-	166	166	-	385	385	-
Stage 2	_	_	-	_	_	-	386	387	_	275	183	-
Critical Hdwy	4.1	-	-	4.4	-	-	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Follow-up Hdwy	2.2	-	-	2.47	-	-	3.635	4	3.39	3.5	4.9	3.3
	1459	-	-	1242	-	-	425	444	860	379	323	919
Stage 1	-	-	-	-	-	-	806	765	-	642	471	-
Stage 2	-	-	-	-	-	-	612	613	-	736	596	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1459	-	-	1242	-	-	391	399	860	262	290	919
Mov Cap-2 Maneuver	-	-	-	-	-	-	391	399	-	262	290	-
Stage 1	-	-	-	-	-	-	805	764	-	641	423	-
Stage 2	-	-	-	-	-	-	549	551	-	550	595	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			3.9			11			18.2		
HCM LOS				3.0			В			C		
200												
Minor Lane/Major Mvmt	N	NBLn11	VIRI n2	EBL	EBT	EBR	WBL	WBT	WBR S	SRI n1		
	ı,											
Capacity (veh/h)		391	860	1459	-		1242	-	-			
HCM Control Polov (a)			0.251		-	-	0.1	-		0.008		
HCM Long LOS		14.8	10.6	7.5	-	-	8.2	-	-			
HCM Lane LOS HCM 95th %tile Q(veh)		В	B 1	A	-	-	A	-	-	С		
HOIVI 9501 76011 WOLLE CALVEU)		0.2	I	0	-	-	0.3	-	-	0		

Intersection												
Int Delay, s/veh	5.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ች	1>		*	<b>\$</b>	
Traffic Vol, veh/h	19	8	13	10	9	64	8	122	14	90	48	4
Future Vol, veh/h	19	8	13	10	9	64	8	122	14	90	48	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	_	<u> </u>	None	-	_	None	-	-	None	-	_	None
Storage Length	_	-	-	-	_	-	150	-	-	230	_	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	22	9	15	11	10	74	9	140	16	103	55	5
Major/Minor N	1inor2		-	Minor1			Major1			Major2		
Conflicting Flow All	472	438	58	442	432	148	60	0	0	156	0	0
Stage 1	264	264	-	166	166	-	-	-	-	-	-	-
Stage 2	208	174	-	276	266	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1	-	-	4.46	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	-	-	2.524	-	-
Pot Cap-1 Maneuver	506	515	991	529	519	846	1556	-	-	1241	-	-
Stage 1	746	694	-	841	765	-	-	-	-	-	-	-
Stage 2	799	759	-	735	692	-	-	-	-	-	-	_
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	424	469	991	479	473	846	1556	-	-	1241	-	-
Mov Cap-2 Maneuver	424	469	-	479	473	-	-	-	-	-	-	-
Stage 1	742	636	-	836	760	-	-	-	-	-	-	-
Stage 2	715	754	-	654	635	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.4			10.8			0.4			5.2		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1556		-			1241	-				
HCM Lane V/C Ratio		0.006	_	_		0.133		_	_			
HCM Control Delay (s)		7.3	-	-	12.4	10.8	8.2	-	-			
HCM Lane LOS		Α	-	-	В	В	A	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.3	0.5	0.3	-	-			

Laterrand						
Intersection	0.1					
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			4
Traffic Vol, veh/h	2	0	25	0	0	220
Future Vol, veh/h	2	0	25	0	0	220
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	_	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	_	_	0
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	0	40	0	0	6
Mymt Flow	2	0	30	0	0	265
IVIVIII( I IOW		U	50	U	U	200
Major/Minor M	linor1		//ajor1	N	/lajor2	
Conflicting Flow All	295	30	0	0	30	0
Stage 1	30	-	-	-	-	-
Stage 2	265	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	_	-	-	_	-
Critical Hdwy Stg 2	5.4	-	_	-	-	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	-
Pot Cap-1 Maneuver	700	1050	_	-	1596	_
Stage 1	998	-	_	_	-	_
Stage 2	784	_				
Platoon blocked, %	7 0-		_	_		_
Mov Cap-1 Maneuver	700	1050	-	-	1596	-
Mov Cap-2 Maneuver	700	-	-	-	-	-
Stage 1	998	-	-	-	-	-
Stage 2	784	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		0	
HCM LOS	В		- 0		0	
1 TOWN LOO	U					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	700	1596	-
HCM Lane V/C Ratio		-	-	0.003	-	-
HCM Control Delay (s)		-	-	10.2	0	-
HCM Lane LOS		_	-	В	Α	-
HCM 95th %tile Q(veh)		_	_	0	0	_
				•	_	

Intersection												
Int Delay, s/veh	0											
•		EDT	<b>EDD</b>	MOL	MOT	WDD	NDI	NDT	NDD	0.01	ODT	000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	39	0	0	179	0
Future Vol, veh/h	0	0	0	0	0	0	0	39	0	0	179	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	73	73	73	73	73	73
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	53	0	0	245	0
Major/Minor N	Minor2		N	Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	298	298	245	298	298	53	245	0	0	53	0	0
						ეა	240	U	U			
Stage 1	245	245	-	53	53	-	-	-	-	-	-	-
Stage 2	53	53	- 6.0	245	245	6.0	11	-	-	11	-	-
Critical Holy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	658	617	799	658	617	1020	1333	-	-	1566	-	-
Stage 1	763	707	-	965	855	-	-	-	-	-	-	-
Stage 2	965	855	-	763	707	-	_	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	658	617	799	658	617	1020	1333	-	-	1566	-	-
Mov Cap-2 Maneuver	658	617	-	658	617	-	-	-	-	-	-	-
Stage 1	763	707	-	965	855	-	-	-	-	-	-	-
Stage 2	965	855	-	763	707	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A			- 0			J		
TIOWI LOO	Λ											
Minor Lane/Major Mvm	t	NBL	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1333	-	-	-	-	1566	-	-			
HCM Lane V/C Ratio		-	-	-	-	-	-	-	-			
HCM Control Delay (s)		0	-	-	0	0	0	-	-			
HCM Lane LOS		Α	-	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	33	0	9	0	23	9	37	150	0
Future Vol, veh/h	0	0	0	33	0	9	0	23	9	37	150	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	75	75	75	75	75	75	75	75	75	75	75
Heavy Vehicles, %	0	0	0	6	0	44	0	39	33	16	13	0
Mvmt Flow	0	0	0	44	0	12	0	31	12	49	200	0
Major/Minor M	1inor2			Minor1		1	Major1		ľ	Major2		
Conflicting Flow All	341	341	200	335	335	37	200	0	0	43	0	0
Stage 1	298	298	-	37	37	-	-	-	-	-	-	-
Stage 2	43	43	-	298	298	-	_	-	_	_	-	_
Critical Hdwy	7.1	6.5	6.2	7.16	6.5	6.64	4.1	-	-	4.26	-	_
Critical Hdwy Stg 1	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.554	4	3.696	2.2	-	-	2.344	-	-
Pot Cap-1 Maneuver	617	584	846	611	589	927	1384	-	-	1480	-	_
Stage 1	715	671	-	968	868	-	-	-	-	-	-	-
Stage 2	976	863	-	702	671	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	592	562	846	594	567	927	1384	-	-	1480	-	-
Mov Cap-2 Maneuver	592	562	-	594	567	-	-	-	-	-	-	-
Stage 1	715	646	-	968	868	-	-	-	-	-	-	-
Stage 2	963	863	-	676	646	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			11.1			0			1.5		
HCM LOS	A			В			_			•		
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1384	-	-	-			-				
HCM Lane V/C Ratio		-	_	_		0.087		_	_			
HCM Control Delay (s)		0	_	_	0	11.1	7.5	0	_			
HCM Lane LOS		A	_	_	A	В	Α.	A	_			
HCM 95th %tile Q(veh)		0	-	-	-	0.3	0.1	-	_			
70th Q(7011)						0.0	7.1					

Intersection						
Int Delay, s/veh	0.1					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	•	4	4	f)	_
Traffic Vol, veh/h	0	0	1	13	43	7
Future Vol, veh/h	0	0	1	13	43	7
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	73	73	73	73	73	73
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	18	59	10
NA - ' /NA' N	<i>I</i> 0		1.1.1		4 - ' 0	
	/linor2		Major1		/lajor2	
Conflicting Flow All	84	64	69	0	-	0
Stage 1	64	-	-	-	-	-
Stage 2	20	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	923	1006	1545	-	-	-
Stage 1	964	-	-	-	-	-
Stage 2	1008	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	922	1006	1545	-	_	-
Mov Cap-2 Maneuver	922	-	-	-	_	_
Stage 1	963	_	_	_	_	_
Stage 2	1008	_	_	_	_	_
Olugo Z	1000					
	EB		NB		SB	
Approach	LD				0	
Approach HCM Control Delay, s	0		0.5		v	
			0.5		v	
HCM Control Delay, s	0		0.5			
HCM Control Delay, s HCM LOS	0 A	ND		EDI »4		CDD
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	0 A	NBL		EBLn1	SBT	SBR
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	0 A	1545	NBT	-	SBT -	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0 A	1545 0.001	NBT I	-	SBT -	SBR -
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	0 A	1545 0.001 7.3	NBT   - - 0	- - 0	SBT -	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	0 A	1545 0.001	NBT I	-	SBT -	-

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	13	0	1	42	0
Future Vol, veh/h	1	0	0	0	0	0	0	13	0	1	42	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	65	65	65	65	65	65	65	65	65	65	65	65
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow	2	0	0	0	0	0	0	20	0	2	65	0
Major/Minor	Minor2		ı	Minor1		N	Major1		N	Major2		
Conflicting Flow All	89	89	65	89	89	20	65	0	0	20	0	0
Stage 1	69	69	- 00	20	20	20	UU	U	U	20	-	-
Stage 2	20	20	-	69	69	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	_	<u>-</u>	4.1	-	<u>-</u>
Critical Hdwy Stg 1	6.1	5.5	0.2	6.1	5.5	0.2	4.1	_	_	4.1	_	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	<u>-</u>	-		-
Follow-up Hdwy	3.5	3.5	3.3	3.5	3.5	3.3	2.2	_	-	2.2	_	-
Pot Cap-1 Maneuver	901	805	1005	901	805	1064	1550	_	<u>-</u>	1609	-	<u>-</u>
Stage 1	946	841	1005	1004	883	1004	1000	_	_	1009	_	-
Stage 2	1004	883	-	946	841	<u>-</u>	<u>-</u>	-	<u>-</u>	<u>-</u>	-	-
Platoon blocked, %	1004	000		340	U <del>4</del> 1	_	_	_	_	_	_	_
Mov Cap-1 Maneuver	900	804	1005	900	804	1064	1550	<del>-</del>	<del>-</del>	1609	-	<del>-</del>
Mov Cap-2 Maneuver	900	804	1005	900	804	1004	1000	_	_	1009	_	_
Stage 1	946	840	-	1004	883	<u>-</u>	<u>-</u>	-	<u>-</u>	<u>-</u>	-	-
Stage 2	1004	883	_	945	840	_	_	_	_	_		_
Olaye Z	1004	000	-	J <del>H</del> J	040	<u>-</u>	_	-	<u>-</u>	_	_	<u>-</u>
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9			0			0			0.2		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR E	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1550		-		-	1609		_			
HCM Lane V/C Ratio		-	_		0.002		0.001	_	<u>-</u>			
HCM Control Delay (s)		0	_	_	9	0	7.2	0	_			
HCM Lane LOS		A	_	_	A	A	Α	A	_			
HCM 95th %tile Q(veh)	\	0	_	_	0	-	0	-	_			
TICIVI JOHN JOHN Q VOII)		U			J		U					

Intersection						
Int Delay, s/veh	5.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	TTDIX.	<b>1</b>	HUIT	002	4
Traffic Vol, veh/h	34	21	9	3	2	23
Future Vol, veh/h	34	21	9	3	2	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		_	0	-	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	72	72	72	72	72	72
Heavy Vehicles, %	3	0	22	0	22	0
Mvmt Flow	47	29	13	4	3	32
WWW.CT IOW		20	10	•		UL.
		_				
	/linor1		Major1		Major2	
Conflicting Flow All	53	15	0	0	17	0
Stage 1	15	-	-	-	-	-
Stage 2	38	-	-	-	-	-
Critical Hdwy	6.43	6.2	-	-	4.32	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	-	-	2.398	-
Pot Cap-1 Maneuver	953	1070	-	-	1479	-
Stage 1	1005	-	-	-	-	-
Stage 2	982	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	951	1070	-	_	1479	-
Mov Cap-2 Maneuver	951	-	-	-	-	-
Stage 1	1005	-	-	_	-	-
Stage 2	980	_	_	_	_	_
Jugo 2	500					
Approach	WB		NB		SB	
HCM Control Delay, s	8.9		0		0.6	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NRRV	VBLn1	SBL	SBT
	l	וטוו		993	1479	
Capacity (veh/h)		-	-			-
HCM Control Doloy (a)		-		0.077		-
HCM Control Delay (s) HCM Lane LOS		-	-		7.4	0 A
		-	-	0.2	A 0	A -
HCM 95th %tile Q(veh)			_			

Intersection												
Int Delay, s/veh	2.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4			4	
Traffic Vol, veh/h	12	8	0	1	95	134	0	1	0	52	0	29
Future Vol., veh/h	12	8	0	1	95	134	0	1	0	52	0	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	80
Heavy Vehicles, %	58	0	0	0	8	10	0	0	0	19	0	10
Mvmt Flow	15	10	0	1	119	168	0	1	0	65	0	36
Major/Minor	Major1		ı	Major2		ı	Minor1		ı	Minor2		
Conflicting Flow All	287	0	0	10	0	0	263	329	10	162	161	119
Stage 1	-	-	-	-	-	-	40	40	-	121	121	-
Stage 2	-	-	-	-	-	-	223	289	-	41	40	-
Critical Hdwy	4.68	-	-	4.1	-	-	7.1	6.5	6.2	7.29	6.5	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Follow-up Hdwy	2.722	-	-	2.2	-	-	3.5	4	3.3	3.671	4	3.39
Pot Cap-1 Maneuver	1013	-	-	1623	-	-	694	593	1077	766	735	912
Stage 1	-	-	-	-	-	-	980	866	-	844	800	-
Stage 2	-	-	-	-	-	-	784	677	-	932	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1013	-	-	1623	-	-	659	584	1077	755	723	912
Mov Cap-2 Maneuver	-	-	-	-	-	-	659	584	-	755	723	-
Stage 1	-	-	-	-	-	-	965	853	-	831	799	-
Stage 2	-	-	-	-	-	-	752	676	-	917	853	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	5.2			0			11.2			10.1		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		584	1013	-	-	1623	-	-	805			
HCM Lane V/C Ratio		0.002		-	-	0.001	-	_	0.126			
HCM Control Delay (s)		11.2	8.6	0	-	7.2	0	-	10.1			
HCM Lane LOS		В	Α	A	-	Α	A	-	В			
HCM 95th %tile Q(veh	)	0	0	-	-	0	-	-	0.4			
	,											

Intersection												
Int Delay, s/veh	2.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.		*	ĵ.			4			4	
Traffic Vol, veh/h	1	35	1	11	335	30	4	0	9	69	2	7
Future Vol, veh/h	1	35	1	11	335	30	4	0	9	69	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	45	1	14	435	39	5	0	12	90	3	9
Major/Minor N	lajor1		ı	Major2		N	/linor1		ı	Minor2		
Conflicting Flow All	474	0	0	46	0	0	537	550	46	537	531	455
Stage 1	-	-	-	-	-	-	48	48	-	483	483	-
Stage 2	-	-	-	-	-	-	489	502	-	54	48	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4	3.3	3.581	4	3.561
Pot Cap-1 Maneuver	723	-	-	1575	-	-	458	446	1029	444	457	553
Stage 1	-	-	-	-	-	-	971	859	-	552	556	-
Stage 2	-	-	-	-	-	-	564	545	-	941	859	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	723	-	-	1575	-	-	445	442	1029	436	452	553
Mov Cap-2 Maneuver	-	-	-	-	-	-	445	442	-	436	452	-
Stage 1	-	-	-	-	-	-	970	858	-	551	551	-
Stage 2	-	-	-	-	-	-	547	540	-	929	858	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			10			15.5		
HCM LOS							В			С		
Minor Lane/Major Mvmt	: 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		733	723	-	-	1575	-	_	445			
HCM Lane V/C Ratio		0.023		-	-	0.009	-	-	0.228			
HCM Control Delay (s)		10	10	-	-	7.3	-	-	15.5			
HCM Lane LOS		В	Α	-	-	Α	-	-	С			
HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	0.9			

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	10	85	8	296	0	0	0	0	2	2	96
Future Vol, veh/h	0	10	85	8	296	0	0	0	0	2	2	96
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	92	92	92	80	80	80
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	13	106	10	370	0	0	0	0	3	3	120
Major/Minor N	1ajor1		N	Major2					Λ	/linor2		
Conflicting Flow All	<u>-</u>	0	0	119	0	0				456	509	370
Stage 1	-	-	-	-	-	-				390	390	-
Stage 2	_	_	_	_	_	-				66	119	-
Critical Hdwy	-	-	-	4.85	-	-				6.4	7	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1118	-	0				566	405	650
Stage 1	0	-	-	-	-	0				689	532	-
Stage 2	0	-	-	-	-	0				962	713	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1118	-	-				560	0	650
Mov Cap-2 Maneuver	-	-	-	-	-	-				560	0	-
Stage 1	-	-	-	-	-	-				689	0	-
Stage 2	-	-	-	-	-	-				951	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						11.9		
HCM LOS	•			0.2						В		
Minor Lang/Major Mumt		EBT	EBR	WBL	WPT	SBLn1						
Minor Lane/Major Mvmt												
Capacity (veh/h)		-		1118 0.009	-							
HCM Control Doloy (s)		-				0.193						
HCM Control Delay (s) HCM Lane LOS		-	-	6.2 A	0 A							
HCM 95th %tile Q(veh)		-	-	0	- A	0.7						
HOW SOUL WILLE CALVELL)		-	-	U	-	0.7						

Int Delay, s/veh
Lane Configurations         Image: Configuration of the confi
Lane Configurations         Image: Configuration of the confi
Traffic Vol, veh/h         9         3         0         0         4         1         300         5         23         0         0         0           Future Vol, veh/h         9         3         0         0         4         1         300         5         23         0         0         0           Conflicting Peds, #/hr         0
Future Vol, veh/h         9         3         0         0         4         1         300         5         23         0         0         0           Conflicting Peds, #/hr         0
Conflicting Peds, #/hr         0
Sign Control Free Free Free Free Free Stop Stop Stop Stop Stop Stop Stop Stop
RT Channelized None
Storage Length
Grade, % - 0 0 0 -
Peak Hour Factor 82 82 82 82 82 82 82 82 92 92 92
Heavy Vehicles, % 14 0 0 0 75 0 10 80 26 2 2 2
Mvmt Flow 11 4 0 0 5 1 366 6 28 0 0 0
Major/Minor Major1 Major2 Minor1
Conflicting Flow All 6 0 0 32 32 4
Stage 1 26 26 -
Stage 2 6 6 -
Critical Hdwy 4.24 6.5 7.3 6.46
Critical Hdwy Stg 1 5.5 6.3 -
Critical Hdwy Stg 2 5.5 6.3 -
Follow-up Hdwy 2.326 3.59 4.72 3.534
·
Mov Cap-1 Maneuver 1540 955 0 1013
Mov Cap-1 Maneuver
Stage 1 969 0 -
•
Stage 2 997 0 -
A
Approach EB WB NB
HCM Control Delay, s 5.5 0 11.4
HCM LOS B
Minor Lane/Major Mvmt NBLn1 EBL EBT WBT WBR
Capacity (veh/h) 959 1540
HCM Lane V/C Ratio 0.417 0.007
HCM Control Delay (s) 11.4 7.4 0
HCM Lane LOS B A A
HCM 95th %tile Q(veh) 2.1 0

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ			र्स						4	
Traffic Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Future Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	_	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	70	70	70	70	70	70	92	92	92	70	70	70
Heavy Vehicles, %	0	20	0	0	50	0	2	2	2	10	83	0
Mvmt Flow	0	7	3	4	16	0	0	0	0	14	9	4
Major/Minor N	/lajor1		N	Major2					N	/linor2		
Conflicting Flow All		0	0	10	0	0				33	34	16
Stage 1	-	-	-	-	-	-				24	24	-
Stage 2	-	-	-	-	-	-				9	10	-
Critical Hdwy	-	-	-	4.1	-	-				6.5	7.33	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.5	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.5	6.33	-
Follow-up Hdwy	-	-	-	2.2	-	-				3.59	4.747	3.3
Pot Cap-1 Maneuver	0	-	-	1623	-	0				960	724	1069
Stage 1	0	-	-	-	-	0				978	739	-
Stage 2	0	-	-	-	-	0				994	750	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1623	-	-				958	0	1069
Mov Cap-2 Maneuver	-	-	-	-	-	-				958	0	-
Stage 1	-	-	-	-	-	-				978	0	-
Stage 2	-	-	-	-	-	-				992	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			1.5						8.8		
HCM LOS										Α		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		_		1623	_							
HCM Lane V/C Ratio		_		0.003		0.028						
HCM Control Delay (s)		-	-		0	8.8						
HCM Lane LOS		_	_	Α	A	A						
HCM 95th %tile Q(veh)		_	-	0	-	0.1						

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			ĵ.			4				
Traffic Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0
Future Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	63	63	63	63	63	63	63	63	63	92	92	92
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2
Mvmt Flow	0	24	0	0	22	6	0	8	13	0	0	0
Major/Minor N	1ajor1		ı	Major2		N	Minor1					
Conflicting Flow All	28	0	-	-	-	0	49	52	24			
Stage 1	-	-	-	-	-	-	24	24	-			
Stage 2	-	-	-	-	-	-	25	28	-			
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.3	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-			
Follow-up Hdwy	2.2	-	-	-	-	-	3.5	4.72	3.408			
Pot Cap-1 Maneuver	1599	-	0	0	-	-	965	710	1024			
Stage 1	-	-	0	0	-	-	1004	743	-			
Stage 2	-	-	0	0	-	-	1003	740	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1599	-	-	-	-	-	965	0	1024			
Mov Cap-2 Maneuver	-	-	-	-	-	-	965	0	-			
Stage 1	-	-	-	-	-	-	1004	0	-			
Stage 2	-	-	-	-	-	-	1003	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	0			0			8.6					
HCM LOS							Α					
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1024	1599	_	_	_						
HCM Lane V/C Ratio		0.02	-	_	_	_						
HCM Control Delay (s)		8.6	0	_	_	-						
HCM Lane LOS		A	A	_	_	_						
HCM 95th %tile Q(veh)		0.1	0	_	_	_						
// (// (//////////////////		J.,										

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Future Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	67	67	67	67	67	67	67	67	67	67	67	67
Heavy Vehicles, %	0	13	0	0	21	0	0	0	0	0	0	50
Mvmt Flow	0	31	3	0	21	0	3	0	0	0	0	3
Major/Minor N	lajor1		ľ	Major2		<u> </u>	/linor1		N	/linor2		
Conflicting Flow All	21	0	0	34	0	0	56	54	33	54	55	21
Stage 1	-	-	-	-	-	-	33	33	-	21	21	-
Stage 2	-	-	-	-	-	-	23	21	-	33	34	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.7
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.75
Pot Cap-1 Maneuver	1608	-	-	1591	-	-	946	841	1046	949	840	933
Stage 1	-	-	-	-	-	-	988	872	-	1003	882	-
Stage 2	-	-	-	-	-	-	1000	882	-	988	871	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1608	-	-	1591	-	-	943	841	1046	949	840	933
Mov Cap-2 Maneuver	-	-	-	-	-	-	943	841	-	949	840	-
Stage 1	-	-	-	-	-	-	988	872	-	1003	882	-
Stage 2	-	-	-	-	-	-	997	882	-	988	871	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			8.8			8.9		
HCM LOS							Α			Α		
Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		943	1608	-	-	1591	-	-	933			
HCM Lane V/C Ratio		0.003	-	-	-	-	-	-	0.003			
HCM Control Delay (s)		8.8	0	-	-	0	-	-	8.9			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			
. ,												

Intersection												
Int Delay, s/veh	7.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ			र्स						4	
Traffic Vol, veh/h	0	29	5	114	10	0	0	0	0	65	1	35
Future Vol, veh/h	0	29	5	114	10	0	0	0	0	65	1	35
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	73	73	73	73	73	73	92	92	92	73	73	73
Heavy Vehicles, %	0	25	0	6	0	0	2	2	2	3	0	0
Mvmt Flow	0	40	7	156	14	0	0	0	0	89	1	48
Major/Minor N	/lajor1		ľ	Major2					<u> </u>	/linor2		
Conflicting Flow All	-	0	0	47	0	0				370	373	14
Stage 1	-	-	-	-	-	-				326	326	-
Stage 2	-	-	-	-	-	-				44	47	-
Critical Hdwy	-	-	-	4.16	-	-				6.43	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	5.5	-
Follow-up Hdwy	-	-	-	2.254	-	-				3.527	4	3.3
Pot Cap-1 Maneuver	0	-	-	1535	-	0				628	561	1072
Stage 1	0	-	-	-	-	0				729	652	-
Stage 2	0	-	-	-	_	0				976	860	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1535	-	-				564	0	1072
Mov Cap-2 Maneuver	-	-	-	-	-	-				564	0	-
Stage 1	-	-	-	-	-	-				729	0	-
Stage 2	-	-	-	-	-	-				876	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			7						11.7		
HCM LOS										В		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-		1535	-							
HCM Lane V/C Ratio		_		0.102	_	0.205						
HCM Control Delay (s)		-	-									
HCM Lane LOS		-	-	A	A	В						
HCM 95th %tile Q(veh)		-	-	0.3	-	0.8						

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			€			4				
Traffic Vol, veh/h	14	80	0	0	103	83	21	2	9	0	0	0
Future Vol, veh/h	14	80	0	0	103	83	21	2	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	92	92	92
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2
Mvmt Flow	18	101	0	0	130	105	27	3	11	0	0	0
Major/Minor N	Major1		ı	Major2		N	Minor1					
Conflicting Flow All	235	0	_	-	_	0	320	372	101			
Stage 1	-	-	_	_	_	-	137	137	-			
Stage 2	_	_	_	_	_	_	183	235	_			
Critical Hdwy	4.67	_	_	_	_	-	6.4	7.5	6.53			
Critical Hdwy Stg 1	-	_	_	_	_	_	5.4	6.5	-			
Critical Hdwy Stg 2	_	_	_	_	_	_	5.4	6.5	_			
Follow-up Hdwy	2.713	_	_	_	_	_	3.5	4.9	3.597			
Pot Cap-1 Maneuver	1067	_	0	0	_	-	678	431	876			
Stage 1	-	_	0	0	_	_	895	629	-			
Stage 2	_	-	0	0	_	-	853	562	_			
Platoon blocked, %		_		•	_	_	000	002				
Mov Cap-1 Maneuver	1067	-	_	_	_	_	666	0	876			
Mov Cap-2 Maneuver	-	_	_	_	_	_	666	0	-			
Stage 1	_	-	_	_	-	-	879	0	_			
Stage 2	_	_	_	_	_	_	853	0	_			
2.530 2							500					
Annroach	EB			WB			NB					
Approach												
HCM Control Delay, s HCM LOS	1.3			0			10.3 B					
HOW LOS							D					
NA' I /NA - ' NA	, ,	NDL 4	EDI	EDT	MOT	WDD						
Minor Lane/Major Mvm	it I	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		718	1067	-	-	-						
HCM Lane V/C Ratio			0.017	-	-	-						
HCM Control Delay (s)		10.3	8.4	0	-	-						
HCM Lane LOS		В	A	Α	-	-						
HCM 95th %tile Q(veh)		0.2	0.1	-	-	-						

Movement   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR
Cane Configurations
Traffic Vol, veh/h         3         84         2         1         182         0         1         0         0         1         0         3           Future Vol, veh/h         3         84         2         1         182         0         1         0         0         1         0         3           Conflicting Peds, #/hr         0
Traffic Vol, veh/h         3         84         2         1         182         0         1         0         0         1         0         3           Future Vol, veh/h         3         84         2         1         182         0         1         0         0         1         0         3           Conflicting Peds, #/hr         0
Conflicting Peds, #/hr         0
Sign Control Free Free Free Free Free Free Stop Stop Stop Stop Stop Stop Stop Stop
RT Channelized None
Storage Length
/eh in Median Storage, # - 0 0 0 0
Grade, % - 0 0 0 -
Peak Hour Factor 81 81 81 81 81 81 81 81 81 81 81
Heavy Vehicles, % 33 38 50 0 21 0 0 0 0 0 67
Mvmt Flow 4 104 2 1 225 0 1 0 0 1 0 4
Major/Minor Major1 Major2 Minor1 Minor2
Conflicting Flow All 225 0 0 106 0 0 342 340 105 340 341 225
Stage 1 113 113 - 227 227 -
Stage 2 229 227 - 113 114 -
Critical Hdwy 4.43 4.1 7.1 6.5 6.2 7.1 6.5 6.87
Critical Hdwy Stg 1 6.1 5.5 - 6.1 5.5 -
Critical Hdwy Stg 2 6.1 5.5 - 6.1 5.5 -
Follow-up Hdwy 2.497 2.2 3.5 4 3.3 3.5 4 3.903
Pot Cap-1 Maneuver 1181 1498 616 585 955 618 584 677
Stage 1 897 806 - 780 720 -
Stage 2 778 720 - 897 805 -
Platoon blocked, %
Mov Cap-1 Maneuver 1181 1498 610 582 955 616 581 677
Mov Cap-2 Maneuver 610 582 - 616 581 -
Stage 1 893 803 - 777 719 -
Stage 2 773 719 - 893 802 -
Approach EB WB NB SB
HCM Control Delay, s 0.3 0 10.9 10.5
HCM LOS B B
Airenten Maior Million All Discontinuity All Discontinuity and a second and a secon
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1
Capacity (veh/h) 610 1181 1498 661
HCM Lane V/C Ratio 0.002 0.003 0.001 0.007
HCM Control Delay (s) 10.9 8.1 0 - 7.4 0 - 10.5
HCM Lane LOS B A A - A A - B
HCM 95th %tile Q(veh) 0 0 0 0

Intersection							
Int Delay, s/veh	5.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
	EBL	_EBT		WDK	SBL	SBR	
Lane Configurations Traffic Vol, veh/h	<b>ገ</b> 6	<b>T</b> 19	<b>1</b> →	15	<u>ግ</u> 11	115	
Future Vol, veh/h	6	19	44	15	11	115	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-		Stop -	None	
Storage Length	180	-	_	-	100	0	
Veh in Median Storage		0	0	_	0	-	
Grade, %	-	0	0	_	0	_	
Peak Hour Factor	85	85	85	85	85	85	
Heavy Vehicles, %	0	5	7	0	0	2	
Mymt Flow	7	22	52	18	13	135	
minici ion	-		- OL	- 13	10	100	
	/lajor1		Major2		/linor2		
Conflicting Flow All	70	0	-	0	97	61	
Stage 1	-	-	-	-	61	-	
Stage 2	-	-	-	-	36	-	
Critical Hdwy	4.1	-	-	-	6.4	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.2	-	-	-		3.318	
Pot Cap-1 Maneuver	1544	-	-	-	907	1004	
Stage 1	-	-	-	-	967	-	
Stage 2	-	-	-	-	992	-	
Platoon blocked, %	4544	-	-	-	000	1001	
Mov Cap-1 Maneuver	1544	-	-	-	902	1004	
Mov Cap-2 Maneuver	-	-	-	-	902	-	
Stage 1	-	-	-	-	962	-	
Stage 2	-	-	-	-	992	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.8		0		9.1		
HCM LOS					Α		
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WRR	SBLn1 S	SRI n2
		1544		1101	7751		1004
Capacity (veh/h) HCM Lane V/C Ratio		0.005	-	-	-	0.014	
HCM Control Delay (s)		7.3	-			0.014	9.1
HCM Lane LOS			-	-	-	9 A	9.1 A
HCM 95th %tile Q(veh)		A 0	-	-	-	A 0	0.5
HOW BOUT WILL Q(VEII)		U	-	-	-	U	0.5

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	,	<del>(</del>		ř	f)			4			4	
Traffic Vol, veh/h	15	35	7	1	24	1	1	1	1	4	11	31
Future Vol, veh/h	15	35	7	1	24	1	1	1	1	4	11	31
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	70	70	70	70	70	70	70	70	70	70	70	70
Heavy Vehicles, %	13	31	0	100	33	0	0	100	100	0	0	16
Mvmt Flow	21	50	10	1	34	1	1	1	1	6	16	44
Major/Minor I	Major1		I	Major2		N	/linor1		N	/linor2		
Conflicting Flow All	35	0	0	60	0	0	164	134	55	135	139	35
Stage 1	-	_	-	-	_	-	97	97	-	37	37	-
Stage 2	_	_	-	_	_	_	67	37	_	98	102	_
Critical Hdwy	4.23	_	_	5.1	_	_	7.1	7.5	7.2	7.1	6.5	6.36
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Follow-up Hdwy	2.317	-	-	3.1	-	-	3.5	4.9	4.2	3.5		3.444
Pot Cap-1 Maneuver	1508	-	-	1094	-	-	805	608	793	841	756	999
Stage 1	-	-	-	-	-	-	914	658	-	984	868	-
Stage 2	-	-	-	-	-	-	948	705	-	913	815	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1508	-	-	1094	-	-	749	599	793	828	745	999
Mov Cap-2 Maneuver	-	-	-	-	-	-	749	599	-	828	745	-
Stage 1	-	-	-	-	-	-	901	649	-	970	867	-
Stage 2	-	-	-	-	-	-	889	704	-	897	804	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2			0.3			8.6			9.3		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		998	1508	-	-	1094	-	-	909			
HCM Lane V/C Ratio			0.014	-	_	0.001	_	_	0.072			
HCM Control Delay (s)		8.6	7.4	-	-	8.3	-	-	9.3			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.2			

Intersection						
Int Delay, s/veh	0.8					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDIX	VVDL	<u>₩</u>	NDL W	אטוז
Traffic Vol, veh/h	36	3	8	<b>T</b> 24	0	0
Future Vol, veh/h	36	3	8	24	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
_	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- Olop	None
Storage Length	<u>-</u>	-	150	-	0	-
Veh in Median Storage, #		_	-	0	0	_
Grade, %	0	_	_	0	0	-
Peak Hour Factor	77	- 77	77	77	77	- 77
Heavy Vehicles, %	8	33	0	13	0	0
	47	4	10	31		
Mvmt Flow	47	4	10	31	0	0
Major/Minor Ma	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	51	0	100	49
Stage 1	-	-	-	-	49	-
Stage 2	-	-	-	-	51	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	_	_	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1568	-	904	1025
Stage 1	_	-	_	-	979	-
Stage 2	_	-	_	_	977	_
Platoon blocked, %	_	_		_	• • • •	
Mov Cap-1 Maneuver	_	_	1568	_	899	1025
Mov Cap-2 Maneuver	_	<u>-</u>	1000	_	899	-
Stage 1	_	_	_	_	979	_
Stage 2	_	_	_	_	971	_
Stage 2		_	_		3/ 1	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.8		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
		NDLIII -	LDI		1568	-
Capacity (veh/h) HCM Lane V/C Ratio					0.007	
		0	-	-	7.3	-
HCM Control Delay (s) HCM Lane LOS		A			7.3 A	-
HCM 95th %tile Q(veh)		- -	-	-	0	_

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>₽</b>			4
Traffic Vol, veh/h	0	0	0	1	0	14
Future Vol, veh/h	0	0	0	1	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	0	-	-	0
Grade, %	0	_	0	-	_	0
Peak Hour Factor	54	54	54	54	54	54
Heavy Vehicles, %	0	0	0	0	0	14
Mvmt Flow	0	0	0	2	0	26
WWIIICI IOW	U	U	U		U	20
Major/Minor N	Minor1	<u> </u>	//ajor1	N	Major2	
Conflicting Flow All	27	1	0	0	2	0
Stage 1	1	-	-	-	-	-
Stage 2	26	-	-	-	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	-
Critical Hdwy Stg 1	5.4	_	_	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	993	1090	_	_	1634	_
Stage 1	1028	-	_	_	-	_
Stage 2	1002	_	_		_	_
Platoon blocked, %	1002	-		-	-	_
	002	1000	-	-	1604	
Mov Cap-1 Maneuver	993	1090	-	-	1634	-
Mov Cap-2 Maneuver	993	-	-	-	-	-
Stage 1	1028	-	-	-	-	-
Stage 2	1002	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		U		U	
TICIVI LOG						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	_	1634	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		_	_	0	0	_
HCM Lane LOS		_	_	A	A	-
HCM 95th %tile Q(veh)		_	_	-	0	_
HOW JOHN JOHN Q(VEII)		_			U	_

Intersection						
Int Delay, s/veh	0.5					
		===			055	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol, veh/h	1	0	0	0	18	0
Future Vol, veh/h	1	0	0	0	18	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	59	59	59	59	59	59
Heavy Vehicles, %	0	0	0	0	11	0
Mvmt Flow	2	0	0	0	31	0
	inor2		/lajor1		/lajor2	
Conflicting Flow All	31	31	31	0	-	0
Stage 1	31	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	988	1049	1595	-	-	-
Stage 1	997	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	988	1049	1595	-	-	_
Mov Cap-2 Maneuver	988	-	-	_	_	_
Stage 1	997	_	_	_	_	_
Stage 2	-	_	_	_	_	_
Olago Z	_	_				
Approach	EB		NB		SB	
HCM Control Delay, s	8.7		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBL	NDT	EBLn1	SBT	SBR
			INDI		ODI	SDK
Capacity (veh/h)		1595	-	988	-	-
HCM Lane V/C Ratio		-		0.002	-	-
HCM Control Delay (s)		0	-	8.7	-	-
HCM Lane LOS		A	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Later and Con-						
Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	î,	
Traffic Vol, veh/h	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	_
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	15	6	1	11	9
IVIVITIT FIOW	2	15	O		11	9
Major/Minor N	/linor2	N	//ajor1	N	/lajor2	
Conflicting Flow All	29	16	20	0		0
Stage 1	16	-		-	-	-
Stage 2	13	_	_	_	_	_
Critical Hdwy	6.4	6.28	4.5	_	_	_
Critical Hdwy Stg 1	5.4	-	1.0	_	_	_
Critical Hdwy Stg 2	5.4	_			_	_
Follow-up Hdwy		3.372	2.56	_	_	_
	991	1046	1381	-		_
Pot Cap-1 Maneuver		1040	1301	-	-	-
Stage 1	1012	-	-	-	-	-
Stage 2	1015	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	987	1046	1381	-	-	-
Mov Cap-2 Maneuver	987	-	-	-	-	-
Stage 1	1008	-	-	-	-	-
Stage 2	1015	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1381		1037	-	
HCM Lane V/C Ratio		0.004		0.016	_	
HCM Control Delay (s)		7.6	0	8.5	-	-
						-
HCM Lane LOS		A	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	0.1	-	-

Intersection												
Int Delay, s/veh	4.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	6	12	1	0	26	2	2	2	1	1	0	29
Future Vol, veh/h	6	12	1	0	26	2	2	2	1	1	0	29
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	76	76	76	76	76	76	76	76	76	76	76	76
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	8	16	1	0	34	3	3	3	1	1	0	38
Major/Minor N	Major1			Major2		<u> </u>	/linor1		<u> </u>	Minor2		
Conflicting Flow All	37	0	0	17	0	0	88	70	17	71	69	36
Stage 1	-	-	-	-	-	-	33	33	-	36	36	-
Stage 2	-	-	-	-	-	-	55	37	-	35	33	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.516
Pot Cap-1 Maneuver	1395	-	-	1613	-	-	902	824	1068	925	825	977
Stage 1	-	-	-	-	-	-	988	872	-	985	869	-
Stage 2	-	-	-	-	-	-	962	868	-	986	872	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1395	-	-	1613	-	-	863	819	1068	918	820	977
Mov Cap-2 Maneuver	-	-	-	-	-	-	863	819	-	918	820	-
Stage 1	-	-	-	-	-	-	982	867	-	979	869	-
Stage 2	-	-	-	-	-	-	924	868	-	976	867	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.4			0			9.1			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)			1395	-		1613	-	-	975			
HCM Lane V/C Ratio		0.007		_	_	-	_	_	0.04			
HCM Control Delay (s)		9.1	7.6	0	_	0	_	_	8.8			
HCM Lane LOS		A	A	A	_	A	_	_	A			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.1			
((****)												

Intersection						
Int Delay, s/veh	2.9					
		WED	NDT	NDD	CDI	CDT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	40	<b>^}</b>	•	4 =	4
Traffic Vol, veh/h	5	18	29	2	15	41
Future Vol, veh/h	5	18	29	2	15	41
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	72	72	72	72	72	72
Heavy Vehicles, %	20	6	21	0	12	13
Mvmt Flow	7	25	40	3	21	57
Major/Minor M	inor1	N	laior1		Majara	
			Major1		Major2	
Conflicting Flow All	141	42	0	0	43	0
Stage 1	42	-	-	-	-	-
Stage 2	99	-	-	-	-	-
Critical Hdwy	6.6	6.26	-	-	4.22	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	3.68	3.354	-	-	2.308	-
Pot Cap-1 Maneuver	811	1017	-	-	1504	-
Stage 1	936	-	-	-	-	-
Stage 2	882	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	800	1017	-	-	1504	-
Mov Cap-2 Maneuver	800	_	-	_	_	_
Stage 1	936	_	_	_	-	-
Stage 2	870	_	_	_	_	_
o tago 2	0.0					
Approach	WB		NB		SB	
HCM Control Delay, s	8.9		0		2	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NRR\	VBLn1	SBL	SBT
		INDI	אוטויי			ופט
Canacity (yah/h)		-	-	960	1504	-
Capacity (veh/h)				$\Lambda$ $\Lambda$ $\Lambda$ $\Lambda$		
HCM Lane V/C Ratio		-		0.033		-
HCM Lane V/C Ratio HCM Control Delay (s)		-	-	8.9	7.4	0
HCM Lane V/C Ratio						

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	₩ <u>₽</u>	וטיי	₩.	אופט
Traffic Vol, veh/h	0	5	55	0	0	0
Future Vol, veh/h	0	5	55	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		- Stop	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage	. # -	0	0	_	0	_
Grade, %	, <del>π</del> - -	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
		2	2			2
Heavy Vehicles, %	2			2	2	
Mvmt Flow	0	5	60	0	0	0
Major/Minor I	Major1	N	Major2	N	Minor2	
Conflicting Flow All	60	0		0	65	60
Stage 1	-		_	_	60	-
Stage 2	_	<u>-</u>	_	_	5	_
Critical Hdwy	4.12	_	_	_	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	-	_	_	_	5.42	_
Follow-up Hdwy	2.218	<u>-</u>	_		3.518	
Pot Cap-1 Maneuver	1544	_	_	_	941	1005
Stage 1	1044	_	<u>-</u>	_	963	1005
		-	-			
Stage 2	-	-	-	-	1018	-
Platoon blocked, %	1511	-	-	-	044	4005
Mov Cap-1 Maneuver	1544	-	-	-	941	1005
Mov Cap-2 Maneuver	-	-	-	-	941	-
Stage 1	-	-	-	-	963	-
Stage 2	-	-	-	-	1018	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	U		U		A	
I IOW LOS						
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR	SBL <sub>n1</sub>
Capacity (veh/h)		1544	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	-	-	0
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh)	)	0	-	-	-	-

Intersection	
Intersection Delay, s/veh	8.2
Intersection LOS	Α

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			↔	
Traffic Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Future Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Peak Hour Factor	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	1	0	4	0	0	0	6	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		8.3				0		8		
HCM LOS		-		Α				-		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	25%	100%	
Vol Thru, %	100%	100%	0%	0%	
Vol Right, %	0%	0%	75%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	0	0	4	4	
LT Vol	0	0	1	4	
Through Vol	0	0	0	0	
RT Vol	0	0	3	0	
Lane Flow Rate	0	0	6	6	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0	0	0.009	0.008	
Departure Headway (Hd)	3.914	3.914	5.21	4.96	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	0	0	690	725	
Service Time	1.924	1.923	3.216	2.968	
HCM Lane V/C Ratio	0	0	0.009	0.008	
HCM Control Delay	6.9	6.9	8.3	8	
HCM Lane LOS	N	N	Α	Α	
HCM 95th-tile Q	0	0	0	0	

Intersection						
Int Delay, s/veh	6.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	EDR	VVDL		INDL	NDR
Traffic Vol, veh/h	20	173	34	<b>↑</b> 62	248	4
Future Vol, veh/h	20	173	34	62	248	4
Conflicting Peds, #/hr	0	0	0	02	240	0
	Free	Free	Free	Free		
Sign Control RT Channelized	riee -	None			Stop	Stop None
	-	None -	130		-	None -
Storage Length			130	0	0	
Veh in Median Storage,						-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	23	197	39	70	282	5
Major/Minor M	lajor1	N	Major2	1	Minor1	
Conflicting Flow All	0	0	220	0	270	122
Stage 1	-	_	-	-	122	-
Stage 2	_	_	_	_	148	<u>-</u>
Critical Hdwy	_	_	4.16	_	6.45	6.7
Critical Hdwy Stg 1	_	_		_	5.45	-
Critical Hdwy Stg 2			_		5.45	_
Follow-up Hdwy	_	_	2.254	_	3.545	3.75
Pot Cap-1 Maneuver		_	1326	_	713	815
	-	-	1320	-	896	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	872	-
Platoon blocked, %	-	-	4000	-	000	045
Mov Cap-1 Maneuver	-	-	1326	-	692	815
Mov Cap-2 Maneuver	-	-	-	-	692	-
Stage 1	-	-	-	-	896	-
Stage 2	-	-	-	-	847	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		2.8		13.8	
HCM LOS	U		2.0		В	
I ICIVI LOS					Ь	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		694	-	_	1326	-
HCM Lane V/C Ratio		0.413	-		0.029	_
HCM Control Delay (s)		13.8	-	-		-
HCM Lane LOS		В	-	_	A	-
HCM 95th %tile Q(veh)		2	-	-	0.1	-

Intersection						
Int Delay, s/veh	2.5					
		EDD	MDI	MOT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>}</b>	000	<b>1</b>	<b>↑</b>	<b>Y</b>	
Traffic Vol, veh/h	179	226	13	297	102	14
Future Vol, veh/h	179	226	13	297	102	14
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	17	8	8	5	25	21
Mvmt Flow	190	240	14	316	109	15
Major/Minor Ma	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	430	0	654	310
Stage 1	-	-	700	-	310	310
Stage 2	_	_		_	344	_
Critical Hdwy		-	4.18	-	6.65	6.41
	-	_	4.10		5.65	0.41
Critical Hdwy Stg 1		-	-	-		
Critical Hdwy Stg 2	-	-	2 272	-	5.65	2 400
Follow-up Hdwy	-		2.272			3.489
Pot Cap-1 Maneuver	-	-	1098	-	397	688
Stage 1	-	-	-	-	695	-
Stage 2	-	-	-	-	669	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1098	-	392	688
Mov Cap-2 Maneuver	-	-	-	-	392	-
Stage 1	-	-	-	-	695	-
Stage 2	-	-	-	-	660	-
Approach	EB		WB		NB	
	0		0.3		17.4	
HCM LOS	U		0.5			
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		413	_	-	1098	-
HCM Lane V/C Ratio		0.299	-		0.013	_
HCM Control Delay (s)		17.4	_	_	8.3	_
HCM Lane LOS		С	_	_	A	_
HCM 95th %tile Q(veh)		1.2	_	_	0	_

Intersection												
Int Delay, s/veh	7.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	î,		ች	ĵ.			सी	7		4	
Traffic Vol, veh/h	0	175	47	211	183	5	82	2	225	5	0	2
Future Vol, veh/h	0	175	47	211	183	5	82	2	225	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	93	93	93
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	188	51	227	197	5	88	2	242	5	0	2
Major/Minor M	lajor1		ı	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	202	0	0	239	0	0	869	870	214	990	893	200
Stage 1	-	-	-	-	-	-	214	214	-	654	654	-
Stage 2	-	-	-	-	-	-	655	656	-	336	239	-
Critical Hdwy	4.1	-	-	4.26	-	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1382	-	-	1250	-	-	261	292	781	227	283	846
Stage 1	-	-	-	-	-	-	764	729	-	459	466	-
Stage 2	-	-	-	-	-	-	437	465	-	682	711	-
Platoon blocked, %		-	-		-	-						
	1382	-	-	1250	-	-	224	239	781	134	231	846
Mov Cap-2 Maneuver	-	-	-	-	-	-	224	239	-	134	231	-
Stage 1	-	-	-	-	-	-	764	729	-	459	381	-
Stage 2	-	-	-	-	-	-	357	380	-	469	711	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.5			17.1			26.4		
HCM LOS							С			D		
Minor Lane/Major Mvmt	N	NBLn1N	NBI n2	EBL	EBT	EBR	WBL	WBT	WBR S	SBL n1		
Capacity (veh/h)		224	781	1382	-		1250	-	-			
HCM Lane V/C Ratio		0.403	0.31	1302	_		0.182	_		0.043		
HCM Control Delay (s)		31.5	11.7	0	_	_	8.5	_	_			
HCM Lane LOS		D D	В	A	_	_	Α	_	<u>-</u>	20.4 D		
HCM 95th %tile Q(veh)		1.8	1.3	0	_	_	0.7	_	_	0.1		
		1.0	1.0				3.1			J. 1		

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		,,,,,,	4	11511	ሻ	\$	TIDIT	ሻ	<b>1</b>	UDIT
Traffic Vol, veh/h	17	20	12	29	25	171	12	85	19	122	120	27
Future Vol, veh/h	17	20	12	29	25	171	12	85	19	122	120	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	_	-	230	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-		0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	6	0	0	7	0	16	0	15	32	29	5	0
Mvmt Flow	18	22	13	32	27	186	13	92	21	133	130	29
Major/Minor	Minor2			Minor1		N	Major1			Major2		
Conflicting Flow All	646	550	145	557	554	103	159	0	0	113	0	0
Stage 1	411	411	-	129	129	-	-	-	-	-	-	-
Stage 2	235	139	-	428	425	-	-	-	_	-	_	-
Critical Hdwy	7.16	6.5	6.2	7.17	6.5	6.36	4.1	-	-	4.39	-	-
Critical Hdwy Stg 1	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.554	4	3.3	3.563	4	3.444	2.2	-	-	2.461	-	-
Pot Cap-1 Maneuver	379	446	908	433	443	915	1433	-	-	1324	-	-
Stage 1	610	598	-	863	793	-	-	-	-	-	-	-
Stage 2	759	785	-	595	590	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	263	398	908	375	395	915	1433	-	-	1324	-	-
Mov Cap-2 Maneuver	263	398	-	375	395	-	-	-	-	-	-	-
Stage 1	605	538	-	855	786	-	-	-	-	-	-	-
Stage 2	579	778	-	506	531	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.9			13.1			0.8			3.6		
HCM LOS	C			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR I	EBLn1V	VRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1433	וטוו	-		687	1324		ODIC			
HCM Lane V/C Ratio		0.009			0.139		0.1	_	_			
HCM Control Delay (s)		7.5	-	_	4	13.1	8	-	_			
HCM Lane LOS		7.5 A		_	13.9 C	13.1 B	A	_	_			
HCM 95th %tile Q(veh	)	0	_	_	0.5	1.6	0.3	_	_			
HOW JOHN JOHN GUVEN	7	U			0.0	1.0	0.0					

Intersection						
Int Delay, s/veh	0.3					
		14/5		NES	05:	05-
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽			4
Traffic Vol, veh/h	3	5	210	8	3	109
Future Vol, veh/h	3	5	210	8	3	109
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	0	20	19	0	0	34
Mvmt Flow	4	6	250	10	4	130
	-				-	
	linor1		//ajor1		Major2	
Conflicting Flow All	393	255	0	0	260	0
Stage 1	255	-	-	-	-	-
Stage 2	138	-	-	-	-	-
Critical Hdwy	6.4	6.4	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	_	-
Follow-up Hdwy	3.5	3.48	-	-	2.2	-
Pot Cap-1 Maneuver	615	742	-	-	1316	-
Stage 1	792	_	_	-	_	-
Stage 2	894	_	_	_	_	_
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	613	742	_	_	1316	_
Mov Cap-1 Maneuver	613	- 142	_	_	1310	_
Stage 1	792	-	_	-	-	-
_			-	-		
Stage 2	891	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.3		0		0.2	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)				688	1316	
HCM Lane V/C Ratio		_		0.014		_
HCM Control Delay (s)		_		10.3	7.7	0
HCM Lane LOS			-			A
HCM 95th %tile Q(veh)		-	-	B 0	A 0	
HOW Sour Mile Q(ven)		_	-	U	U	-

Intersection												
Int Delay, s/veh	0											
• ·												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	187	0	0	99	0
Future Vol, veh/h	1	0	0	0	0	0	0	187	0	0	99	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	231	0	0	122	0
Maiay/Minay	Aire a mo			Aire a red			1-1-4			1-10		
	/linor2	0.50		/linor1	0=0		Major1			//ajor2		
Conflicting Flow All	353	353	122	353	353	231	122	0	0	231	0	0
Stage 1	122	122	-	231	231	-	-	-	-	-	-	-
Stage 2	231	231	-	122	122	-	-	-	_	-	-	-
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	455	575	935	606	575	813	1478	-	-	1349	-	-
Stage 1	692	799	-	776	717	-	-	-	-	-	-	-
Stage 2	595	717	-	887	799	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	455	575	935	606	575	813	1478	-	-	1349	-	-
Mov Cap-2 Maneuver	455	575	-	606	575	-	-	-	-	-	-	-
Stage 1	692	799	-	776	717	-	-	-	-	-	-	-
Stage 2	595	717	-	887	799	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.9			0			0			0		
HCM LOS	В			A								
TOW LOO	U											
Minor Lane/Major Mvm	t	NBL	NBT	NRR	EBLn1V	VRI n1	SBL	SBT	SBR			
			HDT	ואטויו			1349	001	אנטט			
Capacity (veh/h)		1478	-	-	455	-		-	-			
HCM Control Polov (a)		-	-		0.003	-	-	-	-			
HCM Control Delay (s)		0	-	-	12.9	0	0	-	-			
HCM Lane LOS		A	-	-	В	Α	A	-	-			
HCM 95th %tile Q(veh)		0	-	-	0	-	0	-	-			

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	4	0	13	1	58	1	143	42	39	55	2
Future Vol, veh/h	0	4	0	13	1	58	1	143	42	39	55	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	<u>-</u>	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mvmt Flow	0	5	0	15	1	67	1	164	48	45	63	2
Major/Minor N	linor2		I	Minor1		1	Major1		1	Major2		
Conflicting Flow All	378	368	64	347	345	188	65	0	0	212	0	0
Stage 1	154	154	-	190	190	-	-	-	-	-	-	-
Stage 2	224	214	-	157	155	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4		3.572	4	3.426	2.2	-	-	2.452	-	-
Pot Cap-1 Maneuver	583	564	1006	596	581	824	1550	-	-	1218	-	-
Stage 1	853	774	-	798	747	-	-	-	-	-	-	-
Stage 2	783	729	-	831	773	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	519	542	1006	575	558	824	1550	-	-	1218	-	-
Mov Cap-2 Maneuver	519	542	-	575	558	-	-	-	-	-	-	-
Stage 1	852	745	-	797	746	-	-	-	-	-	-	-
Stage 2	718	728	-	794	744	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.7			10.3			0			3.3		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1550	-	-	542	760	1218	-	-			
HCM Lane V/C Ratio		0.001	-	-		0.109		-	-			
HCM Control Delay (s)		7.3	0	-	11.7	10.3	8.1	0	-			
HCM Lane LOS		Α	Α	-	В	В	Α	Α	-			
HCM 95th %tile Q(veh)		0	-	-	0	0.4	0.1	-	-			

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y	LDN	NDL	IND I	) 	אמט
Traffic Vol, veh/h		2	2	<b>€</b> 1	21	12
Future Vol, veh/h	9	2	3	62	21	12
· · · · · · · · · · · · · · · · · · ·				02		0
Conflicting Peds, #/hr	0	0	0		0	
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	0	0	8	14	0
Mvmt Flow	10	2	3	68	23	13
Major/Minor M	inor2	N	Major1	ı	/lajor2	
Conflicting Flow All	104	30	36	0	- najoiz	0
	30	-	-	-	-	-
Stage 1						
Stage 2	74	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	899	1050	1588	-	-	-
Stage 1	998	-	-	-	-	-
Stage 2	954	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	897	1050	1588	-	-	-
Mov Cap-2 Maneuver	897	-	-	-	_	-
Stage 1	996	_	_	_	_	_
Stage 2	954	_	_	_	_	_
Olago Z	001					
	EB		NB		SB	
Approach			0.3		0	
HCM Control Delay, s	9		0.0			
	9 A		0.0			
HCM Control Delay, s			0.0			
HCM Control Delay, s HCM LOS	A	NDL		ERI n1	QDT	QDD
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	A	NBL		EBLn1	SBT	SBR
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	A	1588	NBT I	921	-	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	A	1588 0.002	NBT I	921 0.013	-	SBR -
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	A	1588 0.002 7.3	NBT   - - 0	921 0.013 9	- - -	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	A	1588 0.002	NBT I	921 0.013	-	-

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	1	63	0	0	24	1
Future Vol, veh/h	0	0	0	0	0	0	1	63	0	0	24	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	82	82	82	82	82	82	82	82	82	82	82	82
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	1	77	0	0	29	1
Major/Minor M	linor2		1	Minor1		N	Major1		N	Major2		
Conflicting Flow All	109	109	30	109	109	77	30	0	0	77	0	0
Stage 1	30	30	-	79	79	-	-	-	-	-	-	-
Stage 2	79	79	-	30	30	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	874	785	1050	874	785	990	1596	-	-	1535	-	-
Stage 1	992	874	-	935	833	-	-	-	-	-	-	-
Stage 2	935	833	-	992	874	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	873	784	1050	873	784	990	1596	-		1535	-	-
Mov Cap-2 Maneuver	873	784	-	873	784	-	-	-	-	-	-	-
Stage 1	991	874	-	934	832	-	-	-	-	-	-	-
Stage 2	934	832	-	992	874	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0.1			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1596	-	-	-	-	1535	-	-			
HCM Lane V/C Ratio		0.001	-	-	-	-	-	-	-			
HCM Control Delay (s)		7.3	0	-	0	0	0	-	-			
HCM Lane LOS		Α	A	-	A	A	A	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	2.1					
		WED	NOT	NDD	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		ĵ»		_	4
Traffic Vol, veh/h	12	3	50	40	25	31
Future Vol, veh/h	12	3	50	40	25	31
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	81	81	81	81	81	81
Heavy Vehicles, %	17	0	14	23	12	26
Mvmt Flow	15	4	62	49	31	38
Maiau/Minau	1:1		1-:1		Ma:0	
	Minor1		Major1		Major2	
Conflicting Flow All	187	87	0	0	111	0
Stage 1	87	-	-	-	-	-
Stage 2	100	-	-	-	-	-
Critical Hdwy	6.57	6.2	-	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	-	-	-	-	-
Follow-up Hdwy	3.653	3.3	-	-		-
Pot Cap-1 Maneuver	769	977	-	-	1419	-
Stage 1	900	-	-	-	-	-
Stage 2	888	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	752	977	-	-	1419	-
Mov Cap-2 Maneuver	752	_	-	-	-	-
Stage 1	900	-	_	_	_	_
Stage 2	868	_	_	_	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	9.7		0		3.4	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NRRV	VBLn1	SBL	SBT
		-	-		1419	-
Capacity (veh/h) HCM Lane V/C Ratio				0.024		
HCM Control Delay (s)		-	-	9.7	7.6	0
		-	-	9.7 A	7.6 A	A
HCM Land LOC						A
HCM Lane LOS HCM 95th %tile Q(veh)			_	0.1	0.1	- , ,

Intersection												
Int Delay, s/veh	6.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			स	7		4			4	
Traffic Vol, veh/h	52	64	3	0	22	52	1	0	2	132	1	12
Future Vol, veh/h	52	64	3	0	22	52	1	0	2	132	1	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	76	76	76	76	76	76	76	76	76	76	76	76
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	68	84	4	0	29	68	1	0	3	174	1	16
Major/Minor N	//ajor1		1	Major2		1	Minor1		1	Minor2		
Conflicting Flow All	97	0	0	88	0	0	294	319	86	253	253	29
Stage 1	-	-	-	-	-	-	222	222	-	29	29	-
Stage 2	-	-	-	-	-	-	72	97	-	224	224	-
Critical Hdwy	4.39	-	-	4.1	-	-	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Follow-up Hdwy	2.461	-	-	2.2	-	-	3.5	4	3.3	3.689	4	3.975
Pot Cap-1 Maneuver	1343	-	-	1520	-	-	662	601	978	663	654	870
Stage 1	-	-	-	-	-	-	785	723	-	941	875	-
Stage 2	-	-	-	-	-	-	943	819	-	738	722	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1343	-	-	1520	-	-	623	569	978	634	619	870
Mov Cap-2 Maneuver	-	-	-	-	-	-	623	569	-	634	619	-
Stage 1	-	-	-	-	-	-	743	685	-	891	875	-
Stage 2	-	-	-	-	-	-	924	819	-	697	684	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.4			12.9		
HCM LOS							Α			В		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)			1343	-		1520	-	-	648			
HCM Lane V/C Ratio		0.005		_	_	-	_		0.294			
HCM Control Delay (s)		9.4	7.8	0	_	0	_	_				
HCM Lane LOS		A	A	A	_	A	_	-	В			
HCM 95th %tile Q(veh)		0	0.2	-	-	0	-	-	1.2			
2 22 / 0 2 (1011)												

Intersection												
Int Delay, s/veh	3.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>	LDIK	ሻ	1≯	TIDIC	HUL	4	HOIL	JDL	4	ODIN
Traffic Vol, veh/h	9	280	8	28	98	99	1	5	27	70	7	7
Future Vol, veh/h	9	280	8	28	98	99	1	5	27	70	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	_	_	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	11	329	9	33	115	116	1	6	32	82	8	8
Major/Minor N	Major1		1	Major2		ľ	Minor1			Minor2		
Conflicting Flow All	231	0	0	338	0	0	603	653	334	614	599	173
Stage 1	-	-	-	-	-	-	356	356	-	239	239	-
Stage 2	-	-	-	-	-	-	247	297	-	375	360	-
Critical Hdwy	4.32	-	-	4.17	-	-	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Follow-up Hdwy	2.398	-	-	2.263	-	-	3.5	4	3.3	3.581	4.126	3.426
Pot Cap-1 Maneuver	1228	-	-	1194	-	-	414	389	712	394	400	840
Stage 1	-	-	-	-	-	-	666	633	-	749	686	-
Stage 2	-	-	-	-	-	-	761	671	-	632	606	-
Platoon blocked, %	40	-	-		-	-						
Mov Cap-1 Maneuver	1228	-	-	1194	-	-	392	375	712	362	385	840
Mov Cap-2 Maneuver	-	-	-	-	-	-	392	375	-	362	385	-
Stage 1	-	-	-	-	-	-	660	627	-	742	667	-
Stage 2	-	-	-	-	-	-	724	652	-	593	601	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			11.3			17.7		
HCM LOS							В			С		
Minor Lane/Major Mvm	nt 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)			1228	-	-	1194	-	-				
HCM Lane V/C Ratio		0.063		-		0.028	-	_	0.259			
HCM Control Delay (s)		11.3	8	-	-	8.1	-	-				
HCM Lane LOS		В	Α	-	-	Α	-	-	С			
HCM 95th %tile Q(veh)	)	0.2	0	-	-	0.1	-	-	1			

13· I_82	Southhound	On-Ramp/I-82	2 Southhound	Off-Ramr	& Route	1/1
13. 1-02	Southbound	Oli-Nallip/i-04	2 300011000110	OII-Naiii	) & Noule	14

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₽			4						4	<b>02</b> 11
Traffic Vol, veh/h	0	94	261	13	164	0	0	0	0	1	4	53
Future Vol, veh/h	0	94	261	13	164	0	0	0	0	1	4	53
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	-	-	_	-	-
Veh in Median Storage,	# -	0	_	_	0	_	_	0	-	_	0	-
Grade, %	_	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	89	89	89	89	89	89	92	92	92	89	89	89
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mvmt Flow	0	106	293	15	184	0	0	0	0	1	4	60
Major/Minor M	aior1		N	/laior2					N	Minor2		
	ajor1	^		Major2	^	0					640	104
Conflicting Flow All	-	0	0	399	0	0				467	613	184
Stage 1	-	-	-	-	-	-				214	214	-
Stage 2	-	-	-	4 OF	-	-				253	399	- C E
Critical Hdwy	-	-	-	4.25	-	-				6.4	6.75	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	5.75	-
Critical Hdwy Stg 2	-	-	-	2 225	-	-				5.4	5.75	2 57
Follow-up Hdwy	_	-		2.335	-	-				3.5 558	4.225	3.57 791
Pot Cap-1 Maneuver	0	-	-	1093	-	0				826	685	
Stage 1	0	-	-	-	-	0				794	564	-
Stage 2 Platoon blocked, %	U	-	-	-	-	U				194	304	-
		-	-	1093	-					550	0	791
Mov Cap-1 Maneuver	-	_	-	1093	_	-				550	0	191
Mov Cap-2 Maneuver Stage 1	-	-	-	-	-	-				826	0	-
Stage 1 Stage 2	-	-	-	-	-	-				782	0	-
Slaye Z	-	_	-	-	-	-				102	U	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.6						10		
HCM LOS										В		
Minor Lane/Major Mvmt		EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		_		1093		785						
HCM Lane V/C Ratio		_		0.013		0.083						
HCM Control Delay (s)		-	-	8.3	0	10						
HCM Lane LOS		_	_	A	A	В						
HCM 95th %tile Q(veh)		-	-	0	-	0.3						
						3.0						

Intersection													
Int Delay, s/veh	10.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			f)			4			<u> </u>	02.1	
Traffic Vol, veh/h	91	4	0	0	6	5	171	10	6	0	0	0	
Future Vol, veh/h	91	4	0	0	6	5	171	10	6	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-	
Veh in Median Storage,	# -	0	_	_	0	_	_	0	_	_	0	_	
Grade, %	_	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	93	93	93	93	93	93	93	93	93	92	92	92	
Heavy Vehicles, %	7	25	0	0	50	0	24	50	50	2	2	2	
Mvmt Flow	98	4	0	0	6	5	184	11	6	0	0	0	
WWW.CT IOW	00	•					101	• •			J		
Major/Minor N	/lajor1		P	Major2			Minor1						
Conflicting Flow All	11	0		viajui 2 -	_	0	209	211	4				
Stage 1	- 11	-	-	-	_	-	200	200	-				
Stage 2	_		_	_	_	_	9	11	_				
Critical Hdwy	4.17	-		_		_	6.64	7	6.7				
Critical Hdwy Stg 1	4.17	-	-	_	_	_	5.64	6	0.7				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.64	6	_				
	2.263	_	_	_	-	_		4.45	3.75				
' '	1576	-	0	0			733	610	955				
Pot Cap-1 Maneuver	1370	_	0	0	-	-	784	654					
Stage 1	-	-	0	0			960	800	-				
Stage 2	-	-	U	U	-	-	900	000	-				
Platoon blocked, %	1576	-			-	-	600	٥	OFF				
Mov Cap-1 Maneuver	1576	-	-	-	-	-	688	0	955				
Mov Cap-2 Maneuver	-	-	-	-	-	-	688	0	-				
Stage 1	-	-	-	-	-	-	735	0	-				
Stage 2	-	-	-	-	-	-	960	0	-				
A	ED.			VA/D			ND						
Approach	EB			WB			NB						
HCM Control Delay, s	7.1			0			12.3						
HCM LOS							В						
						1115							
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		695	1576	-	-	-							
HCM Lane V/C Ratio		0.289		-	-	-							
HCM Control Delay (s)		12.3	7.4	0	-	-							
HCM Lane LOS HCM 95th %tile Q(veh)		В	Α	Α	-	-							
		1.2	0.2	_	_	_							

Intersection												
Int Delay, s/veh	6.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		î,			र्स						4	
Traffic Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	_	_	-	-	-	_	-	-	_	-	-	_
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	71	71	71	71	71	71	92	92	92	71	71	71
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	3	10	10	3	0	0	0	0	8	15	8
Major/Minor N	/lajor1		1	Major2					N	Minor2		
Conflicting Flow All	-	0	0	13	0	0				31	36	3
Stage 1	-	-	-	-	-	-				23	23	-
Stage 2	-	-	-	-	-	-				8	13	-
Critical Hdwy	-	-	-	4.27	-	-				6.57	7.23	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.57	6.23	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.57	6.23	-
Follow-up Hdwy	-	-	-	2.353	-	-				3.653	4.657	3.3
Pot Cap-1 Maneuver	0	-	-	1513	-	0				946	736	1087
Stage 1	0	-	-	-	-	0				962	754	-
Stage 2	0	-	-	-	-	0				977	762	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1513	-	-				939	0	1087
Mov Cap-2 Maneuver	-	-	-	-	-	-				939	0	-
Stage 1	-	-	-	-	-	-				962	0	-
Stage 2	-	-	-	-	-	-				970	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.8						8.7		
HCM LOS										Α		
Minor Lane/Major Mvmt	t	EBT	EBR			SBLn1						
Capacity (veh/h)		-	-	1513	-	1008						
HCM Lane V/C Ratio		-	-	0.007		0.032						
HCM Control Delay (s)		-	-	7.4	0	8.7						
HCM Lane LOS		-	-	Α	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	-	0.1						

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ની			<del>(</del> Î			4				
Traffic Vol. veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Future Vol, veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	-	_	None	_	<u> </u>	None	-	_	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	75	75	75	75	75	75	75	75	92	92	92
Heavy Vehicles, %	0	0	0	0	13	11	100	33	0	2	2	2
Mvmt Flow	7	4	0	0	11	24	1	4	12	0	0	0
Major/Minor N	/lajor1		ı	Major2		N	Minor1					
Conflicting Flow All	35	0	-	-	-	0	41	53	4			
Stage 1	-	-	-	-	-	-	18	18	-			
Stage 2	-	-	-	-	-	-	23	35	-			
Critical Hdwy	4.1	-	-	-	-	-	7.4	6.83	6.2			
Critical Hdwy Stg 1	-	-	-	-	-	-	6.4	5.83	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	6.4	5.83	-			
Follow-up Hdwy	2.2	-	-	-	-	-	4.4	4.297	3.3			
Pot Cap-1 Maneuver	1589	-	0	0	-	-	771	782	1085			
Stage 1	-	-	0	0	-	-	801	822	-			
Stage 2	-	-	0	0	-	-	796	808	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1589	-	-	-	-	-	768	0	1085			
Mov Cap-2 Maneuver	-	-	-	-	-	-	768	0	-			
Stage 1	-	-	-	-	-	-	798	0	-			
Stage 2	-	-	-	-	-	-	796	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	4.5			0			8.5					
HCM LOS							Α					
Minor Lane/Major Mvm	t <u></u> 1	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1042	1589	-	-	-						
HCM Lane V/C Ratio		0.017	0.004	-	-	-						
HCM Control Delay (s)		8.5	7.3	0	-	-						
HCM Lane LOS		Α	Α	Α	-	-						
HCM 95th %tile Q(veh)		0.1	0	-	-	-						
,												

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Future Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	75	75	75	75	75	75	75	75	75	75	75	75
Heavy Vehicles, %	0	0	25	0	0	0	18	0	0	0	0	0
Mvmt Flow	1	9	5	0	19	0	16	3	0	0	0	0
Major/Minor M	lajor1		ľ	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	19	0	0	14	0	0	33	33	12	34	35	19
Stage 1	-	-	-	-	-	-	14	14	-	19	19	-
Stage 2	_	-	-	-	_	-	19	19	_	15	16	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.28	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.662	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1611	-	-	1617	-	-	935	864	1074	978	861	1065
Stage 1	-	-	-	-	-	-	966	888	-	1005	884	-
Stage 2	-	-	-	-	-	-	960	884	-	1010	886	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1611	-	-	1617	-	-	934	863	1074	975	860	1065
Mov Cap-2 Maneuver	-	-	-	-	-	-	934	863	-	975	860	-
Stage 1	-	-	-	-	-	-	965	887	-	1004	884	-
Stage 2	-	-	-	-	-	-	960	884	-	1006	885	-
,												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			9			0		
HCM LOS				•			A			A		
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SRI n1			
Capacity (veh/h)	- '	923	1611	-	-	1617	-	ייוטויי	JULIT			
HCM Lane V/C Ratio			0.001	-	-	1017		_	-			
		0.02	7.2	0	-	0	-	-	0			
HCM Control Delay (s) HCM Lane LOS		A	7.2 A	A	-	A	-		A			
HCM 95th %tile Q(veh)		0.1	0	- A	-	0	-	-	- A			
HOW SOUT /OUIE Q(VEII)		U. I	U	-	-	U	-	-	-			

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	64	0	32	7	0	0	0	0	106	1	5
Future Vol, veh/h	0	64	0	32	7	0	0	0	0	106	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	_	None	_	_	None	_	_	None	-	_	None
Storage Length	-	-	-	-	_	-	-	-	-	-	-	-
Veh in Median Storage	.# -	0	-	_	0	-	_	0	-	_	0	_
Grade, %	-	0	_	_	0	_	_	0	_	-	0	-
Peak Hour Factor	91	91	91	91	91	91	92	92	92	91	91	91
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	70	0	35	8	0	0	0	0	116	1	5
Major/Minor N	Major1		N	Major2					N	/linor2		
Conflicting Flow All	viajor i -	0	0	70	0	0				148	148	8
Stage 1			U	70						78	78	
Stage 1	-	-	-		-	-				70	70	-
	-	-	-	4.23	-	-				6.43	7.5	6.4
Critical Hdwy	-	-	-	4.23	-	-						
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	6.5	-
Critical Hdwy Stg 2	-	-	-	- 247	-	-				5.43	6.5	2.40
Follow-up Hdwy	-	-	-	2.317	-	-				3.527	4.9	3.48
Pot Cap-1 Maneuver	0	-	-	1464	-	0				842	596	1024
Stage 1	0	-	-	-	-	0				943	673	-
Stage 2	0	-	-	-	-	0				950	679	-
Platoon blocked, %		-	-	1101	-					000		4004
Mov Cap-1 Maneuver	-	-	-	1464	-	-				822	0	1024
Mov Cap-2 Maneuver	-	-	-	-	-	-				822	0	-
Stage 1	-	-	-	-	-	-				943	0	-
Stage 2	-	-	-	-	_	-				927	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.2						10.1		
HCM LOS										В		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)			-	1464	-	829						
HCM Lane V/C Ratio		<u>-</u>		0.024		0.148						
HCM Control Delay (s)				7.5	0	10.1						
HCM Lane LOS		<u> </u>	_	7.5 A	A	В						
HCM 95th %tile Q(veh)		_	_	0.1	-	0.5						
HOW JOHN JOHN Q(VEII)		_	_	U. I		0.0						

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			<b>f</b>			4				
Traffic Vol, veh/h	41	129	0	0	30	76	9	4	119	0	0	0
Future Vol, veh/h	41	129	0	0	30	76	9	4	119	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	-	-	None	-	-	None	_	-	None
Storage Length	-	-	-	-	_	-	-	-	-	_	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	_	0	-
Grade, %	_	0	-	-	0	-	_	0	-	-	0	-
Peak Hour Factor	93	93	93	93	93	93	93	93	93	92	92	92
Heavy Vehicles, %	17	7	2	0	12	4	0	25	10	2	2	2
Mvmt Flow	44	139	0	0	32	82	10	4	128	0	0	0
Major/Minor N	Major1			Major2		N	Minor1					
	114	0	_	viajoiz -			300	341	139			
Conflicting Flow All Stage 1					-	0	227	227				
•	-	-	-	-	-	-	73	114	-			
Stage 2	4.27	-	-	-	-	-		6.75	6.3			
Critical Hdwy	4.27	-	-	-	-	-	6.4 5.4					
Critical Hdwy Stg 1	-	-	-	-	-	-		5.75	-			
Critical Hdwy Stg 2	2 252	-	-	-	-	-	5.4	5.75	2 20			
Follow-up Hdwy	2.353	-	-	-	-	-	3.5	4.225	3.39			
Pot Cap-1 Maneuver	1387	-	0	0	-	-	696	546	888			
Stage 1	-	-	0	0	-	-	815	675	-			
Stage 2	-	-	0	0	-	-	955	759	-			
Platoon blocked, %	1207	-			-	-	670	0	000			
Mov Cap-1 Maneuver	1387	-	-	-	-	-	672	0	888			
Mov Cap-2 Maneuver	-	-	-	-	-	-	672	0	-			
Stage 1	-	-	-	-	-	-	787	0	-			
Stage 2	-	-	-	-	-	-	955	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	1.9			0			10					
HCM LOS							В					
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		868	1387	-	1101	-						
HCM Lane V/C Ratio			0.032		_	_						
HCM Control Delay (s)		10	7.7	0	<u>-</u>							
HCM Lane LOS		В	7.7 A	A								
HCM 95th %tile Q(veh)		0.6	0.1	- A	-	-						
How som wille Q(ven)		0.0	0.1	-	-	-						

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	245	0	0	99	3	1	1	4	1	0	6
Future Vol, veh/h	3	245	0	0	99	3	1	1	4	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	275	0	0	111	3	1	1	4	1	0	7
Major/Minor N	1ajor1		ľ	Major2		1	Minor1		N	/linor2		
Conflicting Flow All	114	0	0	275	0	0	397	395	275	397	394	113
Stage 1	-	-	-	-	-	-	281	281	-	113	113	-
Stage 2	-	-	-	-	-	-	116	114	-	284	281	-
Critical Hdwy	4.1	_	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.453
Pot Cap-1 Maneuver	1488	-	-	1300	-	-	567	545	769	567	546	901
Stage 1	-	-	-	-	-	-	730	682	-	897	806	-
Stage 2	-	-	-	-	-	-	894	805	-	727	682	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1488	_	-	1300	-	-	562	544	769	562	545	901
Mov Cap-2 Maneuver	-	-	-	-	-	-	562	544	-	562	545	-
Stage 1	-	_	-	-	-	-	729	681	-	895	806	-
Stage 2	-	-	-	-	-	-	887	805	-	720	681	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0			10.3			9.4		
HCM LOS							В			Α		
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		680	1488	-		1300	_	_				
HCM Lane V/C Ratio				_	_		-		0.009			
HCM Control Delay (s)		10.3	7.4	0	_	0	_	_				
HCM Lane LOS		В	Α	A	_	A	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection							
Int Delay, s/veh	5.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	T T	<u></u>	WB1 <b>♣</b>	אטא	SBL 1	JUN.	
Traffic Vol, veh/h	166	55	56	28	24	67	
Future Vol, veh/h	166	55	56	28	24	67	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-		-	None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage	e,# -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	175	58	59	29	25	71	
Major/Minor I	Major1	N	Major2	N	/linor2		
Conflicting Flow All	88	0	-	0	482	74	
Stage 1	-	-	_	-	74	-	
Stage 2	_	_	-	-	408	-	
Critical Hdwy	4.15	_	-	-	6.4	6.26	
Critical Hdwy Stg 1	-	_	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.245	-	-	-	3.5	3.354	
Pot Cap-1 Maneuver	1489	-	-	-	547	977	
Stage 1	-	-	-	-	954	-	
Stage 2	-		-	-	676	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1489	-	-	-	482	977	
Mov Cap-2 Maneuver	-	-	-	-	482	-	
Stage 1	-	-	-	-	841	-	
Stage 2	-	-	-	-	676	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.8		0		10		
HCM LOS					В		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WRD	SBLn1 S	RI n2
	TC .	1489	LDI	AADI	יוטיי	482	
Capacity (veh/h) HCM Lane V/C Ratio		0.117	_	-	_	0.052	977
HCM Control Delay (s)		7.7	-	-	-	12.9	9
HCM Lane LOS		Α.	-	-	-	12.9 B	A
HCM 95th %tile Q(veh	1	0.4	-	-	_	0.2	0.2
HOW JOHN JOHN WINE WINE	1	0.4	_			U.Z	U.Z

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		*	f)			4			4	
Traffic Vol, veh/h	46	32	4	1	45	4	8	16	7	0	2	21
Future Vol, veh/h	46	32	4	1	45	4	8	16	7	0	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	55	39	5	1	54	5	10	19	8	0	2	25
Major/Minor I	Major1		ľ	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	59	0	0	44	0	0	224	213	42	220	213	57
Stage 1	-	-	-	-	-	-	152	152	-	59	59	-
Stage 2	-	_	-	-	-	-	72	61	-	161	154	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.56	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.56	-	6.1	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4.054	3.426	3.5	4	3.39
Pot Cap-1 Maneuver	1489	-	-	1577	-	-	736	677	996	740	688	987
Stage 1	-	-	-	-	-	-	855	764	-	958	850	-
Stage 2	-	-	-	-	-	-	943	836	-	846	774	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1489	-	-	1577	-	-	695	651	996	696	662	987
Mov Cap-2 Maneuver	-	-	-	-	-	-	695	651	-	696	662	-
Stage 1	-	-	-	-	-	-	823	736	-	923	849	-
Stage 2	-	-	-	-	-	-	916	835	-	787	745	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.2			0.1			9.4			8.9		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		859	1489	-	-	1577	_	-	947			
HCM Lane V/C Ratio		0.043		-	-	0.001	-	_	0.029			
HCM Control Delay (s)		9.4	7.5	-	-	7.3	_	-	8.9			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh)	)	0.1	0.1	-	_	0	_	-	0.1			

2.3					
EBT	EBR	WBL	WBT	NBL	NBR
<b>1</b>	LDIT	ነ ነ	<b>†</b>	¥	HEIN
	1				14
					14
					0
					Stop
					None
_					-
# N					_
					<u>-</u>
					61
					7
				-	23
04	2	0	07	10	23
Major1	N	Major2	N	Minor1	
0	0	66	0	148	65
-	-	-	-	65	-
-	-	-	-	83	-
-	-	4.1	-	6.4	6.27
-	_	_	_		_
_	_	-	_		-
_	_	2.2	_		3.363
_	_		_		985
	_	-	_		-
_	_	_			_
	_			J-10	
	_	1549		845	985
	_				-
					-
	_				
-		-	-	940	-
EB		WB		NB	
		0.0			
it N		EBT	EBR		WBT
		-			-
	0.043	-	-		-
	9.1	-	-	7.3	-
	Α	-	-	Α	-
)	0.1	_	_	0	_
	39 39 0 Free - - 0, # 0 0 61 26 64 Major1 0 - - - - - - -	39 1 39 1 39 1 0 0 Free Free - None	39 1 5 39 1 5 39 1 5 0 0 0 0 Free Free Free Free - None 150 0,# 0 61 61 61 61 26 0 0 64 2 8  Major1 Major2 0 0 66 4.1 2.2 - 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549 1549	39	39

Intersection						
Int Delay, s/veh	0.7					
		WDD	NDT	NDD	CDI	CDT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	À	^	<b>}</b>	^	0	र्स्
Traffic Vol, veh/h	2	0	16	0	0	6
Future Vol, veh/h	2	0	16	0	0	6
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	46	46	46	46	46	46
Heavy Vehicles, %	0	0	0	0	0	17
Mvmt Flow	4	0	35	0	0	13
Major/Minor M	linor1	N	/lajor1	N	/lajor2	
Conflicting Flow All	48	35	0	0	35	0
Stage 1	35	-	-	-	-	-
Stage 2	13	_	_		_	_
Critical Hdwy	6.4	6.2	-	_	4.1	_
Critical Hdwy Stg 1	5.4	0.2	_	_	4.1	_
Critical Hdwy Stg 2	5.4	_	-	-		-
Follow-up Hdwy	3.5	3.3	-	_	2.2	_
Pot Cap-1 Maneuver	967	1044	-	-	1589	-
Stage 1	993	-	-	_	1303	_
	1015	-	-	-		-
Platoon blocked, %	1015	-	_	-	_	_
	967	1044	-	_	1589	-
Mov Cap-1 Maneuver			-	-		-
Mov Cap-2 Maneuver	967	-	-	-	-	-
Stage 1	993	-	-	-	-	-
		_	-	-	-	-
	1015					
	1015					
Stage 2			NB		SB	
Stage 2 Approach	WB		NB 0		SB 0	
Stage 2  Approach HCM Control Delay, s	WB 8.7		NB 0		SB 0	
Stage 2 Approach	WB					
Stage 2  Approach HCM Control Delay, s HCM LOS	WB 8.7 A		0	M/DL 4	0	CDT
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt	WB 8.7 A	NBT	0 NBRW	VBLn1	0 SBL	SBT
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h)	WB 8.7 A	NBT -	0 NBRW	967	0 SBL 1589	SBT_
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	WB 8.7 A	NBT -	0 NBRV -	967 0.004	0 SBL 1589	-
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	WB 8.7 A	NBT - -	NBRV - -	967 0.004 8.7	0 SBL 1589	- - -
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	WB 8.7 A	NBT -	0 NBRV -	967 0.004	0 SBL 1589	-

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	7/	LUIT	NDL	4	301 <b>♣</b>	אופט
Traffic Vol. veh/h	0	0	0	21	5	0
Future Vol, veh/h	0	0	0	21	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Clop	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage,		_	_	0	0	_
Grade, %	, # 0	_	_	0	0	_
Peak Hour Factor	65	65	65	65	65	65
Heavy Vehicles, %	0	0	0	5	20	0
Mymt Flow	0	0	0	32	8	0
IVIVIII( I IOW	U	U	U	52	U	U
	/linor2		Major1		/lajor2	
Conflicting Flow All	40	8	8	0	-	0
Stage 1	8	-	-	-	-	-
Stage 2	32	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	977	1080	1625	-	-	-
Stage 1	1020	-	-	-	-	-
Stage 2	996	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	977	1080	1625	-	-	-
Mov Cap-2 Maneuver	977	-	-	-	-	-
Stage 1	1020	-	-	-	-	-
Stage 2	996	-	-	-	-	-
J						
Annroach	EB		ND		SB	
Approach			NB			
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1625	-	_	_	_
HCM Lane V/C Ratio		-	_	_	_	_
HCM Control Delay (s)		0	-	0	_	-
HCM Lane LOS		Ā	_	A	_	_
HCM 95th %tile Q(veh)		0	-	-	_	-
		•				

Intersection						
Int Delay, s/veh	5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	^	40		<b>₽</b>	4
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		_	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	60	60	60	60	60	60
Heavy Vehicles, %	25	33	0	0	0	33
Mvmt Flow	7	10	27	18	5	2
Major/Minor I	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	78	6	7	0	-	0
Stage 1	6	-	-	-	_	-
Stage 2	72	<u>-</u>	_	_	_	_
Critical Hdwy	6.65	6.53	4.1		_	
Critical Hdwy Stg 1	5.65	0.00	7.1	_	_	_
Critical Hdwy Stg 2	5.65		-	-	_	-
Follow-up Hdwy	3.725	3.597	2.2	-	_	-
	871	993	1627	_		_
Pot Cap-1 Maneuver	960		1021	-	-	-
Stage 1		-	-	-	-	-
Stage 2	896	-	-	-	-	-
Platoon blocked, %	050	000	4007	-	-	-
Mov Cap-1 Maneuver	856	993	1627	-	-	-
Mov Cap-2 Maneuver	856	-	-	-	-	-
Stage 1	944	-	-	-	-	-
Stage 2	896	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		4.3		0	
HCM LOS	Α		7.0		U	
TIOW LOO						
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1627	-	933	-	-
HCM Lane V/C Ratio		0.016	-	0.018	-	-
HCM Control Delay (s)		7.2	0	8.9	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)	)	0.1	-	0.1	-	-

Intersection												
Int Delay, s/veh	3.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	28	40	1	1	8	4	1	1	0	4	2	11
Future Vol, veh/h	28	40	1	1	8	4	1	1	0	4	2	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	84	84	84	84	84	84	84	84	84	84
Heavy Vehicles, %	11	18	100	0	0	0	0	0	0	25	0	0
Mvmt Flow	33	48	1	1	10	5	1	1	0	5	2	13
Major/Minor I	Major1		ľ	Major2		ı	Minor1		- 1	Minor2		
Conflicting Flow All	15	0	0	49	0	0	137	132	49	130	130	13
Stage 1	-	-	-	-	-	-	115	115	-	15	15	-
Stage 2	-	-	-	-	-	-	22	17	-	115	115	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.5	6.2	7.35	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4	3.3	3.725	4	3.3
Pot Cap-1 Maneuver	1546	-	-	1571	-	-	838	762	1025	792	764	1073
Stage 1	-	-	-	-	-	-	895	804	-	949	887	-
Stage 2	-	-	-	-	-	-	1002	885	-	837	804	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1546	-	-	1571	-	-	811	744	1025	777	746	1073
Mov Cap-2 Maneuver	-	-	-	-	-	-	811	744	-	777	746	-
Stage 1	-	-	-	-	-	-	875	786	-	928	886	-
Stage 2	-	-	-	-	-	-	986	884	-	817	786	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3			0.6			9.7			8.9		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		776		-	-	1571	-	-	940			
HCM Lane V/C Ratio		0.003		-		0.001	-	-	0.022			
HCM Control Delay (s)		9.7	7.4	0	-	7.3	0	-	8.9			
HCM Lane LOS		Α	Α	A	-	Α	A	-	Α			
HCM 95th %tile Q(veh)	)	0	0.1	-	-	0	-	-	0.1			

Intersection						
Int Delay, s/veh	4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	7/	וטוי	<b>1</b>	TIDIT	ODL	<u>€</u>
Traffic Vol, veh/h	<b>T</b> 5	47	73	8	53	35
Future Vol, veh/h	5	47	73	8	53	35
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	riee -	None		None
	0		-	None	-	None
Storage Length		-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	- 04	0
Peak Hour Factor	81	81	81	81	81	81
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	6	58	90	10	65	43
Major/Minor I	Minor1	N	Major1		Major2	
Conflicting Flow All	268	95	0	0	100	0
Stage 1	95	95	-	-	100	-
Stage 2	173	_	_		_	_
Critical Hdwy	6.6	6.2	_		4.12	-
Critical Hdwy Stg 1	5.6	0.2		_	4.12	_
	5.6		-	_		-
Critical Hdwy Stg 2			-	-		
Follow-up Hdwy	3.68	3.3	-	-	2.218	-
Pot Cap-1 Maneuver	684	967	-	-	1493	-
Stage 1	886	-	-	-	-	-
Stage 2	815	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	653	967	-	-	1493	-
Mov Cap-2 Maneuver	653	-	-	-	-	-
Stage 1	886	-	-	-	-	-
Stage 2	778	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.2		0		4.5	
HCM LOS	9.2 A		U		4.5	
HOW LOS	А					
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	924	1493	-
HCM Lane V/C Ratio		-	-	0.069	0.044	-
HCM Control Delay (s)		-	-	9.2	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Intersection						
Intersection Delay, s/ve	eh 7.3					
Intersection LOS	Α					
M 1	EDI	EDT	MOT	WDD	ODI	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		-4	Þ		¥	
Traffic Vol, veh/h	0	65	15	0	0	0
Future Vol, veh/h	0	65	15	0	0	0
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	71	16	0	0	0
Number of Lanes	0	1	1	0	1	0
Approach		EB	WB		SB	
Opposing Approach		WB	EB			
Opposing Lanes		1	1		0	
Conflicting Approach L	eft	SB			WB	
Conflicting Lanes Left		1	0		1	
Conflicting Approach R	Riaht		SB		EB	
Conflicting Lanes Right		0	1		1	
HCM Control Delay	•	7.3	7.1		0	
HCM LOS		A	A			
TIOWI LOO		71	71			
Lane	E		VBLn1			
Vol Left, %		0%	0%	0%		
Vol Thru, %		100%	100%	100%		
Vol Right, %		0%	0%	0%		
Sign Control		Stop	Stop	Stop		
Traffic Vol by Lane		65	15	0		
LT Vol		0	0	0		
Through Vol		65	15	0		
RT Vol		03	0	0		
Lane Flow Rate		71	16	0		
Geometry Grp		1	1	1		
Degree of Util (X)		0.077		0		
Departure Headway (H	ld)		3.986			
Convergence, Y/N		Yes	Yes	Yes		
Сар		913	901	0		
Service Time		1.949	1.998	2.119		
HCM Lane V/C Ratio		0.078	0.018	0		
HCM Control Delay		7.3	7.1	7.1		
HCM Lane LOS		Α	Α	N		
LIOM OF the tile O		0.0	0.4	^		

0.2

0.1

0

HCM 95th-tile Q

Intersection												
Intersection Delay, s/veh	n 7											
Intersection LOS	A											
intersection Loo	71											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Future Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Peak Hour Factor	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69	0.69
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0
Mvmt Flow	1	0	0	0	0	4	0	1	1	4	3	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB				WB			NB		SB		
Opposing Approach	WB				EB			SB		NB		
Opposing Lanes	1				1			1		1		
Conflicting Approach Le	ft SB				NB			EB		WB		
Conflicting Lanes Left	1				1			1		1		
Conflicting Approach Rig	ghtNB				SB			WB		EB		
Conflicting Lanes Right	1				1			1		1		
HCM Control Delay	7.1				6.3			6.6		7.6		
HCM LOS	Α				Α			Α		Α		
Lane	N	JBI n1	FBI n1\	WBLn1	SBLn1							
Vol Left, %			100%	0%	60%							
Vol Thru, %		50%	0%	0%	40%							
Vol Right, %		50%	0%	100%	0%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		2	1	3	5							
LT Vol		0	1	0	3							
Through Vol		1	0	0	2							
RT Vol		1	0	3	0							
Lane Flow Rate		3	1	4	7							
Geometry Grp		1	1	1	1							
Degree of Util (X)				0.004	0.009							
Departure Headway (Hd			4.121	3.318								
Convergence, Y/N	•/	Yes	Yes	Yes	Yes							
Cap		995	872	1083	784							
Service Time				1.325	2.595							
HCM Lane V/C Ratio		0.003		0.004								
HCM Control Delay		6.6	7.1	6.3	7.6							
HOM Love LOO		0.0	7.1	0.0	7.0							

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HCM Lane LOS

HCM 95th-tile Q

Intersection						
Int Delay, s/veh	4.3					
<u> </u>	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				NDK
Lane Configurations	<b>}</b>	100	<u></u>	<b>†</b>	127	0
Traffic Vol, veh/h	92	103	6	22	137	8
Future Vol, veh/h	92	103	6	22	137	8
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	18	0	18	8	0
Mvmt Flow	92	103	6	22	137	8
Major/Minor NA	oio-1		/oic=0		Mine -1	
	ajor1		Major2		Minor1	444
Conflicting Flow All	0	0	195	0	178	144
Stage 1	-	-	-	-	144	-
Stage 2	-	-	-	-	34	-
Critical Hdwy	-	-	4.1	-	6.48	6.2
Critical Hdwy Stg 1	-	-	-	-	5.48	-
Critical Hdwy Stg 2	-	-	-	-	5.48	-
Follow-up Hdwy	-	-	2.2	-	3.572	3.3
Pot Cap-1 Maneuver	-	-	1390	-	798	909
Stage 1	-	-	-	-	869	-
Stage 2	-	-	-	-	973	-
Platoon blocked, %	-	_		_		
Mov Cap-1 Maneuver	_	_	1390	_	795	909
Mov Cap-2 Maneuver	_	_	-	_	795	-
Stage 1	_	_	_	_	869	_
Stage 2	_	_	_	_	969	_
Glage Z	_	_	_	_	303	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		10.5	
HCM LOS					В	
Miner Lone/Meier Muset		JDI1	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		801	-	-	1390	-
HCM Lane V/C Ratio		0.181	-	-	0.004	-
HCM Control Delay (s)		10.5	-	-	7.6	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.7	-	-	0	-

Intersection						
Int Delay, s/veh	2.7					
		EDD	MDI	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ	407		<u></u>	Y	- 4
Traffic Vol, veh/h	144	187	5	154	84	51
Future Vol, veh/h	144	187	5	154	84	51
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
5	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	9	0	10	31	4
Mvmt Flow	144	187	5	154	84	51
Major/Minor M	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	331	0	402	238
Stage 1	-	-	-	-	238	230
Stage 2	_	-		_	164	_
Critical Hdwy	-	-	4.1		6.71	6.24
Critical Hdwy Stg 1	-	-	4.1	-	5.71	0.24
			-		5.71	
Critical Hdwy Stg 2	-	-	-	-		2 226
Follow-up Hdwy	-	-	2.2			3.336
Pot Cap-1 Maneuver	-	-	1240	-	552	796
Stage 1	-	-	-	-	738	-
Stage 2	-	-	-	-	799	-
Platoon blocked, %	-	-	1010	-		
Mov Cap-1 Maneuver	-	-	1240	-	550	796
Mov Cap-2 Maneuver	-	-	-	-	550	-
Stage 1	-	-	-	-	738	-
Stage 2	-	-	-	-	796	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		12.4	
HCM LOS	U		0.2		12. <del>4</del> B	
TIOWI LOG					Б	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		623	-	-	1240	-
HCM Lane V/C Ratio		0.217	-		0.004	-
HCM Control Delay (s)		12.4	-	-	7.9	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.8	-	-	0	-
2000						

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	ĵ.		ች	₽			स	1		4	
Traffic Vol, veh/h	1	134	31	113	121	4	20	1	196	1	1	0
Future Vol, veh/h	1	134	31	113	121	4	20	1	196	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	134	31	113	121	4	20	1	196	1	1	0
Major/Minor M	1ajor1			Major2		ľ	Minor1		N	/linor2		
Conflicting Flow All	125	0	0	165	0	0	502	503	150	599	516	123
Stage 1	-	-	-	-	-	-	152	152	-	349	349	-
Stage 2	-	-	-	-	-	-	350	351	-	250	167	-
Critical Hdwy	4.1	-	-	4.4	-	-	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Follow-up Hdwy	2.2	-	-	2.47	-	-	3.635	4	3.39	3.5	4.9	3.3
Pot Cap-1 Maneuver	1474	-	-	1260	-	-	459	474	876	416	349	933
Stage 1	-	-	-	-	-	-	821	775	-	671	491	-
Stage 2	-	-	-	-	-	-	640	636	-	759	608	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1474	-	-	1260	-	-	426	431	876	300	317	933
Mov Cap-2 Maneuver	-	-	-	-	-	-	426	431	-	300	317	-
Stage 1	-	-	-	-	-	-	820	774	-	670	447	-
Stage 2	-	-	-	-	-	-	581	579	-	588	607	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			3.9			10.6			16.8		
HCM LOS							В			С		
Minor Lane/Major Mvmt		NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		426	876	1474	_	-	1260	-	-	308		
HCM Lane V/C Ratio			0.224		_	_	0.09	_	_	0.006		
HCM Control Delay (s)		13.9	10.3	7.4	_	_	8.1	_	-	16.8		
HCM Lane LOS		В	В	Α	-	-	A	-	-	С		
HCM 95th %tile Q(veh)		0.2	0.9	0	-	-	0.3	-	-	0		

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	₽		ች	ĵ.	
Traffic Vol, veh/h	19	8	13	10	9	65	8	124	14	92	49	4
Future Vol, veh/h	19	8	13	10	9	65	8	124	14	92	49	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	<u>-</u>	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	19	8	13	10	9	65	8	124	14	92	49	4
Major/Minor N	/linor2		1	Minor1		1	Major1		1	Major2		
Conflicting Flow All	419	389	51	393	384	131	53	0	0	138	0	0
Stage 1	235	235	-	147	147	-	-	-	-	-	-	-
Stage 2	184	154	-	246	237	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1	-	-	4.46	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	-	-	2.524	-	-
Pot Cap-1 Maneuver	548	549	1000	570	553	865	1566	-	-	1261	-	-
Stage 1	773	714	-	860	779	-	-	-	-	-	-	-
Stage 2	822	774	-	762	713	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	470	506	1000	523	510	865	1566	-	-	1261	-	-
Mov Cap-2 Maneuver	470	506	-	523	510	-	-	-	-	-	-	-
Stage 1	769	662	-	856	775	-	-	-	-	-	-	-
Stage 2	748	770	-	689	661	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.7			10.4			0.4			5.1		
HCM LOS	В			В								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1566	-	-	578	751	1261	-	-			
HCM Lane V/C Ratio		0.005	-	_		0.112		-	-			
HCM Control Delay (s)		7.3	-	-	11.7	10.4	8.1	-	-			
HCM Lane LOS		Α	-	-	В	В	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.2	0.4	0.2	-	-			

Intersection						
Int Delay, s/veh	0.1					
Movement V	NBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	₩		<b>f</b>			4
Traffic Vol, veh/h	2	0	26	0	0	224
Future Vol, veh/h	2	0	26	0	0	224
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	<u> </u>	110116
			0			0
Veh in Median Storage, #	<i>+</i> 0	-	0		-	
Grade, %		400		400	400	0
	100	100	100	100	100	100
Heavy Vehicles, %	0	0	40	0	0	6
Mvmt Flow	2	0	26	0	0	224
Major/Minor Mir	nor1	N	/lajor1	N	Major2	
	250	26	0	0	26	0
Stage 1	26	-	-	-	-	-
	224	_	_	_	_	_
Critical Hdwy	6.4	6.2	-	_	4.1	_
			-			
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
	743	1056	-	-	1601	-
	1002	-	-	-	-	-
<u> </u>	818	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	743	1056	-	-	1601	-
· · · · · · · · · · · · · · · · · · ·	743	-	-	-	_	-
•	1002	_	_	_	_	_
<u> </u>	818	_	_	_	_	_
J. W. J. Z.	3.0					
Approach	WB		NB		SB	
HCM Control Delay, s	9.9		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NIDDV	VBLn1	SBL	SBT
IVIII OLLAHE/IVIAIOLIVIVIIII			INDLA	VDLIII	ODL	ODI
				740	1001	
Capacity (veh/h)		-	-		1601	-
Capacity (veh/h) HCM Lane V/C Ratio			-	0.003	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		-	-	0.003 9.9	0	
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.003	-	-

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	40	0	0	183	0
Future Vol, veh/h	0	0	0	0	0	0	0	40	0	0	183	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	_	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	40	0	0	183	0
Major/Minor N	/linor2		N	Minor1		N	/lajor1		N	Major2		
	223	223	183	223	223	40	183	0	0	40	0	0
Conflicting Flow All	183	183		40	40	40	103	U	U	40	U	
Stage 1	40	40	-	183	183	-	_	-	=	-	-	-
Stage 2	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	_	-
Critical Hdwy	6.1	5.5	0.2	6.1	5.5	0.2	4.1	-	=	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5		6.1	5.5			<del>-</del>	<del>-</del>	-	-	<del>-</del>
Critical Hdwy Stg 2		5.5 4	3.3	3.5	5.5 4	3.3	2.2	-	<del>-</del>	2.2	_	-
Follow-up Hdwy	3.5		865					<del>-</del>	<del>-</del>	1583	-	-
Pot Cap-1 Maneuver	737 823	679 752		737 980	679 866	1037	1404	-	-	1303	-	-
Stage 1		866	-	823	752	<del>-</del>	<del>-</del>	<del>-</del>	<del>-</del>	-	-	-
Stage 2	980	000	-	023	152	-	-	-	-	-	-	-
Platoon blocked, %	737	679	865	737	679	1037	1404	-	-	1583	_	-
Mov Cap-1 Maneuver	737	679		737	679	1037	1404	-	=	1000	-	-
Mov Cap-2 Maneuver Stage 1	823	752	-	980	866	-	-	-	-	-	_	-
•	980	866		823	752	-	_	-	=	-		-
Stage 2	300	000	-	023	152	-	_	_	_	_	_	_
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NBR F	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1404	_	_		_	1583	_	_			
HCM Lane V/C Ratio		-	_	_	_	_	-	_	_			
HCM Control Delay (s)		0	_	_	0	0	0	_	_			
HCM Lane LOS		A	_	-	A	A	A	_	_			
HCM 95th %tile Q(veh)		0		_	_	-	0	_	_			
HOW JOHN JUNIO Q(VOII)		U					- 0					

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	34	0	9	0	23	9	38	153	0
Future Vol, veh/h	0	0	0	34	0	9	0	23	9	38	153	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	_	None	-	_	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	6	0	44	0	39	33	16	13	0
Mvmt Flow	0	0	0	34	0	9	0	23	9	38	153	0
Major/Minor N	/linor2			Minor1		<u> </u>	Major1			Major2		
Conflicting Flow All	261	261	153	257	257	28	153	0	0	32	0	0
Stage 1	229	229	-	28	28	-	-	-	-	-	-	-
Stage 2	32	32	-	229	229	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.16	6.5	6.64	4.1	-	-	4.26	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3			3.696	2.2	-	-	2.344	-	-
Pot Cap-1 Maneuver	696	647	898	688	651	938	1440	-	-	1494	-	-
Stage 1	778	718	-	979	876	-	-	-	-	-	-	-
Stage 2	990	872	-	765	718	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	674	629	898	674	633	938	1440	-	-	1494	-	-
Mov Cap-2 Maneuver	674	629	-	674	633	-	-	-	-	-	-	-
Stage 1	778	698	-	979	876	-	-	-	-	-	-	-
Stage 2	981	872	-	744	698	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			10.3			0			1.5		
HCM LOS	Α			В								
Minor Lane/Major Mvmt	t _	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1440	-	-		716	1494	-				
HCM Lane V/C Ratio		-	-	-	-		0.025	-	-			
HCM Control Delay (s)		0	-	-	0	10.3	7.5	0	-			
HCM Lane LOS		Α	-	-	Α	В	Α	Α	-			
HCM 95th %tile Q(veh)		0	-	-	-	0.2	0.1	-	-			

Intersection						
Int Delay, s/veh	0.1					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	^	4	<b>€</b>	₽	7
Traffic Vol, veh/h	0	0	1	13	44	7
Future Vol, veh/h	0	0	1	13	44	7
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		_	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	13	44	7
Major/Minor N	linor2	N	/lajor1	, A	/lajor2	
						^
Conflicting Flow All	63	48	51	0	-	0
Stage 1	48	-	-	-	-	-
Stage 2	15	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	948	1027	1568	-	-	-
Stage 1	980	-	-	-	-	-
Stage 2	1013	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	947	1027	1568	-	-	-
Mov Cap-2 Maneuver	947	-	-	-	-	-
Stage 1	979	-	-	_	-	-
Stage 2	1013	_	-	_	_	_
5 g =						
A	ED		ND		C.D.	
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.5		0	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1568	11011	LULIII	051	יופט
HCM Lane V/C Ratio		0.001	-	_	-	-
		7.3	_	0	-	-
HCM Lang LOS			0		-	-
HCM 05th % file O(vah)		A	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	-	-	-

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Future Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow	1	0	0	0	0	0	0	13	0	1	43	0
Major/Minor I	Minor2			Minor1		N	Major1		N	Major2		
Conflicting Flow All	58	58	43	58	58	13	43	0	0	13	0	0
		45		13	13	13	43	U	U	13		
Stage 1	45 13	13	-	45	45	-	-	-	-	-	-	-
Stage 2	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy	6.1			6.1	5.5	0.Z -		-	-		-	-
Critical Hdwy Stg 1		5.5	-	6.1			-	-	<del>-</del>	-	-	-
Critical Hdwy Stg 2	6.1	5.5	2.2		5.5	2.2	-	-	-	2.2	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	944	837	1033	944	837	1073	1579	-	-	1619	-	-
Stage 1	974	861	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	974	861	-	-	-	-	-	-	-
Platoon blocked, %	0.40	000	1000	040	000	1072	1570	-	-	1010	-	-
Mov Cap-1 Maneuver	943	836	1033	943	836	1073	1579	-	-	1619	-	-
Mov Cap-2 Maneuver	943	836	-	943	836	-	-	-	-	-	-	-
Stage 1	974	860	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	973	860	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.8			0			0			0.2		
HCM LOS	Α			A								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR F	EBLn1V	VRI n1	SBL	SBT	SBR			
	n e	1579	1101	-	0.40			001	OBIT			
Capacity (veh/h) HCM Lane V/C Ratio			-		0.001	-	0.001	-	=			
		_	-					-	-			
HCM Lang LOS		0	-	-	8.8	0	7.2	0	-			
HCM Ceth % tile O(vah)	١	A	-	-	A	Α	A	Α	-			
HCM 95th %tile Q(veh)	)	0	-	-	0	-	0	-	-			

5.5					
	WDD	NDT	NDD	CDI	CDT
	WBK		NRK	SBL	SBT
	04				<del>વ</del>
					23
					23
					_ 0
					Free
	None	-	None	-	None
	-	-	-	-	-
	-		-	-	0
	-		-	-	0
			100		100
3	0	22	0	22	0
35	21	9	3	2	23
Minor1	N	Acior1		Major	
					0
		-	-	-	-
		-	-	-	-
		-	-	4.32	-
	-	-	-	-	-
	-	-	-	-	-
		-	-		-
972	1076	-	-	1486	-
1009	-	-	-	-	-
993	-	-	-	-	-
		-	-		-
971	1076	_	-	1486	-
	-	-	_	_	_
	_	_	_	_	_
	_	_	_	_	_
332					
WB		NB		SB	
		NB 0		SB 0.6	
WB					
WB 8.8					
WB 8.8 A		0	MDI n1	0.6	CDT
WB 8.8	NBT	0 NBRV	VBLn1	0.6 SBL	SBT
WB 8.8 A	NBT -	0 NBRV	1008	0.6 SBL 1486	-
WB 8.8 A	NBT -	0 NBRV -	1008 0.056	0.6 SBL 1486 0.001	-
WB 8.8 A	NBT - -	NBRV - -	1008 0.056 8.8	0.6 SBL 1486 0.001 7.4	- - 0
WB 8.8 A	NBT -	0 NBRV -	1008 0.056	0.6 SBL 1486 0.001	-
	WBL  35 35 0 Stop 0 4,# 0 0 100 3 35  Minor1 38 11 27 6.43 5.43 5.43 3.527 972 1009 993	WBL WBR  35 21 35 21 0 0 Stop Stop - None 0 1,# 0 100 100 3 0 35 21  Minor1 N  38 11 11 27 6.43 6.2 5.43 5.43 3.527 3.3 972 1076 1009 993  971 1076 971	WBL WBR NBT  35 21 9 35 21 9 0 0 0 0 Stop Stop Free - None - None 0 0 - 0 100 100 100 3 0 22 35 21 9  Minor1 Major1  38 11 0 11 27 6.43 6.2 - 5.43 5.43 5.43 5.43 5.43 972 1076 - 1009 993 971 1076 - 971 1009	WBL         WBR         NBT         NBR           35         21         9         3           35         21         9         3           0         0         0         0           Stop         Stop         Free         Free           -         None         -         None           0         -         -         -           0         -         0         -           100         100         100         100           3         0         22         0           35         21         9         3           Minor1         Major1         I           38         11         0         0           11         -         -         -           27         -         -         -           5.43         -         -         -           5.43         -         -         -           972         1076         -         -           1009         -         -         -           971         1076         -         -           971         -         -         -	WBL         WBR         NBT         NBR         SBL           Y         Image: Control of the control of th

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4			4	
Traffic Vol, veh/h	12	8	0	1	97	137	0	1	0	53	0	30
Future Vol, veh/h	12	8	0	1	97	137	0	1	0	53	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	58	0	0	0	8	10	0	0	0	19	0	10
Mvmt Flow	12	8	0	1	97	137	0	1	0	53	0	30
Major/Minor N	//ajor1		ı	Major2		ı	Minor1		ı	Minor2		
Conflicting Flow All	234	0	0	8	0	0	215	268	8	132	131	97
Stage 1	-	-	-	-	-	-	32	32	-	99	99	-
Stage 2	-	-	-	-	-	-	183	236	-	33	32	-
Critical Hdwy	4.68	-	-	4.1	-	-	7.1	6.5	6.2	7.29	6.5	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Follow-up Hdwy	2.722	-	-	2.2	-	-	3.5	4	3.3	3.671	4	3.39
Pot Cap-1 Maneuver	1065	-	-	1625	-	-	746	641	1080	802	763	938
Stage 1	-	-	-	-	-	-	990	872	-	867	817	-
Stage 2	-	-	-		-	-	823	713	-	941	872	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1065	-	-	1625	-	-	715	633	1080	794	754	938
Mov Cap-2 Maneuver	-	-	-	-	-	-	715	633	-	794	754	-
Stage 1	-	-	-	-	-	-	979	862	-	857	816	-
Stage 2	-	-	-	-	-	-	796	712	-	930	862	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	5.1			0			10.7			9.7		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBI n1			
Capacity (veh/h)			1065			1625	-	-				
HCM Lane V/C Ratio		0.002		_		0.001	<u>-</u>		0.099			
HCM Control Delay (s)		10.7	8.4	0	_	7.2	0	_	9.7			
HCM Lane LOS		В	Α	A	_	Α	A	_	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0.3			
2011)												

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	1>	בטול	<u> ነ</u>	1≯	TIDIC	HUL	4	HOIL	ODL	4	ODIN
Traffic Vol, veh/h	1	36	1	11	342	31	4	0	9	70	2	7
Future Vol, veh/h	1	36	1	11	342	31	4	0	9	70	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	_	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	_	-	_	_	-	-	_	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	36	1	11	342	31	4	0	9	70	2	7
Major/Minor M	lajor1			Major2			Minor1			Minor2		
Conflicting Flow All	373	0	0	37	0	0	423	434	37	423	419	358
Stage 1	-	-	-	-	-	-	39	39	-	380	380	-
Stage 2	-	-	-	-	-	-	384	395	-	43	39	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4	3.3	3.581	4	3.561
Pot Cap-1 Maneuver	800	-	-	1587	-	-	545	518	1041	529	528	630
Stage 1	-	-	-	-	-	-	981	866	-	628	617	-
Stage 2	-	-	-	-	-	-	643	608	-	954	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	800	-	-	1587	-	-	534	514	1041	521	524	630
Mov Cap-2 Maneuver	-	-	-	-	-	-	534	514	-	521	524	-
Stage 1	-	-	-	-	-	-	980	865	-	627	613	-
Stage 2	-	-	-	-	-	-	629	604	-	945	865	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			9.5			13		
HCM LOS							Α			В		
Minor Lane/Major Mvmt	1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		806	800	-		1587	-	-	529			
HCM Lane V/C Ratio		0.016		_		0.007	-	_	0.149			
HCM Control Delay (s)		9.5	9.5	-	-	7.3	-	-	13			
HCM Lane LOS		Α	Α	-	-	Α	-	-	В			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0.5			

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		<b>1</b>			सी						4	
Traffic Vol, veh/h	0	10	87	8	302	0	0	0	0	2	2	98
Future Vol, veh/h	0	10	87	8	302	0	0	0	0	2	2	98
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	10	87	8	302	0	0	0	0	2	2	98
Major/Minor N	/lajor1		ľ	Major2					N	/linor2		
Conflicting Flow All		0	0	97	0	0				372	415	302
Stage 1	-	-	-	-	-	-				318	318	-
Stage 2	-	-	-	-	-	-				54	97	-
Critical Hdwy	-	-	-	4.85	_	-				6.4	7	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1142	_	0				633	461	710
Stage 1	0	-	-	-	-	0				742	576	-
Stage 2	0	-	-	-	-	0				974	730	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1142	-	-				628	0	710
Mov Cap-2 Maneuver	-	-	-	-	-	-				628	0	-
Stage 1	-	-	-	-	-	-				742	0	-
Stage 2	-	-	-	-	-	-				966	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						10.9		
HCM LOS										В		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-	-	1142	_	708						
HCM Lane V/C Ratio		-		0.007	_	0.144						
HCM Control Delay (s)		-	-		0	10.9						
HCM Lane LOS		-	-	Α	A	В						
HCM 95th %tile Q(veh)		-	-	0	-	0.5						

Intersection												
Int Delay, s/veh	10.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			ĵ.			4				
Traffic Vol, veh/h	9	3	0	0	4	1	306	5	23	0	0	0
Future Vol, veh/h	9	3	0	0	4	1	306	5	23	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	_	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	14	0	0	0	75	0	10	80	26	2	2	2
Mvmt Flow	9	3	0	0	4	1	306	5	23	0	0	0
Major/Minor N	Major1			Major2		N	/linor1					
Conflicting Flow All	5	0	-	-	-	0	26	26	3			
Stage 1	-	-	-	-	_	-	21	21	-			
Stage 2	-	-	-	-	-	-	5	5	-			
Critical Hdwy	4.24	-	-	-	-	-	6.5	7.3	6.46			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.5	6.3	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.5	6.3	-			
Follow-up Hdwy	2.326	-	-	-	-	-	3.59	4.72	3.534			
Pot Cap-1 Maneuver	1541	-	0	0	-	-	969	736	1015			
Stage 1	-	-	0	0	-	-	981	745	-			
Stage 2	-	-	0	0	-	-	998	759	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1541	-	-	-	-	-	963	0	1015			
Mov Cap-2 Maneuver	-	-	-	-	-	-	963	0	-			
Stage 1	-	-	-	-	-	-	975	0	-			
Stage 2	-	-	-	-	-	-	998	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	5.5			0			10.7					
HCM LOS							В					
Minor Lane/Major Mvm	it l	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		966	1541	-	-	-						
HCM Lane V/C Ratio		0.346		_	_	-						
HCM Control Delay (s)		10.7	7.3	0	-	-						
HCM Lane LOS		В	Α	A	-	-						
HCM 95th %tile Q(veh)		1.6	0	-	-	-						

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ß			4						4	
Traffic Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Future Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	0	0	50	0	2	2	2	10	83	0
Mvmt Flow	0	5	2	3	11	0	0	0	0	10	6	3
Major/Minor N	/lajor1		N	Major2					N	/linor2		
Conflicting Flow All	-	0	0	7	0	0				23	24	11
Stage 1	-	-	-	-	-	-				17	17	-
Stage 2	-	-	-	-	_	-				6	7	-
Critical Hdwy	-	-	-	4.1	-	-				6.5	7.33	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.5	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.5	6.33	-
Follow-up Hdwy	-	-	-	2.2	-	-				3.59	4.747	3.3
Pot Cap-1 Maneuver	0	-	-	1627	_	0				973	734	1076
Stage 1	0	-	-	-	-	0				985	744	-
Stage 2	0	-	-	-	-	0				997	753	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1627	-	-				971	0	1076
Mov Cap-2 Maneuver	-	-	-	-	-	-				971	0	-
Stage 1	-	-	-	-	-	-				985	0	-
Stage 2	-	-	-	-	-	-				995	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			1.5						8.7		
HCM LOS										Α		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		_	_	1627	_	993						
HCM Lane V/C Ratio		-	_	0.002		0.019						
HCM Control Delay (s)		-	-	7.2	0	8.7						
HCM Lane LOS		-	-	A	A	A						
HCM 95th %tile Q(veh)		-	-	0	-	0.1						

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ની			<del>(</del>			4				
Traffic Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0
Future Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2
Mvmt Flow	0	15	0	0	14	4	0	5	8	0	0	0
Major/Minor N	//ajor1		N	Major2		ı	Minor1					
Conflicting Flow All	18	0	-		_	0	31	33	15			
Stage 1	_	_	_	_	_	-	15	15	_			
Stage 2	-	-	-	-	-	-	16	18	-			
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32			
Critical Hdwy Stg 1	_	_	-	_	-	-	5.4	6.3	_			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-			
Follow-up Hdwy	2.2	_	-	_	-	-	3.5		3.408			
Pot Cap-1 Maneuver	1612	-	0	0	-	-	988	729	1036			
Stage 1	-	-	0	0	-	-	1013	750	-			
Stage 2	-	-	0	0	-	-	1012	748	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1612	-	-	-	-	-	988	0	1036			
Mov Cap-2 Maneuver	-	-	-	-	-	-	988	0	-			
Stage 1	-	-	-	-	-	-	1013	0	-			
Stage 2	-	-	-	-	-	-	1012	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	0			0			8.5					
HCM LOS	U			U			Α					
TOW LOO												
Minor Long/Major Marin	4 K	JDI n4	EDI	ГРТ	WDT	WDD						
Minor Lane/Major Mym	t r	VBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1036	1612	-	-	-						
HCM Control Dolov (a)		0.013	-	-	-	-						
HCM Control Delay (s)		8.5	0	-	-	-						
HCM Lane LOS		A	A	-	-	-						
HCM 95th %tile Q(veh)		0	0	-	-	-						

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Future Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	13	0	0	21	0	0	0	0	0	0	50
Mvmt Flow	0	21	2	0	14	0	2	0	0	0	0	2
Major/Minor N	/lajor1		ľ	Major2		<u> </u>	/linor1		N	/linor2		
Conflicting Flow All	14	0	0	23	0	0	37	36	22	36	37	14
Stage 1	-	-	-	-	-	-	22	22	-	14	14	-
Stage 2	-	-	-	-	-	-	15	14	-	22	23	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.7
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	_	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.75
Pot Cap-1 Maneuver	1617	-	-	1605	-	-	973	860	1061	975	859	942
Stage 1	-	-	-	-	-	-	1002	881	-	1011	888	-
Stage 2	-	-	-	-	-	-	1010	888	-	1002	880	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1617	-	-	1605	-	-	971	860	1061	975	859	942
Mov Cap-2 Maneuver	-	-	-	-	-	-	971	860	-	975	859	-
Stage 1	-	-	-	-	-	-	1002	881	-	1011	888	-
Stage 2	-	-	-	-	-	-	1008	888	-	1002	880	-
_												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			8.7			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvmt	t1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		971	1617	-	-	1605	-	-	942			
HCM Lane V/C Ratio		0.002	-	-	-	-	-	-	0.002			
HCM Control Delay (s)		8.7	0	-	-	0	-	-	8.8			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection												
Int Delay, s/veh	7.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			4						4	
Traffic Vol, veh/h	0	30	5	116	10	0	0	0	0	66	1	36
Future Vol, veh/h	0	30	5	116	10	0	0	0	0	66	1	36
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	25	0	6	0	0	2	2	2	3	0	0
Mvmt Flow	0	30	5	116	10	0	0	0	0	66	1	36
Major/Minor I	Major1		ľ	Major2					N	/linor2		
Conflicting Flow All	-	0	0	35	0	0				275	277	10
Stage 1	-	-	-	-	-	-				242	242	-
Stage 2	-	-	-	-	-	-				33	35	-
Critical Hdwy	-	-	-	4.16	-	-				6.43	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	5.5	-
Follow-up Hdwy	-	-	-	2.254	-	-				3.527	4	3.3
Pot Cap-1 Maneuver	0	-	-	1551	-	0				712	634	1077
Stage 1	0	-	-	-	-	0				796	709	-
Stage 2	0	-	-	-	-	0				987	870	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1551	-	-				659	0	1077
Mov Cap-2 Maneuver	-	-	-	-	-	-				659	0	-
Stage 1	-	-	-	-	-	-				796	0	-
Stage 2	-	-	-	-	-	-				913	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.9						10.4		
HCM LOS										В		
Minor Lane/Major Mvm	nt	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)				1551		764						
HCM Lane V/C Ratio		_		0.075		0.135						
HCM Control Delay (s)		_	_		0	10.4						
HCM Lane LOS		_	_	Α	A	В						
HCM 95th %tile Q(veh)	)	_	_	0.2	-	0.5						
				Ų. <u>L</u>		0.0						

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			ĵ.			4				
Traffic Vol, veh/h	14	82	0	0	105	85	21	2	9	0	0	0
Future Vol, veh/h	14	82	0	0	105	85	21	2	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2
Mvmt Flow	14	82	0	0	105	85	21	2	9	0	0	0
Major/Minor I	Major1		ľ	Major2		N	Minor1					
Conflicting Flow All	190	0	-	-	-	0	258	300	82			
Stage 1	-	-	-	-	-	-	110	110	-			
Stage 2	-	-	-	-	-	-	148	190	-			
Critical Hdwy	4.67	-	-	-	-	-	6.4	7.5	6.53			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.5	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.5	-			
Follow-up Hdwy	2.713	-	-	-	-	-	3.5	4.9	3.597			
Pot Cap-1 Maneuver	1113	-	0	0	-	-	735	479	898			
Stage 1	-	-	0	0	-	-	920	649	-			
Stage 2	-	-	0	0	-	-	884	592	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1113	-	-	-	-	-	725	0	898			
Mov Cap-2 Maneuver	-	-	-	-	-	-	725	0	-			
Stage 1	-	-	-	-	-	-	908	0	-			
Stage 2	-	-	-	-	-	-	884	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	1.2			0			9.9					
HCM LOS							Α					
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		769	1113	-	_	-						
HCM Lane V/C Ratio		0.042		_	_	_						
HCM Control Delay (s)		9.9	8.3	0	_	-						
HCM Lane LOS		A	A	A	_	_						
HCM 95th %tile Q(veh)	)	0.1	0	-	_	-						
70th Q(VOI)		J.,										

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	86	2	1	186	0	1	0	0	1	0	3
Future Vol, veh/h	3	86	2	1	186	0	1	0	0	1	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	38	50	0	21	0	0	0	0	0	0	67
Mvmt Flow	3	86	2	1	186	0	1	0	0	1	0	3
Major/Minor N	Major1			Major2		<u> </u>	Minor1		N	/linor2		
Conflicting Flow All	186	0	0	88	0	0	283	281	87	281	282	186
Stage 1	-	-	-	-	-	-	93	93	-	188	188	-
Stage 2	-	-	-	-	-	-	190	188	-	93	94	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.87
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5		3.903
Pot Cap-1 Maneuver	1222	-	-	1520	-	-	673	631	977	675	630	714
Stage 1	-	-	-	-	-	-	919	822	-	818	748	-
Stage 2	-	-	-	-	-	-	816	748	-	919	821	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1222	-	-	1520	-	-	668	628	977	673	627	714
Mov Cap-2 Maneuver	-	-	-	-	-	-	668	628	-	673	627	-
Stage 1	-	-	-	-	-	-	916	820	-	816	747	-
Stage 2	-	-	-	-	-	-	812	747	-	916	819	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0			10.4			10.2		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)	· '	668	1222	-		1520	-	-				
HCM Lane V/C Ratio		0.001		_		0.001	_		0.006			
HCM Control Delay (s)		10.4	8	0		7.4	0	_	10.2			
HCM Lane LOS		В	A	A	_	Α	A	_	В			
HCM 95th %tile Q(veh)	\	0	0	-	_	0		_	0			

Intersection							
Int Delay, s/veh	5.6						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	<u> </u>	<b>1</b>	11511	ሻ	7	
Traffic Vol, veh/h	6	19	45	15	11	117	
Future Vol, veh/h	6	19	45	15	11	117	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage	e, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	0	5	7	0	0	2	
Mvmt Flow	6	19	45	15	11	117	
Major/Minor I	Major1	<b>N</b>	Major2	N	/linor2		
Conflicting Flow All	60	0	-	0	84	53	
Stage 1	-	-	-	-	53	-	
Stage 2	-	-	-	-	31	-	
Critical Hdwy	4.1	-	-	-	6.4	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.2	-	-	-		3.318	
Pot Cap-1 Maneuver	1556	-	-	-	923	1014	
Stage 1	-	-	-	-	975	-	
Stage 2	-	-	-	-	997	-	
Platoon blocked, %	4550	-	-	-	040	1011	
Mov Cap-1 Maneuver	1556	-	-	-	919	1014	
Mov Cap-2 Maneuver	-	-	-	-	919	-	
Stage 1	-	-	-	-	971	-	
Stage 2	-	-	-	-	997	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.8		0		9		
HCM LOS					Α		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1 S	SBLn2
Capacity (veh/h)		1556	_	-	-		1014
HCM Lane V/C Ratio		0.004	-	-	_	0.012	
HCM Control Delay (s)		7.3	_	-	-	9	9
HCM Lane LOS		Α	-	-	-	Α	Α
HCM 95th %tile Q(veh)	)	0	-	-	-	0	0.4

Intersection												
Int Delay, s/veh	4.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		*	f)			4			4	
Traffic Vol, veh/h	15	36	7	1	24	1	1	1	1	4	11	32
Future Vol, veh/h	15	36	7	1	24	1	1	1	1	4	11	32
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	13	31	0	100	33	0	0	100	100	0	0	16
Mvmt Flow	15	36	7	1	24	1	1	1	1	4	11	32
Major/Minor	Major1		ı	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	25	0	0	43	0	0	118	97	40	97	100	25
Stage 1	-	-	-	-	-	-	70	70	-	27	27	-
Stage 2	-	-	-	-	-	-	48	27	-	70	73	-
Critical Hdwy	4.23	-	-	5.1	-	-	7.1	7.5	7.2	7.1	6.5	6.36
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Follow-up Hdwy	2.317	-	-	3.1	-	-	3.5	4.9	4.2	3.5	4	3.444
Pot Cap-1 Maneuver	1521	-	-	1113	-	-	863	641	810	890	794	1012
Stage 1	-	-	-	-	-	-	945	679	-	996	877	-
Stage 2	-	-	-	-	-	-	971	713	-	945	838	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1521	-	-	1113	-	-	820	634	810	880	785	1012
Mov Cap-2 Maneuver	-	-	-	-	-	-	820	634	-	880	785	-
Stage 1	-	-	-	-	-	-	936	672	-	986	876	-
Stage 2	-	-	-	-	-	-	928	712	-	933	830	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.9			0.3			8.4			9		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		1073	1521	-	_	1113	-	-	937			
HCM Lane V/C Ratio		0.003	0.01	-	_	0.001	-	-	0.05			
HCM Control Delay (s)		8.4	7.4	-	-	8.2	-	-	9			
HCM Lane LOS		Α	Α	_	_	Α	-	-	A			
HCM 95th %tile Q(veh	)	0	0	-	-	0	-	-	0.2			
	,											

0.8 EBT 37 37 0 Free	3 3 0 Free None - 100 33 3	WBL  8 8 0 Free - 150 - 100 0 8	WBT 24 24 0 Free None - 0 100 13 24	NBL 0 0 0 Stop - 0 0 100 0	NBR  0 0 0 Stop None 100 0
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37 0 Free - - 4, # 0 0 100 8 37	3 0 Free None - - - 100 33	8 0 Free - 150 - - 100 0	24 0 Free None - 0 0 100 13	0 0 Stop 0 0 0 100	0 0 Stop None - - - 100
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- , # 0 0 100 8 37	- - 100 33	150 - - 100 0	0 0 100 13	0 0 0 100	- - - 100
8, # 0 0 100 8 37	- 100 33	- 100 0	0 0 100 13	0 0 100	- - 100
0 100 8 37	100 33	100 0	0 100 13	0 100	100
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8 37	33	0	13		
37				0	Λ
	3	8	24		
				0	0
Major1	N	Major2	N	Minor1	
					39
		-			-
		-			-
		4.1			6.2
-	-	-	-		-
-	-	-	-		-
-	-		-		3.3
-	-	1583	-		1038
-	-	-	-		-
-	-	-	-	988	-
-	-		-		
-	-	1583	-	924	1038
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Intersection						
Int Delay, s/veh	0					
		MDD	NOT	NDD	051	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			र्न
Traffic Vol, veh/h	0	0	0	1	0	14
Future Vol, veh/h	0	0	0	1	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	14
Mvmt Flow	0	0	0	1	0	14
	•		•	•		
	Minor1	N	//ajor1		/lajor2	
Conflicting Flow All	15	1	0	0	1	0
Stage 1	1	-	-	-	-	-
Stage 2	14	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	_	-	_	_	_
Critical Hdwy Stg 2	5.4	-	-	-	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	1009	1090	_	_	1635	_
Stage 1	1028	-	_	_	-	_
Stage 2	1014	_	_	_	_	_
Platoon blocked, %	1017			_		_
	1000	1090	-	_	1635	_
Mov Cap-1 Maneuver	1009			-		<del>-</del>
Mov Cap-2 Maneuver	1009	-	-	-	-	-
Stage 1	1028	-	_	-	-	-
Stage 2	1014	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		U		v	
TIOW LOO						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	-	1635	-
HCM Lane V/C Ratio		_	-	-	-	-
HCM Control Delay (s)		_	-	0	0	_
HCM Lane LOS		_	_	A	A	_
HCM 95th %tile Q(veh)	)	_	_	- '.	0	_
TOWN JOHN JOHNE Q(VEI)	,				U	

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			ની	Ą.	
Traffic Vol, veh/h	1	0	0	Ö	18	0
Future Vol, veh/h	1	0	0	0	18	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	_	-	_	_	_
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	_	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	11	0
Mvmt Flow	1	0	0	0	18	0
IVIVIII( I IOW		U	U	U	10	U
Major/Minor N	/linor2	N	//ajor1	Λ	/lajor2	
Conflicting Flow All	18	18	18	0	-	0
Stage 1	18	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	_	_	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	1005	1066	1612	_	_	-
Stage 1	1010	-		_	_	_
Stage 2	-	_	_	_	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	1005	1066	1612	<u>-</u>	-	-
•		1000	1012	-		
Mov Cap-2 Maneuver	1005	-	-	-	-	-
Stage 1	1010	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	A		•		•	
	٠,٠					
Minor Lane/Major Mvm	t	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1612		1005	-	-
HCM Lane V/C Ratio		-	-	0.001	-	-
HCM Control Delay (s)		0	-	8.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		LDIX	NDL			SDIX
Lane Configurations	¥	40	-	4	<b>₽</b>	7
Traffic Vol, veh/h	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	12	5	1	9	7
IVIVIIIL I IOW		12	J		9	,
Major/Minor N	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	24	13	16	0		0
Stage 1	13	_		_	_	_
Stage 2	11	_	_	_	_	_
Critical Hdwy	6.4	6.28	4.5	_	_	_
Critical Hdwy Stg 1	5.4	0.20	7.5	_	_	_
	5.4		-	-		
Critical Hdwy Stg 2		- 270	0.50	-	-	-
Follow-up Hdwy		3.372	2.56	-	-	-
Pot Cap-1 Maneuver	997	1050	1386	-	-	-
Stage 1	1015	-	-	-	-	-
Stage 2	1017	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	993	1050	1386	-	-	-
Mov Cap-2 Maneuver	993	-	-	-	-	-
Stage 1	1011	-	-	-	_	-
Stage 2	1017	_	_	_	_	_
Olago Z	1017					
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
J 200						
Minor Lane/Major Mvm	t	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1386		1041	-	-
HCM Lane V/C Ratio		0.004	-	0.013	-	-
HCM Control Delay (s)		7.6	0	8.5	-	-
HCM Lane LOS		A	A	Α	_	_
HCM 95th %tile Q(veh)		0	-	0	_	-
. 13111 00th 70th Q(VOI)		-		J		

Intersection												
Int Delay, s/veh	4.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	6	12	1	0	27	2	2	2	1	1	0	30
Future Vol, veh/h	6	12	1	0	27	2	2	2	1	1	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	6	12	1	0	27	2	2	2	1	1	0	30
Major/Minor I	Major1		1	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	29	0	0	13	0	0	68	54	13	54	53	28
Stage 1	_	-	_	-	-	-	25	25	-	28	28	-
Stage 2	-	-	-	-	-	-	43	29	-	26	25	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.516
Pot Cap-1 Maneuver	1405	-	-	1619	-	-	930	841	1073	949	842	987
Stage 1	-	-	-	-	-	-	998	878	-	994	876	-
Stage 2	-	-	-	-	-	-	976	875	-	997	878	-
Platoon blocked, %	440=	-	-	1010	-	-	000	000	4070	0.40	000	00-
Mov Cap-1 Maneuver	1405	-	-	1619	-	-	899	838	1073	943	839	987
Mov Cap-2 Maneuver	-	-	-	-	-	-	899	838	-	943	839	-
Stage 1	-	-	-	-	-	-	994	874	-	990	876	-
Stage 2	-	-	-	-	-	-	946	875	-	990	874	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.4			0			9			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBL <sub>n1</sub>			
Capacity (veh/h)		902	1405	-	-	1619	-	-	986			
HCM Lane V/C Ratio		0.006		-	-	-	-	-	0.031			
HCM Control Delay (s)		9	7.6	0	-	0	-	-	8.8			
HCM Lane LOS		Α	Α	Α	-	Α	-	-	Α			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.1			

Internaction						
Intersection	0.0					
Int Delay, s/veh	2.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		<del>(</del> Î			ની
Traffic Vol, veh/h	5	18	30	2	15	42
Future Vol, veh/h	5	18	30	2	15	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	6	21	0	12	13
Mvmt Flow	5	18	30	2	15	42
MATERIAL TON				_	.0	
		_				
	Minor1		//ajor1		Major2	
Conflicting Flow All	103	31	0	0	32	0
Stage 1	31	-	-	-	-	-
Stage 2	72	-	-	-	-	-
Critical Hdwy	6.6	6.26	-	-	4.22	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	3.68	3.354	-	-	2.308	-
Pot Cap-1 Maneuver	853	1032	-	-	1518	-
Stage 1	947	-	-	-	-	-
Stage 2	907	-	-	-	-	-
Platoon blocked, %			-	_		-
Mov Cap-1 Maneuver	844	1032	_	_	1518	_
Mov Cap-2 Maneuver	844	-	_	_	-	_
Stage 1	947	_	_	_	_	_
Stage 2	898	_	_	_	_	_
Olugo Z	550					
Approach	WB		NB		SB	
HCM Control Delay, s	8.7		0		1.9	
HCM LOS	Α					
Minor Lane/Major Mvn	nt .	NBT	NIPDV	VBLn1	SBL	SBT
	IC .	NDT				ODT
Capacity (veh/h)		-	-	•••	1518	-
HCM Cartest Dates (a)		-	-	0.023	0.01	-
HCM Control Delay (s)		-	-	8.7	7.4	0
HCM Lane LOS		-	-	A	A	Α
HCM 95th %tile Q(veh	)	-	-	0.1	0	-

Intersection						
Int Delay, s/veh	0					
			14/5=	14/5-	07:	055
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	. ↑		Y	
Traffic Vol, veh/h	0	5	56	0	0	0
Future Vol, veh/h	0	5	56	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	56	0	0	0
N.4. ' (N.4.						
	Major1		//ajor2		Minor2	
Conflicting Flow All	56	0	-	0	61	56
Stage 1	-	-	-	-	56	-
Stage 2	-	-	-	-	5	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1549	-	-	-	945	1011
Stage 1	-	_	-	_	967	_
Stage 2	-	-	-	-	1018	-
Platoon blocked, %		_	-	_		
Mov Cap-1 Maneuver	1549	_	_	_	945	1011
Mov Cap-1 Maneuver	-	_	_	_	945	-
Stage 1	_				967	_
Stage 2	_				1018	_
Staye 2	<u>-</u>	-	-	_	1010	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS					Α	
Minor Long/Major M.	.4	EDI	CDT	WDT	WDD	2DL4
Minor Lane/Major Mvn	IL	EBL	EBT	WBT	WBR :	PRFUI
Capacity (veh/h)		1549	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	-	-	0
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh		0				

Intersection		
Intersection Delay, s/veh	8.1	
Intersection LOS	Α	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Future Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	1	0	3	0	0	0	4	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		8.2				0		8		
HCM LOS		_		Δ				_		Δ		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	25%	100%	
Vol Thru, %	100%	100%	0%	0%	
Vol Right, %	0%	0%	75%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	0	0	4	4	
LT Vol	0	0	1	4	
Through Vol	0	0	0	0	
RT Vol	0	0	3	0	
Lane Flow Rate	0	0	4	4	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0	0	0.006	0.006	
Departure Headway (Hd)	3.911	3.911	5.208	4.958	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	0	0	691	726	
Service Time	1.917	1.917	3.212	2.962	
HCM Lane V/C Ratio	0	0	0.006	0.006	
HCM Control Delay	6.9	6.9	8.2	8	
HCM Lane LOS	N	N	Α	Α	
HCM 95th-tile Q	0	0	0	0	

Intersection						
Int Delay, s/veh	6.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	LDIX	ሻ	<u>₩</u>	¥	אוטוז
Traffic Vol, veh/h	20	177	35	63	253	4
Future Vol, veh/h	20	177	35	63	253	4
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length		-	130	-	0	110116
Veh in Median Storage,	# 0	_	-	0	0	_
Grade, %	# 0 0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	20	177	35	63	253	4
Major/Minor Major/Minor	ajor1	N	Major2	ı	Minor1	
Conflicting Flow All	0	0	197	0	242	109
Stage 1	-	-	-	-	109	-
Stage 2	_	_	_	_	133	_
Critical Hdwy	_	_	4.16	_	6.45	6.7
Critical Hdwy Stg 1	_	_	7.10	_	5.45	-
Critical Hdwy Stg 2	_	_	_	_	5.45	_
Follow-up Hdwy	_	_	2.254		3.545	3.75
		-	1352		740	829
Pot Cap-1 Maneuver	-	-	1332	-	908	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	886	-
Platoon blocked, %	-	-	10-0	-	-01	
Mov Cap-1 Maneuver	-	-	1352	-	721	829
Mov Cap-2 Maneuver	-	-	-	-	721	-
Stage 1	-	-	-	-	908	-
Stage 2	-	-	-	-	863	-
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		2.8		12.7	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		722	-	-		_
HCM Lane V/C Ratio		0.356	_		0.026	<u>-</u>
HCM Control Delay (s)		12.7	_	_	7.7	_
		12.7 B	_	_	Α	_
HUM I and I IIX				-	$\neg$	-
HCM Lane LOS HCM 95th %tile Q(veh)		1.6	_	_	0.1	_

Intersection						
Int Delay, s/veh	2.4					
		EDD	14/5	1A/DT	NE	NES
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		ች	<u></u>	À	
Traffic Vol, veh/h	183	231	13	303	104	14
Future Vol, veh/h	183	231	13	303	104	14
Conflicting Peds, #/hr	0	0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	8	8	5	25	21
Mvmt Flow	183	231	13	303	104	14
Major/Minor M	lajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	414	0	628	299
Stage 1		-	414	-	299	299
	-	-	_	-	329	_
Stage 2		-	110			6.41
Critical Hdwy	-	-	4.18	-	6.65	
Critical Hdwy Stg 1	-	-	-	-	5.65	-
Critical Hdwy Stg 2	-	-	- 0.70	-	5.65	- 100
Follow-up Hdwy	-		2.272		••	3.489
Pot Cap-1 Maneuver	-	-	1113	-	412	698
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	680	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1113	-	407	698
Mov Cap-2 Maneuver	-	-	-	-	407	-
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	672	-
Approach	EB		WB		NB	
			0.3		16.6	
HCM LOS	0		0.5			
HCM LOS					С	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		428	_	_	1113	_
HCM Lane V/C Ratio		0.276	_		0.012	_
HCM Control Delay (s)		16.6	-	_	8.3	-
HCM Lane LOS		С	-	-	A	_
HCM 95th %tile Q(veh)		1.1	-	_	0	-
/ (1011)					_	

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ĵ.		ች	ĵ.			सी	7		4	
Traffic Vol, veh/h	0	179	48	215	187	5	84	2	230	5	0	2
Future Vol, veh/h	0	179	48	215	187	5	84	2	230	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	_	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	179	48	215	187	5	84	2	230	5	0	2
Major/Minor N	1ajor1			Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	192	0	0	227	0	0	824	825	203	939	847	190
Stage 1	-	-	-	-	-	-	203	203	-	620	620	-
Stage 2	-	-	-	-	-	-	621	622	-	319	227	-
Critical Hdwy	4.1	-	-	4.26	-	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1394	-	-	1263	-	-	280	310	792	246	301	857
Stage 1	-	-	-	-	-	-	774	737	-	479	483	-
Stage 2	-	-	-	-	-	-	457	482	-	697	720	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1394	-	-	1263	-	-	243	257	792	151	250	857
Mov Cap-2 Maneuver	-	-	-	-	-	-	243	257	-	151	250	-
Stage 1	-	-	-	-	-	-	774	737	-	479	401	-
Stage 2	-	-	-	-	-	-	378	400	-	493	720	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.5			15.8			23.9		
HCM LOS							С			С		
Minor Lane/Major Mvmt	t N	NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		243	792	1394			1263	-	-			
HCM Lane V/C Ratio		0.354	0.29	-	_	_	0.17	_		0.036		
HCM Control Delay (s)		27.7	11.4	0	_	_	8.4	_	_			
HCM Lane LOS		D	В	A	_	_	A	_	_	C		
HCM 95th %tile Q(veh)		1.5	1.2	0	-	_	0.6	-	-	0.1		

Movement   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBR   SBR   SBR   Configurations   Traffic Vol, veh/h   17   20   12   30   26   174   12   87   19   124   122   28   Conflicting Peds, #/hr   0   0   0   0   0   0   0   0   0	Intersection												
Movement		7											
Lane Configurations			EDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Traffic Vol, veh/h  17 20 12 30 26 174 12 87 19 124 122 28  Future Vol, veh/h  17 20 12 30 26 174 12 87 19 124 122 28  Future Vol, veh/h  17 20 12 30 26 174 12 87 19 124 122 28  Conflicting Peds, #hr  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		ERL		FRK	WRL		WBR			MRK			SBK
Future Vol, veh/h  Future Volume Vol		47		40	00		474			40			00
Conflicting Peds, #/hr													
Sign Control   Stop	·												
RT Channelized													
Storage Length						•							
Veh in Median Storage, # - 0				None									None
Grade, % - 0 0 0 0 0 - 0 0 - 0 0 0 0 0				-									-
Peak Hour Factor						~							
Heavy Vehicles, %   6   0   0   7   0   16   0   15   32   29   5   0													
Mymt Flow         17         20         12         30         26         174         12         87         19         124         122         28           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         605         514         136         521         519         97         150         0         0         106         0         0           Stage 1         384         384         -         121         121         -													
Major/Minor   Minor2   Minor1   Major1   Major2													
Conflicting Flow All 605 514 136 521 519 97 150 0 0 106 0 0  Stage 1 384 384 - 121 121 Stage 2 221 130 - 400 398	Mvmt Flow	1/	20	12	30	26	1/4	12	8/	19	124	122	28
Conflicting Flow All 605 514 136 521 519 97 150 0 0 106 0 0  Stage 1 384 384 - 121 121 Stage 2 221 130 - 400 398													
Conflicting Flow All 605 514 136 521 519 97 150 0 0 106 0 0  Stage 1 384 384 - 121 121 Stage 2 221 130 - 400 398	Major/Minor	Minor2			Minor1			Major1			Major2		
Stage 1         384         384         -         121         121         -	Conflicting Flow All	605	514	136	521	519			0	0	106	0	0
Stage 2   221   130													-
Critical Hdwy         7.16         6.5         6.2         7.17         6.5         6.36         4.1         -         4.39         -         -           Critical Hdwy Stg 1         6.16         5.5         -         6.17         5.5         -	•			-			-	-	-	-	-	-	-
Critical Hdwy Stg 1       6.16       5.5       -       6.17       5.5       - <t< td=""><td>Critical Hdwy</td><td></td><td>6.5</td><td>6.2</td><td>7.17</td><td>6.5</td><td>6.36</td><td>4.1</td><td>-</td><td>-</td><td>4.39</td><td>-</td><td>-</td></t<>	Critical Hdwy		6.5	6.2	7.17	6.5	6.36	4.1	-	-	4.39	-	-
Critical Hdwy Stg 2         6.16         5.5         - 6.17         5.5	Critical Hdwy Stg 1			-				-	-	-	-	-	-
Follow-up Hdwy 3.554 4 3.3 3.563 4 3.444 2.2 - 2.461 Pot Cap-1 Maneuver 404 467 918 458 464 922 1444 - 1333 Stage 1 631 615 - 871 800 Stage 2 772 792 - 616 606	Critical Hdwy Stg 2		5.5	-			-	-	-	-	-	-	-
Pot Cap-1 Maneuver	Follow-up Hdwy		4	3.3		4	3.444	2.2	-	-	2.461	-	-
Stage 1       631       615       -       871       800       -			467			464	922	1444	-	-		-	-
Stage 2         772         792         -         616         606         -				-			-	-	-	-	-	-	-
Platoon blocked, %		772	792	_	616	606	_	-	-	-	-	-	-
Mov Cap-2 Maneuver         288         420         -         402         418         - </td <td>Platoon blocked, %</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td>-</td> <td></td> <td>-</td> <td>-</td>	Platoon blocked, %								-	-		-	-
Mov Cap-2 Maneuver         288         420         -         402         418         - </td <td>Mov Cap-1 Maneuver</td> <td>288</td> <td>420</td> <td>918</td> <td>402</td> <td>418</td> <td>922</td> <td>1444</td> <td>-</td> <td>-</td> <td>1333</td> <td>-</td> <td>-</td>	Mov Cap-1 Maneuver	288	420	918	402	418	922	1444	-	-	1333	-	-
Stage 1         626         558         -         864         794         -	Mov Cap-2 Maneuver	288	420	-	402	418	-	-	-	-	-	-	-
Stage 2         601         786         -         532         550         -		626	558	_	864	794	_	-	-	-	-	-	-
HCM Control Delay, s   15		601	786	-	532	550	-	-	-	-	-	-	-
HCM Control Delay, s   15													
HCM Control Delay, s   15	Annroach	FR			WR			NB			SB		
Minor Lane/Major Mvmt         NBL         NBT         NBR EBLn1WBLn1         SBL         SBT         SBR           Capacity (veh/h)         1444         -         -         409         707         1333         -         -           HCM Lane V/C Ratio         0.008         -         -         0.12         0.325         0.093         -         -           HCM Control Delay (s)         7.5         -         -         15         12.5         8         -         -           HCM Lane LOS         A         -         -         C         B         A         -         -													
Minor Lane/Major Mvmt         NBL         NBT         NBR EBLn1WBLn1         SBL         SBT         SBR           Capacity (veh/h)         1444         -         -         409         707         1333         -         -           HCM Lane V/C Ratio         0.008         -         -         0.12         0.325         0.093         -         -           HCM Control Delay (s)         7.5         -         -         15         12.5         8         -         -           HCM Lane LOS         A         -         -         C         B         A         -         -								0.0			3.0		
Capacity (veh/h)       1444       -       -       409       707       1333       -       -         HCM Lane V/C Ratio       0.008       -       -       0.12       0.325       0.093       -       -         HCM Control Delay (s)       7.5       -       -       15       12.5       8       -       -         HCM Lane LOS       A       -       -       C       B       A       -       -	TIOWI LOO	U			D								
Capacity (veh/h)       1444       -       -       409       707       1333       -       -         HCM Lane V/C Ratio       0.008       -       -       0.12       0.325       0.093       -       -         HCM Control Delay (s)       7.5       -       -       15       12.5       8       -       -         HCM Lane LOS       A       -       -       C       B       A       -       -			.,-										
HCM Lane V/C Ratio       0.008       -       -       0.12       0.325       0.093       -       -         HCM Control Delay (s)       7.5       -       -       15       12.5       8       -       -         HCM Lane LOS       A       -       -       C       B       A       -       -		nt		NBT	NBR I				SBT	SBR			
HCM Control Delay (s) 7.5 15 12.5 8 HCM Lane LOS A C B A	Capacity (veh/h)			-	-				-	-			
HCM Lane LOS A C B A				-	-				-	-			
				-	-				-	-			
HCM 95th %tile Q(veh) 0 0.4 1.4 0.3				-	-				-	-			
	HCM 95th %tile Q(veh	1)	0	-	-	0.4	1.4	0.3	-	-			

Intersection						
Int Delay, s/veh	0.3					
		WDD	NET	NDD	ODL	ODT
	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	_	ĵ,			4
Traffic Vol, veh/h	3	5	214	8	3	111
Future Vol, veh/h	3	5	214	8	3	111
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	20	19	0	0	34
Mvmt Flow	3	5	214	8	3	111
NA -1/NA1 NA1	4		1.1.1		4	
	inor1		//ajor1		Major2	
Conflicting Flow All	335	218	0	0	222	0
Stage 1	218	-	-	-	-	-
Stage 2	117	-	-	-	-	-
Critical Hdwy	6.4	6.4	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.48	-	-	2.2	-
Pot Cap-1 Maneuver	664	779	-	-	1359	-
Stage 1	823	-	-	-	-	-
Stage 2	913	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	663	779	-	-	1359	-
Mov Cap-2 Maneuver	663	_	-	_	_	_
Stage 1	823	-	_	_	-	-
Stage 2	911	_	_	_	_	_
01490 2						
Approach	WB		NB		SB	
HCM Control Delay, s	10		0		0.2	
HCM L OC	В					
HCM LOS						
HCIVI LOS						
		NRT	NRDV	MRI n1	QRI.	CRT
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Minor Lane/Major Mvmt Capacity (veh/h)		-	-	731	1359	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio		-	-	731 0.011	1359 0.002	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		- - -	- - -	731 0.011 10	1359 0.002 7.7	- - 0
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio		-	-	731 0.011	1359 0.002	-

Intersection												
Int Delay, s/veh	0											
• •												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	191	0	0	101	0
Future Vol, veh/h	1	0	0	0	0	0	0	191	0	0	101	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	191	0	0	101	0
Major/Minor N	/linor2		N	/linor1		N	Major1		N	/lajor2		
Conflicting Flow All	292	292	101	292	292	191	101	0	0	191	0	0
	101	101	101	191	191	191	101	-	-	191	-	-
Stage 1	191	191	-	101	101				-	_		
Stage 2	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy	7.1	5.5		6.1	5.5			-			-	
Critical Hdwy Stg 1	7.1	5.5	-	6.1		-	-	-	-	-	-	-
Critical Hdwy Stg 2	4.4		3.3	3.5	5.5	3.3	2.2	-	-	2.2	-	-
Follow-up Hdwy	504	622	960	664	622	3.3 856	1504	-	-	1395	-	-
Pot Cap-1 Maneuver	713	815		815	746	000	1004	-	-	1393	-	-
Stage 1	629	746	-	910	815	-	-	-	<del>-</del>	<del>-</del>	-	-
Stage 2	029	740	-	910	010	-	-	=	-	_		
Platoon blocked, %	504	622	960	664	622	856	1504	-	<del>-</del>	1395	-	-
Mov Cap-1 Maneuver	504	622		664	622	000			-			
Mov Cap-2 Maneuver	713	815	-	815	746	-	-	-	-	-	-	-
Stage 1	629	746	-	910	815	-	-	=	-	_		-
Stage 2	029	740	-	310	010	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.2			0			0			0		
HCM LOS	В			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NRR	EBLn1V	VRI n1	SBL	SBT	SBR			
	•	1504	NDT	NON	504		1395	001	ODIX			
Capacity (veh/h) HCM Lane V/C Ratio			-	-	0.002	-		=	-			
		_	-		12.2	_	_	-	-			
HCM Control Delay (s) HCM Lane LOS		0	-	-		0	0	-	-			
		A 0	-	-	B 0	Α	A 0	-	-			
HCM 95th %tile Q(veh)		U	-	-	U	-	U	-	-			

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	4	0	13	1	59	1	146	43	40	56	2
Future Vol, veh/h	0	4	0	13	1	59	1	146	43	40	56	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mvmt Flow	0	4	0	13	1	59	1	146	43	40	56	2
Major/Minor N	/linor2		l	Minor1		ľ	Major1			Major2		
Conflicting Flow All	337	328	57	309	308	168	58	0	0	189	0	0
Stage 1	137	137	-	170	170	-	-	-	-	-	-	-
Stage 2	200	191	-	139	138	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3		4	3.426	2.2	-	-	2.452	-	-
Pot Cap-1 Maneuver	621	594	1015	632	609	846	1559	-	-	1243	-	-
Stage 1	871	787	-	818	762	-	-	-	-	-	-	-
Stage 2	806	746	-	850	786	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	562	574	1015	612	588	846	1559	-	-	1243	-	-
Mov Cap-2 Maneuver	562	574	-	612	588	-	-	-	-	-	-	-
Stage 1	870	761	-	817	761	-	-	-	-	-	-	-
Stage 2	748	745	-	818	760	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.3			10			0			3.3		
HCM LOS	В			В								
Minor Lane/Major Mvmt	1	NBL	NBT	NRR I	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1559	-	-	574	788	1243	-	021(			
HCM Lane V/C Ratio		0.001	<u>-</u>			0.093		_				
HCM Control Delay (s)		7.3	0	_	11.3	10	8	0	_			
HCM Lane LOS		7.5 A	A	-	В	В	A	A	_			
HCM 95th %tile Q(veh)		0	-	_	0	0.3	0.1	-	_			
HOW JOHN JUNE Q(VEII)		U	_		0	0.0	0.1		_			

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	ĵ.	
Traffic Vol, veh/h	9	2	3	63	21	12
Future Vol, veh/h	9	2	3	63	21	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	14	0
Mymt Flow	9	2	3	63	21	12
IVIVIII( I IOW	9	2	J	00	۷1	12
Major/Minor N	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	96	27	33	0	-	0
Stage 1	27	-	-	-	-	-
Stage 2	69	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	_	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	908	1054	1592	-	-	-
Stage 1	1001	-	-	-	-	-
Stage 2	959	-	_	-	_	-
Platoon blocked, %				_	-	-
Mov Cap-1 Maneuver	906	1054	1592	-	-	_
Mov Cap-2 Maneuver	906	-	-	_	_	_
Stage 1	999	_	_	_	_	_
Stage 2	959	<u>-</u>	_	_	_	_
Olugo Z	303					
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.3		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBL	NRT	EBLn1	SBT	SBR
						אומט
Capacity (veh/h)		1592	-	930	-	-
HCM Control Doloy (a)		0.002		0.012	-	-
HCM Long LOS		7.3	0	8.9	-	-
HCM 05th % tile O(vob)		A	Α	A	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Int Delay, s/veh	Intersection												
Lane Configurations		0.1											
Lane Configurations	Movement	EBI	FRT	FRR	WRI	WRT	WBR	NBI	NBT	NBR	SBI	SBT	SBR
Traffic Vol, veh/h				LJI	1,02		1,51	1100		, , j	UDL		UDIT
Future Vol, veh/h Conflicting Peds, #lhr O O O O O O O O O O O O O O O O O O O		0		0	0		0	1		0	0		1
Conflicting Peds, #/hr						-				-			-
Sign Control   Stop   Stop   Stop   Stop   Stop   Stop   Stop   Free										-			
RT Channelized		Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
Veh in Median Storage, # - 0								-	-	None	-	-	None
Grade, %         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         0         100         20         2         1         2         2         2         2         2         2         2         2         2         2         2         2         2         <	Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Peak Hour Factor	Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, %	Grade, %	-	0	-	-	0	-	-	0	-	-		-
Mynt Flow         0         0         0         0         0         1         64         0         0         24         1           Major/Minor         Minor2         Minor1         Major1         Major2           Conflicting Flow All         91         91         25         91         91         64         25         0         0         64         0         0           Stage 1         25         25         -         66         66         -	Peak Hour Factor	100	100	100	100	100	100	100		100	100	100	100
Major/Minor   Minor2   Minor1   Major1   Major2			0			0	0			0			
Conflicting Flow All	Mvmt Flow	0	0	0	0	0	0	1	64	0	0	24	1
Conflicting Flow All													
Conflicting Flow All	Major/Minor M	linor2		1	Minor1		<b>N</b>	Major1		<b>N</b>	Major2		
Stage 1       25       25       -       66       66       -        -       -       -       -       -       -       -       -       -       -       -       -       -       -       -        -       <		91	91	25	91	91			0	0	64	0	0
Critical Hdwy         7.1         6.5         6.2         7.1         6.5         6.2         4.1         -         4.1         -         -         4.1         -         -         4.1         -         -         4.1         -         -         4.1         -		25	25	-	66	66	-	-	-	-	-	-	_
Critical Hdwy Stg 1       6.1       5.5       -       6.1       5.5       -	Stage 2	66			25	25			-	-	-	-	-
Critical Hdwy Stg 2         6.1         5.5         -         6.1         5.5         -<				6.2			6.2	4.1	-		4.1	-	-
Follow-up Hdwy 3.5 4 3.3 3.5 4 3.3 2.2 - 2.2 2.2 Pot Cap-1 Maneuver 898 803 1057 898 803 1006 1603 - 1551 Stage 1 998 878 - 950 844	, ,			-			-	-	-	-	-	-	-
Pot Cap-1 Maneuver									-	-	-	-	-
Stage 1         998         878         -         950         844         -									-	-		-	-
Stage 2         950         844         -         998         878         -	•			1057			1006	1603	-	-	1551	-	-
Platoon blocked, %				-			-	-	-	-	-	-	-
Mov Cap-1 Maneuver         897         802         1006         1603         -         - 1551         -         -           Mov Cap-2 Maneuver         897         802         -         897         802         -		950	844	-	998	878	-	-	-	-	-	-	-
Mov Cap-2 Maneuver         897         802         -         897         802         - </td <td></td> <td>007</td> <td>000</td> <td>4055</td> <td>007</td> <td>000</td> <td>4000</td> <td>4000</td> <td>-</td> <td>-</td> <td>4554</td> <td>-</td> <td>-</td>		007	000	4055	007	000	4000	4000	-	-	4554	-	-
Stage 1         997         878         -         949         843         -							1006	1603	-	-	1551	-	-
Stage 2         949         843         -         998         878         -	•						-	-	-	-	-	-	-
Approach         EB         WB         NB         SB           HCM Control Delay, s         0         0         0.1         0           HCM LOS         A         A         A         A             Minor Lane/Major Mvmt         NBL         NBT         NBR EBLn1WBLn1         SBL         SBT         SBR           Capacity (veh/h)         1603         -         -         -         1551         -         -           HCM Lane V/C Ratio         0.001         -         -         -         -         -         -         -           HCM Control Delay (s)         7.2         0         -         0         0         -         -           HCM Lane LOS         A         A         -         A         A         -         -         -	•						-	-	-	-	-	-	-
HCM Control Delay, s	Staye 2	545	043	-	330	0/0	_	_	-	_	_	-	_
HCM Control Delay, s													
Minor Lane/Major Mvmt         NBL         NBT         NBR EBLn1WBLn1         SBL         SBT         SBR           Capacity (veh/h)         1603         -         -         -         1551         -         -           HCM Lane V/C Ratio         0.001         -         -         -         -         -         -         -           HCM Control Delay (s)         7.2         0         -         0         0         -         -           HCM Lane LOS         A         A         -         A         A         -         -													
Minor Lane/Major Mvmt         NBL         NBT         NBR EBLn1WBLn1         SBL         SBT         SBR           Capacity (veh/h)         1603         -         -         -         1551         -         -           HCM Lane V/C Ratio         0.001         -         -         -         -         -         -         -           HCM Control Delay (s)         7.2         0         -         0         0         -         -           HCM Lane LOS         A         A         -         A         A         -         -								0.1			0		
Capacity (veh/h) 1603 1551 HCM Lane V/C Ratio 0.001	HCM LOS	Α			Α								
Capacity (veh/h) 1603 1551 HCM Lane V/C Ratio 0.001													
HCM Lane V/C Ratio 0.001 HCM Control Delay (s) 7.2 0 - 0 0 0 HCM Lane LOS A A - A A A	Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBL <sub>n1</sub>	SBL	SBT	SBR			
HCM Lane V/C Ratio 0.001 HCM Control Delay (s) 7.2 0 - 0 0 0 HCM Lane LOS A A - A A A	Capacity (veh/h)		1603	-	-	-	-	1551	-	-			
HCM Lane LOS A A - A A			0.001	-	-	-	-		-	-			
			7.2	0	-	0	0	0	-	-			
HCM 95th %tile Q(veh) 0 0				Α	-	Α	Α		-	-			
	HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	2.1					
•		14/00	NET	NEE	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		f)			4
Traffic Vol, veh/h	12	3	51	41	26	32
Future Vol, veh/h	12	3	51	41	26	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	0	14	23	12	26
Mvmt Flow	12	3	51	41	26	32
	Minor1		//ajor1		Major2	_
Conflicting Flow All	156	72	0	0	92	0
Stage 1	72	-	-	-	-	-
Stage 2	84	-	-	-	-	-
Critical Hdwy	6.57	6.2	-	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	-	-	-	-	-
Follow-up Hdwy	3.653	3.3	-	-	2.308	-
Pot Cap-1 Maneuver	802	996	-	-	1442	-
Stage 1	914	-	-	-	-	-
Stage 2	903	-	-	_	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	788	996	_	_	1442	_
Mov Cap-2 Maneuver	788	-	_	_	-	_
Stage 1	914	_	_	_	_	_
Stage 2	887	_	_	_	_	_
Olugo Z	501					
Approach	WB		NB		SB	
HCM Control Delay, s	9.5		0		3.4	
HCM LOS	Α					
Minor Long/Major Maria	<b>.</b> +	NDT	NDDV	MDI 4	CDI	CDT
Minor Lane/Major Mvn	IL	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1442	-
HCM Lane V/C Ratio		-	-	0.018		-
HCM Control Delay (s		-	-	9.5	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh	)	-	-	0.1	0.1	-

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4			4	
Traffic Vol, veh/h	53	65	3	0	22	53	1	0	2	135	1	12
Future Vol, veh/h	53	65	3	0	22	53	1	0	2	135	1	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	53	65	3	0	22	53	1	0	2	135	1	12
Major/Minor I	Major1			Major2		Minor1		N		Minor2		
Conflicting Flow All	75	0	0	68	0	0	228	248	67	196	196	22
Stage 1	-	-	-	-	-	-	173	173	-	22	22	-
Stage 2	-	-	-	-	-	-	55	75	-	174	174	-
Critical Hdwy	4.39	-	-	4.1	-	-	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Follow-up Hdwy	2.461	-	-	2.2	-	-	3.5	4	3.3	3.689	4	3.975
Pot Cap-1 Maneuver	1369	-	-	1546	-	-	731	658	1002	723	703	879
Stage 1	-	-	-	-	-	-	834	760	-	950	881	-
Stage 2	-	-	-	-	-	-	962	836	-	785	759	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1369	-	-	1546	-	-	698	632	1002	700	675	879
Mov Cap-2 Maneuver	-	-	-	-	-	-	698	632	-	700	675	-
Stage 1	-	-	-	-	-	-	801	730	-	912	881	-
Stage 2	-	-	-	-	-	-	948	836	-	752	729	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.1			11.4		
HCM LOS							Α			В		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		875	1369	-	-	1546	-	-	712			
HCM Lane V/C Ratio		0.003		-	-	-	-	-	0.208			
HCM Control Delay (s)		9.1	7.7	0	-	0	-	-	11.4			
HCM Lane LOS		Α	Α	Α	-	Α	-	-	В			
HCM 95th %tile Q(veh	)	0	0.1	-	-	0	-	-	0.8			

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	- ↑		ሻ	<b>\$</b>			4			4	
Traffic Vol, veh/h	9	286	8	29	100	101	1	5	28	71	7	7
Future Vol, veh/h	9	286	8	29	100	101	1	5	28	71	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	9	286	8	29	100	101	1	5	28	71	7	7
Major/Minor N	Major1		1	Major2		1	Minor1			Minor2		
Conflicting Flow All	201	0	0	294	0	0	524	567	290	534	521	151
Stage 1	-	-	-	-	-	-	308	308	-	209	209	-
Stage 2	-	-	-	-	-	-	216	259	-	325	312	-
Critical Hdwy	4.32	-	-	4.17	-	-	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Follow-up Hdwy	2.398	-	-	2.263	-	-	3.5	4	3.3	3.581	4.126	3.426
Pot Cap-1 Maneuver	1260	-	-	1239	-	-	467	436	754	446	443	865
Stage 1	-	-	-	-	-	-	706	664	-	777	707	-
Stage 2	-	-	-	-	-	-	791	697	-	673	637	-
Platoon blocked, %		-	-		-	-						_
Mov Cap-1 Maneuver	1260	-	-	1239	-	-	447	423	754	416	430	865
Mov Cap-2 Maneuver	-	-	-	-	-	-	447	423	-	416	430	-
Stage 1	-	-	-	-	-	-	701	659	-	772	691	-
Stage 2	-	-	-	-	-	-	758	681	-	638	633	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			10.7			15.2		
HCM LOS							В			С		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		664				1239	-	-				
HCM Lane V/C Ratio		0.051		_		0.023	_		0.195			
HCM Control Delay (s)		10.7	7.9	-	-	8	-	-	15.2			
HCM Lane LOS		В	A	_	_	A	_	-	C			
HCM 95th %tile Q(veh)	)	0.2	0	_	_	0.1	_	_	0.7			

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		î,			सी						4	
Traffic Vol, veh/h	0	96	266	13	167	0	0	0	0	1	4	54
Future Vol, veh/h	0	96	266	13	167	0	0	0	0	1	4	54
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mvmt Flow	0	96	266	13	167	0	0	0	0	1	4	54
Major/Minor N	/lajor1		1	Major2					N	/linor2		
Conflicting Flow All	-	0	0	362	0	0				422	555	167
Stage 1	_	-	-	-	-	-				193	193	-
Stage 2	_	_	_	<u>-</u>	_	<u>-</u>				229	362	_
Critical Hdwy	_	_	_	4.25	_	_				6.4	6.75	6.5
Critical Hdwy Stg 1	_	_	_	20	_	_				5.4	5.75	-
Critical Hdwy Stg 2	_	-	_	-	_	-				5.4	5.75	_
Follow-up Hdwy	-	_	-	2.335	_	-				3.5	4.225	3.57
Pot Cap-1 Maneuver	0	-	-	1128	-	0				592	410	809
Stage 1	0	_	_		_	0				845	700	-
Stage 2	0	-	-	-	-	0				814	587	-
Platoon blocked, %		_	_		_							
Mov Cap-1 Maneuver	_	_	_	1128	-	_				584	0	809
Mov Cap-2 Maneuver	_	_	_	-	_	_				584	0	-
Stage 1	_	_	_	_	-	_				845	0	-
Stage 2	_	_	-	_	_	_				803	0	_
Approach	EB			WB						SB		
HCM LOS	0			0.6						9.8		
HCM LOS										Α		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-		1128	-	803						
HCM Lane V/C Ratio		-	-	0.012	-	0.073						
HCM Control Delay (s)		-	-	8.2	0	9.8						
HCM Lane LOS		-	-	Α	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	-	0.2						

Intersection												
Int Delay, s/veh	10											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			<del>(</del> Î			4				
Traffic Vol, veh/h	93	4	0	0	6	5	174	10	6	0	0	0
Future Vol, veh/h	93	4	0	0	6	5	174	10	6	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	7	25	0	0	50	0	24	50	50	2	2	2
Mvmt Flow	93	4	0	0	6	5	174	10	6	0	0	0
Major/Minor N	Major1		I	Major2			Minor1					
Conflicting Flow All	11	0		- viajoiz	_	0	199	201	4			
Stage 1	-	-	_	_	_	-	190	190	-			
Stage 2	<u>-</u>	_	_	<u> </u>	_	_	9	11	<u>-</u>			
Critical Hdwy	4.17		_	_		_	6.64	7	6.7			
Critical Hdwy Stg 1		_	_	<u> </u>	_	_	5.64	6	-			
Critical Hdwy Stg 2	_	_	_	_	_	_	5.64	6	_			
Follow-up Hdwy	2.263	<u>-</u>	_	_	_	_	0 740	4.45	3.75			
Pot Cap-1 Maneuver	1576	_	0	0	_	_	742	618	955			
Stage 1	-	<u>-</u>	0	0	_	_	792	661	-			
Stage 2	-		0	0	_	_	960	800	_			
Platoon blocked, %		_	U	U	_	_	300	000				
Mov Cap-1 Maneuver	1576		_	_	_	_	698	0	955			
Mov Cap-1 Maneuver	1070	_	_	<u> </u>	_	_	698	0	-			
Stage 1	_		_			_	745	0	_			
Stage 2	_	_	_	_		-	960	0	_			
Olago Z							300	J				
Approach	ED			WD			ND					
Approach	EB			WB			NB					
HCM Control Delay, s	7.1			0			12					
HCM LOS							В					
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		704	1576	-	-	-						
HCM Lane V/C Ratio			0.059	-	-	-						
HCM Control Delay (s)		12	7.4	0	-	-						
HCM Lane LOS		В	Α	Α	-	-						
HCM 95th %tile Q(veh)		1.1	0.2	-	-	-						

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	-	-	-	_	-	-	-	-	-	_	-
Veh in Median Storage	,# -	0	-	_	0	-	-	0	-	_	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	2	7	7	2	0	0	0	0	6	11	6
Major/Minor N	Major1		N	Major2					N	Minor2		
Conflicting Flow All	- -	0	0	9	0	0				22	25	2
Stage 1	_	-	-	-	-	-				16	16	-
Stage 2	_	_	_	_	_	_				6	9	_
Critical Hdwy	_	_	_	4.27	_	_				6.57	7.23	6.2
Critical Hdwy Stg 1	_	_	_	7.21	_	_				5.57	6.23	0.2
Critical Hdwy Stg 2	_	_		_	_	_				5.57	6.23	_
Follow-up Hdwy	_	<u>-</u>	_	2.353	<u>-</u>	_					4.657	3.3
Pot Cap-1 Maneuver	0	_	_	1518	_	0				957	747	1088
Stage 1	0	_	_	-	_	0				969	760	-
Stage 2	0	_		_	_	0				979	766	_
Platoon blocked, %		_	_		_	U				313	700	
Mov Cap-1 Maneuver	_	_	_	1518	_	_				952	0	1088
Mov Cap-1 Maneuver	<u>-</u>	<u>-</u>	_	-	<u>-</u>	_				952	0	-
Stage 1	_	_	_	_	_	_				969	0	_
Stage 2	_	_	_	_	_	_				974	0	_
Olugo Z										317	J	
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.7						8.6		
HCM LOS				0.1						Α		
										, \		
Minor Lane/Major Mvm	it	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-	-	1518	-	1015						
HCM Lane V/C Ratio		_	_	0.005		0.023						
HCM Control Delay (s)		-	-	7.4	0	8.6						
HCM Lane LOS		_	-	Α	A	Α						
HCM 95th %tile Q(veh)		_	_	0	-	0.1						
						J. 1						

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			<b>₽</b>			4				
Traffic Vol, veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Future Vol, veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	13	11	100	33	0	2	2	2
Mvmt Flow	5	3	0	0	8	18	1	3	9	0	0	0
Major/Minor N	/lajor1		ı	Major2		N	/linor1					
Conflicting Flow All	26	0		- viajuiz	_	0	30	39	3			
Stage 1	- 20	-	-	-	_	-	13	13	-			
Stage 2	_	_	-	-	_	-	17	26	-			
Critical Hdwy	4.1		_	_	-	_	7.4	6.83	6.2			
Critical Hdwy Stg 1	7.1	_		_	_	_	6.4	5.83	0.2			
Critical Hdwy Stg 2	_		_	_	_	_	6.4	5.83	_			
Follow-up Hdwy	2.2	_		_	_	_	4.4	4.297	3.3			
Pot Cap-1 Maneuver	1601		0	0	_	_	783	796	1087			
Stage 1	-	_	0	0	_	_	806	827	-			
Stage 2		_	0	0	_	_	802	816				
Platoon blocked, %		_	U	U	_	_	002	010				
Mov Cap-1 Maneuver	1601	_	_	_	-	_	781	0	1087			
Mov Cap-1 Maneuver	-	_		_	_	_	781	0	-			
Stage 1	_		_		_	_	804	0	_			
Stage 2	_	_	_	_	_	-	802	0	_			
Glaye Z	-	_	_	<u>-</u>	-	_	002	U	-			
Approach	EB			WB			NB					
HCM Control Delay, s	4.5			0			8.5					
HCM LOS							Α					
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1046	1601									
HCM Lane V/C Ratio			0.003	_	_	_						
HCM Control Delay (s)		8.5	7.3	0	_	_						
HCM Lane LOS		0.5 A	7.5 A	A	_	_						
HCM 95th %tile Q(veh)		0	0	_	_	_						
		- 3										

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Future Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	25	0	0	0	18	0	0	0	0	0
Mvmt Flow	1	7	4	0	14	0	12	2	0	0	0	0
Major/Minor M	lajor1		ľ	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	14	0	0	11	0	0	25	25	9	26	27	14
Stage 1	-	-	-	-	-	-	11	11	-	14	14	-
Stage 2	-	-	-	-	-	-	14	14	-	12	13	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.28	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.662	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1617	-	-	1621	-	-	947	872	1079	990	870	1072
Stage 1	-	-	-	-	-	-	970	890	-	1011	888	-
Stage 2	-	-	-	-	-	-	966	888	-	1014	889	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1617	-	-	1621	-	-	946	871	1079	987	869	1072
Mov Cap-2 Maneuver	-	-	-	-	-	-	946	871	-	987	869	-
Stage 1	-	-	-	-	-	-	969	889	-	1010	888	-
Stage 2	-	-	-	-	-	-	966	888	-	1011	888	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			8.9			0		
HCM LOS							Α			Α		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		935	1617			1621			_			
HCM Lane V/C Ratio		0.015		_	_	-	_	_	_			
HCM Control Delay (s)		8.9	7.2	0	_	0	_	_	0			
HCM Lane LOS		A	A	A	_	A	_	-	A			
HCM 95th %tile Q(veh)		0	0	-	_	0	-	-	-			
2 22 /0 2(.011)												

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.			4						4	
Traffic Vol, veh/h	0	65	0	33	7	0	0	0	0	109	1	5
Future Vol, veh/h	0	65	0	33	7	0	0	0	0	109	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	65	0	33	7	0	0	0	0	109	1	5
Major/Minor Ma	ajor1		N	//ajor2					N	/linor2		
Conflicting Flow All	-	0	0	65	0	0				138	138	7
Stage 1	-	-	-	-	-	-				73	73	-
Stage 2	-	-	-	-	-	-				65	65	-
Critical Hdwy	-	-	-	4.23	-	-				6.43	7.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	6.5	-
Follow-up Hdwy	-	-	-	2.317	-	-				3.527	4.9	3.48
Pot Cap-1 Maneuver	0	-	-	1470	-	0				853	604	1025
Stage 1	0	-	-	-	-	0				947	676	-
Stage 2	0	-	-	-	-	0				955	683	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1470	-	-				833	0	1025
Mov Cap-2 Maneuver	-	-	-	-	-	-				833	0	-
Stage 1	-	-	-	-	-	-				947	0	-
Stage 2	-	-	-	-	-	-				933	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.2						10		
HCM LOS	•			7.2						В		
Minor Lane/Major Mvmt		EBT	EBR	WBL	WBT :	SBLn1						
Capacity (veh/h)				1470	-							
HCM Lane V/C Ratio		_		0.022		0.137						
HCM Control Delay (s)		_	_		0	10						
HCM Lane LOS		_	<u>-</u>	Α.	A	В						
HCM 95th %tile Q(veh)		_	_	0.1	-	0.5						
σσαι /σαισ α(νσιι)				<b>V</b> . 1		0.0						

Intersection													
Int Delay, s/veh	3.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			<b>1</b>			4					
Traffic Vol, veh/h	42	132	0	0	31	78	9	4	121	0	0	0	
Future Vol, veh/h	42	132	0	0	31	78	9	4	121	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	17	7	2	0	12	4	0	25	10	2	2	2	
Mvmt Flow	42	132	0	0	31	78	9	4	121	0	0	0	
Major/Minor N	Major1		N	Major2		N	Minor1						
Conflicting Flow All	109	0	-	-	-	0	286	325	132				
Stage 1	-	-	-	-	-	-	216	216	-				
Stage 2	-	-	-	-	-	-	70	109	-				
Critical Hdwy	4.27	-	-	-	-	-	6.4	6.75	6.3				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.75	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.75	-				
Follow-up Hdwy	2.353	-	-	-	-	-	3.5	4.225	3.39				
Pot Cap-1 Maneuver	1393	-	0	0	-	-	709	557	896				
Stage 1	-	-	0	0	-	-	825	683	-				
Stage 2	-	-	0	0	-	-	958	763	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1393	-	-	-	-	-	686	0	896				
Mov Cap-2 Maneuver	-	-	-	-	-	-	686	0	-				
Stage 1	-	-	-	-	-	-	798	0	-				
Stage 2	-	-	-	-	-	-	958	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	1.9			0			9.8						
HCM LOS							Α						
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		877	1393	-	_	-							
HCM Lane V/C Ratio		0.153	0.03	_	_	-							
HCM Control Delay (s)		9.8	7.7	0	_	-							
HCM Lane LOS		A	Α	A	-	-							
HCM 95th %tile Q(veh)		0.5	0.1	-	-	-							

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	250	0	0	102	3	1	1	4	1	0	6
Future Vol, veh/h	3	250	0	0	102	3	1	1	4	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	250	0	0	102	3	1	1	4	1	0	6
Major/Minor N	//ajor1		ı	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	105	0	0	250	0	0	363	361	250	363	360	104
Stage 1	-	-	-		-	-	256	256	-	104	104	-
Stage 2	_	_	-	_	-	_	107	105	_	259	256	-
Critical Hdwy	4.1	_	-	4.1	_	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	_	_	-	_	_	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	_	6.1	5.5	_	6.1	5.5	_
Follow-up Hdwy	2.2	_	_	2.2	_	_	3.5	4	3.3	3.5	4	3.453
Pot Cap-1 Maneuver	1499	-	-	1327	-	_	597	569	794	597	570	911
Stage 1	-	-	_	-	_	_	753	699	-	907	813	-
Stage 2	-	-	_	_	_	_	903	812	-	750	699	_
Platoon blocked, %		-	_		_	_		•				
Mov Cap-1 Maneuver	1499	_	-	1327	_	-	592	568	794	592	569	911
Mov Cap-2 Maneuver	-	_	-	_	-	-	592	568	-	592	569	-
Stage 1	-	-	-	-	-	-	751	698	-	905	813	-
Stage 2	-	_	-	-	-	-	897	812	-	744	698	-
Ü												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0			10.1			9.3		
HCM LOS							В			A		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		707	1499	-	-	1327	-	-	846			
HCM Lane V/C Ratio				-	-	_	-	-	0.008			
HCM Control Delay (s)		10.1	7.4	0	-	0	-	_				
HCM Lane LOS		В	Α	A	-	A	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection							
Int Delay, s/veh	5.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	EBL Š			WDK	SBL	SBK	
Traffic Vol, veh/h	<b>1</b> 69	<b>↑</b> 56	<b>1</b> → 57	29	<b>1</b> 24	68	
Future Vol, veh/h	169	56	57	29	24	68	
Conflicting Peds, #/hr	0	0	0	0	0	00	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-		-		- Olop	None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage		0	0	_	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	169	56	57	29	24	68	
Major/Minor	Major1	N	Major2	ı	Minor2		
Conflicting Flow All	86	0	- viajoiz	0	466	72	
Stage 1	-	-	_	-	72	-	
Stage 2	_	-	_	-	394	-	
Critical Hdwy	4.15	-	_	-	6.4	6.26	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.245	-	-	-	3.5	3.354	
Pot Cap-1 Maneuver	1492	-	-	-	559	979	
Stage 1	-	-	-	-	956	-	
Stage 2	-	-	-	-	686	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1492	-	-	-	496	979	
Mov Cap-2 Maneuver	-	-	-	-	496	-	
Stage 1	-	-	-	-	848	-	
Stage 2	-	-	-	-	686	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.8		0		9.9		
HCM LOS					Α		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WRR	SBLn1	SBI n2
Capacity (veh/h)		1492		-	-		979
HCM Lane V/C Ratio		0.113		-		0.048	
HCM Control Delay (s)		7.7	_	_	_		9
HCM Lane LOS		A	_	_	_	В	A
HCM 95th %tile Q(veh)	)	0.4	-	-	-	0.2	0.2

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>	בטול	ሻ	1≯	11011	TIDE	4	וטו	UDL	4	ODIN
Traffic Vol, veh/h	47	33	4	1	46	4	8	16	7	0	2	21
Future Vol, veh/h	47	33	4	1	46	4	8	16	7	0	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	_	_	None	-	-	Stop	-	-	None
Storage Length	100	_	-	150	-	-	-	_	-	_	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	47	33	4	1	46	4	8	16	7	0	2	21
Major/Minor N	Major1			Major2			Minor1		N	/linor2		
Conflicting Flow All	50	0	0	37	0	0	191	181	35	187	181	48
Stage 1	-	-	-	-	_	-	129	129	-	50	50	-
Stage 2	-	-	-	-	-	-	62	52	-	137	131	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.56	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.56	-	6.1	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4.054	3.426	3.5	4	3.39
Pot Cap-1 Maneuver	1501	-	-	1587	-	-	773	706	1005	778	717	999
Stage 1	-	-	-	-	-	-	880	782	-	968	857	-
Stage 2	-	-	-	-	-	-	954	844	-	871	792	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1501	-	-	1587	-	-	737	683	1005	741	694	999
Mov Cap-2 Maneuver	-	-	-	-	-	-	737	683	-	741	694	-
Stage 1	-	-	-	-	-	-	853	758	-	938	856	-
Stage 2	-	-	-	-	-	-	931	843	-	820	767	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.2			0.1			9.1			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		904				1587	-	-	962			
HCM Lane V/C Ratio		0.034		_		0.001	_		0.024			
HCM Control Delay (s)		9.1	7.5	-	-	7.3	-	-	8.8			
HCM Lane LOS		A	A	_	_	A	_	_	A			
HCM 95th %tile Q(veh)		0.1	0.1	-	-	0	-	-	0.1			
			***									

Intersection						
Int Delay, s/veh	2.2					
		EDD	\\/DI	WDT	NDI	NDD
Movement Lana Configurations	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>♣</b>	1	<u>ኝ</u>	<b>†</b>	10	14
Traffic Vol, veh/h Future Vol, veh/h	40	1	5	42 42	10	14
· · · · · · · · · · · · · · · · · · ·	40	0	0	42	0	0
Conflicting Peds, #/hr	-	Free	-	Free	Stop	Stop
Sign Control RT Channelized	Free	None	Free	None		None
Storage Length	-		150	None -	-	None -
	- # 0	-	150		0	
Veh in Median Storage,		-		0		-
Grade, %	100	100	100	100	100	100
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	26	0	0	15	0	7
Mvmt Flow	40	1	5	42	10	14
Major/Minor M	ajor1	N	//ajor2	1	Minor1	
Conflicting Flow All	0	0	41	0	93	41
Stage 1	_	_	_	-	41	_
Stage 2	-	_	-	_	52	_
Critical Hdwy	-	-	4.1	-	6.4	6.27
Critical Hdwy Stg 1	_	_	-	_	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	_
Follow-up Hdwy	_	_	2.2	_		3.363
Pot Cap-1 Maneuver	-	-	1581	-	912	1016
Stage 1	_	_	-	_	987	-
Stage 2	_	_	_	_	976	_
Platoon blocked, %	_	_		_	0.0	
Mov Cap-1 Maneuver	_	_	1581	_	909	1016
Mov Cap-2 Maneuver	_	_	-	_	909	-
Stage 1	_	_	_	_	987	_
Stage 2	_	_	_	_	973	_
Staye 2		-	-		913	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.8	
HCM LOS					Α	
Minor Lang/Major Mumt		NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Major Mvmt						
Capacity (veh/h)		968	-		1581	-
HCM Lane V/C Ratio		0.025	-		0.003	-
HCM Control Delay (s)		8.8	-	-	7.3	-
		Α	-	-	Α	-
HCM Lane LOS HCM 95th %tile Q(veh)		0.1	_	_	0	_

Intersection						
Int Delay, s/veh	0.7					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	^	<b>^</b>	^	0	र्
Traffic Vol, veh/h	2	0	16	0	0	6
Future Vol, veh/h	2	0	16	0	0	6
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	17
Mvmt Flow	2	0	16	0	0	6
NA=:==/NA:===	1:1		1-:1		4-:0	
	/linor1		Major1		Major2	
Conflicting Flow All	22	16	0	0	16	0
Stage 1	16	-	-	-	-	-
Stage 2	6	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	1000	1069	-	-	1615	-
Stage 1	1012	-	-	-	-	-
Stage 2	1022	-	-	-	-	-
Platoon blocked, %			-	-		_
Mov Cap-1 Maneuver	1000	1069	_	_	1615	_
Mov Cap-2 Maneuver	1000	-	_	_	-	_
Stage 1	1012	_	_	_	_	_
Stage 2	1012	_	_	_	_	_
Staye 2	1022	_	_	-	_	_
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
I IOW LOO						
TIOM EOO						
	t	NRT	NRRV	WRI n1	SBI	SRT
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL 1615	SBT
Minor Lane/Major Mvm Capacity (veh/h)	t	-	-	1000	1615	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	t	-	-	1000 0.002	1615 -	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t	- - -	- - -	1000 0.002 8.6	1615 - 0	- - -
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio		-	-	1000 0.002	1615 -	-

Intersection						
Int Delay, s/veh	0					
	EBL	EBR	NDI	NDT	CDT	CDD
Movement		EBK	NBL	NBT	SBT	SBR
Lane Configurations	¥	•	•	4	î,	•
Traffic Vol, veh/h	0	0	0	21	5	0
Future Vol, veh/h	0	0	0	21	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	5	20	0
Mvmt Flow	0	0	0	21	5	0
NA ' (NA)						
	linor2		//ajor1		/lajor2	
Conflicting Flow All	26	5	5	0	-	0
Stage 1	5	-	-	-	-	-
Stage 2	21	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	995	1084	1630	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1007	-	-	-	-	-
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	995	1084	1630	_	_	_
Mov Cap-2 Maneuver	995	-	-	_	<u>-</u>	_
Stage 1	1023		_	_	_	_
Stage 2	1023	_		_	_	_
Stage 2	1001	_	-	_	-	_
Annroach	EB		NB		SB	
Approach			0		0	
HCM Control Delay, s	0		U			
	0 A		U			
HCM Control Delay, s			0			
HCM Control Delay, s HCM LOS	Α	NDI		⊏DI ∞1	CDT	CDD
HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	Α	NBL		EBLn1	SBT	SBR
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h)	Α	1630	NBT I	-	-	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	Α	1630		-	SBT - -	SBR -
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	Α	1630 - 0	NBT I	- - 0	-	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	Α	1630	NBT   - -	-	- -	-

Intersection						
Int Delay, s/veh	5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		40	4	<b>₽</b>	
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	25	33	0	0	0	33
Mvmt Flow	4	6	16	11	3	1
	•		. •	• •		•
		_				
	Minor2		//ajor1		/lajor2	
Conflicting Flow All	47	4	4	0	-	0
Stage 1	4	-	-	-	-	-
Stage 2	43	-	-	-	-	-
Critical Hdwy	6.65	6.53	4.1	-	-	-
Critical Hdwy Stg 1	5.65	-	-	-	-	-
Critical Hdwy Stg 2	5.65	_	-	-	-	-
Follow-up Hdwy	3.725	3.597	2.2	-	_	-
Pot Cap-1 Maneuver	908	996	1631	-	-	-
Stage 1	962	-	_	_	_	_
Stage 2	924	_	_	_	_	_
Platoon blocked, %	JZT			_	_	<u>-</u>
Mov Cap-1 Maneuver	899	996	1631	<del>-</del>	_	<del>-</del>
	899	330	1031	-	_	-
Mov Cap-2 Maneuver		-	-	-		-
	$\Delta \Gamma \Delta$					
Stage 1	952	-	-	-	-	_
Stage 1 Stage 2	952 924	-	- -	-	-	-
_		-		-		-
Stage 2	924	-	-	-	-	-
Stage 2 Approach	924 EB	-	- NB	-	SB	
Stage 2  Approach HCM Control Delay, s	924 EB 8.8	-	-	-	-	
Stage 2 Approach	924 EB	-	- NB	-	SB	-
Stage 2  Approach HCM Control Delay, s HCM LOS	924 EB 8.8 A		NB 4.3	-	SB 0	
Stage 2  Approach HCM Control Delay, s	924 EB 8.8 A	- - NBL	NB 4.3	EBLn1	SB	SBR
Stage 2  Approach HCM Control Delay, s HCM LOS	924 EB 8.8 A		NB 4.3		SB 0	SBR -
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm	924 EB 8.8 A	NBL	NB 4.3		SB 0	
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h)	924 EB 8.8 A	NBL 1631	NB 4.3	955	SB 0	-
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	924 EB 8.8 A	NBL 1631 0.01 7.2	NB 4.3  NBT I	955 0.01 8.8	SB 0	-
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	924  EB  8.8  A	NBL 1631 0.01	NB 4.3	955 0.01	SB 0	- - -

| Intersection   Int Delay, s/veh   3.8   SBT   SBR   |---|
| Lane Configurations   |
| Lane Configurations   |
| Traffic Vol, veh/h   29   |
| Future Vol, veh/h         29         41         1         1         8         4         1         1         0         4         2         11           Conflicting Peds, #/hr         0   |
| Conflicting Peds, #/hr  |
| Sign Control         Free RTOR         Free RTOR         Free RTOR         Free RTOR         Free RTOR         Free RTOR         Stop None         Stop None         Stop None         Stop RT Channelized         - None  |
| RT Channelized  |
| Veh in Median Storage, #         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         0         -         0         0         -         0  |
| Grade, %         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         0         100         100         100         100         100         100         100         100         100         100         100         100         4         2         11           Major/Minor         Major1         Major2         Minor1         Minor1         Minor2         Minor2         Minor2         Minor2         Minor2         Minor3         Minor3         Minor4   |
| Peak Hour Factor  |
| Heavy Vehicles, %   |
| Mymt Flow         29         41         1         1         8         4         1         1         0         4         2         11           Major/Minor         Major1         Major2         Minor1         Minor2           Conflicting Flow All         12         0         0         42         0         0         119         114         42         112         112         10           Stage 1         -         -         -         -         -         100         100         -         12         12         -           Stage 2         -         -         -         -         19         14         -         100         100         -         12         12         -         -         -         -         100         100         -         12         12         -   |
| Major/Minor         Major1         Major2         Minor1         Minor2           Conflicting Flow All         12         0         0         42         0         0         119         114         42         112         112         10           Stage 1         -         -         -         -         -         100         100         -         12         12         -           Stage 2         -         -         -         -         19         14         -         100         100         -         12         12         -           Critical Hdwy         4.21         -         4.1         -         -         7.1         6.5         6.2         7.35         6.5         6.2           Critical Hdwy Stg 1         -         -         -         -         6.1         5.5         -         6.35         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.1         5.5         -         6.35         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.1         5.5         -         6.35         5.5         -   |
| Conflicting Flow All 12 0 0 42 0 0 119 114 42 112 112 10 Stage 1 100 100 - 12 12 - Stage 2 19 14 - 100 100 - Critical Hdwy 4.21 4.1 7.1 6.5 6.2 7.35 6.5 6.2 Critical Hdwy Stg 1 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 861 780 1034 814 782 1077 Stage 1 911 816 - 952 890 - Stage 2 1005 888 - 853 816 - Platoon blocked, % 1005 888 - 853 816 - Platoon blocked, % 838 764 1034 801 766 1077 Mov Cap-1 Maneuver 1550 1580 838 764 1034 801 766 1077 Mov Cap-2 Maneuver 894 800 - 934 889 - Stage 1 894 800 - 934 889 - Stage 2 894 800 - 934 889 - Stage 2 992 887 - 836 800 Approach EB WB NB SB HCM Control Delay, \$ 3   |
| Conflicting Flow All 12 0 0 42 0 0 119 114 42 112 112 10 Stage 1 100 100 - 12 12 - Stage 2 19 14 - 100 100 - Critical Hdwy 4.21 4.1 7.1 6.5 6.2 7.35 6.5 6.2 Critical Hdwy Stg 1 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 861 780 1034 814 782 1077 Stage 1 911 816 - 952 890 - Stage 2 911 816 - 952 890 - Stage 2 1005 888 - 853 816 - Platoon blocked, % 1005 888 - 853 816 - Platoon blocked, % 838 764 1034 801 766 1077 Mov Cap-1 Maneuver 1550 1580 838 764 1034 801 766 1077 Mov Cap-2 Maneuver 894 800 - 934 889 - Stage 1 894 800 - 934 889 - Stage 2 894 800 - 934 889 - Stage 2 894 800 - 934 889 - Stage 2 894 800 - 934 889 - Stage 2 894 800 - 934 889 - Stage 2  |
| Conflicting Flow All 12 0 0 42 0 0 1119 114 42 112 112 10 Stage 1 100 100 - 12 12 - Stage 2 19 14 - 100 100 - Critical Hdwy 4.21 - 4.1 7.1 6.5 6.2 7.35 6.5 6.2 Critical Hdwy Stg 1 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2 6.1 5.5 - 6.35 5.5 - Critical Hdwy Stg 2   |
| Stage 1       -       -       -       -       100       100       -       12       12       -         Stage 2       -       -       -       -       -       19       14       -       100       100       -         Critical Hdwy       4.21       -       -       4.1       -       -       7.1       6.5       6.2       7.35       6.5       6.2         Critical Hdwy       Stg 1       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy       Stg 2       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy       Stg 2       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy       Stg 2       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy       Stg 2       -       -       2.2       -       3.5       4       3.3       3.725       4       3.3         Poll Cand Hdwy       150       -       1580       <   |
| Critical Hdwy       4.21       -       4.1       -       7.1       6.5       6.2       7.35       6.5       6.2         Critical Hdwy Stg 1       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy Stg 2       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Follow-up Hdwy       2.299       -       -       2.2       -       -       6.1       5.5       -       6.35       5.5       -         Follow-up Hdwy       2.299       -       -       2.2       -       -       3.5       4       3.3       3.725       4       3.3         Pot Cap-1 Maneuver       1550       -       1580       -       861       780       1034       814       782       1077         Mov Cap-1 Maneuver       1550       -       1580       -       838       764       1034       801       766       1077         Mov Cap-2 Maneuver       -       -       -       -       838       764       -       801       766       -         Stage 1       -       -       -<  |
| Critical Hdwy Stg 1       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Critical Hdwy Stg 2       -       -       -       -       6.1       5.5       -       6.35       5.5       -         Follow-up Hdwy       2.299       -       -       2.2       -       -       3.5       4       3.3       3.725       4       3.3         Pot Cap-1 Maneuver       1550       -       -       1580       -       861       780       1034       814       782       1077         Stage 2       -       -       -       -       -       -       -       952       890       -         Stage 2       - <td< td=""></td<>   |
| Critical Hdwy Stg 2       -       -       -       6.1       5.5       -       6.35       5.5       -         Follow-up Hdwy       2.299       -       -       2.2       -       3.5       4       3.3       3.725       4       3.3         Pot Cap-1 Maneuver       1550       -       -       1580       -       861       780       1034       814       782       1077         Stage 1       -       -       -       -       -       911       816       -       952       890       -         Stage 2       -       -       -       -       -       1005       888       -       853       816       -         Platoon blocked, %       -  |
| Follow-up Hdwy 2.299 - 2.2 - 3.5 4 3.3 3.725 4 3.3  Pot Cap-1 Maneuver 1550 - 1580 - 861 780 1034 814 782 1077  Stage 1 911 816 - 952 890 - 1005 888 - 853 816 - 9100 888 - 9100     |
| Pot Cap-1 Maneuver         1550         -         -         1580         -         -         861         780         1034         814         782         1077           Stage 1         -         -         -         -         -         911         816         -         952         890         -           Stage 2         -         -         -         -         -         1005         888         -         853         816         -           Platoon blocked, %         -<   |
| Stage 1       -       -       -       -       952       890       -         Stage 2       -       -       -       -       1005       888       -       853       816       -         Platoon blocked, %       -       <   |
| Stage 2       -       -       -       -       1005       888       -       853       816       -         Platoon blocked, %       -       <   |
| Platoon blocked, %       -       -       -       -         Mov Cap-1 Maneuver 1550       -       1580       -       -       838 764 1034 801 766 1077         Mov Cap-2 Maneuver -       -       -       -       -       838 764 -       801 766 -         Stage 1 -       -       -       -       -       894 800 -       934 889 -         Stage 2 -       -       -       -       -       992 887 -       836 800 -    Approach EB WB NB SB HCM Control Delay, s 3 0.6 9.5 8.8 HCM LOS A A   |
| Mov Cap-1 Maneuver         1550         -         -         1580         -         -         838         764         1034         801         766         1077           Mov Cap-2 Maneuver         -         -         -         -         -         838         764         -         801         766         -           Stage 1         -         -         -         -         -         894         800         -         934         889         -           Stage 2         -         -         -         -         -         992         887         -         836         800         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         3         0.6         9.5         8.8           HCM LOS         A         A         A   |
| Mov Cap-2 Maneuver       -       -       -       -       838       764       -       801       766       -         Stage 1       -       -       -       -       894       800       -       934       889       -         Stage 2       -       -       -       -       -       992       887       -       836       800       -         Approach       EB       WB       NB       SB         HCM Control Delay, s       3       0.6       9.5       8.8         HCM LOS       A       A  |
| Stage 1         -         -         -         -         894         800         -         934         889         -           Stage 2         -         -         -         -         -         992         887         -         836         800         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         3         0.6         9.5         8.8           HCM LOS         A         A         A   |
| Stage 2         -         -         -         -         992         887         -         836         800         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         3         0.6         9.5         8.8           HCM LOS         A         A         A   |
| Approach EB WB NB SB HCM Control Delay, s 3 0.6 9.5 8.8 HCM LOS A A   |
| HCM Control Delay, s         3         0.6         9.5         8.8           HCM LOS         A         A         A  |
| HCM Control Delay, s         3         0.6         9.5         8.8           HCM LOS         A         A         A  |
| HCM LOS A A   |
|   |
| Mind and Mind Alpha Ent. Ent. Ent. Web Web Web Alpha A  |
| Ministry Marin Marin Applied EDI EDD AND AND AND AND AND AND AND AND AND A  |
| Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1   |
| Capacity (veh/h) 799 1550 1580 954  |
| HCM Lane V/C Ratio 0.003 0.019 0.001 0.018  |
| HCM Control Delay (s) 9.5 7.4 0 - 7.3 0 - 8.8   |
| HCM Lane LOS A A A - A A - A  |
| HCM 95th %tile Q(veh) 0 0.1 0 0.1   |

Intersection						
Int Delay, s/veh	3.9					
	WDL	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	.10	ĵ»	^	-,	<del>ન</del>
Traffic Vol, veh/h	5	48	74	8	54	36
Future Vol, veh/h	5	48	74	8	54	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	5	48	74	8	54	36
Miller 1011					0.	00
Major/Minor N	/linor1	N	//ajor1		Major2	
Conflicting Flow All	222	78	0	0	82	0
Stage 1	78	-	-	-	-	-
Stage 2	144	-	-	-	-	-
Critical Hdwy	6.6	6.2	_	-	4.12	_
Critical Hdwy Stg 1	5.6	-	_	_	_	_
Critical Hdwy Stg 2	5.6	_	_	_	_	_
Follow-up Hdwy	3.68	3.3	_	_	2.218	_
Pot Cap-1 Maneuver	728	988	_	_	1515	_
Stage 1	901	-	_	_	1313	_
				_	_	
Stage 2	841	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	702	988	-	-	1515	-
Mov Cap-2 Maneuver	702	-	-	-	-	-
Stage 1	901	-	-	-	-	-
Stage 2	811	-	-	-	-	-
Annyonah	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		4.5	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NRRV	VBLn1	SBL	SBT
•		-	-	951	1515	-
Capacity (veh/h)						-
HCM Control Polov (a)		-	-	0.056		-
HCM Control Delay (s)		-	-	9	7.5	0
HCM Lane LOS		-	-	A	A	Α
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Movement	Intersection						
BBT   BBR   WBL   WBT   NBL   NBR		0					
Care   Configurations   Care			EDD	///DI	WDT	NIDI	NDD
Traffic Vol, veh/h Future Vol, veh/future Vol, veh/future Future Fu			EBK	WBL			NBK
Future Vol, veh/h Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Peds, #/hr O Conflicting Length O Conflicting Storage, # O Conflicting Flow All Conflicting Flow All Conflicting Flow All Conflicting Flow All Conflicting Howy			^	^			^
Conflicting Peds, #/hr							
Sign Control         Free         Free         Free         Free         Free         Stop         Stop           RT Channelized         -         None         -         None         -         None           Storage Length         -         -         -         0         -         -         0         -           Grade, %         0         -         -         0         0         -           Peak Hour Factor         100         100         100         100         100         100           Heavy Vehicles, %         2							
RT Channelized							
Storage Length   -							
Weh in Median Storage, #         0         -         -         0         0         -           Grade, %         0         -         -         0         0         -           Peak Hour Factor         100         100         100         100         100         100           Heavy Vehicles, %         2		-		-			None
Grade, %         0         -         -         0         0         -           Peak Hour Factor         100         100         100         100         100         100           Heavy Vehicles, %         2         3         3         3         0         0         0         0         0         0         2         2         2         2         2         2         2         2         2         2         2         2         3         5.18         3.518         3.518         3.518		-		-			-
Peak Hour Factor         100         0				-			-
Heavy Vehicles, %							
Mount Flow         4         0         0         3         0         0           Major/Minor         Major1         Major2         Minor1           Conflicting Flow All         0         0         4         0         7         4           Stage 1         -         -         -         4         -         -         4         -           Stage 2         -         -         -         -         -         4         -         -         -         -         4         -							
Major/Minor         Major1         Major2         Minor1           Conflicting Flow All         0         0         4         0         7         4           Stage 1         -         -         -         4         -         -         4         -         -         4         -         -         4         -         -         4         -         -         -         4         -         -         -         4         -         -         -         4         -         -         -         -         4         -         <	Heavy Vehicles, %		2			2	2
Conflicting Flow All	Mvmt Flow	4	0	0	3	0	0
Conflicting Flow All							
Conflicting Flow All	Majay/Minay Ma	-:1		Maia#0		Minard	
Stage 1       -       -       -       4       -         Stage 2       -       -       -       3       -         Critical Hdwy       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42       -         Critical Hdwy Stg 2       -       -       -       5.42       -         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       1618       -       1014       1080         Stage 1       -       -       -       1020       -         Platoon blocked, %       -       -       -       1019       -         Mov Cap-1 Maneuver       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       1014       -         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1019       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0       0         HCM							
Stage 2       -       -       -       3       -         Critical Hdwy       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42       -         Critical Hdwy Stg 2       -       -       -       5.42       -         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pollow-up Hdwy       -       -       1618       -       1014       1080         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       1014       -       -         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0       0			0				
Critical Hdwy       -       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42       -         Critical Hdwy Stg 2       -       -       -       5.42       -         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020       -         Platoon blocked, %       -       -       -       -       -       -       1020       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       -       -         Stage 1       -       -       -       1019       -       -       -       -       1014       -       -       -       -       1014       -	<u> </u>	-	-	-			-
Critical Hdwy Stg 1       -       -       -       5.42       -         Critical Hdwy Stg 2       -       -       -       5.42       -         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pol Cap-1 Maneuver       -       1618       -       1014       1080         Stage 1       -       -       -       1020       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       -       -         Stage 1       -       -       -       1019       -       -         Stage 2       -       -       -       1020       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0         HCM Lane/Major Mvmt       NBLn1       EB       WB       WB         Capacity (veh/h)       -       -       -       -       -         HCM Control Delay (s)       0       -       - <t< td=""><td></td><td>-</td><td>-</td><td></td><td>-</td><td></td><td></td></t<>		-	-		-		
Critical Hdwy Stg 2       -       -       -       5.42       -         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1014       1080         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Platoon blocked, %       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       -       -       -       1014       -       -       -       1014       -       -       -       1014       -       -       -       1014       -       -       -       -       1014       -       -       -       -       1014       -       -       -       -       1019       -       -       -       -       1019       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -		-	-	4.12	-		6.22
Follow-up Hdwy - 2.218 - 3.518 3.318  Pot Cap-1 Maneuver - 1618 - 1014 1080  Stage 1 1019 -  Stage 2 1020 -  Platoon blocked, %  Mov Cap-1 Maneuver - 1618 - 1014 1080  Mov Cap-2 Maneuver - 1618 - 1014 1080  Mov Cap-2 Maneuver 1618 - 1014 -  Stage 1 1019 -  Stage 2 1020 -  Approach EB WB NB  HCM Control Delay, s 0 0 0  HCM LOS A  Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT  Capacity (veh/h) 1618 -  HCM Lane V/C Ratio		-	-	-	-		-
Pot Cap-1 Maneuver		-	-		-		
Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Platoon blocked, %       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       -         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0         HCM LOS       A       -       -       1618       -         Minor Lane/Major Mvmt       NBLn1       EBT       EBR       WBL       WBT         Capacity (veh/h)       -       -       -       -       -       -         HCM Lane V/C Ratio       -       -       -       -       -       -       -         HCM Control Delay (s)       0       -       -       0       -       -       -       -       -       -       -       -       -		-	-		-		
Stage 2       -       -       -       1020       -         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       -         Stage 1       -       -       -       1019       -         Stage 2       -       -       -       1020       -         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0       0         HCM LOS       A       -       -       1618       -         Minor Lane/Major Mvmt       NBLn1       EBR       WBL       WBT         Capacity (veh/h)       -       -       -       1618       -         HCM Lane V/C Ratio       -       -       -       -       -         HCM Control Delay (s)       0       -       -       0       -         HCM Lane LOS       A       -       -       A       -	Pot Cap-1 Maneuver	-	-	1618	-		1080
Platoon blocked, %         -         -         -           Mov Cap-1 Maneuver         -         1618         -         1014         1080           Mov Cap-2 Maneuver         -         -         -         1014         -           Stage 1         -         -         -         1019         -           Stage 2         -         -         -         1020         -           Approach         EB         WB         NB         NB           HCM Control Delay, s         0         0         0         0           HCM LOS         A         -         -         1618         -           Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         -         -         -           HCM Lane V/C Ratio         -         -         -         -         -         -           HCM Lane LOS         A         -         -         A         -	Stage 1	-	-	-	-		-
Mov Cap-1 Maneuver         -         -         1618         -         1014         1080           Mov Cap-2 Maneuver         -         -         -         1014         -           Stage 1         -         -         -         1019         -           Stage 2         -         -         -         1020         -    Approach  EB  WB  NB  HCM Control Delay, s  0 0 0 0 HCM LOS  A  Minor Lane/Major Mvmt  NBLn1  EBT  EBR  WBL  WBT  Capacity (veh/h)	Stage 2	-	-	-	-	1020	-
Mov Cap-2 Maneuver         -         -         -         1014         -           Stage 1         -         -         -         1019         -           Stage 2         -         -         -         1020         -   Approach           EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A         -         1618         -           Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618         -           HCM Lane V/C Ratio         -         -         -         -         -           HCM Control Delay (s)         0         -         -         0         -           HCM Lane LOS         A         -         -         A         -	Platoon blocked, %	-	-		-		
Mov Cap-2 Maneuver         -         -         -         1014         -           Stage 1         -         -         -         1019         -           Stage 2         -         -         -         1020         -   Approach           EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A         -         1618         -           Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618         -           HCM Lane V/C Ratio         -         -         -         -         -           HCM Control Delay (s)         0         -         -         0         -           HCM Lane LOS         A         -         -         A         -		-	-	1618	_	1014	1080
Stage 1         -         -         -         1019         -           Stage 2         -         -         -         1020         -           Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A    Minor Lane/Major Mvmt  NBLn1  EBT  EBR  WBL  WBT  Capacity (veh/h)  1618  - HCM Lane V/C Ratio		-	_		_		
Stage 2         -         -         -         -         1020         -           Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618         -           HCM Lane V/C Ratio         -         -         -         -         -           HCM Control Delay (s)         0         -         -         0         -           HCM Lane LOS         A         -         -         A         -		_	_	_	_		_
Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618         -           HCM Lane V/C Ratio         -         -         -         -         -           HCM Control Delay (s)         0         -         -         0         -           HCM Lane LOS         A         -         -         A         -		_	_	_	_		_
HCM Control Delay, s	Olago 2					1020	
HCM Control Delay, s							
Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618         -           HCM Lane V/C Ratio         -         -         -         -         -           HCM Control Delay (s)         0         -         -         0         -           HCM Lane LOS         A         -         -         A         -	Approach	EB					
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 1618 - HCM Lane V/C Ratio HCM Control Delay (s) 0 0 - HCM Lane LOS A - A -	HCM Control Delay, s	0		0		0	
Capacity (veh/h)       -       -       1618       -         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0       -         HCM Lane LOS       A       -       -       A       -	HCM LOS					Α	
Capacity (veh/h)       -       -       1618       -         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0       -         HCM Lane LOS       A       -       -       A       -							
Capacity (veh/h)       -       -       1618       -         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0       -         HCM Lane LOS       A       -       -       A       -	Minor Lane/Major Mymt	N	VRI n1	FRT	FRR	WRI	WRT
HCM Lane V/C Ratio       -			ADLIII	LDI	LDIX		VVDI
HCM Control Delay (s) 0 0 - HCM Lane LOS A - A -			-	-	-		-
HCM Lane LOS A A -							
			А	-	-		-
HCM 95th %tile Q(veh) 0 -	HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection						
Intersection Delay, s/veh	7.3					
Intersection LOS	Α					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	f)		¥	
Traffic Vol, veh/h	0	66	15	0	0	0
Future Vol, veh/h	0	66	15	0	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	66	15	0	0	0
Number of Lanes	0	1	1	0	1	0
Approach		EB	WB		SB	
Opposing Approach		WB	EB			
Opposing Lanes		1	1		0	
Conflicting Approach Left		SB			WB	
Conflicting Lanes Left		1	0		1	
Conflicting Approach Right			SB		EB	
Conflicting Lanes Right		0	1		1	
HCM Control Delay		7.3	7.1		0	
HCM LOS		Α	Α		-	
Lane	E	EBLn1	WBLn1	SBLn1		
Vol Left, %		201				
		0%	0%	0%		
Vol Thru, %						
Vol Thru, % Vol Right, %		0% 100% 0%	100% 0%	100%		
Vol Right, %		100%	100% 0%	100% 0%		
Vol Right, % Sign Control		100%	100% 0% Stop	100%		
Vol Right, %		100% 0% Stop 66	100% 0% Stop 15	100% 0% Stop 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol		100% 0% Stop	100% 0% Stop	100% 0% Stop		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		100% 0% Stop 66 0 66	100% 0% Stop 15	100% 0% Stop 0 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol		100% 0% Stop 66 0	100% 0% Stop 15 0	100% 0% Stop 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		100% 0% Stop 66 0 66	100% 0% Stop 15 0 15	100% 0% Stop 0 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		100% 0% Stop 66 0 66 0	100% 0% Stop 15 0 15	100% 0% Stop 0 0 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		100% 0% Stop 66 0 66 0 66	100% 0% Stop 15 0 15 15	100% 0% Stop 0 0 0 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		100% 0% Stop 66 0 66 0 66 1 0.072 3.945	100% 0% Stop 15 0 15 1 0 15 1 0.017	100% 0% Stop 0 0 0 0 1 0 4.074		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		100% 0% Stop 66 0 66 1 0.072 3.945 Yes	100% 0% Stop 15 0 15 15 0 15	100% 0% Stop 0 0 0 0 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% Stop 66 0 66 1 0.072 3.945 Yes 913	100% 0% Stop 15 0 15 1 0.017 3.983 Yes 902	100% 0% Stop 0 0 0 0 1 0 4.074 Yes 0		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		100% 0% Stop 66 0 66 1 0.072 3.945 Yes	100% 0% Stop 15 0 15 1 0.017 3.983 Yes	100% 0% Stop 0 0 0 0 0 1 0 4.074 Yes		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		100% 0% Stop 66 0 66 1 0.072 3.945 Yes 913 1.948	100% 0% Stop 15 0 15 1 0.017 3.983 Yes 902 1.994	100% 0% Stop 0 0 0 0 1 0 4.074 Yes 0 2.107		
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		100% 0% Stop 66 0 66 1 0.072 3.945 Yes 913 1.948 0.072	100% 0% Stop 15 0 15 1 0.017 3.983 Yes 902 1.994 0.017	100% 0% Stop 0 0 0 0 1 0 4.074 Yes 0 2.107		

Intersection Delay, s/veh 7	7
Intersection LOS A	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0	
Future Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0	
Mvmt Flow	1	0	0	0	0	3	0	1	1	3	2	0	
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0	
Approach	EB				WB			NB		SB			
Opposing Approach	WB				EB			SB		NB			
Opposing Lanes	1				1			1		1			
Conflicting Approach Le	ft SB				NB			EB		WB			
Conflicting Lanes Left	1				1			1		1			
Conflicting Approach Ri	gh <b>N</b> B				SB			WB		EB			
Conflicting Lanes Right	1				1			1		1			
HCM Control Delay	7.1				6.3			6.6		7.6			
HCM LOS	Α				Α			Α		Α			

Lane	NBLn1	EBLn1\	NBLn1	SBLn1
Vol Left, %	0%	100%	0%	60%
Vol Thru, %	50%	0%	0%	40%
Vol Right, %	50%	0%	100%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	2	1	3	5
LT Vol	0	1	0	3
Through Vol	1	0	0	2
RT Vol	1	0	3	0
Lane Flow Rate	2	1	3	5
Geometry Grp	1	1	1	1
Degree of Util (X)	0.002	0.001	0.003	0.006
Departure Headway (Hd)	3.611	4.114	3.313	4.591
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	996	874	1086	784
Service Time	1.613	2.118	1.316	2.591
HCM Lane V/C Ratio	0.002	0.001	0.003	0.006
HCM Control Delay	6.6	7.1	6.3	7.6
HCM Lane LOS	Α	Α	Α	Α
HCM 95th-tile Q	0	0	0	0

Intersection						
Int Delay, s/veh	4.3					
		EDD.	MAID	MOT	ND	NIDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		<u>ች</u>	<u></u>	Y	
Traffic Vol, veh/h	92	103	6	22	138	8
Future Vol, veh/h	92	103	6	22	138	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	18	0	18	8	0
Mvmt Flow	92	103	6	22	138	8
IVIVIIIL I IUW	JZ	100	U	ZZ	100	U
Major/Minor Ma	ajor1	N	/lajor2	ı	Minor1	
Conflicting Flow All	0	0	195	0	178	144
Stage 1	-	-	-	-	144	_
Stage 2	_	_	_	_	34	_
Critical Hdwy	_	_	4.1	_	6.48	6.2
Critical Hdwy Stg 1			7.1	_	5.48	0.2
	-	-	-		5.48	
Critical Hdwy Stg 2	-	-	-	-		-
Follow-up Hdwy	-	-	2.2	-	3.572	3.3
Pot Cap-1 Maneuver	-	-	1390	-	798	909
Stage 1	-	-	-	-	869	-
Stage 2	-	-	-	-	973	-
Platoon blocked, %	-			-		
Mov Cap-1 Maneuver	-	-	1390	-	795	909
Mov Cap-2 Maneuver	-	-	-	-	795	-
Stage 1	_	-	_	_	869	-
Stage 2	_	_	_	_	969	_
Jugo L					300	
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		10.5	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		801	-	-	1390	-
HCM Lane V/C Ratio		0.182	-	-	0.004	-
HCM Control Delay (s)		10.5	_	-	7.6	-
HCM Lane LOS		В	_	-	A	_
HCM 95th %tile Q(veh)		0.7	_	_	0	_
		J.1			J	

Intersection						
Int Delay, s/veh	2.7					
		EDD	MOL	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		<u>ነ</u>	<b>↑</b>	Y	
Traffic Vol, veh/h	144	202	5	155	84	51
Future Vol, veh/h	144	202	5	155	84	51
Conflicting Peds, #/hr	0	0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	9	0	10	31	4
Mvmt Flow	144	202	5	155	84	51
	ajor1		//ajor2		Minor1	
Conflicting Flow All	0	0	346	0	410	245
Stage 1	-	-	-	-	245	-
Stage 2	-	-	-	-	165	-
Critical Hdwy	-	-	4.1	-	6.71	6.24
Critical Hdwy Stg 1	-	-	-	-	5.71	-
Critical Hdwy Stg 2	-	-	-	-	5.71	-
Follow-up Hdwy	-	-	2.2	-	3.779	3.336
Pot Cap-1 Maneuver	-	-	1224	-	546	789
Stage 1	-	-	-	-	733	-
Stage 2	-	-	-	-	799	-
Platoon blocked, %	_	-		_		
Mov Cap-1 Maneuver	_	_	1224	_	544	789
Mov Cap-2 Maneuver	_	<u>-</u>	-	_	544	-
Stage 1	_	_	_	_	733	_
Stage 2	_	_	_	_	796	_
Glage Z	_	-	-	-	1 30	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		12.5	
HCM LOS					В	
Minan Lana/Maian Munat		UDL 4	EDT		WDI	WDT
Minor Lane/Major Mvmt	ſ	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		616	-	-	1224	-
HCM Lane V/C Ratio		0.219	-	-	0.004	-
HCM Control Delay (s)		12.5	-	-	8	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		8.0	-	-	0	-

Intersection												
Int Delay, s/veh	5.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	ĵ.		ች	ĵ.			सी	1		4	
Traffic Vol, veh/h	1	134	31	114	121	4	20	1	211	1	1	0
Future Vol, veh/h	1	134	31	114	121	4	20	1	211	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	134	31	114	121	4	20	1	211	1	1	0
Major/Minor N	/lajor1		ı	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	125	0	0	165	0	0	504	505	150	609	518	123
Stage 1	-	-	-	-	-	-	152	152	-	351	351	-
Stage 2	-	-	-	-	-	-	352	353	-	258	167	-
Critical Hdwy	4.1	-	-	4.4	-	-	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Follow-up Hdwy	2.2	-	-	2.47	-	-	3.635	4	3.39	3.5	4.9	3.3
Pot Cap-1 Maneuver	1474	-	-	1260	-	-	458	473	876	410	348	933
Stage 1	-	-	-	-	-	-	821	775	-	670	490	-
Stage 2	-	-	-		-	-	639	634	-	751	608	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1474	-	-	1260	-	-	425	430	876	289	316	933
Mov Cap-2 Maneuver	-	-	-	-	-	-	425	430	-	289	316	-
Stage 1	-	-	-	-	-	-	820	774	-	669	446	-
Stage 2	-	-	-	-	-	-	580	577	-	569	607	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			3.9			10.7			17		
HCM LOS							В			С		
							_					
Minor Lane/Major Mvm	t N	NBLn1 I	VBI n2	EBL	EBT	EBR	WBL	WBT	WBR S	SBI n1		
Capacity (veh/h)	<u> </u>	425	876	1474	-		1260	-	-			
HCM Lane V/C Ratio			0.241		_	_	0.09	_		0.007		
HCM Control Delay (s)		13.9	10.4	7.4	_	_	8.1	_	_	17		
HCM Lane LOS		В	В	Α.	_	_	Α	_	<u>-</u>	C		
HCM 95th %tile Q(veh)		0.2	0.9	0	_	_	0.3	_	_	0		
		V. <u>—</u>	3.0				3.0					

2025 Build Phase 1 LY 1 AM
Weekday Morning Peak Hour
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Intersection												
Int Delay, s/veh	5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	ĵ.		ሻ	f)	
Traffic Vol, veh/h	19	8	13	10	9	65	8	139	14	92	50	4
Future Vol, veh/h	19	8	13	10	9	65	8	139	14	92	50	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	19	8	13	10	9	65	8	139	14	92	50	4
Major/Minor M	linor2		N	Minor1		1	Major1		N	Major2		
Conflicting Flow All	435	405	52	409	400	146	54	0	0	153	0	0
Stage 1	236	236	-	162	162	-	-	-	-	-	-	-
Stage 2	199	169	-	247	238	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1	-	-	4.46	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	-	-	2.524	-	-
Pot Cap-1 Maneuver	535	538	999	556	541	848	1564	-	-	1244	-	-
Stage 1	772	713	-	845	768	-	-	-	-	-	-	-
Stage 2	807	763	-	761	712	-	-	-		-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	458	495	999	509	498	848	1564	-		1244	-	-
Mov Cap-2 Maneuver	458	495	-	509	498	-	-	-	-	-	-	-
Stage 1	768	660	-	841	764	-	-	-	-	-	-	-
Stage 2	733	759	-	687	659	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.8			10.5			0.4			5.1		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1564	-	-	566	734	1244	-				
HCM Lane V/C Ratio		0.005	_			0.114		<u>-</u>	<u>-</u>			
HCM Control Delay (s)		7.3	_	_	11.8	10.5	8.1	_	_			
HCM Lane LOS		Α.	_	<u>-</u>	В	В	Α	<u>-</u>	<u>-</u>			
HCM 95th %tile Q(veh)		0		_	0.2	0.4	0.2	_				
					J.L	J. 1	J.L					

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>1</b>			4
Traffic Vol, veh/h	2	0	26	0	0	224
Future Vol, veh/h	2	0	26	0	0	224
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	_	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage,		_	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	40	0	0	6
Mymt Flow	2	0	26	0	0	224
WWW.CT IOW	_	J	20	•		
				_		
	/linor1		Major1		Major2	
Conflicting Flow All	250	26	0	0	26	0
Stage 1	26	-	-	-	-	-
Stage 2	224	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	743	1056	-	-	1601	-
Stage 1	1002	-	-	-	-	-
Stage 2	818	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	743	1056	_	_	1601	_
Mov Cap-2 Maneuver	743	-	-	_	-	_
Stage 1	1002	-	-	-	_	-
Stage 2	818	_	_	_	_	_
Jugo 2	010					
Approach	WB		NB		SB	
HCM Control Delay, s	9.9		0		0	
HCM LOS	Α					
NA: 1 (NA : NA		NBT	NIPDV	VBLn1	SBL	SBT
Minor Lang/Major Misses		INDI				
Minor Lane/Major Mymt			-	743	1601	-
Capacity (veh/h)		-				
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.003	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		-	-	0.003 9.9	0	-
Capacity (veh/h) HCM Lane V/C Ratio			-	0.003		

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	40	0	0	183	0
Future Vol, veh/h	0	0	0	0	0	0	0	40	0	0	183	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	40	0	0	183	0
Major/Minor N	Minor2		ı	Minor1			Major1		N	Major2		
Conflicting Flow All	223	223	183	223	223	40	183	0	0	40	0	0
Stage 1	183	183		40	40	40	100	U	U	40		
	40	40	-	183	183	-	-	-	=	-	-	-
Stage 2			6.2			6.2	4.1	-	<del>-</del>	4.1	-	<del>-</del>
Critical Hdwy	7.1	6.5		7.1	6.5			-	-		-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	737	679	865	737	679	1037	1404	-	-	1583	-	-
Stage 1	823	752	-	980	866	-	-	-	-	-	-	-
Stage 2	980	866	-	823	752	-	-	-	-	-	-	-
Platoon blocked, %	707	070	005	707	070	4007	4404	-	-	4500	-	-
Mov Cap-1 Maneuver	737	679	865	737	679	1037	1404	-	-	1583	-	-
Mov Cap-2 Maneuver	737	679	-	737	679	-	-	-	-	-	-	-
Stage 1	823	752	-	980	866	-	-	-	-	-	-	-
Stage 2	980	866	-	823	752	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A								
	,,			,,								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1404				-	1583					
HCM Lane V/C Ratio		1404		<u> </u>	<u> </u>	-	1303	_	<u> </u>			
HCM Control Delay (s)		0	-	<u>-</u>	0	0	0	_				
HCM Lane LOS		A		_	A	A	A	-	<u>-</u>			
HCM 95th %tile Q(veh)	\	0	-				0					
How som whe Q(ven)	)	U	-	-	-	-	U	-	-			

Intersection												
Int Delay, s/veh	2.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	4	0	34	0	9	0	23	9	38	153	0
Future Vol, veh/h	0	4	0	34	0	9	0	23	9	38	153	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	_	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	6	0	44	0	39	33	16	13	0
Mvmt Flow	0	4	0	34	0	9	0	23	9	38	153	0
Major/Minor N	/linor2			Minor1			Major1			Major2		
Conflicting Flow All	261	261	153	259	257	28	153	0	0	32	0	0
Stage 1	229	229	-	28	28	-	-	-	-	-	-	-
Stage 2	32	32	-	231	229	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.16	6.5	6.64	4.1	-	-	4.26	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3		4	3.696	2.2	-	-	2.344	-	-
Pot Cap-1 Maneuver	696	647	898	686	651	938	1440	-	-	1494	-	-
Stage 1	778	718	-	979	876	-	-	-	-	-	-	-
Stage 2	990	872	-	763	718	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	674	629	898	668	633	938	1440	-	-	1494	-	-
Mov Cap-2 Maneuver	674	629	-	668	633	-	-	-	-	-	-	-
Stage 1	778	698	-	979	876	-	-	-	-	-	-	-
Stage 2	981	872	-	737	698	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.8			10.4			0			1.5		
HCM LOS	В			В								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1440	_	-	629	711	1494	-	-			
HCM Lane V/C Ratio		-	-	-	0.006		0.025	-	-			
HCM Control Delay (s)		0	-	-	10.8	10.4	7.5	0	-			
HCM Lane LOS		A	-	-	В	В	A	A	-			
HCM 95th %tile Q(veh)		0	-	-	0	0.2	0.1	-	-			

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	LDI	NUL	4	\$	ODIN
Traffic Vol, veh/h	0	0	1	13	44	7
Future Vol, veh/h	0	0	1	13	44	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
	0	None -	-		_	None
Storage Length Veh in Median Storage			-	0	0	
		-	-			-
Grade, %	0	400	400	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	13	44	7
Major/Minor N	Minor2	N	Major1	N	/lajor2	
Conflicting Flow All	63	48	51	0	-	0
Stage 1	48	_	_	_	_	-
Stage 2	15	_	-	_	_	_
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-		_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	948	1027	1568	_	_	
Stage 1	980	1021	1500	_	_	_
Stage 2	1013	-	-	_	_	_
Platoon blocked, %	1013	_	_	_	_	_
	047	1007	1568	-	-	_
Mov Cap-1 Maneuver	947	1027	1000	-		-
Mov Cap-2 Maneuver	947	-	-	-	-	-
Stage 1	979	-	-	-	-	-
Stage 2	1013	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.5		0	
HCM LOS	A		0.0			
	, ,					
					25-	
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1568	-	-	-	-
HCM Lane V/C Ratio		0.001	-	-	-	-
HCM Control Delay (s)		7.3	0	0	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	-	-	-
now your wille Q(ven)		U	-	-	-	-

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Future Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	<u> </u>	<u>-</u>	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	_	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow	1	0	0	0	0	0	0	13	0	1	43	0
Major/Minor N	Minor2		N	Minor1			Major1		N	/lajor2		
Conflicting Flow All	58	58	43	58	58	13	43	0	0	13	0	0
Stage 1	45	45	-	13	13	-	-	-	-	-	-	-
Stage 2	13	13	-	45	45	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	944	837	1033	944	837	1073	1579	-	-	1619	-	-
Stage 1	974	861	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	974	861	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	943	836	1033	943	836	1073	1579	-	-	1619	-	-
Mov Cap-2 Maneuver	943	836	-	943	836	-	-	-	-	-	-	-
Stage 1	974	860	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	973	860	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.8			0			0			0.2		
HCM LOS	Α			Α								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1579	-	-	0.0		1619	-	-			
HCM Lane V/C Ratio		-	-	-	0.001		0.001	-	-			
HCM Control Delay (s)		0	-	-	8.8	0	7.2	0	-			
HCM Lane LOS		Α	-	-	Α	Α	Α	Α	-			
HCM 95th %tile Q(veh)		0	-	-	0	-	0	-	-			

Intersection						
Int Delay, s/veh	5.5					
		WDD	NDT	NDD	CDI	CDT
Movement Configurations	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<b>\</b>	04	<b>f</b>	2	C	4
Traffic Vol, veh/h	35	21	9	3	6	23
Future Vol, veh/h	35	21	9	3	6	23
Conflicting Peds, #/hr	0	0	0	_ 0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	3	0	22	0	22	0
Mvmt Flow	35	21	9	3	6	23
Maiau/Minau	N 4: 4		1-:1		M-:0	
	Minor1		//ajor1		Major2	
Conflicting Flow All	46	11	0	0	12	0
Stage 1	11	-	-	-	-	-
Stage 2	35	-	-	-	-	-
Critical Hdwy	6.43	6.2	-	-	4.32	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	-	-	2.398	-
Pot Cap-1 Maneuver	962	1076	-	-	1486	-
Stage 1	1009	-	-	-	-	-
Stage 2	985	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	958	1076	_	-	1486	-
Mov Cap-2 Maneuver	958	_	-	_	_	_
Stage 1	1009	_	_	_	_	_
Stage 2	981	_	_	_	_	_
Olage 2	301					
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		1.5	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-	999	1486	-
HCM Lane V/C Ratio		_			0.004	_
HCM Control Delay (s)		_	-	8.8	7.4	0
HCM Lane LOS			<u> </u>	0.0 A	7.4 A	A
	١	-		0.2	0	
HCM 95th %tile Q(veh	)	-	-	0.2	U	-

Intersection Int Delay, s/veh  2.7  Movement  EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
MOVEMENT EDL EDT EDR WELL WELL WELL INDT INDR SBL SBT SBR
Lane Configurations & d 7 7 4
Lane Configurations
Future Vol, veh/h 12 8 0 1 97 137 0 1 0 53 0 30
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0
Sign Control Free Free Free Free Free Stop Stop Stop Stop Stop
RT Channelized None None None
Storage Length 200
Veh in Median Storage, # - 0 0 0 0
Grade, % - 0 0 0 0
Peak Hour Factor 100 100 100 100 100 100 100 100 100 10
Heavy Vehicles, % 58 0 0 0 8 10 0 0 19 0 10
Mvmt Flow 12 8 0 1 97 137 0 1 0 53 0 30
Major/Minor Major1 Major2 Minor1 Minor2
Conflicting Flow All 234 0 0 8 0 0 215 268 8 132 131 97
Stage 1 32 32 - 99 99 -
Stage 2 183 236 - 33 32 -
Critical Hdwy 4.68 4.1 7.1 6.5 6.2 7.29 6.5 6.3
Critical Hdwy Stg 1 6.1 5.5 - 6.29 5.5 -
Critical Hdwy Stg 2 6.1 5.5 - 6.29 5.5 -
Follow-up Hdwy 2.722 2.2 3.5 4 3.3 3.671 4 3.39
Pot Cap-1 Maneuver 1065 1625 746 641 1080 802 763 938
Stage 1 990 872 - 867 817 -
Stage 2 823 713 - 941 872 -
Platoon blocked, %
Mov Cap-1 Maneuver 1065 1625 715 633 1080 794 754 938
Mov Cap-2 Maneuver 715 633 - 794 754 -
Stage 1 979 862 - 857 816 -
Stage 2 796 712 - 930 862 -
Approach EB WB NB SB
HCM Control Delay, s 5.1 0 10.7 9.7
HCM LOS B A
<u>D</u> //
Miner Lene/Major Mumt NDL n4 FDL FDT FDD W/DL W/DT W/DD CDL n4
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1
Capacity (veh/h) 633 1065 1625 841
HCM Lane V/C Ratio 0.002 0.011 0.001 0.099
HCM Control Delay (s) 10.7 8.4 0 - 7.2 0 - 9.7
HCM Lane LOS B A A - A A - A
HCM 95th %tile Q(veh) 0 0 0 0.3

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ķ	÷		ř	ĵ.			4			4	
Traffic Vol, veh/h	1	36	1	11	342	31	4	0	9	70	2	7
Future Vol, veh/h	1	36	1	11	342	31	4	0	9	70	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	36	1	11	342	31	4	0	9	70	2	7
Major/Minor N	lajor1		ľ	Major2		ľ	Minor1		1	Minor2		
Conflicting Flow All	373	0	0	37	0	0	423	434	37	423	419	358
Stage 1	-	-	-	-	-	-	39	39	-	380	380	-
Stage 2	-	-	-	-	-	-	384	395	-	43	39	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4	3.3	3.581	4	3.561
Pot Cap-1 Maneuver	800	-	-	1587	-	-	545	518	1041	529	528	630
Stage 1	_	-	-	-	-	-	981	866	-	628	617	-
Stage 2	-	-	-	-	-	-	643	608	_	954	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	800	-	-	1587	-	-	534	514	1041	521	524	630
Mov Cap-2 Maneuver	-	-	-	-	-	-	534	514	-	521	524	-
Stage 1	-	_	-	-	-	-	980	865	-	627	613	-
Stage 2	_	-	-	-	-	-	629	604	-	945	865	-
0 / -											- 2 3	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			9.5			13		
HCM LOS							Α			В		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		806	800	-	-	1587	_	_	529			
HCM Lane V/C Ratio				-	-	0.007	-	-	0.149			
HCM Control Delay (s)		9.5	9.5	-	-	7.3	-	-	13			
HCM Lane LOS		A	A	_	-	A	-	-	В			
HCM 95th %tile Q(veh)		0	0	_	-	0	-	_	0.5			

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			4						4	
Traffic Vol, veh/h	0	10	87	8	302	0	0	0	0	2	2	98
Future Vol, veh/h	0	10	87	8	302	0	0	0	0	2	2	98
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	10	87	8	302	0	0	0	0	2	2	98
Major/Minor N	/lajor1		N	Major2					Λ	/linor2		
Conflicting Flow All	-	0	0	97	0	0				372	415	302
Stage 1	-	-	-	-	_	-				318	318	-
Stage 2	-	-	-	-	-	-				54	97	-
Critical Hdwy	-	-	-	4.85	-	-				6.4	7	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	-	_				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1142	-	0				633	461	710
Stage 1	0	-	-	-	-	0				742	576	-
Stage 2	0	-	-	-	-	0				974	730	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1142	-	-				628	0	710
Mov Cap-2 Maneuver	-	-	-	-	-	-				628	0	-
Stage 1	-	-	-	-	-	-				742	0	-
Stage 2	-	-	-	-	-	-				966	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						10.9		
HCM LOS										В		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)				1142	-							
HCM Lane V/C Ratio		<u>-</u>		0.007		0.144						
HCM Control Delay (s)					0	10.9						
HCM Lane LOS		<u>-</u>	<u>-</u>	Α	A	В						
HCM 95th %tile Q(veh)		_	_	0	-	0.5						
. 13111 0041 704110 ((1011)						0.0						

Intersection												
Int Delay, s/veh	10.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			ĵ.			4				
Traffic Vol, veh/h	9	3	0	0	4	1	306	5	23	0	0	0
Future Vol, veh/h	9	3	0	0	4	1	306	5	23	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	.# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	, -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	14	0	0	0	75	0	10	80	26	2	2	2
Mymt Flow	9	3	0	0	4	1	306	5	23	0	0	0
						•						
Major/Minor N	Major1		1	Major2		N	/linor1					
Conflicting Flow All	5	0	_	-	_	0	26	26	3			
Stage 1	-	-	-	_	-	-	21	21	-			
Stage 2	_	_	-	_	_	-	5	5	_			
Critical Hdwy	4.24	-	_	_	-	-	6.5	7.3	6.46			
Critical Hdwy Stg 1	-	_	-	_	_	_	5.5	6.3	-			
Critical Hdwy Stg 2	_	_	_	_	_	-	5.5	6.3	-			
Follow-up Hdwy	2.326	_	-	_	_	_	3.59		3.534			
Pot Cap-1 Maneuver	1541	_	0	0	_	-	969	736	1015			
Stage 1	-	_	0	0	_	_	981	745	-			
Stage 2	_	_	0	0	_	-	998	759	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1541	-	-	-	-	-	963	0	1015			
Mov Cap-2 Maneuver	-	-	-	-	-	-	963	0	-			
Stage 1	-	-	_	-	-	-	975	0	-			
Stage 2	-	-	-	-	-	-	998	0	-			
<b>3</b> -												
Approach	EB			WB			NB					
HCM Control Delay, s	5.5			0			10.7					
HCM LOS							В					
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		966	1541	-	-	-						
HCM Lane V/C Ratio		0.346		-	-	-						
HCM Control Delay (s)		10.7	7.3	0	-	-						
HCM Lane LOS		В	Α	Α	-	-						
HCM 95th %tile Q(veh)		1.6	0	-	-	-						

Movement
Lane Configurations         Image: Configuration of the confi
Traffic Vol, veh/h         0         5         2         5         11         0         0         0         10         6         3           Future Vol, veh/h         0         5         2         5         11         0         0         0         0         10         6         3           Conflicting Peds, #/hr         0
Traffic Vol, veh/h         0         5         2         5         11         0         0         0         10         6         3           Future Vol, veh/h         0         5         2         5         11         0         0         0         0         10         6         3           Conflicting Peds, #/hr         0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Sign Control Free Free Free Free Free Free Stop Stop Stop Stop Stop Stop Stop Stop
RT Channelized None None None
Storage Length
Veh in Median Storage, # - 0 0 0
Grade, % - 0 0 0 -
Peak Hour Factor 100 100 100 100 100 100 100 100 100 10
Heavy Vehicles, % 0 20 0 0 50 0 2 2 10 83 0
Mvmt Flow 0 5 2 5 11 0 0 0 10 6 3
Major/Minor Major1 Major2 Minor2
Conflicting Flow All - 0 0 7 0 0 27 28 11
Stage 1 21 21 -
Stage 2 6 7 -
Critical Hdwy 4.1 6.5 7.33 6.2
Critical Hdwy Stg 1 5.5 6.33 -
Critical Hdwy Stg 2 5.5 6.33 -
Follow-up Hdwy 2.2 3.59 4.747 3.3
Pot Cap-1 Maneuver 0 1627 - 0 968 730 1076
Stage 1 0 0 981 741 -
Stage 2 0 0 997 753 -
Platoon blocked, %
Mov Cap-1 Maneuver 1627 965 0 1076
Mov Cap-2 Maneuver 965 0 -
Stage 1 981 0 -
Stage 2 994 0 -
Approach EB WB SB
HCM Control Delay, s 0 2.3 8.7
HCM LOS A
TIOM LOC
W
Minor Lane/Major Mvmt EBT EBR WBL WBT SBLn1
Capacity (veh/h) 1627 - 989
HCM Lane V/C Ratio 0.003 - 0.019
HCM Control Delay (s) 7.2 0 8.7
HCM Lane LOS A A A
HCM 95th %tile Q(veh) 0 - 0.1

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ની			<del>(</del> î			4				
Traffic Vol, veh/h	0	15	0	0	16	4	0	5	70	0	0	0
Future Vol, veh/h	0	15	0	0	16	4	0	5	70	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2
Mvmt Flow	0	15	0	0	16	4	0	5	70	0	0	0
Major/Minor M	/lajor1		N	Major2		N	/linor1					
Conflicting Flow All	20	0	-		-	0	33	35	15			
Stage 1	-	-	-	-	-	-	15	15	-			
Stage 2	-	-	-	-	-	-	18	20	-			
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.3	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-			
Follow-up Hdwy	2.2	-	-	-	-	-	3.5	4.72	3.408			
Pot Cap-1 Maneuver	1609	-	0	0	-	-	986	727	1036			
Stage 1	-	-	0	0	-	-	1013	750	-			
Stage 2	-	-	0	0	-	-	1010	746	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1609	-	-	-	-	-	986	0	1036			
Mov Cap-2 Maneuver	-	-	-	-	-	-	986	0	-			
Stage 1	-	-	-	-	-	-	1013	0	-			
Stage 2	-	-	-	-	-	-	1010	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	0			0			8.7					
HCM LOS	•						Α					
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)	_	1036	1609	_	_	_						
HCM Lane V/C Ratio		0.072	-	_	_	_						
HCM Control Delay (s)		8.7	0	_	_	_						
HCM Lane LOS		A	A	_	_	_						
HCM 95th %tile Q(veh)		0.2	0	_	-	-						
, , , , , , , , , , , , , , , , , , ,												

Intersection												
Int Delay, s/veh	4.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	62	21	2	0	14	0	2	0	0	0	0	4
Future Vol, veh/h	62	21	2	0	14	0	2	0	0	0	0	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	13	0	0	21	0	0	0	0	0	0	50
Mvmt Flow	62	21	2	0	14	0	2	0	0	0	0	4
Major/Minor M	lajor1		ľ	Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	14	0	0	23	0	0	162	160	22	160	161	14
Stage 1	_	_	_	_	_	_	146	146		14	14	_
Stage 2	_	-	_	-	-	_	16	14	_	146	147	_
Critical Hdwy	4.1	-	_	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.7
Critical Hdwy Stg 1	_	_	_	-	_	_	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	_	_	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	_	2.2	-	_	3.5	4	3.3	3.5	4	3.75
	1617	_	-	1605	_	-	808	736	1061	810	735	942
Stage 1	_	_	_	-	_	-	861	780	_	1011	888	-
Stage 2	_	_	-	_	_	-	1009	888	-	861	779	_
Platoon blocked, %		_	_		_	-						
	1617	-	-	1605	-	-	781	707	1061	786	706	942
Mov Cap-2 Maneuver	-	-	-	-	-	-	781	707	-	786	706	-
Stage 1	-	-	-	-	-	-	827	750	-	972	888	-
Stage 2	-	-	-	-	-	-	1005	888	-	827	749	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	5.3			0			9.6			8.8		
HCM LOS	0.0			- 0			Α.			Α		
							, ,			,,		
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SRI n1			
Capacity (veh/h)		781	1617	LDI	LDIX -	1605	-	- VIDIV	942			
HCM Lane V/C Ratio		0.003		-	_	1003	-		0.004			
HCM Control Delay (s)		9.6	7.3	0	-	0	-	-	8.8			
HCM Lane LOS		9.6 A	7.3 A	A	-	A	-	-	0.0 A			
HCM 95th %tile Q(veh)		0	0.1	- -	_	0	-	-	0			
HOW JOHN JOHN Q(VEII)		U	0.1	<u>-</u>	_	U	_	_	U			

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**HCM Lane LOS** 

HCM 95th %tile Q(veh)

Intersection												
	0.8											
Int Delay, s/veh	0.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		सी			Þ			4				
Traffic Vol, veh/h	14	384	0	0	105	95	21	2	9	0	0	0
Future Vol, veh/h	14	384	0	0	105	95	21	2	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2
Mvmt Flow	14	384	0	0	105	95	21	2	9	0	0	0
Major/Minor	Major1		N	Major2		N	/linor1					
Conflicting Flow All	200	0	-		-	0	565	612	384			
Stage 1	-	-	-	-	-	-	412	412	-			
Stage 2	-	-	-	-	-	-	153	200	-			
Critical Hdwy	4.67	-	-	-	-	-	6.4	7.5	6.53			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.5	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.5	-			
Follow-up Hdwy	2.713	-	-	-	-	-	3.5	4.9	3.597			
Pot Cap-1 Maneuver	1103	-	0	0	-	-	490	303	601			
Stage 1	-	-	0	0	-	-	673	456	-			
Stage 2	-	-	0	0	-	-	880	585	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1103	-	-	-	-	-	482	0	601			
Mov Cap-2 Maneuver	-	-	-	-	-	-	482	0	-			
Stage 1	-	-	-	-	-	-	662	0	-			
Stage 2	-	-	-	-	-	-	880	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	0.3			0			12.5					
HCM LOS							В					
Minor Lane/Major Mvn	nt N	NBLn1	EBL	EBT	WBT	WBR						
	it I			LDI	WDI	WDR						
Capacity (veh/h)		512		-	-	-						
HCM Central Delay (a)	\	0.063		-	-	-						
HCM Control Delay (s)	)	12.5	8.3	0	-	-						

В

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HCM Lane LOS

HCM 95th %tile Q(veh)

Intersection												
Int Delay, s/veh	0.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	86	304	24	186	0	11	0	1	1	0	3
Future Vol, veh/h	3	86	304	24	186	0	11	0	1	1	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	38	50	0	21	0	0	0	0	0	0	67
Mvmt Flow	3	86	304	24	186	0	11	0	1	1	0	3
Major/Minor I	Major1		ľ	Major2		ľ	Minor1		N	/linor2		
Conflicting Flow All	186	0	0	390	0	0	480	478	238	479	630	186
Stage 1	-	-	-	-	-	-	244	244	-	234	234	-
Stage 2	-	-	-	-	-	-	236	234	-	245	396	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.87
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.903
Pot Cap-1 Maneuver	1222	-	-	1180	-	-	499	489	806	500	401	714
Stage 1	-	-	-	-	-	-	764	708	-	774	715	-
Stage 2	-	-	-	-	-	-	772	715	-	763	607	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1222	-	-	1180	-	-	487	476	806	490	391	714
Mov Cap-2 Maneuver	-	-	-	-	-	-	487	476	-	490	391	-
Stage 1	-	-	-	-	-	-	762	706	-	772	699	-
Stage 2	-	-	-	-	-	-	751	699	-	760	605	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.9			12.3			10.7		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)			1222	-		1180	-	-				
HCM Lane V/C Ratio		0.024		_	_	0.02	_		0.006			
HCM Control Delay (s)		12.3	8	0	_	8.1	0	_				
HCM Lane LOS		В	A	A	_	A	A	_	В			
HCM 95th %tile Q(veh)	)	0.1	0	-	_	0.1	-	_	0			
		<b>-</b>				<b>J</b> .,						

Intersection							
Int Delay, s/veh	5.9						
		ERT	WOT	WED	ODI	000	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	<u> </u>	<b>†</b>	<b>}</b>	45	<u>ነ</u>	420	
Traffic Vol, veh/h	7	19	49	15	11	136	
Future Vol, veh/h	7	19	49	15	11	136	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	100	None	-	None	100		
Storage Length	180	-	-	-	100	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	100	100	100	100	100	100	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	7	5	7 49	15	0 11	136	
Mvmt Flow	1	19	49	15	11	130	
Major/Minor	Major1	<u> </u>	Major2	N	Minor2		
Conflicting Flow All	64	0	-	0	90	57	
Stage 1	-	-	_	-	57	-	
Stage 2	-	-	-	-	33	-	
Critical Hdwy	4.1	_	-	_	6.4	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	_	-	-	5.4	-	
Follow-up Hdwy	2.2	-	-	-	3.5	3.318	
Pot Cap-1 Maneuver	1551	-	-	-	915	1009	
Stage 1	-	-	-	-	971	-	
Stage 2	-	-	-	-	995	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1551	-	-	-	910	1009	
Mov Cap-2 Maneuver	-	-	-	-	910	-	
Stage 1	-	-	-	-	966	-	
Stage 2	-	-	-	-	995	-	
Approach	EB		WB		SB		
			0		9.1		
HCM Control Delay, s HCM LOS	2		U				
I IOWI LOS					Α		
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)		1551	-	-	-	910	1009
HCM Lane V/C Ratio		0.005	-	-	-	0.012	
HCM Control Delay (s)		7.3	-	-	-	9	9.1
HCM Lane LOS		Α	-	-	-	Α	Α
HCM 95th %tile Q(veh	)	0	_	-	-	0	0.5
2 2 2 7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	,						

Int Delay, s/veh	Intersection												
Lane Configurations		4.1											
Lane Configurations	Movement	FRI	FRT	FRR	WRI	WRT	WRR	NRI	NRT	NRR	SRI	SRT	SBR
Traffic Vol, veh/h  15				LDIX			WDIX	INDL		HUIT	ODL		ODIT
Future Vol, veh/h  15 36 7 1 28 1 1 1 1 1 1 4 11 32  Conflicting Peds, #ihr  0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				7			1	1		1	4		32
Conflicting Peds, #/hr   Free   Stop   Sto				-	•			-	-	•			
Sign Control   Free   None   -   None   -   None   -   None   -   None				•					•				
RT Channelized													
Storage Length   100		-										•	
Veh in Median Storage, #         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         0         -         0 <td>Storage Length</td> <td>100</td> <td>-</td> <td>-</td> <td>150</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>•</td> <td>-</td> <td>-</td> <td>-</td>	Storage Length	100	-	-	150	-	-	-	-	•	-	-	-
Peak Hour Factor	Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, %	Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Mymt Flow         15         36         7         1         28         1         1         1         4         11         32           Major/Minor         Major1         Major2         Minor1         Minor2           Conflicting Flow All         29         0         0         43         0         0         122         101         40         101         104         29           Stage 1         -         -         -         -         -         -         70         70         -         31         31         -           Stage 2         -         -         -         -         52         31         -         70         73         -           Critical Hdwy Stg 1         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Critical Hdwy Stg 2         -	Peak Hour Factor	100		100			100	100			100	100	100
Major/Minor   Major1					100								
Conflicting Flow All   29   0   0   43   0   0   122   101   40   101   104   29	Mvmt Flow	15	36	7	1	28	1	1	1	1	4	11	32
Conflicting Flow All   29   0   0   43   0   0   122   101   40   101   104   29													
Conflicting Flow All   29   0   0   43   0   0   122   101   40   101   104   29	Major/Minor N	Major1		ľ	Major2		ľ	Minor1		N	/linor2		
Stage 1			0			0			101	40	101	104	29
Stage 2         -         -         -         -         52         31         -         70         73         -           Critical Hdwy         4.23         -         5.1         -         7.1         7.5         7.2         7.1         6.5         6.6         6.6         6.6         6.6         6.6         6.6         6.6         6.6         5.5         -         Critical Hdwy Stg 2         -         -         -         6.1         6.5         -         6.1         5.5         -         Critical Hdwy Stg 2         -         -         -         6.1         6.5         -         6.1         5.5         -         Critical Hdwy Stg 2         -         -         -         6.1         6.5         -         6.1         5.5         -         Critical Hdwy Stg 2         -         -         6.1         6.5         -         6.1         5.5         -         Critical Hdwy Stg 2         -         -         6.1         6.5         -         6.1         8.5         790         1007         Med 2         2.2         -         -         -         2.2         -         -         1007         8.2         -         -         9.2         -         -			-	-	-	-	-						
Critical Hdwy Stg 1         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Follow-up Hdwy         2.317         -         -         3.1         -         -         3.5         4.9         4.2         3.5         4         3.444           Pot Cap-1 Maneuver         1516         -         11113         -         -         858         637         810         885         790         1007           Stage 1         -         -         -         -         -         966         709         -         991         873         -           Stage 2         -         -         -         -         -         -         -         -         -         945         838         -           Platoon blocked, %         -         -         -         -         -         815         630         810         875         781         1007           Mov Cap-1 Maneuver         1516         -         -         -         - <td>•</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>52</td> <td>31</td> <td>-</td> <td>70</td> <td>73</td> <td>-</td>	•	-	-	-	-	-	-	52	31	-	70	73	-
Critical Hdwy Stg 2         -         -         -         -         6.1         6.5         -         6.1         5.5         -           Follow-up Hdwy         2.317         -         3.1         -         -         3.5         4.9         4.2         3.5         4         3.444           Pot Cap-1 Maneuver         1516         -         1113         -         -         858         637         810         885         790         1007           Stage 1         -         -         -         -         -         945         679         -         991         873         -           Stage 2         -         -         -         -         -         966         709         -         945         838         -           Platoon blocked, %         -         -         -         1113         -         -         815         630         810         875         781         1007           Mov Cap-1 Maneuver         1516         -         1113         -         -         815         630         810         875         781         1007           Mov Cap-2 Maneuver         -         -         -         -	Critical Hdwy	4.23	-	-	5.1	-	-			7.2			6.36
Follow-up Hdwy 2.317 3.1 3.5 4.9 4.2 3.5 4 3.444  Pot Cap-1 Maneuver 1516 1113 858 637 810 885 790 1007  Stage 1 945 679 - 991 873 -  Stage 2 966 709 - 945 838 -  Platoon blocked, % 966 709 - 945 838 -  Mov Cap-1 Maneuver 1516 - 1113 - 815 630 810 875 781 1007  Mov Cap-2 Maneuver 815 630 810 875 781 1007  Mov Cap-2 Maneuver 815 630 - 875 781 -  Stage 1 936 672 - 981 872 -  Stage 2 923 708 - 933 830 -  Approach EB WB NB SB  HCM Control Delay, s 1.9 0.3 8.4 9.1  HCM LOS A A A - 1113 - 932  HCM Lane V/C Ratio 0.003 0.01 - 0.001 - 0.005  HCM Control Delay (s) 8.4 7.4 - 8.2 - 9.1  HCM Lane LOS A A A - A		-	-	-	-	-	-			-			-
Pot Cap-1 Maneuver         1516         -         -         1113         -         858         637         810         885         790         1007           Stage 1         -         -         -         -         -         945         679         -         991         873         -           Stage 2         -         -         -         -         966         709         -         945         838         -           Plation blocked, %         -         -         -         -         -         -         -         -         945         838         -           Plation blocked, %         -<	, ,		-	-	-	-	-						
Stage 1         -         -         -         945         679         -         991         873         -           Stage 2         -         -         -         -         966         709         -         945         838         -           Platoon blocked, %         -<			-	-		-	-						
Stage 2         -         -         -         -         966         709         -         945         838         -           Platoon blocked, %         -         <	•	1516	-	-	1113	-	-						1007
Platoon blocked, %         -		-	-	-	-	-	-						-
Mov Cap-1 Maneuver         1516         -         -         1113         -         -         815         630         810         875         781         1007           Mov Cap-2 Maneuver         -         -         -         -         -         815         630         -         875         781         -           Stage 1         -         -         -         -         936         672         -         981         872         -           Stage 2         -         -         -         -         923         708         -         933         830         -           Approach         EB         WB         NB         SB         SB           HCM Control Delay, s         1.9         0.3         8.4         9.1           HCM Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Lane LOS         A		-	-	-	-			966	709	-	945	838	-
Mov Cap-2 Maneuver         -         -         -         -         -         815         630         -         875         781         -           Stage 1         -         -         -         -         -         936         672         -         981         872         -           Stage 2         -         -         -         -         923         708         -         933         830         -           Approach         EB         WB         NB         NB         SB           HCM Control Delay, s         1.9         0.3         8.4         9.1           HCM Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Control Delay (s)         8.4         7.4         -         -         8.2         -         -         9.1           HCM Lane LOS         A		1510	-	-	1110			0.45	000	040	075	704	4007
Stage 1         -         -         -         -         936         672         -         981         872         -           Stage 2         -         -         -         -         923         708         -         933         830         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         1.9         0.3         8.4         9.1           HCM LOS         A         A         A           Minor Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Control Delay (s)         8.4         7.4         -         -         8.2         -         -         9.1           HCM Lane LOS         A         A         -         -         A         -         -         A	•		-	-	1113								
Stage 2         -         -         -         -         923         708         -         933         830         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         1.9         0.3         8.4         9.1           HCM LOS         A         A         A           Minor Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Control Delay (s)         8.4         7.4         -         -         8.2         -         -         9.1           HCM Lane LOS         A         A         -         A         -         A         -         A			-	-	-								
Approach EB WB NB SB  HCM Control Delay, s 1.9 0.3 8.4 9.1  HCM LOS A A A  Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1  Capacity (veh/h) 1066 1516 - 1113 - 932  HCM Lane V/C Ratio 0.003 0.01 - 0.001 - 0.05  HCM Control Delay (s) 8.4 7.4 - 8.2 - 9.1  HCM Lane LOS A A - A - A		-	-	-	-	-	-						
HCM Control Delay, s   1.9   0.3   8.4   9.1     HCM LOS	Staye 2	<u>-</u>	-	-	-	-	-	923	708	-	933	030	-
HCM Control Delay, s   1.9   0.3   8.4   9.1     HCM LOS													
Minor Lane/Major Mvmt         NBLn1         EBL         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Control Delay (s)         8.4         7.4         -         -         8.2         -         -         9.1           HCM Lane LOS         A         A         -         -         A         -         -         A													
Minor Lane/Major Mvmt         NBLn1         EBL         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         1066         1516         -         -         1113         -         -         932           HCM Lane V/C Ratio         0.003         0.01         -         -         0.001         -         -         0.05           HCM Control Delay (s)         8.4         7.4         -         -         8.2         -         -         9.1           HCM Lane LOS         A         A         -         -         A         -         -         A		1.9			0.3								
Capacity (veh/h) 1066 1516 1113 932  HCM Lane V/C Ratio 0.003 0.01 0.001 0.05  HCM Control Delay (s) 8.4 7.4 8.2 9.1  HCM Lane LOS A A A - A	HCM LOS							Α			Α		
Capacity (veh/h) 1066 1516 1113 932  HCM Lane V/C Ratio 0.003 0.01 0.001 0.05  HCM Control Delay (s) 8.4 7.4 8.2 9.1  HCM Lane LOS A A A - A													
HCM Lane V/C Ratio       0.003       0.01       -       -       0.001       -       -       0.05         HCM Control Delay (s)       8.4       7.4       -       -       8.2       -       -       9.1         HCM Lane LOS       A       A       -       A       -       A       -       A	Minor Lane/Major Mvm	ıt N	NBL <sub>n1</sub>	EBL	EBT	EBR	WBL	WBT	WBR S	SBL <sub>n1</sub>			
HCM Lane V/C Ratio       0.003       0.01       -       -       0.001       -       -       0.05         HCM Control Delay (s)       8.4       7.4       -       -       8.2       -       -       9.1         HCM Lane LOS       A       A       -       A       -       A       -       A	Capacity (veh/h)		1066	1516	-	-	1113	-	-	932			
HCM Lane LOS A A A			0.003		-	-	0.001	-	-	0.05			
	HCM Control Delay (s)		8.4	7.4	-	-	8.2	-	-	9.1			
HCM 95th %tile Q(veh) 0 0 0 0.2					-	-		-	-				
	HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0.2			

Intersection						
Int Delay, s/veh	0.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽	LDIX	VVDL	<u>₩</u>	NDL W	אטוז
Traffic Vol, veh/h	37	3	8	28	0	0
Future Vol, veh/h	37	3	8	28	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	riee -	None	Stop -	None
Storage Length	<u>-</u>	None -	150	-	0	None -
			150			
Veh in Median Storage,		-		0	0	-
Grade, %	0	400	400	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	33	0	13	0	0
Mvmt Flow	37	3	8	28	0	0
Major/Minor M	lajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	40	0	83	39
Stage 1	_	_	-	_	39	_
Stage 2	_	_	_	_	44	_
Critical Hdwy	_	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	<u>-</u>	- '	_	5.4	-
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	_	<u>-</u>	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	_	_	1583	_	924	1038
•	-	-	1000	-	989	1030
Stage 1			-		984	
Stage 2	-	-	-	-	904	-
Platoon blocked, %	-	-	4500	-	040	4000
Mov Cap-1 Maneuver	-	-	1583	-	919	1038
Mov Cap-2 Maneuver	-	-	-	-	919	-
Stage 1	-	-	-	-	989	-
Stage 2	-	-	-	-	979	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		0	
HCM LOS	U		1.0		A	
TIOWI EOU					Α.	
Minor Lane/Major Mvmt	: 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-		1583	-
HCM Lane V/C Ratio		-	-	-	0.005	-
HCM Control Delay (s)		0	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection Int Delay, s/veh	0					
		MDD	NDT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	^	<b>₽</b>		^	4
Traffic Vol, veh/h	0	0	0	1	0	14
Future Vol, veh/h	0	0	0	1	0	14
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	14
Mvmt Flow	0	0	0	1	0	14
NA = : = = /NA:== = =	\ A: A		1-:1		A-:O	
	Minor1		/lajor1		/lajor2	
Conflicting Flow All	15	1	0	0	1	0
Stage 1	1	-	-	-	-	-
Stage 2	14	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	1009	1090	-	-	1635	-
Stage 1	1028	-	-	-	-	-
Stage 2	1014	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	1009	1090	-	-	1635	-
Mov Cap-2 Maneuver	1009	_	_	_	_	_
•						
Stage 1		-	-	-	_	_
Stage 1	1028	- -	-	-	-	- -
Stage 1 Stage 2			-	-		-
Stage 2	1028 1014			-	-	-
Stage 2 Approach	1028		NB	-		-
Stage 2  Approach HCM Control Delay, s	1028 1014 WB			-	-	-
Stage 2 Approach	1028 1014 WB		NB	-	SB	-
Stage 2  Approach HCM Control Delay, s	1028 1014 WB		NB	-	SB	-
Stage 2  Approach HCM Control Delay, s HCM LOS	1028 1014 WB 0 A		NB 0		SB 0	_
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm	1028 1014 WB 0 A		NB	VBLn1	SB 0	SBT
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h)	1028 1014 WB 0 A	NBT -	NB 0 NBRV	VBLn1	SB 0 SBL 1635	SBT -
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	1028 1014 WB 0 A	NBT	NB 0 NBRV	VBLn1 - -	SB 0 SBL 1635	SBT -
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	1028 1014 WB 0 A	NBT -	NBRV	VBLn1 - - 0	SBL 1635	SBT -
Stage 2  Approach HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	1028 1014 WB 0 A	NBT	NB 0 NBRV	VBLn1 - -	SB 0 SBL 1635	SBT -

Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	\$	
Traffic Vol. veh/h	1	0	0	0	18	0
Future Vol, veh/h	1	0	0	0	18	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage,		_	_	0	0	_
Grade, %	0	<u>-</u>	<u>-</u>	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	11	0
Mymt Flow	1	0	0	0	18	0
IVIVIIIL FIOW	I	U	U	U	10	U
Major/Minor N	/linor2		//ajor1		/lajor2	
Conflicting Flow All	18	18	18	0	-	0
Stage 1	18	-	-	-	-	-
Stage 2	0	-	_	-	-	-
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	_	-	_	_	-
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	1005	1066	1612			
Stage 1	1010	1000	1012	_	_	_
Stage 2	1010	-	<u>-</u>	-	_	<u>-</u>
	-	-	-	-	-	-
Platoon blocked, %	4005	1000	1010	-	-	-
Mov Cap-1 Maneuver	1005	1066	1612	-	-	-
Mov Cap-2 Maneuver	1005	-	-	-	-	-
Stage 1	1010	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
- ' '						
HCM LOS	8.6		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1612		1005		-
HCM Lane V/C Ratio		1012		0.001	_	_
HCM Control Delay (s)		0	_	8.6	_	
HCM Lane LOS		A	_			
HCM 95th %tile Q(veh)		0 0		A 0	-	-
H(\)\\\ ()\(\)\(\)\(\)\(\)\(\)\(\)\(\)\(\			_		_	

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		EDR	NDL			SDR
Traffic Vol., veh/h	<b>**</b>	10		र्स	<b>ન</b>	7
•	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	12	5	1	9	7
Major/Minor N	linor2	N	/lajor1	A	/lajor2	
						^
Conflicting Flow All	24	13	16	0	-	0
Stage 1	13	-	-	-	-	-
Stage 2	11	-	-	-	-	-
Critical Hdwy	6.4	6.28	4.5	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy		3.372	2.56	-	-	-
Pot Cap-1 Maneuver	997	1050	1386	-	-	-
Stage 1	1015	-	-	-	-	-
Stage 2	1017	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	993	1050	1386	-	-	-
Mov Cap-2 Maneuver	993	-	-	-	-	-
Stage 1	1011	-	-	-	-	-
Stage 2	1017	_	_	-	_	_
2.0.33 -						
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
TICIVI LOS						
TICIVI LOS						
		NIRI	NRT	ERI n1	CRT	SBD
Minor Lane/Major Mvmt	t	NBL		EBLn1	SBT	SBR
Minor Lane/Major Mvmt Capacity (veh/h)	t	1386	-	1041	-	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	1	1386 0.004	-	1041 0.013	-	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t .	1386 0.004 7.6	- - 0	1041 0.013 8.5	- - -	- - -
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	t .	1386 0.004	-	1041 0.013	-	-

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	6	16	1	0	27	2	2	2	1	1	0	30
Future Vol, veh/h	6	16	1	0	27	2	2	2	1	1	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	6	16	1	0	27	2	2	2	1	1	0	30
Major/Minor I	Major1		1	Major2		N	/linor1		N	/linor2		
Conflicting Flow All	29	0	0	17	0	0	72	58	17	58	57	28
Stage 1	-	-	-	-	-	-	29	29	-	28	28	-
Stage 2	-	-	-	-	-	-	43	29	-	30	29	-
Critical Hdwy	4.43	_	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	_	-	-	-	-	-	6.1	5.5	-	6.1	5.5	_
Critical Hdwy Stg 2	-	_	_	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5		3.516
Pot Cap-1 Maneuver	1405	_	-	1613	-	-	924	837	1068	944	838	987
Stage 1	-	-	-	_	-	-	993	875	-	994	876	-
Stage 2	-	_	_	-	-	-	976	875	-	992	875	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1405	-	-	1613	-	-	894	834	1068	938	835	987
Mov Cap-2 Maneuver	-	-	-	-	-	-	894	834	-	938	835	-
Stage 1	-	-	-	-	-	-	989	872	-	990	876	-
Stage 2	-	-	-	-	-	-	946	875	-	985	872	-
Ŭ												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2			0			9			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		897	1405	-	-	1613	-	-	985			
HCM Lane V/C Ratio		0.006	0.004	-	-	-	-	-	0.031			
HCM Control Delay (s)		9	7.6	0	-	0	-	-	8.8			
HCM Lane LOS		A	A	A	-	A	-	-	Α			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.1			

30 0 Free Fre - Non -	2 15 2 15 0 0 e Free e -	\$BT 42 42 42 0 Free
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30 30 0 Free Fre - Non -	2 15 2 15 0 0 e Free e -	42 42 42 0
30 30 0 Free Fre - Non -	2 15 0 0 e Free e -	42 42 0
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- 8.	A A	
	ajor1  0 NB 0	ajor1 Major2 0 0 32 4.22 2.308 1518 1518 1518 1518

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u></u>			4	¥	
Traffic Vol, veh/h	4	387	0	4	13	0
Future Vol, veh/h	4	387	0	4	13	0
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	_	-	-	-	0	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	387	0	4	13	0
WWIIICT IOW	-	001	U	7	10	U
	Major1		Major2	N	Minor1	
Conflicting Flow All	0	0	391	0	202	198
Stage 1	-	-	-	-	198	-
Stage 2	-	-	-	-	4	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1168	-	787	843
Stage 1	-	-	-	-	835	-
Stage 2	-	_	-	-	1019	_
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	1168	-	787	843
Mov Cap-2 Maneuver		_	_	_	787	_
Stage 1	_	_	_	_	835	_
Stage 2	_	_	_	_	1019	_
otago 2					1010	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		9.7	
HCM LOS					Α	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		787	-	-	1168	-
HCM Lane V/C Ratio		0.017	_	<u> </u>	1 100	<u>-</u>
HCM Control Delay (s	.)	9.7	_		0	<u>-</u>
HCM Lane LOS	7)	9.1 A	_	<u>-</u>	A	_
HCM 95th %tile Q(veh	n)	0.1	_		0	<u>-</u>
HOW JOHN JUHIE W(VEI	7	0.1	_		- 0	

Weekday	Mornina	Peak Hour
vvoonaay	wicining	i oak i loai

0					
EBL	EBT	WBT	WBR	SBL	SBR
0	9		0		0
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			-		0
					Stop
					None
_	-	_			-
ـ # ـ					_
-, π					<u>-</u>
100					100
					2
U	9	56	U	U	0
Maior1	N	Maior2	1	Minor2	
		-			56
		_			-
					_
		_			6.22
					0.22
					-
	-	-			
	-	-			1011
					-
-	-	-	-	1014	-
	-	-	-		
1549	-	-	-		1011
-	-	-	-	941	-
_	-	-	-	967	-
_	-	_	_		-
0		0			
				Α	
nt	FRI	FRT	WRT	WRP	SRI n1
IL		LDI	VVDI	VVDIX	ODLIII
		-	-	-	-
	-	-	-	-	-
)	0	-	-	-	0
)	A 0	-	-	-	Α
	BBL  0 0 7 Free - 100 2 0  Major1 56 - 4.12 - 2.218 1549 - 1549	BBL EBT  0 9 0 9 0 0 0 Free Free - None - 0 100 100 2 2 0 9  Major1 N 56 0 4.12 2.218 - 1549 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 1549 5 - 5 - 1549 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 -	EBL EBT WBT	EBL         EBT         WBT         WBR           0         9         56         0           0         9         56         0           0         0         0         0           Free         Free         Free         Free           -         None         -         None           -         -         0         0           -         -         0         0         -           100         100         100         100         2           2         2         2         2         2         2           0         9         56         0         0         0           Major1         Major2         I         I         0         0           Major1         Major2         I         I         0         0           Major1         Major2         I         I         0         0           Major2         I         I         I         0         0         0         0         0         0         0         0         I         0         0         0         0         0         0         0         0	EBL         EBT         WBT         WBR         SBL           0         9         56         0         0           0         9         56         0         0           0         0         0         0         0           Free         Free         Free         Stop           None         -         None         -           -         0         0         -         0           -         0         0         -         0           -         0         0         -         0         100           100         100         100         100         100         100         100           2

Intersection		
Intersection Delay, s/veh	11.4	
Intersection LOS	В	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	3	0	14	0	0	62	329	0	0
Future Vol, veh/h	0	0	0	3	0	14	0	0	62	329	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	3	0	14	0	0	62	329	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		9.3				7		12.3		
HCM LOS		-		Α				Α		В		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	18%	100%	
Vol Thru, %	0%	100%	0%	0%	
Vol Right, %	100%	0%	82%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	62	0	17	329	
LT Vol	0	0	3	329	
Through Vol	0	0	0	0	
RT Vol	62	0	14	0	
Lane Flow Rate	62	0	17	329	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.064	0	0.029	0.459	
Departure Headway (Hd)	3.738	4.931	6.149	5.027	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	963	0	585	717	
Service Time	1.742	2.936	4.153	3.062	
HCM Lane V/C Ratio	0.064	0	0.029	0.459	
HCM Control Delay	7	7.9	9.3	12.3	
HCM Lane LOS	Α	N	Α	В	
HCM 95th-tile Q	0.2	0	0.1	2.4	

Intersection						
Int Delay, s/veh	6.7					
	EBT	EBR	WBL	\\/DT	NBL	NBR
		EBK		WBT		NBK
Lane Configurations	<b>}</b>	177	<u>ች</u>	<b>↑</b>	760	1
Traffic Vol, veh/h	20	177	35	63	268	4
Future Vol, veh/h	20	177	35	63	268	4
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	20	177	35	63	268	4
Major/Minor NA	oior1	_N	Majora		Minor1	
	ajor1		Major2		Minor1	400
Conflicting Flow All	0	0	197	0	242	109
Stage 1	-	-	-	-	109	-
Stage 2	-	-		-	133	-
Critical Hdwy	-	-	4.16	-	6.45	6.7
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.254	-	3.545	3.75
Pot Cap-1 Maneuver	-	-	1352	-	740	829
Stage 1	-	-	-	-	908	-
Stage 2	-	-	-	-	886	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	_	1352	_	721	829
Mov Cap-2 Maneuver	_	_	-	_	721	-
Stage 1	_	_	_	_	908	_
Stage 2	_	_	_	_	863	_
Staye 2	-	-	_	_	003	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		2.8		13	
HCM LOS					В	
		IDI (			14/5-	14/5-
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		722	-		1352	-
HCM Lane V/C Ratio		0.377	-	-	0.026	-
HCM Control Delay (s)		13	-	-	7.7	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		1.8	-	-	0.1	-

I-82 Southbound Ramps & Wine Country Road	Weekday Afternoon Peak Hou

Intersection						
Int Delay, s/veh	2.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>		ሻ		¥	11511
Traffic Vol, veh/h	183	232	13	318	104	14
Future Vol, veh/h	183	232	13	318	104	14
Conflicting Peds, #/hr		232	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -		riee -	None	Stop -	None
	-	None -	150	None -	0	None -
Storage Length Veh in Median Storage						
		-	-	0	0	-
Grade, %	0	400	400	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	8	8	5	25	21
Mvmt Flow	183	232	13	318	104	14
Major/Minor	Major1	ı	Major2		Minor1	
Conflicting Flow All	0	0	415	0	643	299
Stage 1	-	-	-	-	299	233
Stage 2	_	_	_	_	344	_
Critical Hdwy	_		4.18	_	6.65	6.41
Critical Hdwy Stg 1	-		4.10	_	5.65	0.41
Critical Hdwy Stg 2	-	<u>-</u>		-	5.65	-
	-	-	2.272		3.725	
Follow-up Hdwy			1112			
Pot Cap-1 Maneuver	-	-		-	403	698
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	669	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver		-	1112	-	398	698
Mov Cap-2 Maneuver	-	-	-	-	398	-
Stage 1	-	-	-	-	703	-
Stage 2	-	-	-	-	661	-
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		0.3		16.9	
HCM LOS					С	
Minor Lane/Major Mvr	nt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		419	-	_	1112	
HCM Lane V/C Ratio		0.282	_	_	0.012	-
HCM Control Delay (s	()	16.9	_	_	8.3	_
HCM Lane LOS		C	_	-	Α	-
HCM 95th %tile Q(veh	n)	1.1	_		0	_
	'/	1.1			U	

Intersection												
Int Delay, s/veh	7.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	₽		*	ĵ.			र्स	7		4	
Traffic Vol, veh/h	0	179	48	230	187	5	84	2	231	5	0	2
Future Vol, veh/h	0	179	48	230	187	5	84	2	231	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	179	48	230	187	5	84	2	231	5	0	2
Major/Minor I	Major1			Major2			Minor1		N	Minor2		
Conflicting Flow All	192	0	0	227	0	0	854	855	203	970	877	190
Stage 1	-	_	_		_	_	203	203		650	650	-
Stage 2	_	_	-	-	_	_	651	652	_	320	227	_
Critical Hdwy	4.1	-	_	4.26	_	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	_	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1394	-	-	1263	-	-	267	298	792	235	289	857
Stage 1	-	-	-	-	-	-	774	737	-	461	468	-
Stage 2	-	-	-	-	-	-	440	467	-	696	720	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1394	-	-	1263	-	-	229	244	792	142	236	857
Mov Cap-2 Maneuver	-	-	-	-	-	-	229	244	-	142	236	-
Stage 1	-	-	-	-	-	-	774	737	-	461	383	-
Stage 2	-	-	-	-	-	-	359	382	-	492	720	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.6			16.4			25.1		
HCM LOS				1.0			С			D		
										_		
Minor Lang/Major Myss	<b>,</b> <del>+</del>	NBLn11	VIDI 22	EBL	EBT	EBR	WBL	WBT	WBR S	201 51		
Minor Lane/Major Mvm	IL							VVDI				
Capacity (veh/h)		229	792	1394	-		1263	-	-	100		
HCM Control Doloy (a)		0.376		-	-	-	0.182	-		0.038		
HCM Long LOS		29.9	11.4	0	-	-	8.5	-	-	25.1 D		
HCM Lane LOS HCM 95th %tile Q(veh)	\	D 1.7	1.2	A 0	-	-	A 0.7	-	- -	0.1		
HOW SOUT WHIE Q(VEI)		1.7	1.2	U	-	-	0.7	-	-	U. I		

Intersection												
Int Delay, s/veh	6.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	- ↑		ሻ	1	
Traffic Vol, veh/h	17	20	12	30	26	174	12	88	19	124	137	28
Future Vol, veh/h	17	20	12	30	26	174	12	88	19	124	137	28
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	6	0	0	7	0	16	0	15	32	29	5	0
Mvmt Flow	17	20	12	30	26	174	12	88	19	124	137	28
Major/Minor I	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	621	530	151	537	535	98	165	0	0	107	0	0
Stage 1	399	399	-	122	122	-	-	-	-	-	-	-
Stage 2	222	131	-	415	413	-	-	-	-	-	-	-
Critical Hdwy	7.16	6.5	6.2	7.17	6.5	6.36	4.1	-	_	4.39	-	-
Critical Hdwy Stg 1	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.554	4	3.3	3.563	4	3.444	2.2	-	-	2.461	-	-
Pot Cap-1 Maneuver	394	457	901	447	454	921	1426	-	-	1331	-	-
Stage 1	619	606	-	870	799	-	-	-	-	-	-	-
Stage 2	771	792	-	605	597	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	281	411	901	392	409	921	1426	-	-	1331	-	-
Mov Cap-2 Maneuver	281	411	-	392	409	-	-	-	-	-	-	-
Stage 1	614	550	-	863	793	-	-	-	-	-	-	-
Stage 2	600	786	-	522	541	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.3			12.7			0.8			3.4		
HCM LOS	С			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR I	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1426	-	-	400	699	1331		0211			
HCM Lane V/C Ratio		0.008	<u> </u>			0.329		_				
HCM Control Delay (s)		7.5		_	15.3	12.7	8	_	_			
HCM Lane LOS		Α.5	_	_	C	12.7 B	A	_	_			
HCM 95th %tile Q(veh	)	0	_	_	0.4	1.4	0.3	_	_			
TOW JOHN JOHN WING WING	1	- 0			J.7	1.7	0.0					

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>1</b>			4
Traffic Vol, veh/h	3	5	214	8	3	111
Future Vol, veh/h	3	5	214	8	3	111
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		_	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage,		_	0	_	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	20	19	0	0	34
Mymt Flow	3	5	214	8	3	111
WWW. C. LOW	- 3	- 0	<b>L</b> 17	- 0		111
	/linor1		Major1		Major2	
Conflicting Flow All	335	218	0	0	222	0
Stage 1	218	-	-	-	-	-
Stage 2	117	-	-	-	-	-
Critical Hdwy	6.4	6.4	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.48	-	-	2.2	-
Pot Cap-1 Maneuver	664	779	-	-	1359	-
Stage 1	823	-	-	-	-	-
Stage 2	913	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	663	779	_	-	1359	-
Mov Cap-2 Maneuver	663	-	-	_	-	_
Stage 1	823	-	-	-	-	-
Stage 2	911	_	_	_	_	_
J	911					
Approach	WB		NB		SB	
HCM Control Delay, s	10		0		0.2	
HCM LOS	В					
Minor Lane/Major Mvm	t	NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		וטוו			1359	
HCM Lane V/C Ratio		-	-	0.011		-
HCM Control Delay (s)		-		10	7.7	0
HCM Lane LOS		-	-	В	Α.	A
HCM 95th %tile Q(veh)		-		0	0	- A
How som while Q(ven)		-	-	U	U	-

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	191	0	0	101	0
Future Vol, veh/h	1	0	0	0	0	0	0	191	0	0	101	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	191	0	0	101	0
Major/Minor N	/linor2			Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	292	292	101	292	292	191	101	0	0	191	0	0
Stage 1	101	101	-	191	191	_	-	-	_	-	-	_
Stage 2	191	191	-	101	101	-	-	-	-	-	-	-
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	_	-	_	_	-	-	-
Critical Hdwy Stg 2	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	504	622	960	664	622	856	1504	-	-	1395	-	-
Stage 1	713	815	-	815	746	-	-	-	-	-	-	-
Stage 2	629	746	-	910	815	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	504	622	960	664	622	856	1504	-		1395	-	-
Mov Cap-2 Maneuver	504	622	-	664	622	-	-	-	-	-	-	-
Stage 1	713	815	-	815	746	-	-	-	-	-	-	-
Stage 2	629	746	-	910	815	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.2			0			0			0		
HCM LOS	В			A						•		
Minor Lane/Major Mvmt	t	NBL	NBT	NBR I	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1504	-	-		-	1395	-				
HCM Lane V/C Ratio		-	_		0.002	_	-	_	_			
HCM Control Delay (s)		0	_	_		0	0	_	_			
HCM Lane LOS		A	_	_	В	A	A	_	_			
HCM 95th %tile Q(veh)		0	-	-	0	-	0	-	-			
							•					

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	CDL		EDK	VVDL		WDIX	INDL		INDIX	ODL		SDR
Lane Configurations	۸	4	٥	13	- ♣	E0	1	446	43	40	4	2
Traffic Vol, veh/h	0	4	0	13	5	59 59	1 1	146 146	43	40	56 56	2
Future Vol, veh/h	0	0	0	0	5	0	0	0	43	0	0	0
Conflicting Peds, #/hr								Free	Free		Free	
Sign Control RT Channelized	Stop	Stop	Stop None	Stop -	Stop -	Stop None	Free -	riee -	None	Free -		Free None
Storage Length	_	-	None	_	-	None	-	-	NONE -	_	-	NOHE
Veh in Median Storage		0	-		0			0	-		0	
Grade, %	, # - -	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mymt Flow	0	4	0	13	5	59	1	146	43	40	56	2
IVIVIIIL I IOW	U	4	U	10	J	33		140	40	40	50	
	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	339	328	57	309	308	168	58	0	0	189	0	0
Stage 1	137	137	-	170	170	-	-	-	-	-	-	-
Stage 2	202	191	-	139	138	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4		3.572	4	3.426	2.2	-	-	2.452	-	-
Pot Cap-1 Maneuver	619	594	1015	632	609	846	1559	-	-	1243	-	-
Stage 1	871	787	-	818	762	-	-	-	-	-	-	-
Stage 2	805	746	-	850	786	-	-	-	-	-	-	-
Platoon blocked, %		F7.4	1045	040	E00	0.40	4550	-	-	1040	-	-
Mov Cap-1 Maneuver	557	574	1015	612	588	846	1559	-	-	1243	-	-
Mov Cap-2 Maneuver	557	574	-	612	588	-	-	-	-	-	-	-
Stage 1	870	761	-	817	761	-	-	-	-	-	-	-
Stage 2	743	745	-	818	760	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.3			10.2			0			3.3		
HCM LOS	В			В								
Minor Lane/Major Mvm	t	NBL	NBT	NRR I	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1559	וטו	-		774	1243	ODT	UDIN			
HCM Lane V/C Ratio		0.001	-				0.032	-	<u>-</u>			
HCM Control Delay (s)		7.3	0	<u>-</u>		10.2	8	0	_			
HCM Lane LOS		7.3 A	A	<u>-</u>	11.3 B	10.2 B	A	A	_			
HCM 95th %tile Q(veh)		0	-	<u>-</u>	0	0.3	0.1	-	_			
HOW JOHN JUHIE W(VEII)		U			U	0.0	0.1					

Intersection						
Int Delay, s/veh	1.1					
	EBL	EDD	NDI	NDT	CDT	CDD
Movement Lang Configurations		EBR	NBL	NBT	SBT	SBR
Lane Configurations	74	0	2	<b>€</b>	<b>}</b>	40
Traffic Vol, veh/h	9	2	3	63	21	12
Future Vol, veh/h	9	2	3	63	21	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	14	0
Mvmt Flow	9	2	3	63	21	12
Major/Minor N	/linor2	N	Major1	N	Major2	
Conflicting Flow All	96	27	33	0	- viajoiz	0
Stage 1	27	-	-	-	<u>-</u>	-
Stage 2	69	_	_		_	_
Critical Hdwy	6.4	6.2	4.1	-	-	-
	5.4	0.2	4.1	-	_	-
Critical Hdwy Stg 1	5.4		-	-	-	-
Critical Hdwy Stg 2	3.5	3.3	2.2	-	-	-
Follow-up Hdwy				-	-	-
Pot Cap-1 Maneuver	908	1054	1592	-	-	-
Stage 1	1001	-	_	-	-	-
Stage 2	959	-	-	-	-	-
Platoon blocked, %	000	4054	4500	-	-	-
Mov Cap-1 Maneuver	906	1054	1592	-	-	-
Mov Cap-2 Maneuver	906	-	-	-	-	-
Stage 1	999	_	-	-	-	-
Stage 2	959	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.3		0	
HCM LOS			0.3		U	
I IOIVI LOS	Α					
Minor Lane/Major Mvmt	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1592	-	930	_	_
HCM Lane V/C Ratio		0.002	-	0.012	_	-
HCM Control Delay (s)		7.3	0	8.9	-	_
HCM Lane LOS		A	A	A	_	-
HCM 95th %tile Q(veh)		0	-	0	_	-
I ICIVI SOIII MINE CILVEITI						

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	1	64	0	0	24	1
Future Vol, veh/h	0	0	0	0	0	0	1	64	0	0	24	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	1	64	0	0	24	1
Major/Minor M	linor2		ľ	Minor1		- 1	Major1		N	//ajor2		
Conflicting Flow All	91	91	25	91	91	64	25	0	0	64	0	0
Stage 1	25	25	-	66	66	-	-	-	-	-	-	-
Stage 2	66	66	-	25	25	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	_	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	898	803	1057	898	803	1006	1603	-	-	1551	-	-
Stage 1	998	878	-	950	844	-	-	-	-	-	-	-
Stage 2	950	844	-	998	878	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	897	802	1057	897	802	1006	1603	-	-	1551	-	-
Mov Cap-2 Maneuver	897	802	-	897	802	-	-	-	-	-	-	-
Stage 1	997	878	-	949	843	-	-	-	-	-	-	-
Stage 2	949	843	-	998	878	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0.1			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1603	-	-	_	-	1551	-	-			
HCM Lane V/C Ratio		0.001	-	-	-	-	-	-	-			
HCM Control Delay (s)		7.2	0	-	0	0	0	-	-			
HCM Lane LOS		Α	Α	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	2.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDIX		NDIX	ODL	- <del>3</del>
Lane Configurations	12	7	<b>Љ</b> 51	11	26	32
Traffic Vol, veh/h	12			41		32
Future Vol, veh/h		7	51	41	26	
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	0	14	23	12	26
Mvmt Flow	12	7	51	41	26	32
N.A'/N.A'	N4' 4		1.1.1		M - ' - O	
	Minor1		//ajor1		Major2	
Conflicting Flow All	156	72	0	0	92	0
Stage 1	72	-	-	-	-	-
Stage 2	84	-	-	-	-	-
Critical Hdwy	6.57	6.2	-	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	-	-	-	-	-
Follow-up Hdwy	3.653	3.3	-	-	2.308	-
Pot Cap-1 Maneuver	802	996	_	_	1442	_
Stage 1	914	-	_	_		_
Stage 2	903	_			_	_
Platoon blocked, %	303	_	_	_	_	_
	700	006	-	-	1110	
Mov Cap-1 Maneuver		996	-	-	1442	-
Mov Cap-2 Maneuver	788	-	-	-	-	-
Stage 1	914	-	-	-	-	-
Stage 2	887	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		3.4	
HCM LOS			U		5.4	
I IOWI LOS	А					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	_	854	1442	_
HCM Lane V/C Ratio		_	_	0.022		_
HCM Control Delay (s	١			9.3	7.5	0
HCM Lane LOS	)			9.3 A	7.5 A	A
		-	-			
HCM 95th %tile Q(veh	1)	-	-	0.1	0.1	-

Intersection												
Int Delay, s/veh	6.1											
		CDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	<b>-</b> -0	- ♣	2	^	4	<b>7</b>	4	₩,	0	405	4	40
Traffic Vol, veh/h	53	65	3	0	22	53	1	0	2	135	1	12
Future Vol, veh/h	53 0	65 0	3	0	22	53 0	1 0	0	2	135	1 0	12
Conflicting Peds, #/hr	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	0 Stop	Stop	Stop
Sign Control RT Channelized	-	riee -	None	riee -	riee -	None	Stop -	Stop -	None	Stop -	Stop -	None
Storage Length	_	_	NOHE	-	-	200	_	-	NONE -	_	_	NOHE
Veh in Median Storage		0		_	0	200		0	_	_	0	_
Grade, %	-, π	0	_	<u>-</u>	0	_	_	0	_	_	0	<u>-</u>
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	53	65	3	0	22	53	1	0	2	135	1	12
							•		_		•	
Major/Minor I	Major1		N	Major2		ı	Minor1			Minor2		
Conflicting Flow All	75	0	0	68	0	0	228	248	67	196	196	22
Stage 1	-	-	-	-	-	-	173	173	-	22	22	-
Stage 2	_	_	_	_	_	_	55	75	_	174	174	_
Critical Hdwy	4.39	_	_	4.1	_	_	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	03	_	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Critical Hdwy Stg 2	_	-	-	-	-	-	6.1	5.5	_	6.31	5.5	-
Follow-up Hdwy	2.461	_	-	2.2	-	_	3.5	4	3.3	3.689	4	3.975
Pot Cap-1 Maneuver	1369	-	-	1546	-	-	731	658	1002	723	703	879
Stage 1	-	-	-	-	-	-	834	760	-	950	881	-
Stage 2	-	-	-	-	-	-	962	836	-	785	759	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1369	-	-	1546	-	-	698	632	1002	700	675	879
Mov Cap-2 Maneuver	-	-	-	-	-	-	698	632	-	700	675	-
Stage 1	-	-	-	-	-	-	801	730	-	912	881	-
Stage 2	-	-	-	-	-	-	948	836	-	752	729	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.1			11.4		
HCM LOS							Α			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		875	1369	-	-	1546	-	-	712			
HCM Lane V/C Ratio		0.003	0.039	-	-	-	-	-	0.208			
HCM Control Delay (s)		9.1	7.7	0	-	0	-	-				
HCM Lane LOS		Α	Α	Α	-	Α	-	-	В			
HCM 95th %tile Q(veh)	)	0	0.1	-	-	0	-	-	0.8			

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	ĵ.		ች	ĵ.			4			4	
Traffic Vol, veh/h	9	286	8	29	100	101	1	5	28	71	7	7
Future Vol, veh/h	9	286	8	29	100	101	1	5	28	71	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	_	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage		0	_	_	0	-	_	0	-	-	0	-
Grade, %	-	0	_	_	0	_	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	9	286	8	29	100	101	1	5	28	71	7	7
Major/Minor	Major1		ı	Major2		ı	Minor1			Minor2		
Conflicting Flow All	201	0	0	294	0	0	524	567	290	534	521	151
Stage 1	201	-	-	-	-	-	308	308	-	209	209	.01
Stage 2	_	<u>-</u>	_	_	_	_	216	259	_	325	312	_
Critical Hdwy	4.32	_	_	4.17	_	_	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	- 1.02	_	_		_	_	6.1	5.5	- 0.2	6.19	5.64	- 0.01
Critical Hdwy Stg 2	_	_	_	_	_	_	6.1	5.5	_	6.19	5.64	_
Follow-up Hdwy	2.398	_	_	2.263	_	_	3.5	4		3.581	4.126	3.426
Pot Cap-1 Maneuver	1260	_	_	1239	_	_	467	436	754	446	443	865
Stage 1		_	_	-	_	_	706	664	-	777	707	-
Stage 2	_	_	_	-	-	_	791	697	-	673	637	-
Platoon blocked, %		_	-		_	_				J. <b>J</b>		
Mov Cap-1 Maneuver	1260	_	_	1239	_	_	447	423	754	416	430	865
Mov Cap-2 Maneuver	-	-	_	-	_	_	447	423	-	416	430	-
Stage 1	_	-	_	_	-	_	701	659	_	772	691	-
Stage 2	_	_	-	-	-	-	758	681	-	638	633	-
0 -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			10.7			15.2		
HCM LOS	0.2						В			C		
Minor Lane/Major Mvm	nt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SRI n1			
Capacity (veh/h)		664	1260		LDI	1239	1101		436			
HCM Lane V/C Ratio			0.007	_	-	0.023			0.195			
HCM Control Delay (s)		10.7	7.9	<u>-</u>	-	8	-	_	15.2			
HCM Lane LOS		10.7 B	7.9 A	_	-	A	-	_	13.2 C			
HCM 95th %tile Q(veh)	)	0.2	0		-	0.1	_	_	0.7			
	1	0.2	- 0			0.1			0.1			

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	96	266	13	167	0	0	0	0	1	4	54
Future Vol, veh/h	0	96	266	13	167	0	0	0	0	1	4	54
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mvmt Flow	0	96	266	13	167	0	0	0	0	1	4	54
Major/Minor N	//ajor1		1	Major2					N	Minor2		
Conflicting Flow All	- -	0	0	362	0	0				422	555	167
Stage 1	_	-	-	-	-	-				193	193	-
Stage 2	-	_	_	_	_	_				229	362	_
Critical Hdwy	_	_	_	4.25	_	_				6.4	6.75	6.5
Critical Hdwy Stg 1	-	-	-		_	-				5.4	5.75	-
Critical Hdwy Stg 2	-	-	_	-	_	-				5.4	5.75	_
Follow-up Hdwy	-	_	_	2.335	_	_				3.5	4.225	3.57
Pot Cap-1 Maneuver	0	-	-	1128	-	0				592	410	809
Stage 1	0	_	-	-	_	0				845	700	-
Stage 2	0	-	_	-	_	0				814	587	-
Platoon blocked, %		-	-		_							
Mov Cap-1 Maneuver	-	-	-	1128	_	-				584	0	809
Mov Cap-2 Maneuver	-	-	-	-	-	-				584	0	-
Stage 1	-	-	-	-	_	-				845	0	-
Stage 2	-	-	-	-	-	-				803	0	-
, i												
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.6						9.8		
HCM LOS										A		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)					-	803						
HCM Lane V/C Ratio		_		0.012		0.073						
HCM Control Delay (s)		_	_	8.2	0	9.8						
HCM Lane LOS		_	_	A	A	Α						
HCM 95th %tile Q(veh)		-	_	0	-	0.2						
7000 (1011)						7.2						

Intersection												
Int Delay, s/veh	10											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			f)			4				
Traffic Vol, veh/h	93	4	0	0	6	5	174	10	6	0	0	0
Future Vol, veh/h	93	4	0	0	6	5	174	10	6	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	7	25	0	0	50	0	24	50	50	2	2	2
Mvmt Flow	93	4	0	0	6	5	174	10	6	0	0	0
Major/Minor N	1ajor1			Major2		_ 1	Minor1					
Conflicting Flow All	11	0	_	- -	_	0	199	201	4			
Stage 1		-	_	_	_	-	190	190				
Stage 2	_	_	_	_	_	_	9	11	_			
Critical Hdwy	4.17	_	-	-	_	-	6.64	7	6.7			
Critical Hdwy Stg 1	-	_	_	_	_	_	5.64	6	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.64	6	_			
	2.263	_	_	_	_	_	3.716	4.45	3.75			
Pot Cap-1 Maneuver	1576	_	0	0	_	-	742	618	955			
Stage 1	-	_	0	0	-	_	792	661	-			
Stage 2	_	_	0	0	_	-	960	800	-			
Platoon blocked, %		_			_	-						
Mov Cap-1 Maneuver	1576	-	-	-	-	-	698	0	955			
Mov Cap-2 Maneuver	-	-	-	-	-	-	698	0	-			
Stage 1	-	-	-	-	-	-	745	0	-			
Stage 2	-	-	-	-	-	-	960	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	7.1			0			12					
HCM LOS							В					
Minor Lane/Major Mvmt	<b>.</b> .	NBLn1	EBL	EBT	WBT	WBR						
		704	1576	LDT	VVDT	אטוע						
Capacity (veh/h) HCM Lane V/C Ratio			0.059	-	-	-						
		12		- 0	-	-						
HCM Control Delay (s) HCM Lane LOS		12 B	7.4	0	-	-						
HCM 95th %tile Q(veh)		1.1	A 0.2	Α	-	-						
HOW SOUL WILLE Q(Ven)		1.1	0.2	-	-	-						

Intersection												
Int Delay, s/veh	7.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		Ĥ			र्स						4	
Traffic Vol, veh/h	0	2	7	69	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	69	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	2	7	69	2	0	0	0	0	6	11	6
Major/Minor N	/lajor1		N	Major2					N	/linor2		
Conflicting Flow All	-	0	0	9	0	0				146	149	2
Stage 1	-	-	-	-	-	-				140	140	-
Stage 2	-	-	-	-	-	-				6	9	-
Critical Hdwy	-	-	-	4.27	-	-				6.57	7.23	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.57	6.23	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.57	6.23	-
Follow-up Hdwy	-	-	-	2.353	-	-				3.653	4.657	3.3
Pot Cap-1 Maneuver	0	-	-	1518	-	0				812	630	1088
Stage 1	0	-	-	-	-	0				851	663	-
Stage 2	0	-	-	-	-	0				979	766	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1518	-	-				775	0	1088
Mov Cap-2 Maneuver	-	-	-	-	-	-				775	0	-
Stage 1	-	-	-	-	-	-				851	0	-
Stage 2	-	-	-	-	-	-				934	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			7.3						9.1		
HCM LOS										Α		
Minor Lang/Major Mumt		EBT	EBR	WBL	WPT	SBLn1						
Minor Lane/Major Mvmt												
Capacity (veh/h) HCM Lane V/C Ratio		-		1518 0.045	-	905 0.025						
		-	-		0	9.1						
HCM Control Delay (s) HCM Lane LOS		-	-	7.5 A	A	9.1 A						
HCM 95th %tile Q(veh)		-	-	0.1	- -	0.1						
HOW JOHN JOHN (VEH)		-	_	0.1	_	0.1						

Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			f)			4				
Traffic Vol, veh/h	5	3	0	0	70	18	1	3	11	0	0	0
Future Vol, veh/h	5	3	0	0	70	18	1	3	11	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	13	11	100	33	0	2	2	2
Mvmt Flow	5	3	0	0	70	18	1	3	11	0	0	0
Major/Minor N	Major1		ı	Major2		N	Minor1					
Conflicting Flow All	88	0		-	_	0	92	101	3			
Stage 1	-	-	_	_	_	-	13	13	-			
Stage 2	<u>-</u>	_	_	_	_	_	79	88	<u>-</u>			
Critical Hdwy	4.1	_	_	_	_	_	7.4	6.83	6.2			
Critical Hdwy Stg 1	- 1. 1	_	_	_	_	_	6.4	5.83	-			
Critical Hdwy Stg 2	_	_	_	_	_	-	6.4	5.83	_			
Follow-up Hdwy	2.2	_	_	_	_	_	4.4	4.297	3.3			
Pot Cap-1 Maneuver	1520	_	0	0	_	-	716	734	1087			
Stage 1	-	_	0	0	_	_	806	827	-			
Stage 2	_	_	0	0	_	-	746	765	_			
Platoon blocked, %		_			_	_	. 13	. 00				
Mov Cap-1 Maneuver	1520	_	_	_	_	-	714	0	1087			
Mov Cap-2 Maneuver	-	_	_	_	_	_	714	0	-			
Stage 1	_	_	_	_	_	-	804	0	_			
Stage 2	_	_	_	_	_	_	746	0	_			
2.0.30 2												
Annroach	EB			\\/D			ND					
Approach				WB			NB					
HCM Control Delay, s	4.6			0			8.5					
HCM LOS							Α					
Minor Lane/Major Mvm	t I	VBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1042	1520	-	-	-						
HCM Lane V/C Ratio			0.003	-	-	-						
HCM Control Delay (s)		8.5	7.4	0	-	-						
HCM Lane LOS		Α	Α	Α	-	-						
HCM 95th %tile Q(veh)		0	0	-	-	-						

Intersection												
Int Delay, s/veh	6.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol. veh/h	3	7	4	0	14	0	12	2	0	0	0	62
Future Vol, veh/h	3	7	4	0	14	0	12	2	0	0	0	62
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	_	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	25	0	0	0	18	0	0	0	0	0
Mvmt Flow	3	7	4	0	14	0	12	2	0	0	0	62
Major/Minor N	/lajor1		ľ	Major2		ľ	Minor1		<u> </u>	/linor2		
Conflicting Flow All	14	0	0	11	0	0	60	29	9	30	31	14
Stage 1	-	-	-	-	-	-	15	15	-	14	14	-
Stage 2	-	-	-	-	-	-	45	14	-	16	17	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.28	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	_	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.662	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1617	-	-	1621	-	-	898	868	1079	984	866	1072
Stage 1	-	-	-	-	-	-	965	887	-	1011	888	-
Stage 2	-	-	-	-	-	-	930	888	-	1009	885	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1617	-	-	1621	-	-	845	866	1079	981	864	1072
Mov Cap-2 Maneuver	-	-	-	-	-	-	845	866	-	981	864	-
Stage 1	-	-	-	-	-	-	963	885	-	1009	888	-
Stage 2	-	-	-	-	-	-	876	888	-	1005	883	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.5			0			9.3			8.6		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		848	1617	-	-	1621	-	_	1072			
HCM Lane V/C Ratio		0.017		-	-	-	-	-	0.058			
HCM Control Delay (s)		9.3	7.2	0	-	0	-	-	8.6			
HCM Lane LOS		Α	Α	Α	-	Α	-	-	Α			
HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	0.2			

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			4						4	
Traffic Vol, veh/h	0	65	0	33	11	0	0	0	0	119	1	5
Future Vol, veh/h	0	65	0	33	11	0	0	0	0	119	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	65	0	33	11	0	0	0	0	119	1	5
Major/Minor N	Major1	Major2							N	/linor2		
Conflicting Flow All		0	0	65	0	0				142	142	11
Stage 1	-	-	-	-	-	-				77	77	-
Stage 2	-	-	-	-	_	-				65	65	-
Critical Hdwy	-	-	-	4.23	-	-				6.43	7.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	6.5	-
Follow-up Hdwy	-	-	-	2.317	-	-				3.527	4.9	3.48
Pot Cap-1 Maneuver	0	-	-	1470	-	0				848	601	1020
Stage 1	0	-	-	-	-	0				943	673	-
Stage 2	0	-	-	-	-	0				955	683	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1470	-	-				828	0	1020
Mov Cap-2 Maneuver	-	-	-	-	-	-				828	0	-
Stage 1	-	-	-	-	-	-				943	0	-
Stage 2	-	-	-	-	-	-				933	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.6						10.1		
HCM LOS	•			3.0						В		
Minor Long/Major Mare		EDT	EDD	WBL	WDT	CDI ~1						
Minor Lane/Major Mvm	l	EBT	EBR			SBLn1						
Capacity (veh/h)		-	-		-	834						
HCM Control Doloy (a)		-		0.022 7.5	-	0.15						
HCM Control Delay (s) HCM Lane LOS		-	-		0	10.1						
HCM 95th %tile Q(veh)		-	-	0.1	A -	0.5						
HOW YOUR WILLE WING		-	-	U. I	-	0.5						

Intersection													
Int Delay, s/veh	2.3												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			<b>1</b>			4				02.1	
Traffic Vol, veh/h	42	142	0	0	35	376	9	4	121	0	0	0	
Future Vol, veh/h	42	142	0	0	35	376	9	4	121	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	_	-	None	_	-	None	-	-	None	-	-	None	
Storage Length	_	_	-	-	-	-	_	_	-	_	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	_	0	-	_	0	-	-	0	-	_	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	17	7	2	0	12	4	0	25	10	2	2	2	
Mvmt Flow	42	142	0	0	35	376	9	4	121	0	0	0	
Major/Minor N	Major1		ľ	Major2		N	Minor1						
Conflicting Flow All	411	0	-	-	-	0	449	637	142				
Stage 1	-	-	-	-	-	-	226	226	-				
Stage 2	-	_	-	-	-	-	223	411	-				
Critical Hdwy	4.27	-	-	-	-	-	6.4	6.75	6.3				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.75	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.75	-				
Follow-up Hdwy	2.353	-	-	-	-	-	3.5	4.225	3.39				
Pot Cap-1 Maneuver	1072	-	0	0	-	_	571	366	885				
Stage 1	-	-	0	0	-	-	816	676	-				
Stage 2	-	-	0	0	-	-	819	557	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1072	-	-	-	-	-	546	0	885				
Mov Cap-2 Maneuver	-	-	-	-	-	-	546	0	-				
Stage 1	-	-	-	-	-	-	781	0	-				
Stage 2	-	-	-	-	-	-	819	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	1.9			0			10						
HCM LOS							В						
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		849	1072	-	-	-							
HCM Lane V/C Ratio		0.158		-	-	-							
HCM Control Delay (s)		10	8.5	0	-	-							
HCM Lane LOS		В	Α	A	-	-							
HCM 95th %tile Q(veh)		0.6	0.1	-	-	-							

Intersection												
Int Delay, s/veh	8.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LOIK	11.52	4	11511	1100	4	11511	- U.D.L	4	USIN
Traffic Vol, veh/h	3	250	10	1	102	3	303	1	27	1	0	6
Future Vol, veh/h	3	250	10	1	102	3	303	1	27	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	250	10	1	102	3	303	1	27	1	0	6
Major/Minor M	ajor1		ľ	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	105	0	0	260	0	0	370	368	255	381	372	104
Stage 1	-	-	-	-	-	-	261	261	-	106	106	-
Stage 2	-	-	-	-	-	-	109	107	-	275	266	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5		3.453
	1499	-	-	1316	-	-	590	564	789	581	561	911
Stage 1	-	-	-	-	-	-	748	696	-	905	811	-
Stage 2	-	-	-	-	-	-	901	811	-	736	692	-
Platoon blocked, %	1.100	-	-	1010	-	-	F0F	500	700	550	550	044
	1499	-	-	1316	-	-	585	562	789	559	559	911
Mov Cap-2 Maneuver	-	-	-	-	-	-	585	562	-	559	559	-
Stage 1	-	-	-	-	-	-	747	695	-	903	810	-
Stage 2	-	-	-	-	-	<u>-</u>	894	810	-	708	691	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0.1			18.2			9.3		
HCM LOS							С			Α		
Minor Lane/Major Mvmt	1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		598	1499	-	-	1316	-	-	836			
HCM Lane V/C Ratio		0.554	0.002	-	-	0.001	-	-	800.0			
HCM Control Delay (s)		18.2	7.4	0	-	7.7	0	-	9.3			
HCM Lane LOS		С	Α	Α	-	Α	Α	-	Α			
HCM 95th %tile Q(veh)		3.4	0	-	-	0	-	-	0			

Intersection							
Int Delay, s/veh	5.6						
		EDT	WDT	WDD	CDI	CDD	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	<b>ነ</b>	<b>†</b>	<b>}</b>	00	<u>ች</u>	7	
Traffic Vol, veh/h	188	60	57	29	24	69	
Future Vol, veh/h	188	60	57	29	24	69	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	188	60	57	29	24	69	
Major/Minor N	//ajor1	N	Major2	N	Minor2		J
Conflicting Flow All	86	0	<u>viajuiz</u> -	0	508	72	
Stage 1	- 00	U		-	72	-	
Stage 2	-	-	-	-	436	_	
Critical Hdwy	4.15		-		6.4	6.26	
•	4.13	-	-	-	5.4		
Critical Hdwy Stg 1	<del>-</del>	-	-	-	5.4	-	
Critical Hdwy Stg 2	- 0.045	-	-	-		2 254	
	2.245	-	-	-		3.354	
Pot Cap-1 Maneuver	1492	-	_	-	528	979	
Stage 1	-	-	-	-	956	-	
Stage 2	-	-	-	-	656	-	
Platoon blocked, %	4.455	-	-	-	4		
Mov Cap-1 Maneuver	1492	-	-	-	461	979	
Mov Cap-2 Maneuver	-	-	-	-	461	-	
Stage 1	-	-	-	-	836	-	
Stage 2	-	-	-	-	656	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.9		0		10.1		
HCM LOS					В		
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WRR	SBLn1 S	
			LDI	VVDI	יוטיי		ار
Capacity (veh/h)		1492	-	-	-	461	
HCM Control Dolay (a)		0.126	-	-		0.052	
HCM Control Delay (s)		7.8	-	-	-	13.2	
HCM Lane LOS		Α	-	-	-	В	
HCM 95th %tile Q(veh)		0.4				0.2	

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ħ	4		ሻ	ĵ.			4			4	
Traffic Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	21
Future Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	47	37	4	1	46	4	8	16	7	0	2	21
Major/Minor	Major1		ı	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	50	0	0	41	0	0	195	185	39	191	185	48
Stage 1	-	-	-	_	-	-	133	133	-	50	50	-
Stage 2	-	_	_	_	_	_	62	52	_	141	135	_
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1		_	-	-	_	_	6.1	5.56	-	6.1	5.5	-
Critical Hdwy Stg 2	_	_	-	_	_	_	6.1	5.56	-	6.1	5.5	_
Follow-up Hdwy	2.299	_	-	2.2	_	_		4.054	3.426	3.5	4	3.39
Pot Cap-1 Maneuver	1501	-	-	1581	-	-	769	702	999	773	713	999
Stage 1	-	-	-	-	-	-	875	779	-	968	857	-
Stage 2	-	-	-	-	-	-	954	844	-	867	789	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1501	-	-	1581	-	-	733	680	999	735	690	999
Mov Cap-2 Maneuver	-	-	-	-	-	-	733	680	-	735	690	-
Stage 1	-	-	-	-	-	-	848	755	-	938	856	-
Stage 2	-	-	-	-	-	-	931	843	-	816	765	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4			0.1			9.1			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		900	1501	-	-	1581	-	-	962			
HCM Lane V/C Ratio		0.034	0.031	-	_	0.001	-	_	0.024			
HCM Control Delay (s)		9.1	7.5	-	-	7.3	_	-	8.8			
HCM Lane LOS		Α	A	-	-	A	-	-	Α			
HCM 95th %tile Q(veh)	)	0.1	0.1	-	-	0	-	-	0.1			

Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	LDIN	YVDL	<u>₩</u>	₩.	ווטוז
Traffic Vol, veh/h	44	1	5	<b>T</b> 42	10	14
Future Vol, veh/h	44	1	5	42	10	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized		None		None	•	None
	-		150	None -	-	None
Storage Length	- 4 0	-			0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	400	400	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	26	0	0	15	0	7
Mvmt Flow	44	1	5	42	10	14
Major/Minor N	1ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	45	0	97	45
Stage 1	-	-	-	-	45	-
Stage 2	_	<u>-</u>	_	<u>-</u>	52	_
Critical Hdwy	_	_	4.1	_	6.4	6.27
Critical Hdwy Stg 1	_	_	4.1	_	5.4	0.21
Critical Hdwy Stg 2		-	-		5.4	-
		-	2.2	-		3.363
Follow-up Hdwy	-		1576			
Pot Cap-1 Maneuver	-	-		-	907	1011
Stage 1	-	-	-	-	983	-
Stage 2	-	-	-	-	976	-
Platoon blocked, %	-	-	4	-	224	
Mov Cap-1 Maneuver	-	-	1576	-	904	1011
Mov Cap-2 Maneuver	-	-	-	-	904	-
Stage 1	-	-	-	-	983	-
Stage 2	-	-	-	-	973	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.8	
HCM LOS	U		0.0		Α	
TION LOS						
Minor Lane/Major Mvmt	: 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		963	-	-	1576	-
HCM Lane V/C Ratio		0.025	-	-	0.003	-
HCM Control Delay (s)		8.8	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		f)			4
Traffic Vol, veh/h	2	0	16	0	0	6
Future Vol, veh/h	2	0	16	0	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	17
Mvmt Flow	2	0	16	0	0	6
IVIVIIICT IOW		U	10	U	U	U
Major/Minor N	Minor1	N	//ajor1	N	/lajor2	
Conflicting Flow All	22	16	0	0	16	0
Stage 1	16	-	-	-	-	-
Stage 2	6	_	-	_	_	_
Critical Hdwy	6.4	6.2	_	-	4.1	-
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	1000	1069	_	_	1615	_
Stage 1	1012	-	_	<u>_</u>	-	_
Stage 2	1012	_	_		_	_
Platoon blocked, %	1022	_	_	_	_	_
	1000	1060	-	<del>-</del>	1615	
Mov Cap-1 Maneuver	1000	1069	-	-	1615	-
Mov Cap-2 Maneuver	1000	-	-	-	-	-
Stage 1	1012	-	-	-	-	-
Stage 2	1022	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α		U		U	
I IOIVI LOO						
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			_	1000	1615	
HCM Lane V/C Ratio		_	_	0.002	-	-
HCM Control Delay (s)		_	_	8.6	0	_
HCM Lane LOS		_	-	A	A	-
HCM 95th %tile Q(veh)		_	_	0	0	_
		_	_	U	U	_

Intersection						
Int Delay, s/veh	0					
		ED.5	NE	Not	057	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			ની	f)	
Traffic Vol, veh/h	0	0	0	21	5	0
Future Vol, veh/h	0	0	0	21	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	5	20	0
Mvmt Flow	0	0	0	21	5	0
NA=:==/NA:===	A: O		1-:1		4-10	
	Minor2		//ajor1		Major2	
Conflicting Flow All	26	5	5	0	-	0
Stage 1	5	-	-	-	-	-
Stage 2	21	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	995	1084	1630	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1007	-	-	_	_	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	995	1084	1630	_	_	_
Mov Cap-2 Maneuver	995	-	-	-	_	_
Stage 1	1023	_	_	_	_	_
Stage 2	1007	_	_	_	_	_
Olago Z	1007					
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Long/Major Mym		NBL	NDT	EBLn1	SBT	SBR
Minor Lane/Major Mvm	ll .		INDI	EDLIII	ODI	SDK
Capacity (veh/h)		1630	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	-	-	-

Intersection						
Int Delay, s/veh	5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			र्स	Þ	
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	_	-	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	25	33	0	0	0	33
Mymt Flow	4	6	16	11	3	1
IVIVIIIL I IUVV	7	U	10	11	J	
Major/Minor	Minor2	N	//ajor1	N	/lajor2	
Conflicting Flow All	47	4	4	0	-	0
Stage 1	4	-	-	-	-	-
Stage 2	43	-	_	-	_	-
Critical Hdwy	6.65	6.53	4.1	_	_	_
Critical Hdwy Stg 1	5.65	-	- '	_	_	_
Critical Hdwy Stg 2	5.65	_	_	_	_	_
Follow-up Hdwy	3.725	3.597	2.2	_	_	_
Pot Cap-1 Maneuver	908	996	1631			_
Stage 1	962	-	1001	_	_	_
	902	-	-	-	-	-
Stage 2	924	-	-		=	
Platoon blocked, %	000	000	1604	-	-	-
Mov Cap-1 Maneuver	899	996	1631	-	-	-
Mov Cap-2 Maneuver	899	-	-	-	-	-
Stage 1	952	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		4.3		0	
HCM LOS	Α		7.0		U	
I IOIVI LOG	Α					
Minor Lane/Major Mvm	nt	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1631	-	955	_	_
HCM Lane V/C Ratio		0.01	-	0.01	_	-
HCM Control Delay (s)		7.2	0	8.8	-	-
HCM Lane LOS		Α	A	А	_	-
HCM 95th %tile Q(veh	)	0	-	0	-	-
. ISM Sour Jours Selver	1	U		v		

Intersection												
Int Delay, s/veh	3.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	29	41	1	1	12	4	1	1	0	4	2	11
Future Vol, veh/h	29	41	1	1	12	4	1	1	0	4	2	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	18	100	0	0	0	0	0	0	25	0	0
Mvmt Flow	29	41	1	1	12	4	1	1	0	4	2	11
Major/Minor I	Major1		1	Major2		<u> </u>	Minor1		- 1	Minor2		
Conflicting Flow All	16	0	0	42	0	0	123	118	42	116	116	14
Stage 1	-	-	-	-	-	-	100	100	-	16	16	-
Stage 2	-	-	-	-	-	-	23	18	-	100	100	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.5	6.2	7.35	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4	3.3	3.725	4	3.3
Pot Cap-1 Maneuver	1545	-	-	1580	-	-	856	776	1034	809	778	1072
Stage 1	-	-	-	-	-	-	911	816	-	947	886	-
Stage 2	-	-	-	-	-	-	1000	884	-	853	816	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1545	-	-	1580	-	-	833	760	1034	796	762	1072
Mov Cap-2 Maneuver	-	-	-	-	-	-	833	760	-	796	762	-
Stage 1	-	-	-	-	-	-	894	800	-	929	885	-
Stage 2	-	-	-	-	-	-	987	883	-	836	800	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3			0.4			9.5			8.9		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		795	1545	-	-	1580	-	-	949			
HCM Lane V/C Ratio		0.003		-	-	0.001	-	-	0.018			
HCM Control Delay (s)		9.5	7.4	0	-	7.3	0	-	8.9			
HCM Lane LOS		Α	Α	Α	-	Α	Α	-	Α			
HCM 95th %tile Q(veh)	)	0	0.1	-	-	0	-	-	0.1			

Intersection		_		_		_
Int Delay, s/veh	3.9					
	WDI	WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	40	ĵ.	•	- 4	4
Traffic Vol, veh/h	5	48	74	8	54	36
Future Vol, veh/h	5	48	74	8	54	36
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	5	48	74	8	54	36
WWW.CT IOW		10			O I	00
	/linor1		//ajor1		Major2	
Conflicting Flow All	222	78	0	0	82	0
Stage 1	78	-	-	-	-	-
Stage 2	144	-	-	-	-	-
Critical Hdwy	6.6	6.2	-	-	4.12	-
Critical Hdwy Stg 1	5.6	-	-	-	-	_
Critical Hdwy Stg 2	5.6	-	-	-	_	-
Follow-up Hdwy	3.68	3.3	_	_	2.218	_
Pot Cap-1 Maneuver	728	988	_	_	1515	_
Stage 1	901	-	_	_	-	_
Stage 2	841	_		_	_	
Platoon blocked, %	041	_	_	_	_	_
•	700	000	-	_	1515	_
Mov Cap-1 Maneuver	702	988	-	-	1515	-
Mov Cap-2 Maneuver	702	-	-	-	-	-
Stage 1	901	-	-	-	-	-
Stage 2	811	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		4.5	
			U		4.5	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_		951	1515	_
HCM Lane V/C Ratio		_	_	0.056		_
HCM Control Delay (s)				9	7.5	0
HCM Lane LOS			_	A	7.5 A	A
HCM 95th %tile Q(veh)		-	-	0.2	0.1	- -
How som while Q(ven)		-	-	0.2	U. I	_

Intersection						
Int Delay, s/veh	10.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>	בטוע	TYDL	₩ <u>₩</u>	₩.	אטא
Traffic Vol, veh/h	4	13	0	3	387	0
Future Vol, veh/h	4	13	0	3	387	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- Stop	None
Storage Length		-	_	-	0	TNOTIC
Veh in Median Storage,	# 0	_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	13	0	3	387	0
Major/Minor N	/lajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	17	0	14	11
Stage 1	_	_	-	_	11	
Stage 2	_	_	_	_	3	_
Critical Hdwy	_	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	_	_		_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	_	_	2.218		3.518	
Pot Cap-1 Maneuver	_		1600	_	1005	1070
Stage 1	_	<u>-</u>	-	_	1012	-
Stage 1		-	-	-	1012	
	-	-	_	-	1020	-
Platoon blocked, %	-	-	1000	-	1005	1070
Mov Cap-1 Maneuver	-	-	1600	-	1005	1070
Mov Cap-2 Maneuver	-	-	-	-	1005	-
Stage 1	-	-	-	-	1012	-
Stage 2	-	-	-	-	1020	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		10.8	
HCM LOS	U		U		В	
I IOIVI LOS					D	
Minor Lane/Major Mvmt	t	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		1005	-	-	1600	-
HCM Lane V/C Ratio		0.385	-	-	-	-
HCM Control Delay (s)		10.8	-	-	0	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		1.8	-	-	0	-

Weekday	<sup>,</sup> Afternoon	Peak Hour

Intersection							ı
Intersection Delay, s/veh	7.3						
Intersection LOS	A						
Movement	EDI	EDT	WDT	WDD	CDI	CDD	
	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		<b>€</b>	<b>}</b>	^	<b>**</b> **	^	
Traffic Vol, veh/h	0	66	19	0	0	0	
Future Vol, veh/h	0	66	19	0	0	0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	0	66	19	0	0	0	
Number of Lanes	0	1	1	0	1	0	
Approach		EB	WB		SB		
Opposing Approach		WB	EB				
Opposing Lanes		1	1		0		
Conflicting Approach Left		SB			WB		
Conflicting Lanes Left		1	0		1		
Conflicting Approach Right			SB		EB		
Conflicting Lanes Right		0	1		1		
HCM Control Delay		7.3	7.1		0		
HCM LOS		Α.	A		_		
		- / \	, ,				
		EDI 4	MD: 4	ODL 4			
Lane		EBLn1	WBLn1	SBLn1			
Vol Left, %		0%	0%	0%			
Vol Left, % Vol Thru, %		0% 100%	0% 100%	0% 100%			
Vol Left, % Vol Thru, % Vol Right, %		0% 100% 0%	0% 100% 0%	0% 100% 0%			
Vol Left, % Vol Thru, % Vol Right, % Sign Control		0% 100% 0% Stop	0% 100% 0% Stop	0% 100% 0% Stop			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane		0% 100% 0% Stop 66	0% 100% 0% Stop 19	0% 100% 0% Stop 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 100% 0% Stop 66	0% 100% 0% Stop 19	0% 100% 0% Stop 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 100% 0% Stop 66	0% 100% 0% Stop 19	0% 100% 0% Stop 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol		0% 100% 0% Stop 66	0% 100% 0% Stop 19	0% 100% 0% Stop 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		0% 100% 0% Stop 66 0	0% 100% 0% Stop 19 0	0% 100% 0% Stop 0 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		0% 100% 0% Stop 66 0 66	0% 100% 0% Stop 19 0 19	0% 100% 0% Stop 0 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		0% 100% 0% Stop 66 0 66	0% 100% 0% Stop 19 0 19	0% 100% 0% Stop 0 0 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		0% 100% 0% Stop 66 0 66 0	0% 100% 0% Stop 19 0 19 0	0% 100% 0% Stop 0 0 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 100% 0% Stop 66 0 66 0 66 1 0.072 3.948	0% 100% 0% Stop 19 0 19 0 19 0.021 3.983	0% 100% 0% Stop 0 0 0 0 4.082			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes	0% 100% 0% Stop 19 0 19 0 19 3.983 Yes	0% 100% 0% Stop 0 0 0 0 4.082 Yes			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes 912	0% 100% 0% Stop 19 0 19 0 19 3.983 Yes 902	0% 100% 0% Stop 0 0 0 0 4.082 Yes 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes 912 1.951	0% 100% 0% Stop 19 0 19 0 19 1 3.983 Yes 902 1.994	0% 100% 0% Stop 0 0 0 0 4.082 Yes 0 2.115			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes 912 1.951 0.072	0% 100% 0% Stop 19 0 19 0 19 3.983 Yes 902 1.994 0.021	0% 100% 0% Stop 0 0 0 0 4.082 Yes 0 2.115 0			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio HCM Control Delay		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes 912 1.951 0.072 7.3	0% 100% 0% Stop 19 0 19 0 19 1 0.021 3.983 Yes 902 1.994 0.021 7.1	0% 100% 0% Stop 0 0 0 0 4.082 Yes 0 2.115 0 7.1			
Vol Left, % Vol Thru, % Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		0% 100% 0% Stop 66 0 66 1 0.072 3.948 Yes 912 1.951 0.072	0% 100% 0% Stop 19 0 19 0 19 3.983 Yes 902 1.994 0.021	0% 100% 0% Stop 0 0 0 0 4.082 Yes 0 2.115 0			

Intersection	
Intersection Delay, s/veh 8.6 Intersection LOS A	
Intersection LOS A	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	1	0	0	62	0	328	0	1	3	14	2	0	
Future Vol, veh/h	1	0	0	62	0	328	0	1	3	14	2	0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0	
Mvmt Flow	1	0	0	62	0	328	0	1	3	14	2	0	
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0	
Approach	EB			WB				NB		SB			
Opposing Approach	WB			EB				SB		NB			
Opposing Lanes	1			1				1		1			
Conflicting Approach Le	ft SB			NB				EB		WB			
Conflicting Lanes Left	1			1				1		1			
Conflicting Approach Ri	gh <b>N</b> B			SB				WB		EB			
<b>Conflicting Lanes Right</b>	1			1				1		1			
HCM Control Delay	7.5			8.6				7.2		8.5			
HCM LOS	Α			Α				Α		Α			

Lane	NBLn1	EBLn1\	WBLn1	SBLn1
Vol Left, %	0%	100%	16%	88%
Vol Thru, %	25%	0%	0%	12%
Vol Right, %	75%	0%	84%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	4	1	390	16
LT Vol	0	1	62	14
Through Vol	1	0	0	2
RT Vol	3	0	328	0
Lane Flow Rate	4	1	390	16
Geometry Grp	1	1	1	1
Degree of Util (X)	0.005	0.001	0.375	0.024
Departure Headway (Hd)	4.143	4.427	3.463	5.326
Convergence, Y/N	Yes	Yes	Yes	Yes
Сар	856	805	1037	669
Service Time	2.208	2.474	1.485	3.382
HCM Lane V/C Ratio	0.005	0.001	0.376	0.024
HCM Control Delay	7.2	7.5	8.6	8.5
HCM Lane LOS	Α	Α	Α	Α
HCM 95th-tile Q	0	0	1.8	0.1

Intersection						
Int Delay, s/veh	4.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>₽</b>	LUIX	WDL T		₩.	NOIN
Traffic Vol, veh/h	92	105	6	22	137	8
Future Vol, veh/h	92	105	6	22	137	8
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	130	-	0	-
Veh in Median Storage, #		_	-	0	0	
Grade, %	0	_	_	0	0	<u>-</u>
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	18	0	18	8	0
Mymt Flow	92	105	6	22	137	8
IVIVIIIL I IOW	JZ	100	- 0		101	- 0
	ajor1		/lajor2		Minor1	
Conflicting Flow All	0	0	197	0	179	145
Stage 1	-	-	-	-	145	-
Stage 2	-	-	-	-	34	-
Critical Hdwy	-	-	4.1	-	6.48	6.2
Critical Hdwy Stg 1	-	-	-	-	5.48	-
Critical Hdwy Stg 2	-		-	-	5.48	-
Follow-up Hdwy	-	-	2.2	-	3.572	3.3
Pot Cap-1 Maneuver	-	-	1388	-	797	908
Stage 1	-	-	-	-	868	-
Stage 2	-	-	-	-	973	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-		1388	-	794	908
Mov Cap-2 Maneuver	-	-	_	-	794	-
Stage 1	-	-	-	-	868	-
Stage 2	_	_	_	_	969	_
Jugo 2					505	
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		10.5	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
		800	LDI		1388	
Capacity (veh/h)						-
HCM Control Dolay (a)		0.181	-		0.004	-
HCM Control Delay (s) HCM Lane LOS		10.5	-	-	7.6	-
LICIVI LAME LUS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.7	_	_	0	_

Intersection						
Int Delay, s/veh	3.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	LDIX	VVDL Š		₩.	NOI
Traffic Vol, veh/h	146	187	5	154	138	51
Future Vol, veh/h	146	187	5	154	138	51
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storag		_	-	0	0	-
Grade, %	0, 11 0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	9	0	10	31	4
Mvmt Flow	146	187	5	154	138	51
WWINCTION	140	101	U	10-7	100	O I
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	333	0	404	240
Stage 1	-	-	-	-	240	-
Stage 2	-	-	-	-	164	-
Critical Hdwy	-	-	4.1	-	6.71	6.24
Critical Hdwy Stg 1	-	-	-	-	5.71	-
Critical Hdwy Stg 2	-	-	-	-	5.71	-
Follow-up Hdwy	-	-	2.2	-	•	3.336
Pot Cap-1 Maneuver	-	-	1238	-	550	794
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	799	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver		-	1238	-	548	794
Mov Cap-2 Maneuver	-	-	-	-	548	-
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	796	-
Approach	EB		WB		NB	
HCM Control Delay, s			0.2		13.8	
HCM LOS	0		0.2		В	
110111 200					U	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		598	-	-	1238	-
LICIAL and MIC Datio		0.246			0.004	

0.316

13.8

В

1.4

- 0.004

7.9

Α

0

HCM Lane V/C Ratio

HCM Lane LOS

HCM Control Delay (s)

HCM 95th %tile Q(veh)

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	f)		ች	ĵ.			4	7		4	
Traffic Vol, veh/h	1	134	31	167	121	4	20	1	198	1	1	0
Future Vol, veh/h	1	134	31	167	121	4	20	1	198	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	134	31	167	121	4	20	1	198	1	1	0
Major/Minor N	/lajor1		1	Major2		1	Minor1		N	Minor2		
Conflicting Flow All	125	0	0	165	0	0	610	611	150	708	624	123
Stage 1	-	-	-	-	-	-	152	152	-	457	457	-
Stage 2	-	-	-	-	-	-	458	459	-	251	167	-
Critical Hdwy	4.1	-	-	4.4	-	-	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Follow-up Hdwy	2.2	-	-	2.47	-	-	3.635	4	3.39	3.5	4.9	3.3
Pot Cap-1 Maneuver	1474	-	-	1260	-	-	388	411	876	352	297	933
Stage 1	-	-	-	-	-	-	821	775	-	587	432	-
Stage 2	-	-	-	-	-	-	559	570	-	758	608	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1474	-	-	1260	-	-	347	356	876	244	257	933
Mov Cap-2 Maneuver	-	-	-	-	-	-	347	356	-	244	257	-
Stage 1	-	-	-	-	-	-	820	774	-	586	375	-
Stage 2	-	-	-	-	-	-	484	494	-	586	607	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.7			10.8			19.5		
HCM LOS							В			С		
Minor Lane/Major Mvmt	t I	NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		347		1474	-		1260	-	-			
HCM Lane V/C Ratio			0.226		_		0.133	_		0.008		
HCM Control Delay (s)		16	10.3	7.4	_	_	8.3	_	-			
HCM Lane LOS		C	В	A	_	_	A	_	_	C		
HCM 95th %tile Q(veh)		0.2	0.9	0	_	_	0.5	_	-	0		
		V. <u>_</u>	0.0				3.0					

Intersection												
Int Delay, s/veh	5.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	ĵ.		ሻ	î,	
Traffic Vol, veh/h	19	8	13	11	9	67	8	124	30	146	49	4
Future Vol, veh/h	19	8	13	11	9	67	8	124	30	146	49	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	19	8	13	11	9	67	8	124	30	146	49	4
Major/Minor N	1inor2		ľ	Minor1		-	Major1		ľ	Major2		
Conflicting Flow All	536	513	51	509	500	139	53	0	0	154	0	0
Stage 1	343	343	-	155	155	-	-	-	-	-	-	-
Stage 2	193	170	-	354	345	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1	-	-	4.46	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	-	-	2.524	-	-
Pot Cap-1 Maneuver	459	468	1000	478	476	856	1566	-	-	1243	-	-
Stage 1	676	641	-	852	773	-	-	-	-	-	-	-
Stage 2	813	762	-	667	640	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	377	411	1000	422	418	856	1566	-	-	1243	-	-
Mov Cap-2 Maneuver	377	411	-	422	418	-	-	-	-	-	-	-
Stage 1	673	566	-	848	769	-	-	-	-	-	-	-
Stage 2	737	758	-	573	565	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.1			11			0.4			6.1		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1566	-	-	483	691	1243	-	-			
HCM Lane V/C Ratio		0.005	-	_		0.126		_	-			
HCM Control Delay (s)		7.3	-	-	13.1	11	8.3	-	-			
HCM Lane LOS		Α	-	-	В	В	Α	-	_			
HCM 95th %tile Q(veh)		0	-	-	0.3	0.4	0.4	-	-			
., .,												

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>1</b>			4
Traffic Vol, veh/h	2	0	29	0	0	294
Future Vol, veh/h	2	0	29	0	0	294
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage,		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	40	0	0	6
Mymt Flow	2	0	29	0	0	294
IVIVIII( I IOW		U	23	U	U	234
	/linor1		Major1		/lajor2	
Conflicting Flow All	323	29	0	0	29	0
Stage 1	29	-	-	-	-	-
Stage 2	294	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	675	1052	-	-	1597	-
Stage 1	999	-	-	-	-	-
Stage 2	761	-	-	-	-	-
Platoon blocked, %	-		-	-		-
Mov Cap-1 Maneuver	675	1052	-	-	1597	-
Mov Cap-2 Maneuver	675	-	_	_	-	_
Stage 1	999	-	_	_	_	_
Stage 2	761	<u>-</u>	_	_	_	_
Olugo Z	701					
Approach	WB		NB		SB	
HCM Control Delay, s	10.3		0		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		1401	- INDIX		1597	
HCM Lane V/C Ratio		-		0.003		-
		-			-	-
HCM Control Delay (s) HCM Lane LOS		-	-		0	-
HOIVI LAITE LUS		-	-	В	Α	-
HCM 95th %tile Q(veh)			_	0	0	_

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	43	0	0	253	0
Future Vol, veh/h	0	0	0	0	0	0	0	43	0	0	253	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	43	0	0	253	0
Major/Minor N	/linor2			Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	296	296	253	296	296	43	253	0	0	43	0	0
Stage 1	253	253	-	43	43	-	-	-	-	-	-	-
Stage 2	43	43	-	253	253	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	660	619	791	660	619	1033	1324	-	-	1579	-	-
Stage 1	756	701	-	976	863	-	-	-	-	-	-	-
Stage 2	976	863	-	756	701	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	660	619	791	660	619	1033	1324	-	-	1579	-	-
Mov Cap-2 Maneuver	660	619	-	660	619	-	-	-	-	-	-	-
Stage 1	756	701	-	976	863	-	-	-	-	-	-	-
Stage 2	976	863	-	756	701	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A								
Minor Lane/Major Mvm	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1324	_	_	_	_	1579	_	_			
HCM Lane V/C Ratio		-	_	_	_	_	-	_	_			
HCM Control Delay (s)		0	_	_	0	0	0	-	-			
HCM Lane LOS		A	_	_	A	A	A	_	_			
HCM 95th %tile Q(veh)		0	-	-		-	0	-	-			
							•					

Int Delay, s/veh
Care Configurations
Traffic Vol, veh/h         0         4         0         34         0         12         0         23         9         108         153         0           Future Vol, veh/h         0         4         0         34         0         12         0         23         9         108         153         0           Conflicting Peds, #/hr         0 <t< td=""></t<>
Traffic Vol, veh/h         0         4         0         34         0         12         0         23         9         108         153         0           Future Vol, veh/h         0         4         0         34         0         12         0         23         9         108         153         0           Conflicting Peds, #/hr         0 <t< td=""></t<>
Future Vol, veh/h Conflicting Peds, #/hr O O O O O O O O O O O O O O O O O O O
Conflicting Peds, #/hr         0
Sign Control         Stop         Stop         Stop         Stop         Stop         Stop         Stop         Free
RT Channelized         -         -         None         -         -         None         -         -         None           Storage Length         -
Veh in Median Storage, #         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         0         -         -         0         0         -         -         0         0         -         -         0         0         -         -         0         0         -         -         0         0         -         0         0         -         0
Veh in Median Storage, #         0         -         -         0         -         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         -         0         0         -         0         0         -         0         0         100 <th< td=""></th<>
Grade, %         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         100
Heavy Vehicles, %         0         0         0         6         0         44         0         39         33         16         13         0           Mymt Flow         0         4         0         34         0         12         0         23         9         108         153         0           Major/Minor         Minor1         Major1         Major2         Major2         10         0         30         0         0         32
Mount Flow         0         4         0         34         0         12         0         23         9         108         153         0           Major/Minor         Minor1         Major1         Major2         And a jor3         And a jor3
Major/Minor         Minor2         Minor1         Major1         Major2           Conflicting Flow All         403         401         153         399         397         28         153         0         0         32         0         0           Stage 1         369         369         -         28         28         -
Conflicting Flow All         403         401         153         399         397         28         153         0         0         32         0         0           Stage 1         369         369         -         28         28         -<
Conflicting Flow All         403         401         153         399         397         28         153         0         0         32         0         0           Stage 1         369         369         -         28         28         -<
Stage 1       369       369       -       28       28       -       <
Stage 2         34         32         -         371         369         -         <
Stage 2         34         32         -         371         369         -         <
Critical Hdwy       7.1       6.5       6.2       7.16       6.5       6.64       4.1       -       -       4.26       -       -         Critical Hdwy Stg 1       6.1       5.5       -       6.16       5.5       -
Critical Hdwy Stg 1       6.1       5.5       -       6.16       5.5       - <td< td=""></td<>
Critical Hdwy Stg 2       6.1       5.5       -       6.16       5.5       - <td< td=""></td<>
Follow-up Hdwy 3.5 4 3.3 3.554 4 3.696 2.2 2.344 Pot Cap-1 Maneuver 562 541 898 554 544 938 1440 1494 Stage 1 655 624 - 979 876 Stage 2 987 872 - 641 624 Platoon blocked, %
Stage 1       655       624       -       979       876       -
Stage 1       655       624       -       979       876       -
Stage 2       987       872       -       641       624       -
Mov Cap-1 Maneuver       522       498       898       517       501       938       1440       -       -       1494       -       -         Mov Cap-2 Maneuver       522       498       -       517       501       -       -       -       -       -       -       -
Mov Cap-2 Maneuver 522 498 - 517 501
Stage 1 655 575 - 979 876
Stage 2 974 872 - 586 575
Approach EB WB NB SB
HCM Control Delay, s 12.3 11.7 0 3.1
HCM LOS B B
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR
Capacity (veh/h) 1440 498 586 1494
HCM Lane V/C Ratio 0.008 0.078 0.072
HCM Control Delay (s) 0 12.3 11.7 7.6 0 -
HCM Lane LOS A B B A A -
HCM 95th %tile Q(veh) 0 0 0.3 0.2

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We	ekday	Morn	ina Pe	eak Hour
***	onaay		9 . 、	Jak i loui

Int Delay, s/veh						
	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	<u>₽</u>	
Traffic Vol. veh/h	0	0	1	15	110	7
Future Vol, veh/h	0	0	1	15	110	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		-	_	0	0	-
Grade, %	0	_	-	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	15	110	7
IVIVIIIL I IOW	U	U		10	110	I
Major/Minor	Minor2		Major1		/lajor2	
Conflicting Flow All	131	114	117	0	-	0
Stage 1	114	-	-	-	-	-
Stage 2	17	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	_	-	-	_	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	868	944	1484	-	_	-
Stage 1	916	-		_	_	_
Stage 2	1011	_	_	_	-	_
Platoon blocked, %	.511			_	_	_
Mov Cap-1 Maneuver	867	944	1484	_	_	_
Mov Cap-1 Maneuver		-	-	_	_	_
Stage 1	915	_	_	_		_
Stage 2	1011	-	_	_	_	_
Staye 2	1011	-	-	-	_	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.5		0	
HCM LOS	A		3.0			
	,,					
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1484	-	-	-	-
HCM Lane V/C Ratio		0.001	-	-	-	-
HCM Control Delay (s	)	7.4	0	0	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh	1)	0	-	-	-	-

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	EDL		EDK	VVDL		WDIN	INDL		INDIX	SDL		SDR
Lane Configurations	4	- ♣	٥	٥	- ♣	0	۸	4	٥	1	400	0
Traffic Vol., veh/h	1	0	0	0	0	0	0	15	0	1	109	0
Future Vol, veh/h	1	0	0	0	0	0	0	15	0	1 0	109	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0		0	0
Sign Control RT Channelized	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length		_	-	-	-	-	-	_	-	-	-	-
Veh in Median Storage,		0	-	-	0	-	-	0	-	-	0	-
Grade, %	100	100	100	100	100	100	100	100	100	100	100	100
Peak Hour Factor				100	100	100		100	100			100
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow		0	0	0	0	0	0	15	0		109	0
Major/Minor N	/linor2			Minor1		<u> </u>	//ajor1		<u> </u>	Major2		
Conflicting Flow All	126	126	109	126	126	15	109	0	0	15	0	0
Stage 1	111	111	-	15	15	-	-	-	-	-	-	-
Stage 2	15	15	-	111	111	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	852	768	950	852	768	1070	1494	-	-	1616	-	-
Stage 1	899	807	-	1010	887	-	-	-	-	-	-	-
Stage 2	1010	887	-	899	807	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	851	767	950	851	767	1070	1494	-	-	1616	-	-
Mov Cap-2 Maneuver	851	767	-	851	767	-	-	-	-	-	-	-
Stage 1	899	806	-	1010	887	-	-	-	-	-	-	-
Stage 2	1010	887	-	898	806	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.2			0			0			0.1		
HCM LOS	Α.Δ			A			U			U. I		
I IOW LOO	Α											
N.C. 1 (0.4.1. N.1.		MBI	Not	NDD.	-DI 41	VDL 4	05:	057	000			
Minor Lane/Major Mvmt	τ	NBL	NBT		EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1494	-	-	•••	-		-	-			
HCM Lane V/C Ratio		-	-		0.001		0.001	-	-			
HCM Control Delay (s)		0	-	-	9.2	0	7.2	0	-			
HCM Lane LOS		A	-	-	A	Α	Α	Α	-			
HCM 95th %tile Q(veh)		0	-	-	0	-	0	-	-			

Intersection						
Int Delay, s/veh	5.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	WDIX	\$	NDIX	ODL	<u>લ</u>
Traffic Vol, veh/h	37	26	9	65	142	23
Future Vol, veh/h	37	26	9	65	142	23
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	3	0	22	0	22	0
Mvmt Flow	37	26	9	65	142	23
	Minor1		//ajor1		Major2	
Conflicting Flow All	349	42	0	0	74	0
Stage 1	42	-	-	-	-	-
Stage 2	307	_	-	-	-	-
Critical Hdwy	6.43	6.2	_	_	4.32	-
Critical Hdwy Stg 1	5.43	-	_	_		_
Critical Hdwy Stg 2	5.43	_	_	_	_	_
Follow-up Hdwy	3.527	3.3	_	_	2.398	_
	646	1034			1408	
Pot Cap-1 Maneuver			-	-	1400	-
Stage 1	978	-	-	-	-	-
Stage 2	744	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	580	1034	-	-	1408	-
Mov Cap-2 Maneuver	580	-	-	-	-	-
Stage 1	978	-	-	-	-	-
Stage 2	668	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.6		0		6.7	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NRDV	VBLn1	SBL	SBT
	IC .					
Capacity (veh/h)		-	-	708	1408	-
HCM Lane V/C Ratio		-	-	0.089		-
HCM Control Delay (s)		-	-	10.6	7.8	0
HCM Lane LOS		-	-	В	Α	Α
HCM 95th %tile Q(veh	)	-	-	0.3	0.3	-

Intersection Int Delay, s/veh  2.7  Movement  EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
MOVEMENT EDL EDL EDR WELL WELL WELL INDI INDR SBL SBI SBR
Lane Configurations & d 7 7 4
Lane Configurations
Future Vol, veh/h 12 8 0 1 97 137 0 1 0 53 0 30
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0
Sign Control Free Free Free Free Free Stop Stop Stop Stop Stop
RT Channelized None None None
Storage Length 200
Veh in Median Storage, # - 0 0 0 0
Grade, % - 0 0 0 0
Peak Hour Factor 100 100 100 100 100 100 100 100 100 10
Heavy Vehicles, % 58 0 0 0 8 10 0 0 19 0 10
Mvmt Flow 12 8 0 1 97 137 0 1 0 53 0 30
Major/Minor Major1 Major2 Minor1 Minor2
Conflicting Flow All 234 0 0 8 0 0 215 268 8 132 131 97
Stage 1 32 32 - 99 99 -
Stage 2 183 236 - 33 32 -
Critical Hdwy 4.68 4.1 7.1 6.5 6.2 7.29 6.5 6.3
Critical Hdwy Stg 1 6.1 5.5 - 6.29 5.5 -
Critical Hdwy Stg 2 6.1 5.5 - 6.29 5.5 -
Follow-up Hdwy 2.722 2.2 3.5 4 3.3 3.671 4 3.39
Pot Cap-1 Maneuver 1065 1625 746 641 1080 802 763 938
Stage 1 990 872 - 867 817 -
Stage 2 823 713 - 941 872 -
Platoon blocked, %
Mov Cap-1 Maneuver 1065 1625 715 633 1080 794 754 938
Mov Cap-2 Maneuver 715 633 - 794 754 -
Stage 1 979 862 - 857 816 -
Stage 2 796 712 - 930 862 -
Approach EB WB NB SB
HCM Control Delay, s 5.1 0 10.7 9.7
HCM LOS B A
<u>D</u> //
Miner Lene/Major Mumt NDL n4 FDL FDT FDD W/DL W/DT W/DD CDL n4
Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1
Capacity (veh/h) 633 1065 1625 841
HCM Lane V/C Ratio 0.002 0.011 0.001 0.099
HCM Control Delay (s) 10.7 8.4 0 - 7.2 0 - 9.7
HCM Lane LOS B A A - A A - A
HCM 95th %tile Q(veh) 0 0 0 0.3

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.		ች	ĵ.			4			4	
Traffic Vol, veh/h	1	36	1	11	342	93	4	0	9	72	2	7
Future Vol, veh/h	1	36	1	11	342	93	4	0	9	72	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	-	-	None	_	-	None	-		None
Storage Length	75	-	-	75	-	-	-	_	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	36	1	11	342	93	4	0	9	72	2	7
Major/Minor N	1ajor1		ľ	Major2		1	Minor1		ľ	Minor2		
Conflicting Flow All	435	0	0	37	0	0	454	496	37	454	450	389
Stage 1	-	-	-	-	-	-	39	39	-	411	411	-
Stage 2	_	-	-	-	-	-	415	457	-	43	39	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4	3.3	3.581	4	3.561
Pot Cap-1 Maneuver	752	-	-	1587	-	-	520	478	1041	505	508	604
Stage 1	-	-	-	-	-	-	981	866	-	604	598	-
Stage 2	-	-	-	-	-	-	619	571	-	954	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	752	-	-	1587	-	-	509	474	1041	497	504	604
Mov Cap-2 Maneuver	-	-	-	-	-	-	509	474	-	497	504	-
Stage 1	-	-	-	-	-	-	980	865	-	603	594	-
Stage 2	-	-	-	-	-	-	606	567	-	944	865	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			9.6			13.5		
HCM LOS							Α			В		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		788	752	_		1587	_	-	505			
HCM Lane V/C Ratio		0.016		_		0.007	_	_	0.16			
HCM Control Delay (s)		9.6	9.8	_	_	7.3	_	_				
HCM Lane LOS		Α.	Α	_	_	Α	_	_	В			
HCM 95th %tile Q(veh)		0.1	0	-	_	0	_	_	0.6			
		V. 1							0.0			

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			4						4	
Traffic Vol, veh/h	0	10	89	8	364	0	0	0	0	2	2	98
Future Vol, veh/h	0	10	89	8	364	0	0	0	0	2	2	98
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	10	89	8	364	0	0	0	0	2	2	98
Major/Minor N	1ajor1		N	Major2					N	/linor2		
Conflicting Flow All		0	0	99	0	0				435	479	364
Stage 1	_	_	_	_	_	_				380	380	-
Stage 2	-	-	-	-	-	-				55	99	-
Critical Hdwy	-	-	-	4.85	-	-				6.4	7	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	_	-				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1140	_	0				582	422	655
Stage 1	0	-	-	-	-	0				696	538	-
Stage 2	0	-	-	-	-	0				973	729	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1140	-	-				577	0	655
Mov Cap-2 Maneuver	-	-	-	-	-	-				577	0	-
Stage 1	-	-	-	-	-	-				696	0	-
Stage 2	-	-	-	-	-	-				964	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						11.5		
HCM LOS				J.L						В		
Minor Lane/Major Mvmt		EBT	EBR	WBL	WRT	SBLn1						
				1140								
Capacity (veh/h) HCM Lane V/C Ratio		-		0.007	-	0.156						
		-	-		0	11.5						
HCM Control Delay (s) HCM Lane LOS		-		0.2 A	A	11.5 B						
HCM 95th %tile Q(veh)		-	-	0	- -	0.6						
HOW JOHN JOHN (VEH)		_	_	U	-	0.0						

Intersection												
Int Delay, s/veh	11											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LDIT	1100	<b>1</b>	11511	1102	4	TIDIT.	ODL	051	OBIT
Traffic Vol, veh/h	9	3	0	0	4	1	368	5	23	0	0	0
Future Vol, veh/h	9	3	0	0	4	1	368	5	23	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	14	0	0	0	75	0	10	80	26	2	2	2
Mvmt Flow	9	3	0	0	4	1	368	5	23	0	0	0
Major/Minor N	Major1		ľ	Major2		N	/linor1					
Conflicting Flow All	5	0	-	-	-	0	26	26	3			
Stage 1	-	-	-	-	-	-	21	21	-			
Stage 2	-	-	-	-	-	-	5	5	-			
Critical Hdwy	4.24	-	-	-	-	-	6.5	7.3	6.46			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.5	6.3	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.5	6.3	-			
Follow-up Hdwy	2.326	-	-	-	-	-	3.59	4.72	3.534			
Pot Cap-1 Maneuver	1541	-	0	0	-	-	969	736	1015			
Stage 1	-	-	0	0	-	-	981	745	-			
Stage 2	-	-	0	0	-	-	998	759	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1541	-	-	-	-	-	963	0	1015			
Mov Cap-2 Maneuver	-	-	-	-	-	-	963	0	-			
Stage 1	-	-	-	-	-	-	975	0	-			
Stage 2	-	-	-	-	-	-	998	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	5.5			0			11.3					
HCM LOS							В					
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		966	1541	-	-	-						
HCM Lane V/C Ratio			0.006	-	-	-						
HCM Control Delay (s)		11.3	7.3	0	_	-						
HCM Lane LOS		В	Α	Α	-	-						
HCM 95th %tile Q(veh)		2	0	-	-	-						

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.			र्स						4	
Traffic Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Future Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	0	0	50	0	2	2	2	10	83	0
Mvmt Flow	0	5	2	3	11	0	0	0	0	10	6	3
Major/Minor N	1ajor1			/lajor2					N	/linor2		
Conflicting Flow All	-	0	0	7	0	0				23	24	11
Stage 1	-	-	-	-	-	-				17	17	-
Stage 2	-	-	-	-	-	-				6	7	-
Critical Hdwy	-	-	-	4.1	-	-				6.5	7.33	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.5	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.5	6.33	-
Follow-up Hdwy	-	-	-	2.2	-	-				3.59	4.747	3.3
Pot Cap-1 Maneuver	0	-	-	1627	-	0				973	734	1076
Stage 1	0	-	-	-	-	0				985	744	-
Stage 2	0	-	-	-	-	0				997	753	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1627	-	-				971	0	1076
Mov Cap-2 Maneuver	-	-	-	-	-	-				971	0	-
Stage 1	-	-	-	-	-	-				985	0	-
Stage 2	-	-	-	-	-	-				995	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			1.5						8.7		
HCM LOS										Α		
Minor Lane/Major Mvmt		EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)				1627	-	993						
HCM Lane V/C Ratio		_	_	0.002		0.019						
HCM Control Delay (s)		_	_	7.2	0	8.7						
HCM Lane LOS		_	_	Α	A	A						
HCM 95th %tile Q(veh)		-	_	0	-	0.1						
						<b>J</b> .,						

	ICM 6th TWS 6: I-82 Northb
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Ir	tersection
Īr	t Delay_s/veh

Intersection													
Int Delay, s/veh	2.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स			<del>(</del> Î			4					
Traffic Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0	
Future Vol, veh/h	0	15	0	0	14	4	0	5	8	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2	
Mvmt Flow	0	15	0	0	14	4	0	5	8	0	0	0	
Major/Minor N	1ajor1		N	Major2			Minor1						
Conflicting Flow All	18	0	-	-	-	0	31	33	15				
Stage 1	-	-	-	-	-	-	15	15	-				
Stage 2	-	-	-	-	-	-	16	18	-				
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.3	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-				
Follow-up Hdwy	2.2	-	-	-	-	-	3.5	4.72	3.408				
Pot Cap-1 Maneuver	1612	-	0	0	-	-	988	729	1036				
Stage 1	-	-	0	0	-	-	1013	750	-				
Stage 2	-	-	0	0	-	-	1012	748	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1612	-	-	-	-	-	988	0	1036				
Mov Cap-2 Maneuver	-	-	-	-	-	-	988	0	-				
Stage 1	-	-	-	-	-	-	1013	0	-				
Stage 2	-	-	-	-	-	-	1012	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	0			0			8.5						
HCM LOS							Α						
Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		1036	1612	-	-	-							
HCM Lane V/C Ratio		0.013	-	-	-	-							
HCM Control Delay (s)		8.5	0	-	-	-							
HCM Lane LOS		Α	Α	-	-	-							

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Future Vol, veh/h	0	21	2	0	14	0	2	0	0	0	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	13	0	0	21	0	0	0	0	0	0	50
Mvmt Flow	0	21	2	0	14	0	2	0	0	0	0	2
Major/Minor N	/lajor1		ľ	Major2		<u> </u>	/linor1		N	/linor2		
Conflicting Flow All	14	0	0	23	0	0	37	36	22	36	37	14
Stage 1	-	-	-	-	-	-	22	22	-	14	14	-
Stage 2	-	-	-	-	-	-	15	14	-	22	23	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.7
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	_	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.75
Pot Cap-1 Maneuver	1617	-	-	1605	-	-	973	860	1061	975	859	942
Stage 1	-	-	-	-	-	-	1002	881	-	1011	888	-
Stage 2	-	-	-	-	-	-	1010	888	-	1002	880	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1617	-	-	1605	-	-	971	860	1061	975	859	942
Mov Cap-2 Maneuver	-	-	-	-	-	-	971	860	-	975	859	-
Stage 1	-	-	-	-	-	-	1002	881	-	1011	888	-
Stage 2	-	-	-	-	-	-	1008	888	-	1002	880	-
_												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			8.7			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvmt	t1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		971	1617	-	-	1605	-	-	942			
HCM Lane V/C Ratio		0.002	-	-	-	-	-	-	0.002			
HCM Control Delay (s)		8.7	0	-	-	0	-	-	8.8			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection												
Int Delay, s/veh	7.7											
• •												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₽			- 4						4	
Traffic Vol, veh/h	0	33	5	116	25	0	0	0	0	66	1	125
Future Vol, veh/h	0	33	5	116	25	0	0	0	0	66	1	125
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	25	0	6	0	0	2	2	2	3	0	0
Mvmt Flow	0	33	5	116	25	0	0	0	0	66	1	125
Major/Minor N	Major1		ı	Major2					N	/linor2		
Conflicting Flow All	viajui i -	0	0	38	0	0			l'	293	295	25
Stage 1	-	-	-	J0 -	-	-				257	257	25
Stage 1		-		_	-	-				36	38	-
Critical Hdwy	-	-	-	4.16	-	-				6.43	6.5	6.2
		-								5.43	5.5	
Critical Hdwy Stg 1	-	-	_	-	-	-				5.43	5.5	-
Critical Hdwy Stg 2			-	2.254		-				3.527	5.5	3.3
Follow-up Hdwy Pot Cap-1 Maneuver	-	-		1547	-	0				696	620	1057
•	0	-	-		-							
Stage 1	0	-	-	-	-	0				784 984	699 867	-
Stage 2	U	-	-	-	-	U				904	007	-
Platoon blocked, %		-	-	1547	-	_				643	0	1057
Mov Cap-1 Maneuver	-	-	-		-							
Mov Cap-2 Maneuver	-	-	-	-	-	-				643	0	-
Stage 1	-	-	-	-	-	-				784	0	-
Stage 2	-	-	-	-	-	-				909	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.2						10.3		
HCM LOS										В		
Minor Long /Maiss NA		EDT	EDD	WDI	MOT	CDL 4						
Minor Lane/Major Mvm	l	EBT	EBR	WBL	WBI	SBLn1						
Capacity (veh/h)		-	-	1547	-	865						
HCM Lane V/C Ratio		-	-	0.075		0.222						
HCM Control Delay (s)		-	-	7.5	0	10.3						
HCM Lane LOS		-	-	Α	Α	В						
HCM 95th %tile Q(veh)		-	-	0.2	-	0.8						

Intersection													
Int Delay, s/veh	1.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			ĵ.			4					
Traffic Vol, veh/h	17	82	0	0	120	85	21	2	9	0	0	0	
Future Vol, veh/h	17	82	0	0	120	85	21	2	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	,# -	0	_	-	0	-	_	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2	
Mvmt Flow	17	82	0	0	120	85	21	2	9	0	0	0	
Major/Minor N	/lajor1			Major2		N	Minor1						
Conflicting Flow All	205	0	_	-	_	0	279	321	82				
Stage 1		-	-	-	_	-	116	116	-				
Stage 2	_	_	_	_	_	_	163	205	_				
Critical Hdwy	4.67	_	_	_	_	_	6.4	7.5	6.53				
Critical Hdwy Stg 1	_	-	_	_	-	_	5.4	6.5	-				
Critical Hdwy Stg 2	-	_	_	_	_	-	5.4	6.5	-				
	2.713	-	-	-	-	-	3.5		3.597				
Pot Cap-1 Maneuver	1098	-	0	0	-	-	715	465	898				
Stage 1	_	-	0	0	-	-	914	644	_				
Stage 2	-	-	0	0	-	-	871	581	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1098	-	-	-	-	-	704	0	898				
Mov Cap-2 Maneuver	-	-	-	-	-	-	704	0	-				
Stage 1	-	-	-	-	-	-	899	0	-				
Stage 2	_	-	-	_	-	-	871	0	_				
, and the second													
Approach	EB			WB			NB						
HCM Control Delay, s	1.4			0			10						
HCM LOS	17			- 0			В						
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		753	1098	_									
HCM Lane V/C Ratio		0.042		_	_	<u>-</u>							
HCM Control Delay (s)		10	8.3	0	_	_							
HCM Lane LOS		В	Α	A	_	_							
I IOIVI LUIIO LOO		ט		$\overline{}$									

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	87	2	1	201	0	1	0	0	1	0	3
Future Vol, veh/h	3	87	2	1	201	0	1	0	0	1	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	_	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	38	50	0	21	0	0	0	0	0	0	67
Mvmt Flow	3	87	2	1	201	0	1	0	0	1	0	3
Major/Minor N	Major1			Major2		N	Minor1		N	/linor2		
Conflicting Flow All	201	0	0	89	0	0	299	297	88	297	298	201
Stage 1		-	-	-	-	-	94	94	-	203	203	
Stage 2	-	_	_	_	_	_	205	203	_	94	95	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.87
Critical Hdwy Stg 1	_	_	-	_	_	-	6.1	5.5	_	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.903
Pot Cap-1 Maneuver	1206	-	-	1519	-	-	657	618	976	659	617	699
Stage 1	-	-	-	-	-	-	918	821	-	804	737	-
Stage 2	-	-	-	-	-	-	802	737	-	918	820	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1206	-	-	1519	-	-	652	616	976	657	615	699
Mov Cap-2 Maneuver	-	-	-	-	-	-	652	616	-	657	615	-
Stage 1	-	-	-	-	-	-	915	819	-	802	736	-
Stage 2	-	-	-	-	-	-	798	736	-	915	818	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0			10.5			10.3		
HCM LOS	0.0						В			В		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		652	1206	-	_	1519	-	-	688			
HCM Lane V/C Ratio		0.002		-		0.001	-	-	0.006			
HCM Control Delay (s)		10.5	8	0	-	7.4	0	-				
HCM Lane LOS		В	A	A	-	Α	A	-	В			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			

Intersection							
Int Delay, s/veh	5.7						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
				MOL			
Lane Configurations	ዃ	10	<b>}</b>	15	<u>ነ</u>	120	
Traffic Vol, veh/h	6	19	49	15	11	128	
Future Vol, veh/h	6	19	49	15	11	128	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-		
Storage Length	180	-	-	-	100	0	
Veh in Median Storage	, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	0	5	7	0	0	2	
Mvmt Flow	6	19	49	15	11	128	
Major/Minor	Major1		/laior2	N	/liner?		
	Major1		Major2		Minor2		
Conflicting Flow All	64	0	-	0	88	57	
Stage 1	-	-	-	-	57	-	
Stage 2	-	-	-	-	31	-	
Critical Hdwy	4.1	-	-	-	6.4	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.2	-	-	-		3.318	
Pot Cap-1 Maneuver	1551	-	-	-	918	1009	
Stage 1	-	-	-	-	971	-	
Stage 2	-	-	-	-	997	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1551	-	-	-	914	1009	
Mov Cap-2 Maneuver	-	-	-	-	914	-	
Stage 1	-	-	_	-	967	-	
Stage 2	_	_	_	_	997	_	
2.5.30 -							
Approach	EB		WB		SB		
HCM Control Delay, s	1.8		0		9.1		
HCM LOS					Α		
Mineral and Marin PA		EDI	EDT	MOT	MDD	ODL 4	ODI - 0
Minor Lane/Major Mvm	ΙŢ	EBL	EBT	WBT	WRK :	SBLn1	
Capacity (veh/h)		1551	-	-	-		1009
HCM Lane V/C Ratio		0.004	-	-	-	0.012	
HCM Control Delay (s)		7.3	-	-	-	9	9.1
HCM Lane LOS		Α	-	-	-	Α	Α
HCM 95th %tile Q(veh)	)	0	-	-	-	0	0.4

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		ሻ	ĵ.			4			4	
Traffic Vol, veh/h	15	36	7	1	28	1	1	1	1	4	11	32
Future Vol, veh/h	15	36	7	1	28	1	1	1	1	4	11	32
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	13	31	0	100	33	0	0	100	100	0	0	16
Mvmt Flow	15	36	7	1	28	1	1	1	1	4	11	32
Major/Minor	Major1		ı	Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	29	0	0	43	0	0	122	101	40	101	104	29
Stage 1	-	-	-	-	-	-	70	70	-	31	31	-
Stage 2	_	-	_	_	_	_	52	31	_	70	73	-
Critical Hdwy	4.23	-	-	5.1	_	_	7.1	7.5	7.2	7.1	6.5	6.36
Critical Hdwy Stg 1		-	_	-	_	_	6.1	6.5	-	6.1	5.5	-
Critical Hdwy Stg 2	_	-	-	-	_	_	6.1	6.5	-	6.1	5.5	-
Follow-up Hdwy	2.317	-	_	3.1	_	_	3.5	4.9	4.2	3.5	4	3.444
Pot Cap-1 Maneuver	1516	-	-	1113	_	_	858	637	810	885	790	1007
Stage 1	-	-	_	-	_	_	945	679	-	991	873	-
Stage 2	_	-	-	-	_	_	966	709	-	945	838	-
Platoon blocked, %		-	_		_	_						
Mov Cap-1 Maneuver	1516	-	-	1113	-	-	815	630	810	875	781	1007
Mov Cap-2 Maneuver	-	-	_	-	_	_	815	630	-	875	781	-
Stage 1	-	-	-	-	-	-	936	672	-	981	872	-
Stage 2	-	-	-	-	-	-	923	708	-	933	830	-
<u></u>												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.9			0.3			8.4			9.1		
HCM LOS	1.0			3.0			A			A		
							, ,			, ,		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		1066	1516			1113	-	-	932			
HCM Lane V/C Ratio		0.003	0.01	<u>-</u>		0.001	_	_	0.05			
HCM Control Delay (s)		8.4	7.4	_	_	8.2	_	_	9.1			
HCM Lane LOS		Α	Α.	_	<u>-</u>	Α	_	<u>-</u>	Α			
HCM 95th %tile Q(veh)	)	0	0	_	_	0	_	_	0.2			
1.13111 00til 70tilo Q(VOII)			- 0						J.L			

Intersection						
Int Delay, s/veh	0.8					
	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				INDK
Lane Configurations	<b>}</b>	2	<u>ነ</u>	<b>†</b>	¥	٨
Traffic Vol, veh/h	37	3	8	28	0	0
Future Vol, veh/h	37	3	8	28	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	33	0	13	0	0
Mvmt Flow	37	3	8	28	0	0
Major/Minor Ma	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	40	0	83	39
Stage 1	-	-	-	-	39	-
Stage 2	_	_	_	_	44	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
	_	-		-	5.4	0.2
Critical Hdwy Stg 1	-	_	-	-		
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1583	-	924	1038
Stage 1	-	-	-	-	989	-
Stage 2	-	-	-	-	984	-
Platoon blocked, %	-	-	1500	-	0.40	1000
Mov Cap-1 Maneuver	-	-	1583	-	919	1038
Mov Cap-2 Maneuver	-	-	-	-	919	-
Stage 1	-	-	-	-	989	-
Stage 2	-	-	-	-	979	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		0	
HCM LOS	U		1.0		A	
TION LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1583	-
HCM Lane V/C Ratio		-	-	-	0.005	-
HCM Control Delay (s)		0	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		-	-	-	0	-
2(1311)						

0					
	WDD	NDT	NDD	CDI	CDT
	WBK		NRK	SBL	SBT
	0		4	. 0	<del>ન</del>
			•	-	14
					14
					0
					Free
					None
					-
•			-		0
			-	-	0
					100
					14
0	0	0	1	0	14
Minor1	N	Major1	N	Major2	
					0
	-	-	-	-	-
	_	_	_	_	_
	6.2		_	<b>4</b> 1	_
		_	_	T. I	_
		_	_	_	_
			_		_
					_
		_	_	1000	_
		-	-	-	_
1014	_	-	_	-	
1000	1000	-	-	4005	-
		-	-		-
		-	-		-
		-	-	-	-
1014	-	-	-	-	-
WB		NB		SB	
				0	
0		()		•	
0 A		0			
0 A		U			
Α					
	NBT		VBLn1	SBL	SBT
Α	NBT -		VBLn1 -		SBT -
A It	NBT - -	NBRV	-	1635 -	
Α	-	NBRV -	- - 0	1635 - 0	-
A It	-	NBRV - -	-	1635 -	- -
	WBL 0 0 0 Stop - 0 100 0 100 0 4,# 0 0 100 0 0 Minor1 15 1 4 6.4 5.4 5.4 3.5 1009 1028 1014 1009 1028 1014	WBL WBR  0 0 0 0 0 0 0 0 0 Stop Stop - None 0 - 1,# 0 - 100 100 0 0 0  Minor1   N 15 1 1 - 14 - 6.4 6.2 5.4 - 5.4 - 3.5 3.3 1009 1090 1028 - 1014 - 1009 1090 1028 - 1014 -	WBL WBR NBT  0 0 0 0 0 0 0 0 0 0 0 0 0 Stop Stop Free - None - None 0 0 0 0 100 100 100 0 0 0 0 0 0  Minor1 Major1  15 1 0 1 14 14 5.4 5.4 5.4 5.4 1014 1009 1090 - 1008 1009 1090 - 1009 1028 1014 1009 1090 - 1028 1014 1009 1090 - 1028 1014	WBL         WBR         NBT         NBR           0         0         0         1           0         0         0         1           0         0         0         0           0         0         0         0           0         -         -         None           0         -         -         -           0         -         0         -           100         100         100         100           0         0         0         0           0         0         0         0           0         0         0         0           0         0         0         0           0         0         0         0           0         0         0         0           0         0         0         0           10         0         0         0           10         0         0         0           10         1         -         -           10         1         -         -           10         1         -         -           10 </td <td>WBL         WBR         NBT         NBR         SBL           VY         Image: Control of the control of t</td>	WBL         WBR         NBT         NBR         SBL           VY         Image: Control of the control of t

Intersection						
Int Delay, s/veh	0.5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	^	^	र्	<b>∱</b>	^
Traffic Vol, veh/h	1	0	0	0	18	0
Future Vol, veh/h	1	0	0	0	18	0
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	11	0
Mvmt Flow	1	0	0	0	18	0
Major/Minor N	Minor2	N	/lajor1	N	/lajor2	
Conflicting Flow All	18	18	18	0	//ajuiz -	Λ
						0
Stage 1	18	-	-	-	-	-
Stage 2	0	- 6.0	- 1 1	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	1005	1066	1612	-	-	-
Stage 1	1010	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	1005	1066	1612	-	-	-
Mov Cap-2 Maneuver	1005	-	-	-	-	-
Stage 1	1010	-	-	-	-	-
Stage 2	-	-	-	-	-	-
-						
Annroach	EB		NB		SB	
Approach						
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBL	NBTI	EBLn1	SBT	SBR
Capacity (veh/h)		1612	_	1005	_	
HCM Lane V/C Ratio		-		0.001	_	_
HCM Control Delay (s)		0	_	8.6	_	_
HCM Lane LOS		A	_	A	_	_
HCM 95th %tile Q(veh)		0	_	0	_	_
How Jour Joure Q(Ver)		U		U		

Intersection						
Int Delay, s/veh	4.4					
<u> </u>		EDD	NDI	NDT	CDT	SBR
Movement	EBL	EBR	NBL	NBT	SBT	SBK
Lane Configurations	¥	40	-	4	<b>₽</b>	7
Traffic Vol, veh/h	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	12	5	1	9	7
Major/Minor N	Minor2		Major1		Major2	
Conflicting Flow All	24	13	16	0	-	0
Stage 1	13	-	-	-	-	-
Stage 2	11	-	-	-	-	-
Critical Hdwy	6.4	6.28	4.5	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.372	2.56	-	-	-
Pot Cap-1 Maneuver	997	1050	1386	-	-	-
Stage 1	1015	-	-	-	-	-
Stage 2	1017	_	-	-	-	-
Platoon blocked, %				-	_	-
Mov Cap-1 Maneuver	993	1050	1386	_	-	-
Mov Cap-2 Maneuver	993	-	-	_	_	_
Stage 1	1011	_	_	_	_	_
Stage 2	1017	<u>-</u>	_	_	_	_
Olage 2	1017					
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
			NDT	ΓDI1	CDT	CDD
Minor Long/Mair - M	.1	NID1		EBLn1	SBT	SBR
Minor Lane/Major Mvm	nt	NBL				
Capacity (veh/h)	nt	1386	-	1041	-	-
Capacity (veh/h) HCM Lane V/C Ratio		1386 0.004	-	1041 0.013	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1386 0.004 7.6	- - 0	1041 0.013 8.5		- -
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS		1386 0.004 7.6 A	-	1041 0.013 8.5 A	-	
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1386 0.004 7.6	- - 0	1041 0.013 8.5	-	-

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	9	15	1	0	131	2	2	2	1	1	0	111
Future Vol, veh/h	9	15	1	0	131	2	2	2	1	1	0	111
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	9	15	1	0	131	2	2	2	1	1	0	111
Major/Minor N	Major1			Major2		<u> </u>	Minor1		<u> </u>	Minor2		
Conflicting Flow All	133	0	0	16	0	0	222	167	16	167	166	132
Stage 1	-	-	-	-	-	-	34	34	-	132	132	-
Stage 2	-	-	-	-	-	-	188	133	-	35	34	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.516
Pot Cap-1 Maneuver	1281	-	-	1615	-	-	738	729	1069	802	730	862
Stage 1	-	-	-	-	-	-	987	871	-	876	791	-
Stage 2	-	-	-	-	-	-	818	790	-	986	871	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1281	-	-	1615	-	-	640	724	1069	796	725	862
Mov Cap-2 Maneuver	-	-	-	-	-	-	640	724	-	796	725	-
Stage 1	-	-	-	-	-	-	980	865	-	870	791	-
Stage 2	-	-	-	-	-	-	713	790	-	976	865	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.8			0			9.9			9.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		733		-		1615	-	-	861			
HCM Lane V/C Ratio		0.007		-	-	-	_	-	0.13			
HCM Control Delay (s)		9.9	7.8	0	-	0	-	-	9.8			
HCM Lane LOS		Α	A	A	-	A	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0.4			

Intersection						
Int Delay, s/veh	1.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
		WDK		NDK	ODL	
Lane Configurations	Y	40	<b>}</b>	0	45	<u>र्</u> स
Traffic Vol, veh/h	5	18	32	2	15	108
Future Vol, veh/h	5	18	32	2	15	108
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None		None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	6	21	0	12	13
Mvmt Flow	5	18	32	2	15	108
Major/Minor N	1inor1	N.	Major1	The state of the s	Major2	
	171	33		0	34	0
Conflicting Flow All			0		34	
Stage 1	33	-	-	-	-	-
Stage 2	138	-	-	-	4.00	-
Critical Hdwy	6.6	6.26	-	-	4.22	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy		3.354	-		2.308	-
Pot Cap-1 Maneuver	779	1029	-	-	1515	
Stage 1	945	-	-	-	-	-
Stage 2	846	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	770	1029	-	-	1515	-
Mov Cap-2 Maneuver	770	-	-	-	-	-
Stage 1	945	-	-	-	-	-
Stage 2	837	-	-	-	-	-
<u> </u>						
Annroach	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		0.9	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NBRV	WBLn1	SBL	SBT
Capacity (veh/h)			-		1515	-
HCM Lane V/C Ratio		-		0.024	0.01	_
HCM Control Delay (s)		_	_	8.8	7.4	0
HCM Lane LOS		_	<u>-</u>	Α	7.4 A	A
		-	-	$\overline{}$	$\neg$	$\overline{}$
HCM 95th %tile Q(veh)		_	_	0.1	0	_

0 EBT	EBR	WBL	WBT	NDI	
ĵ.	EBR	WBL	WRT	NDI	
ĵ.				NBL	NBR
			4	Y	H.SIK
4	0	0	4	0	0
4	0	0	4	0	0
0		0	0	0	0
Free	Free	Free	Free	Stop	Stop
-					None
_	-	_			-
e. # 0	_	-			-
		_			_
					100
					2
					0
7	J		<b>-</b>		
0	0	4	0		4
-	-	-	-	4	-
-	-	-	-	4	-
-	-	4.12	-	6.42	6.22
-	-	-	-	5.42	-
-	-	-	-	5.42	-
-	-	2.218	-	3.518	3.318
-	-	1618	-	1013	1080
-	-	-	-	1019	-
-	-	-	-	1019	-
_	_		_		
_		1618	_	1013	1080
		-	_		-
_		_			_
					_
				1010	_
EB		WB		NB	
0		0		0	
				Α	
<b>.</b>	NIDI -1	EDT	EDD	WDI	WDT
IL I	INDLIII				WBT
	-	-			-
	-	-	-	-	-
	0	-	-	0	-
	-				
)	Α	-	-	A 0	-
		Section   Sect	- None	- None - None 0 0 0 100 100 100 100 2 2 2 2 2 4 0 0 4  Major1 Major2 0 0 0 4 0	- None - None - O O O O O O O O O O O O O O O O O O

Intersection						
Int Delay, s/veh	3.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	LDL			WDK		אמט
Lane Configurations	000	4	<b>\$</b>	405	À	7
Traffic Vol, veh/h	202	5	56	185	6	7
Future Vol, veh/h	202	5	56	185	6	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	э,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	202	5	56	185	6	7
	202			.00	•	•
	Major1	<b>N</b>	Major2	1	Minor2	
Conflicting Flow All	241	0	-	0	558	149
Stage 1	-	-	-	-	149	-
Stage 2	-	-	-	-	409	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	_	_	_	_	5.42	_
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_		3.518	3 318
Pot Cap-1 Maneuver	1326	_	_	_	491	898
Stage 1	1320	_	_	_	879	- 030
	-	-			671	-
Stage 2	-	-	_	-	0/1	_
Platoon blocked, %	4000	-	-	-	440	000
Mov Cap-1 Maneuver	1326	-	-	-	416	898
Mov Cap-2 Maneuver	-	-	-	-	416	-
Stage 1	-	-	-	-	745	-
Stage 2	-	-	-	-	671	-
A	ED		WD		OD.	
Approach	EB		WB		SB	
HCM Control Delay, s	8		0		11.3	
HCM LOS					В	
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SRLn1
	10		LDI	VVDI		
Capacity (veh/h)		1326	-	-	-	
HCM Lane V/C Ratio		0.152	-	-		0.022
HCM Control Delay (s)	)	8.2	0	-		11.3
HCM Lane LOS		Α	Α	-	-	В
HCM 95th %tile Q(veh	1)	0.5	-	-	-	0.1

Intersection		
Intersection Delay, s/veh	8.1	
Intersection LOS	Α	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Future Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	1	0	3	0	0	0	4	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		8.2				0		8		
HCM LOS		-		Α				-		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	25%	100%	
Vol Thru, %	100%	100%	0%	0%	
Vol Right, %	0%	0%	75%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	0	0	4	4	
LT Vol	0	0	1	4	
Through Vol	0	0	0	0	
RT Vol	0	0	3	0	
Lane Flow Rate	0	0	4	4	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0	0	0.006	0.006	
Departure Headway (Hd)	3.911	3.911	5.208	4.958	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	0	0	691	726	
Service Time	1.917	1.917	3.212	2.962	
HCM Lane V/C Ratio	0	0	0.006	0.006	
HCM Control Delay	6.9	6.9	8.2	8	
HCM Lane LOS	N	N	Α	Α	
HCM 95th-tile Q	0	0	0	0	

Intersection						
Int Delay, s/veh	6.1					
		EDD	WDI	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>}</b>	004	<b>\</b>	<b>↑</b>	<b>Y</b>	4
Traffic Vol, veh/h	20	231	35	63	253	4
Future Vol, veh/h	20	231	35	63	253	4
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	20	231	35	63	253	4
Major/Minor Ma	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	251	0	269	136
			201		136	
Stage 1	-	-	-	-		-
Stage 2	-	-	4.40	-	133	- C 7
Critical Hdwy	-	-	4.16	-	6.45	6.7
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.254	-	3.545	3.75
Pot Cap-1 Maneuver	-	-	1291	-	714	799
Stage 1	-	-	-	-	883	-
Stage 2	-	-	-	-	886	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1291	-	695	799
Mov Cap-2 Maneuver	-	-	-	-	695	-
Stage 1	-	-	-	-	883	-
Stage 2	-	-	-	-	862	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		2.8		13.2	
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		696	-		1291	_
HCM Lane V/C Ratio		0.369	_		0.027	_
HCM Control Delay (s)		13.2	_	_	7.9	_
HCM Lane LOS		В	_	_	Α.5	<u>-</u>
HCM 95th %tile Q(veh)		1.7	_	_	0.1	
HOW JOHN MINE Q(VEII)		1.7			0.1	

Intersection						
Int Delay, s/veh	2.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>	LDI	VVDL Š	VVD1	NDL W	NDI
Traffic Vol, veh/h	237	231	13	303	106	14
Future Vol, veh/h	237	231	13	303	106	14
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	150	-	0	-
Veh in Median Storag	e,# 0	_	-	0	0	-
Grade, %	0	_	-	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	8	8	5	25	21
Mvmt Flow	237	231	13	303	106	14
Major/Minor	Major1		Majara		Minar1	
Major/Minor	Major1	0	Major2		Minor1	353
Conflicting Flow All	0		468	0	682	
Stage 1	-	-	-	-	353	-
Stage 2	-	-	- 4.40	-	329	- C 44
Critical Hdwy	-	-	4.18	-	6.65	6.41
Critical Hdwy Stg 1	-	-	-	-	5.65	-
Critical Hdwy Stg 2	-	-	-	-	5.65	- 100
Follow-up Hdwy	-	-	2.272			3.489
Pot Cap-1 Maneuver	-	-	1063	-	382	650
Stage 1	-	-	-	-	663	-
Stage 2	-	-	-	-	680	-
Platoon blocked, %	-	-	4000	-	077	050
Mov Cap-1 Maneuver		-	1063	-	377	650
Mov Cap-2 Maneuver		-	-	-	377	-
Stage 1	-	-	-	-	663	-
Stage 2	-	-	-	-	672	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.3		18	
HCM LOS					C	
Minor Lane/Major Mvr	mt I	NBLn1	EBT	EBR	WBL	WBT
	iit l	396	EDI	LDIX	1063	VVDT
Capacity (veh/h) HCM Lane V/C Ratio		0.303	-	-	0.012	-
HUM Lane V/C Ratio		0.303	-	-	0.012	-

8.4

Α

0

18

С

1.3

HCM Control Delay (s)

HCM 95th %tile Q(veh)

HCM Lane LOS

Intersection												
Int Delay, s/veh	7.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	<u> </u>	LDIT	ሻ	\$	TTDIX.	1102	4	7	052	4	ODIT
Traffic Vol, veh/h	0	179	48	217	187	5	84	2	284	5	0	2
Future Vol, veh/h	0	179	48	217	187	5	84	2	284	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	179	48	217	187	5	84	2	284	5	0	2
Major/Minor N	1ajor1		1	Major2		1	Minor1		_ [	Minor2		
Conflicting Flow All	192	0	0	227	0	0	828	829	203	970	851	190
Stage 1	-	-	-	-	-	-	203	203	-	624	624	-
Stage 2	-	-	-	-	-	-	625	626	-	346	227	-
Critical Hdwy	4.1	-	-	4.26	-	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1394	-	-	1263	-	-	278	308	792	235	299	857
Stage 1	-	-	-	-	-	-	774	737	-	477	481	-
Stage 2	-	-	-	-	-	-	454	480	-	674	720	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1394	-	-	1263	-	-	241	255	792	130	248	857
Mov Cap-2 Maneuver	-	-	-	-	-	-	241	255	-	130	248	-
Stage 1	-	-	-	-	-	-	774	737	-	477	398	-
Stage 2	-	-	-	-	-	-	375	397	-	431	720	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.5			15.8			26.8		
HCM LOS							С			D		
Minor Lane/Major Mvmt		NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		241	792	1394			1263	-	-			
HCM Lane V/C Ratio		0.357		-	_		0.172	_		0.041		
HCM Control Delay (s)		28	12.1	0	_	_	8.4	_	-			
HCM Lane LOS		D	В	A	_	_	A	_	_	D		
HCM 95th %tile Q(veh)		1.5	1.6	0	-	-	0.6	-	-	0.1		

Intersection												
Int Delay, s/veh	8.1											
		EDT		MDI	WDT	WDD	NDI	NDT	NDD	ODI	ODT	000
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	4-	4	40	4=	4	222	<b>\</b>	4	00	- ነ	<b>^</b>	00
Traffic Vol, veh/h	17	20	12	45	26	228	12	87	20	126	122	28
Future Vol, veh/h	17	20	12	45	26	228	12	87	20	126	122	28
Conflicting Peds, #/hr	0	0	0	0	0	0	_ 0	_ 0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage	9,# -	0	-	-	0	-	-	0	-	-	0	_
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	6	0	0	7	0	16	0	15	32	29	5	0
Mvmt Flow	17	20	12	45	26	228	12	87	20	126	122	28
Major/Minor	Minor2			Minor1			Major1		- 1	Major2		
Conflicting Flow All	636	519	136	525	523	97	150	0	0	107	0	0
Stage 1	388	388	-	121	121		-	-	-	-	-	-
Stage 2	248	131	_	404	402	_	_	_	_	_	_	_
Critical Hdwy	7.16	6.5	6.2	7.17	6.5	6.36	4.1	_	_	4.39	_	_
Critical Hdwy Stg 1	6.16	5.5	J.L	6.17	5.5	-		_	_		_	_
Critical Hdwy Stg 2	6.16	5.5	_	6.17	5.5	_	_	_	_	_	_	_
Follow-up Hdwy	3.554	4		3.563	4	3.444	2.2	_	_	2.461	_	_
Pot Cap-1 Maneuver	385	464	918	455	462	922	1444	_	_	1331	_	_
Stage 1	628	612	-	871	800	-	-	_	_	-	_	<u>-</u>
Stage 2	747	792		613	604		_			_	_	_
Platoon blocked, %	771	102		010	- JU-7			_	_		_	<u>-</u>
Mov Cap-1 Maneuver	254	417	918	399	415	922	1444	_	_	1331	_	_
Mov Cap-2 Maneuver	254	417	-	399	415	-	-	_	_	-	_	<u>-</u>
Stage 1	623	554		864	794		_	_	_	_	_	_
Stage 2	539	786	_	528	547	_	_	_	_	_	_	_
Olugo Z	555	7 00		020	U+1							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.8			13.8			0.8			3.6		
HCM LOS	С			В								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1444		-	383	707	1331	_				
HCM Lane V/C Ratio		0.008	_		0.128	0.423	0.095	<u>-</u>	_			
HCM Control Delay (s)		7.5	_	_	4	13.8	8	_	_			
HCM Lane LOS		7.5 A	_	_	C	В	A	<u>-</u>	_			
HCM 95th %tile Q(veh	)	0	_	_	0.4	2.1	0.3	_	_			
TION JOHT JUHE WIVEH	1	U			J. <del>1</del>	۷. ۱	0.0					

0.3					
	\\/PD	NPT	NIPD	CDI	SBT
	WDK		NDK	ODL	
	E		0	2	<del>ન</del>
					114
					114
					0
					Free
					None
					-
	-		-	-	0
	-		-	-	0
					100
0			0	0	34
3	5	284	8	3	114
linar1	N	Jaior1		Major?	
					0
					0
		-	-		-
		-	-		-
		-	-	4.1	-
	-	-	-	-	-
	-	-	-	-	-
		-	-		-
	710	-	-	1281	-
	-	-	-	-	-
910	-	-	-	-	-
		-	-		-
601	710	-	-	1281	-
601	-	-	-	-	-
766	-	-	-	-	-
	_	-	_	_	_
- •					
NA/D		NB		SB	
WB				0.2	
10.5		0		0.2	
		0		0.2	
10.5		0		0.2	
10.5 B	NDT		MDI n1		QDT
10.5	NBT	NBRV	VBLn1	SBL	SBT
10.5 B	-	NBRV -	665	SBL 1281	-
10.5 B	-	NBRV - -	665 0.012	SBL 1281 0.002	-
10.5 B	- - -	NBRV - - -	665 0.012 10.5	SBL 1281 0.002 7.8	- - 0
10.5 B	-	NBRV - -	665 0.012	SBL 1281 0.002	-
	WBL  3 3 0 Stop 0 ,# 0 0 100 0 3  Minor1 408 288 120 6.4 5.4 5.4 5.4 3.5 603 766 910 601 601 766 907	WBL WBR  3 5 3 5 0 0 0 Stop Stop - None 0 - ,# 0 - 100 100 0 20 3 5  Minor1 N 408 288 288 - 120 - 6.4 6.4 5.4 - 5.4 - 5.4 - 3.5 3.48 603 710 766 - 910 - 601 710 601 - 766 - 907 -	WBL         WBR         NBT           3         5         284           0         0         0           Stop         Free           None         -           0         -         0           0         -         0           100         100         100           0         20         19           3         5         284    Minor1  Major1  408  288  0  288  - 120  - 6.4  6.4  6.4  - 5.4  - 5.4  - 5.4  - 5.4  - 3.5  3.48  603  710  - 766  - 910  - 601  710  - 601  766  - 910  - 766  - 907	WBL         WBR         NBT         NBR           3         5         284         8           0         0         0         0           Stop         Stop         Free         Free           -         None         -         None           0         -         -         -           0         -         0         -           100         100         100         100           0         20         19         0           3         5         284         8    Alinor1  Major1  Major1  408  288  0  0  288   120   6.4  6.4  6.4   5.4   5.4   3.5  3.48   603  710   766   910   601  710   601  710   601  710   601  710   766   907   766   907   766   907    766   907    766   907    766   907    766   907    766   907         -	WBL         WBR         NBT         NBR         SBL           3         5         284         8         3           0         0         0         0         0           Stop         Stop         Free         Free         Free           - None         -         None         -           0         -         -         -           0         -         0         -           100         100         100         100           0         20         19         0         0           3         5         284         8         3     Minor1  Major1  Major2   Minor1  Major2    Major2    Major2    Major2    Major2    Major2    Major2    Major2     Major2    Major2     Major2     Major2     Major2     Major2     Major2     Major2     Major2     Major2     Major2      Major2      Major2        Major2        Major2         Major2           Major2

Intersection												
Int Delay, s/veh	0											
	EBL	EBT	EDD	\\/DI	WDT	WBR	NDI	NBT	NDD	CDI	SBT	SBR
Movement Configurations	EBL		EBR	WBL	WBT	WBK	NBL		NBR	SBL		SBK
Lane Configurations	4	₩.	٥	٥	- ♣	۸	^	<b>4</b>	^	^	404	٥
Traffic Vol, veh/h	1	0	0	0	0	0	0	261	0	0	104	0
Future Vol, veh/h	1	0	0	0	0	0	0	261	0	0	104	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage		0	-	-	0	-	-	0	-	-	0	-
Grade, %	100	100	100	100	100	100	100	100	100	100	100	100
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	261	0	0	104	0
Major/Minor I	Minor2		1	Minor1		N	/lajor1		N	//ajor2		
Conflicting Flow All	365	365	104	365	365	261	104	0	0	261	0	0
Stage 1	104	104	-	261	261	-	-	-	-	-	-	-
Stage 2	261	261	-	104	104	-	-	-	-	-	-	-
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	446	566	956	595	566	783	1500	-	-	1315	-	-
Stage 1	710	813	-	748	696	-	-	-	-	-	-	-
Stage 2	571	696	-	907	813	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	446	566	956	595	566	783	1500	-	-	1315	-	-
Mov Cap-2 Maneuver	446	566	-	595	566	-	-	-	-	-	-	-
Stage 1	710	813	-	748	696	-	-	-	-	-	-	-
Stage 2	571	696	-	907	813	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.1			0			0			0		
HCM LOS	В			A								
				,,								
Minor Lane/Major Mvm	nt	NBL	NBT	NIPD	EBLn1V	VRI n1	SBL	SBT	SBR			
	IC.	1500	INDT		110		1315	001	אמט			
Capacity (veh/h) HCM Lane V/C Ratio			-	-	0.002	-		-	-			
		-	-			-	_	-	-			
HCM Control Delay (s) HCM Lane LOS		0	-	-	13.1 B	0	0	-	-			
HCM 95th %tile Q(veh)	١	A 0	-	-	0	A -	A 0	-	-			
	)	U	-	-	U	-	U	-	-			

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	CDL		EDK	VVDL		WDK	INDL		INDIX	ODL		SDR
Lane Configurations	٥	4	٥	10	- ♣	100	1	446	40	42	4	2
Traffic Vol, veh/h	0	4	0	13	5	129	1	146 146	43 43	43 43	56 56	2
Future Vol, veh/h	0	0	0	13	5	129	1 0	0	43	43	0	0
Conflicting Peds, #/hr								Free	Free		Free	
Sign Control RT Channelized	Stop	Stop	Stop None	Stop -	Stop -	Stop None	Free -	riee -	None	Free -		Free None
Storage Length		-	None	_	-	None	-	-	NONE -	_	-	NOHE
Veh in Median Storage		0	-		0	-		0	-		0	
Grade, %	, # - -	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mvmt Flow	0	4	0	13	5	129	1	146	43	43	56	2
IVIVIIIL I IOW	U	4	U	10	J	123		140	40	40	50	
	/linor2			Minor1			Major1			Major2		
Conflicting Flow All	380	334	57	315	314	168	58	0	0	189	0	0
Stage 1	143	143	-	170	170	-	-	-	-	-	-	-
Stage 2	237	191	-	145	144	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4		3.572	4	3.426	2.2	-	-	2.452	-	-
Pot Cap-1 Maneuver	581	589	1015	626	605	846	1559	-	-	1243	-	-
Stage 1	865	782	-	818	762	-	-	-	-	-	-	-
Stage 2	771	746	-	844	782	-	-	-	-	-	-	-
Platoon blocked, %	,		1015	00-	E 0.5	0.10	4===	-	-	40.10	-	-
Mov Cap-1 Maneuver	475	567	1015	605	583	846	1559	-	-	1243	-	-
Mov Cap-2 Maneuver	475	567	-	605	583	-	-	-	-	-	-	-
Stage 1	864	754	-	817	761	-	-	-	-	-	-	-
Stage 2	648	745	-	809	754	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.4			10.5			0			3.4		
HCM LOS	В			В								
Minor Lane/Major Mvm	+	NBL	NBT	NIPD	EBLn1V	MRI n1	SBL	SBT	SBR			
			INDI					ומט	SDR			
Capacity (veh/h)		1559	-	-		805	1243	-	-			
HCM Control Doloy (a)		0.001	-			0.183	0.035	-	-			
HCM Lang LOS		7.3	0	-		10.5	8	0	-			
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	Α	-	В	0.7	0.1	Α	-			
HOW Sour Wille Q(Ven)		U	-	-	0	0.7	U. I	-	-			

Intersection						
Int Delay, s/veh	0.7					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Å	•	0	4	<b>♣</b>	40
Traffic Vol, veh/h	9	2	3	129	23	12
Future Vol, veh/h	9	2	3	129	23	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	_	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	14	0
Mvmt Flow	9	2	3	129	23	12
			*			
		_				
	linor2		//ajor1		/lajor2	
Conflicting Flow All	164	29	35	0	-	0
Stage 1	29	_	-	-	-	-
Stage 2	135	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	_	_	-	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	831	1052	1589	_	_	_
Stage 1	999	-	-	_	_	_
Stage 2	896		_	_	_	_
Platoon blocked, %	030				_	_
	920	1050	1500	-	-	-
Mov Cap-1 Maneuver	829	1052	1589	-	-	_
Mov Cap-2 Maneuver	829	-	-	-	-	-
Stage 1	997	-	-	-	-	-
Stage 2	896	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.2		0.2		0	
HCM LOS	9.2 A		0.2		U	
I IOIVI LOS	А					
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1589	-		_	-
HCM Lane V/C Ratio		0.002		0.013	_	_
HCM Control Delay (s)		7.3	0	9.2	_	_
HCM Lane LOS		Α.5	A	Α.Δ	_	_
HCM 95th %tile Q(veh)		0	-	0	_	_
HOW JOHN JOHN Q (VEH)		U	_	U	_	_

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	1	130	0	0	26	1
Future Vol, veh/h	0	0	0	0	0	0	1	130	0	0	26	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	1	130	0	0	26	1
Major/Minor M	1inor2		ı	Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	159	159	27	159	159	130	27	0	0	130	0	0
Stage 1	27	27	-	132	132	-	-	-	-	-	-	-
Stage 2	132	132	-	27	27	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	811	737	1054	811	737	925	1600	-	-	1468	-	-
Stage 1	996	877	-	876	791	-	-	-	-	-	-	-
Stage 2	876	791	-	996	877	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	810	736	1054	810	736	925	1600	-	-	1468	-	-
Mov Cap-2 Maneuver	810	736	-	810	736	-	-	-	-	-	-	-
Stage 1	995	877	-	875	790	-	-	-	-	-	-	-
Stage 2	875	790	-	996	877	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0.1			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1600	-	-	-	-	1468	-	-			
HCM Lane V/C Ratio		0.001	-	-	-	-	-	-	-			
HCM Control Delay (s)		7.3	0	-	0	0	0	-	-			
HCM Lane LOS		Α	Α	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	6.5					
•						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		₽			4
Traffic Vol, veh/h	74	143	51	43	31	32
Future Vol, veh/h	74	143	51	43	31	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	0	14	23	12	26
Mvmt Flow	74	143	51	43	31	32
			•		· ·	<b>V</b> _
	Minor1		//ajor1		Major2	
Conflicting Flow All	167	73	0	0	94	0
Stage 1	73	-	-	-	-	-
Stage 2	94	-	-	-	-	-
Critical Hdwy	6.57	6.2	-	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	-	-	-	-	-
Follow-up Hdwy	3.653	3.3	-	-	2.308	-
Pot Cap-1 Maneuver	790	995	-	_	1440	-
Stage 1	913	-	_	_		-
Stage 2	893	_	_	_	_	_
Platoon blocked, %	- 500		_	_		_
Mov Cap-1 Maneuver	773	995	_	_	1440	_
Mov Cap-1 Maneuver	773	-	_		-	_
Stage 1	913	_				_
_	873	-	-	_	_	_
Stage 2	0/3	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		3.7	
HCM LOS	В					
		NET	NID D	VDL 1	05:	057
	nt .	NBT	NBRV	VBLn1	SBL	SBT
Minor Lane/Major Mvn	п	1101				
Capacity (veh/h)	п	-	-	906	1440	-
Capacity (veh/h) HCM Lane V/C Ratio		-		0.24	0.022	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		-	-	0.24 10.2	0.022 7.6	- - 0
Capacity (veh/h) HCM Lane V/C Ratio	)	-	-	0.24	0.022	- 0 A

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	EDL		EDK	VVDL		WBR	INDL		אסור	ODL		SDK
Lane Configurations Traffic Vol, veh/h	53	<b>♣</b> 65	3	0	<b>र्स</b> 22	53	1	<b>4</b>	2	135	<b>↔</b> 1	12
Future Vol, veh/h	53	65	3	0	22	53	1	0	2	135	1	12
Conflicting Peds, #/hr	0	00	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	- Otop	- Otop	None	- Olop	-	None
Storage Length	_	_	-	_	_	200	_	_	-	_	_	-
Veh in Median Storage	e.# -	0	_	_	0	-	_	0	_	_	0	_
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	53	65	3	0	22	53	1	0	2	135	1	12
Major/Minor	Major1		N	Major2		ı	Minor1		ı	Minor2		
Conflicting Flow All	75	0	0	68	0	0	228	248	67	196	196	22
Stage 1	13	-	U	- -	U	Ū	173	173	-	22	22	22
Stage 2	_	_	_	_	_	_	55	75	_	174	174	_
Critical Hdwy	4.39	_	-	4.1	_	_	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	4.03		_	4.1	_	_	6.1	5.5	0.2	6.31	5.5	0.95
Critical Hdwy Stg 2			_	_	_		6.1	5.5	_	6.31	5.5	_
Follow-up Hdwy	2.461	_	_	2.2	_	_	3.5	4		3.689		3.975
Pot Cap-1 Maneuver	1369	_	_	1546	-	-	731	658	1002	723	703	879
Stage 1	-	_	_	-	_	_	834	760	-	950	881	-
Stage 2	-	_	-	-	-	-	962	836	-	785	759	-
Platoon blocked, %		-	-		_	-						
Mov Cap-1 Maneuver	1369	-	-	1546	-	-	698	632	1002	700	675	879
Mov Cap-2 Maneuver	-	-	-	-	_	-	698	632	-	700	675	-
Stage 1	-	-	-	-	-	-	801	730	-	912	881	-
Stage 2	-	-	-	-	-	-	948	836	-	752	729	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.1			11.4		
HCM LOS	J. 1						A			В		
							, ,					
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBI n1			
	it I			ED I			VVDI					
Capacity (veh/h)		875	1369	-	-	1546	-	-	–			
HCM Central Delay (a)		0.003	0.039	-	-	_	-		0.208			
HCM Control Delay (s) HCM Lane LOS		9.1	7.7	0	-	0	-	-				
HCM 95th %tile Q(veh	1	A 0	0.1	Α	-	A 0	-	-	B 0.8			
HOW SOUL WILLE CALVED	)	U	0.1	-	-	U	-	-	0.0			

Intersection												
Int Delay, s/veh	4.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ĵ.		ሻ	ĵ.			4			4	
Traffic Vol, veh/h	9	286	8	29	100	103	1	5	28	133	7	7
Future Vol, veh/h	9	286	8	29	100	103	1	5	28	133	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	9	286	8	29	100	103	1	5	28	133	7	7
Major/Minor N	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	203	0	0	294	0	0	525	569	290	535	522	152
Stage 1	-	-	-	-	-	-	308	308	-	210	210	-
Stage 2	-	-	-	-	-	-	217	261	-	325	312	-
Critical Hdwy	4.32	-	-	4.17	-	-	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.64	-
Follow-up Hdwy	2.398	-	-	2.263	-	-	3.5	4	3.3	3.581	4.126	3.426
Pot Cap-1 Maneuver	1258	-	-	1239	-	-	466	435	754	445	443	864
Stage 1	-	-	-	-	-	-	706	664	-	776	706	-
Stage 2	-	-	-	-	-	-	790	696	-	673	637	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1258	-	-	1239	-	-	446	422	754	415	430	864
Mov Cap-2 Maneuver	-	-	-	-	-	-	446	422	-	415	430	-
Stage 1	-	-	-	-	-	-	701	659	-	771	690	-
Stage 2	-	-	-	-	-	-	757	680	-	638	633	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			10.7			17.8		
HCM LOS							В			С		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		664				1239	-	-				
HCM Lane V/C Ratio		0.051		_		0.023	_		0.345			
HCM Control Delay (s)		10.7	7.9	-	-	8	-	-				
HCM Lane LOS		В	A	_	_	A	-	-	С			
HCM 95th %tile Q(veh)	)	0.2	0	-	-	0.1	-	-	1.5			

Intersection												
Int Delay, s/veh	1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	LUL	₽	ופם	TIDE	4	ופוז	HUL	1101	אפאו	ODL	4	ODIN
Traffic Vol, veh/h	0	96	328	13	169	0	0	0	0	1	4	54
Future Vol, veh/h	0	96	328	13	169	0	0	0	0	1	4	54
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	- 13110
Veh in Median Storage,	.# -	0	_	_	0	_	_	0	_	-	0	_
Grade, %	, <i>''</i> -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mymt Flow	0	96	328	13	169	0	0	0	0	1	4	54
			020	10	.00						f	O r
N.A /N.A.												
	/lajor1			Major2					<b>N</b>	/linor2		4
Conflicting Flow All	-	0	0	424	0	0				455	619	169
Stage 1	-	-	-	-	-	-				195	195	-
Stage 2	-	-	-	-	-	-				260	424	-
Critical Hdwy	-	-	-	4.25	-	-				6.4	6.75	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	5.75	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	5.75	-
Follow-up Hdwy	-	-		2.335	-	-				3.5		3.57
Pot Cap-1 Maneuver	0	-	-	1069	-	0				567	376	807
Stage 1	0	-	-	-	-	0				843	698	-
Stage 2	0	-	-	-	-	0				788	549	-
Platoon blocked, %		-	-	1000	-					F00	_	00-
Mov Cap-1 Maneuver	-	-	-	1069	-	-				560	0	807
Mov Cap-2 Maneuver	-	-	-	-	-	-				560	0	-
Stage 1	-	-	-	-	-	-				843	0	-
Stage 2	-	-	-	-	-	-				778	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.6						9.9		
HCM LOS										Α		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-	-	1069	-	801						
HCM Lane V/C Ratio		_	_	0.012	_	0.074						
HCM Control Delay (s)		-	-	8.4	0	9.9						
HCM Lane LOS		_	_	A	A	A						
HCM 95th %tile Q(veh)		-	-	0	-	0.2						
				•		7.2						

Intersection												
Int Delay, s/veh	10											
-	EBL	EBT	EDD	\A/DI	WBT	WBR	NDI	NDT	NBR	SBL	CDT	SBR
Movement	EBL		EBR	WBL		WBK	NBL	NBT	INBK	SBL	SBT	SBK
Lane Configurations	02	<u>र्</u>	٥	٥	<b>€</b>	Е	170	40	c	۸	٥	٥
Traffic Vol, veh/h	93	4	0	0	6	5	176	10 10	6	0	0	0
Future Vol, veh/h	93	4	0	0	6	5	176 0	0	6	0	0	0
Conflicting Peds, #/hr	Free	Free	Free	Free	Free	Free	Stop	Stop		Stop	Stop	Stop
Sign Control RT Channelized	riee -	riee -	None	riee -	riee -	None	Stop -	Stop -	Stop None	Stop -	Stop -	None
Storage Length	_	_	-	-	_	INOHE -	_	-	-	_	_	NOITE
Veh in Median Storage		0	_	<u>-</u> -	0	_		0		_	0	
Grade, %	z, <del>π</del> - -	0	_	-	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	7	25	0	0	50	0	24	50	50	2	2	2
Mymt Flow	93	4	0	0	6	5	176	10	6	0	0	0
MATERIAL TOWN	- 55	7		U			170	10	U	U	0	
Majay/Minay	Maia - 1			Mais =0			Ain cut					
	Major1			Major2			Minor1	004				
Conflicting Flow All	11	0	-	-	-	0	199	201	4			
Stage 1	-	-	-	-	-	-	190	190	-			
Stage 2	117	-	-	-	-	-	9	11	- 6.7			
Critical Hdwy	4.17	-	-	-	-	-	6.64	7	6.7			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.64 5.64	6	-			
Critical Hdwy Stg 2		-	-	-	-	-	3.716	4.45	2 75			
Follow-up Hdwy Pot Cap-1 Maneuver	2.263 1576	-	0	0	-	-	742	618	3.75 955			
	15/0	-	0	0	-	-	792	661	900			
Stage 1 Stage 2	-	-	0	0	-	-	960	800	-			
Platoon blocked, %	_	_	U	U	_	_	900	000	_			
Mov Cap-1 Maneuver	1576	_	_	_	-	_	698	0	955			
Mov Cap-2 Maneuver	1370	_	_	_	_	_	698	0	300			
Stage 1			-	_		_	745	0	_			
Stage 2	_	_	_	_	_	_	960	0	_			
Olugo Z							500	J				
Approach	EB			WB			NB					
HCM Control Delay, s	7.1			0			12					
HCM LOS	7.1			U			B					
TOW LOO							D					
Minor Long/Maior Ma		NDL 1	EDI	EDT	WDT	WDD						
Minor Lane/Major Mvm	IL I	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)			1576	-	-	-						
HCM Cartest Dalay (a)		0.273	0.059	-	-	-						
HCM Control Delay (s)		12	7.4	0	-	-						
HCM CEth (/tile O/coh)	\	В	A	Α	-	-						
HCM 95th %tile Q(veh)	)	1.1	0.2	-	-	-						

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.			सी						4	
Traffic Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	2	7	7	2	0	0	0	0	6	11	6
Major/Minor N	//ajor1			Major2					N	Minor2		
Conflicting Flow All	- -	0	0	9	0	0				22	25	2
Stage 1	_	-	-	-	-	-				16	16	-
Stage 2	<u>-</u>	<u>-</u>	_	_	_	_				6	9	_
Critical Hdwy		_	_	4.27	_	_				6.57	7.23	6.2
Critical Hdwy Stg 1	_	_	_		_	_				5.57	6.23	- 0.2
Critical Hdwy Stg 2	_	-	-	-	_	-				5.57	6.23	_
Follow-up Hdwy	_	_	_	2.353	_	_				3.653	4.657	3.3
Pot Cap-1 Maneuver	0	-	_	1518	_	0				957	747	1088
Stage 1	0	_	_	-	_	0				969	760	-
Stage 2	0	-	-	-	_	0				979	766	_
Platoon blocked, %		_	_		_					<b>J</b>		
Mov Cap-1 Maneuver	-	-	_	1518	-	-				952	0	1088
Mov Cap-2 Maneuver	_	_	_	-	_	-				952	0	-
Stage 1	_	-	-	_	-	-				969	0	-
Stage 2	_	_	_	_	_	-				974	0	_
- 15-13-1												
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.7						8.6		
HCM LOS	U			J.1						Α		
TOW LOO										٨		
Minor Lanc/Major Mum	+	EBT	EBR	WBL	\\/DT	SBLn1						
Minor Lane/Major Mym												
Capacity (veh/h)		-		1518	-							
HCM Control Dolov (a)		-	-	0.005		0.023						
HCM Control Delay (s)		-	-	7.4	0	8.6						
HCM OF the 9/ tile O(web)		-	-	A	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	-	0.1						

Intersection												
Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			- €			4				
Traffic Vol, veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Future Vol, veh/h	5	3	0	0	8	18	1	3	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	13	11	100	33	0	2	2	2
Mvmt Flow	5	3	0	0	8	18	1	3	9	0	0	0
Major/Minor M	1ajor1		N	Major2		N	/linor1					
Conflicting Flow All	26	0	_	-	_	0	30	39	3			
Stage 1	-	-	-	_	_	-	13	13	-			
Stage 2	_	_	_	_	_	-	17	26	_			
Critical Hdwy	4.1	-	-	_	-	-	7.4	6.83	6.2			
Critical Hdwy Stg 1	_	_	_	_	_	-	6.4	5.83	-			
Critical Hdwy Stg 2	-	_	-	_	_	-	6.4	5.83	-			
Follow-up Hdwy	2.2	-	-	-	-	-		4.297	3.3			
Pot Cap-1 Maneuver	1601	-	0	0	-	-	783	796	1087			
Stage 1	-	-	0	0	-	-	806	827	-			
Stage 2	-	-	0	0	-	-	802	816	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1601	-	-	-	-	-	781	0	1087			
Mov Cap-2 Maneuver	-	-	-	-	-	-	781	0	-			
Stage 1	-	-	-	-	-	-	804	0	-			
Stage 2	-	-	-	-	-	-	802	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	4.5			0			8.5					
HCM LOS	7.0			U			Α					
1.5141 2.55							, , ,					
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		1046	1601			יום וע						
HCM Lane V/C Ratio		0.012		-	_	-						
HCM Control Delay (s)		8.5	7.3	0	-	-						
HCM Lane LOS		6.5 A	7.3 A	A	-	-						
HCM 95th %tile Q(veh)		0	0	- A	-	<del>-</del>						
HOW JOHN JOHNE Q(VEII)		U	U	<u>-</u>	_	_						

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LDIX	****	4	VVDIX	HDL	4	HUIT	ODL	4	ODIT
Traffic Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Future Vol, veh/h	1	7	4	0	14	0	12	2	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	25	0	0	0	18	0	0	0	0	0
Mvmt Flow	1	7	4	0	14	0	12	2	0	0	0	0
Major/Minor M	lajor1		ľ	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	14	0	0	11	0	0	25	25	9	26	27	14
Stage 1	-	-	-	-	-	-	11	11	-	14	14	-
Stage 2	-	-	-	-	-	-	14	14	-	12	13	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.28	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.662	4	3.3	3.5	4	3.3
	1617	-	-	1621	-	-	947	872	1079	990	870	1072
Stage 1	-	-	-	-	-	-	970	890	-	1011	888	-
Stage 2	-	-	-	-	-	-	966	888	-	1014	889	-
Platoon blocked, %	1017	-	-	4004	-	-	0.40	074	4070	007	000	4070
	1617	-	-	1621	-	-	946	871	1079	987	869	1072
Mov Cap-2 Maneuver Stage 1	_	-	-	-	-	-	946 969	871 889	-	987 1010	869 888	-
Stage 1	-	-	-	-	-	-	969	888	<u>-</u>	1010	888	-
Slaye Z	<u>-</u>	_	_	_	-	_	900	000	<u>-</u>	1011	000	_
				16/5						65		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			8.9			0		
HCM LOS							Α			Α		
Minor Lane/Major Mvmt	1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		935	1617	-	-	1621	-	-	-			
HCM Lane V/C Ratio		0.015		-	-	-	-	-	-			
HCM Control Delay (s)		8.9	7.2	0	-	0	-	-	0			
HCM Lane LOS		A	A	Α	-	A	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	-			

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.			र्स						4	
Traffic Vol, veh/h	0	169	0	33	7	0	0	0	0	109	1	8
Future Vol, veh/h	0	169	0	33	7	0	0	0	0	109	1	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	169	0	33	7	0	0	0	0	109	1	8
Major/Minor N	/lajor1		N	Major2					N	/linor2		
Conflicting Flow All	- iajui i	0	0	169	0	0				242	242	7
Stage 1	-	-	U	109	-	-				73	73	-
Stage 2	-	-	-	_	-	_				169	169	-
Critical Hdwy	-	-	-	4.23	-	-				6.43	7.5	6.4
Critical Hdwy Stg 1	-	-	-	4.23	-	-				5.43	6.5	0.4
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	6.5	-
Follow-up Hdwy	-	_	-	2.317	-	-				3.527	4.9	3.48
	0	-	-	1344	-	0				744	521	1025
Pot Cap-1 Maneuver	0		-	1344		0				947	676	1025
Stage 1 Stage 2	0	-	-	_	-	0				858	606	
Platoon blocked, %	U	-	-	-	-	U				000	000	-
Mov Cap-1 Maneuver	_	-	-	1344	-	_				725	0	1025
Mov Cap-1 Maneuver		-	-	1344		-				725	0	1025
	-	-	-		-	-				947		
Stage 1	-	-	-	-	-	-				837	0	-
Stage 2	-	-	-	-	-	-				03/	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.4						10.8		
HCM LOS										В		
Minor Lane/Major Mvmt	1	EBT	EBR	WBL	WRT	SBLn1						
Capacity (veh/h)		-	-		-	740						
HCM Lane V/C Ratio		<u>-</u>		0.025		0.159						
HCM Control Delay (s)				7.7	0	10.8						
HCM Lane LOS		<u>-</u>	_	Α.	A	В						
HCM 95th %tile Q(veh)		<u>-</u>		0.1	-	0.6						
HOW JOHN JOHN Q(VEH)		_	_	U. I		0.0						

Intersection												
Int Delay, s/veh	4.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			<b>1</b>			4		<u> </u>	<b>U</b>	02.1
Traffic Vol, veh/h	131	147	0	0	31	78	9	4	121	0	0	0
Future Vol, veh/h	131	147	0	0	31	78	9	4	121	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	_	None	-	-	None	-		None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	_	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	17	7	2	0	12	4	0	25	10	2	2	2
Mvmt Flow	131	147	0	0	31	78	9	4	121	0	0	0
Major/Minor N	Major1		ı	Major2		N	/linor1					
Conflicting Flow All	109	0	-	-	-	0	479	518	147			
Stage 1	-	-	-	-	-	-	409	409	-			
Stage 2	-	-	-	-	-	-	70	109	-			
Critical Hdwy	4.27	-	-	-	-	-	6.4	6.75	6.3			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	5.75	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	5.75	-			
Follow-up Hdwy	2.353	-	-	-	-	-	3.5	4.225	3.39			
Pot Cap-1 Maneuver	1393	-	0	0	-	-	549	431	879			
Stage 1	-	-	0	0	-	-	675	558	-			
Stage 2	-	_	0	0	-	-	958	763	_			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1393	-	-	-	-	-	493	0	879			
Mov Cap-2 Maneuver	-	-	-	-	-	-	493	0	-			
Stage 1	-	-	-	-	-	-	606	0	-			
Stage 2	-	-	-	-	-	-	958	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	3.7			0			10.1					
HCM LOS							В					
Minor Lane/Major Mvm	ıt l	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		834	1393	-	-	_						
HCM Lane V/C Ratio			0.094	_	_	-						
HCM Control Delay (s)		10.1	7.9	0	-	-						
HCM Lane LOS		В	Α	A	-	-						
HCM 95th %tile Q(veh)		0.6	0.3	-	-	-						

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	265	0	0	102	3	1	1	4	1	0	6
Future Vol, veh/h	3	265	0	0	102	3	1	1	4	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	265	0	0	102	3	1	1	4	1	0	6
Major/Minor N	/lajor1		N	Major2		ľ	Minor1		N	/linor2		
Conflicting Flow All	105	0	0	265	0	0	378	376	265	378	375	104
Stage 1	-	-	-	-	-	-	271	271	-	104	104	-
Stage 2	-	-	-	-	-	-	107	105	-	274	271	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	_	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.453
Pot Cap-1 Maneuver	1499	-	-	1311	-	-	583	558	779	583	559	911
Stage 1	-	-	-	-	-	-	739	689	-	907	813	-
Stage 2	-	-	-	-	-	-	903	812	-	736	689	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1499	-	-	1311	-	-	578	557	779	578	558	911
Mov Cap-2 Maneuver	-	-	-	-	-	-	578	557	-	578	558	-
Stage 1	-	-	-	-	-	-	738	688	-	905	813	-
Stage 2	-	-	-	-	-	-	897	812	-	730	688	-
-												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0			10.2			9.3		
HCM LOS				-			В			A		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		693	1499	-		1311	-	-				
HCM Lane V/C Ratio		0.009		_	_		_		0.008			
HCM Control Delay (s)		10.2	7.4	0	_	0	_	_	9.3			
HCM Lane LOS		В	Α	A	_	A	_	_	A			
HCM 95th %tile Q(veh)		0	0	-	_	0	-	-	0			

Intersection							
Int Delay, s/veh	5.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	T T	<u></u>	₩ <b>(</b>	וטייי	JDL Š	JUIN 7	
Traffic Vol, veh/h	180	60	57	29	24	68	
Future Vol, veh/h	180	60	57	29	24	68	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None		None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage	e, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	180	60	57	29	24	68	
Major/Minor I	Major1	N	Major2	N	/linor2		
Conflicting Flow All	86	0	-	0	492	72	
Stage 1	-	-	-	-	72	-	
Stage 2	-	_	_	-	420	-	
Critical Hdwy	4.15	-	-	-	6.4	6.26	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.245	-	-	-		3.354	
Pot Cap-1 Maneuver	1492	-	-	-	540	979	
Stage 1	-	-	-	-	956	-	
Stage 2	-	-	-	-	667	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1492	-	-	-	475	979	
Mov Cap-2 Maneuver	-	-	-	-	475	-	
Stage 1	-	-	-	-	840	-	
Stage 2	-	-	-	-	667	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.8		0		10		
HCM LOS					В		
Minor Long/Major Mare	.+	EDI	EDT	WDT	WDD	CDI 51 C	מי וםי
Minor Lane/Major Mvm	IL	EBL	EBT	WBT		SBLn1 S	
Capacity (veh/h)		1492	_	-	-	475	979
HCM Control Doloy (c)		0.121	-	-	-	0.051	
HCM Control Delay (s) HCM Lane LOS		7.7	-	-	-	13 B	9 A
HCM 95th %tile Q(veh)	١	A 0.4	-	-	-	0.2	0.2
How som while Q(ven)	)	0.4	-	-	-	U.Z	U.Z

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ሻ	ĵ.			4			4	
Traffic Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	21
Future Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	_	_	None	_	-	Stop	-	-	None
Storage Length	100	_	-	150	_	_	_	_	_	-	-	-
Veh in Median Storage		0	-	_	0	_	_	0	-	_	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	47	37	4	1	46	4	8	16	7	0	2	21
Major/Minor	Major1		-	Major2		N	Minor1		N	Minor2		
Conflicting Flow All	50	0	0	41	0	0	195	185	39	191	185	48
Stage 1	-	-		-	-	-	133	133	-	50	50	-
Stage 2	_	- -		_	_	_	62	52	<u>-</u>	141	135	_
Critical Hdwy	4.21	<u>-</u>	-	4.1	_		7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1	7.41	_		- <b>7.</b> I		_	6.1	5.56	0.04	6.1	5.5	0.5
Critical Hdwy Stg 1		-	-	_	_	_	6.1	5.56	_	6.1	5.5	_
Follow-up Hdwy	2.299	_		2.2		_	3.5	4.054		3.5	4	3.39
Pot Cap-1 Maneuver	1501	-	-	1581	_	-	769	702	999	773	713	999
Stage 1	1001	_		1001		_	875	779	-	968	857	-
Stage 2	-	-	_	_	_	_	954	844	-	867	789	_
Platoon blocked, %	_	_		_	_	_	304	044	_	007	103	
Mov Cap-1 Maneuver	1501	<u>-</u>	-	1581	_	-	733	680	999	735	690	999
Mov Cap-1 Maneuver	1501	_		1001	_	_	733	680	999	735	690	333
Stage 1		<u>-</u>	-	<u>-</u>	_	-	848	755	-	938	856	-
Stage 2				_		_	931	843	<u>-</u>	816	765	_
Slaye Z	<u>-</u>	<u>-</u>	_	<u>-</u>	_	<u>-</u>	301	043	<u>-</u>	010	100	_
Annragah	ED			WD			ND			CD		
Approach	EB			WB			NB 0.1			SB		
HCM Control Delay, s	4			0.1			9.1			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:				
Capacity (veh/h)		900		-		1581	-	-	002			
HCM Lane V/C Ratio		0.034		-	-	0.001	-	-	0.024			
HCM Control Delay (s)		9.1	7.5	-	-	7.3	-	-	8.8			
HCM Lane LOS		Α	Α	-	-	Α	-	-	Α			
HCM 95th %tile Q(veh	)	0.1	0.1	-	-	0	-	-	0.1			

Intersection						
Int Delay, s/veh	2.1					
		EDD	\\/DI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>	4	ዃ	<b>†</b>	<b>Y</b>	4.4
Traffic Vol, veh/h	44	1	5	42	10	14
Future Vol, veh/h	44	1	5	42	10	14
Conflicting Peds, #/hr	0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	26	0	0	15	0	7
Mvmt Flow	44	1	5	42	10	14
Majay/Minay	1-:1		10:00		11:00 - 11	
	1ajor1		Major2		Minor1	4=
Conflicting Flow All	0	0	45	0	97	45
Stage 1	-	-	-	-	45	-
Stage 2	-	-	-	-	52	-
Critical Hdwy	-	-	4.1	-	6.4	6.27
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.363
Pot Cap-1 Maneuver	-	-	1576	-	907	1011
Stage 1	-	-	-	-	983	-
Stage 2	-	-	-	-	976	-
Platoon blocked, %	_	-		_		
Mov Cap-1 Maneuver	-	_	1576	_	904	1011
Mov Cap-2 Maneuver	_	_	-	_	904	-
Stage 1	_	_	_	_	983	_
Stage 2	_	_	_	_	973	_
Olage 2					313	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.8	
HCM LOS					Α	
NA: 1 /NA: NA (		IDL 4	EDT	<b>500</b>	MA	MOT
Minor Lane/Major Mvmt		NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		963	-		1576	-
HCM Lane V/C Ratio		0.025	-	-	0.003	-
HCM Control Delay (s)		8.8	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-

Intersection						
Int Delay, s/veh	0.7					
		14/00	Not	NES	051	057
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		₽			4
Traffic Vol, veh/h	2	0	16	0	0	6
Future Vol, veh/h	2	0	16	0	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	17
Mvmt Flow	2	0	16	0	0	6
	_				•	
	Minor1		//ajor1		/lajor2	
Conflicting Flow All	22	16	0	0	16	0
Stage 1	16	-	-	-	-	-
Stage 2	6	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	1000	1069	-	-	1615	-
Stage 1	1012	-	_	-	-	-
Stage 2	1022	_	_	-	-	-
Platoon blocked, %	IVLL		_	_		_
Mov Cap-1 Maneuver	1000	1069			1615	
Mov Cap-1 Maneuver	1000	1009		_	1015	_
Stage 1	1012	-	-	-	<u>-</u>	-
•				-	=	_
Stage 2	1022	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	A					
Minor Lane/Major Mvm	ıt	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-		1000	1615	-
HCM Lane V/C Ratio		-	-	0.002	-	-
HCM Control Delay (s)		-	-	8.6	0	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)	)	-	_	0	0	-

Intersection						
	0					
Int Delay, s/veh						
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	- W			सी	Þ	
Traffic Vol, veh/h	0	0	0	21	5	0
Future Vol, veh/h	0	0	0	21	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	5	20	0
Mvmt Flow	0	0	0	21	5	0
Major/Minor M	/linor2	N	Major1	N	Major2	
Conflicting Flow All	26	5	5	0	-	0
Stage 1	5	-	-	-	_	-
Stage 2	21	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
		2.2	2.2	_	-	-
Follow-up Hdwy	3.5	3.3	۷.۷			
	3.5 995	1084	1630	-	-	-
Follow-up Hdwy				-	-	-
Follow-up Hdwy Pot Cap-1 Maneuver	995			- -		
Follow-up Hdwy Pot Cap-1 Maneuver Stage 1	995 1023	1084		- - -		

Approach	EB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	Α		

Minor Lane/Major Mvmt	NBL	NBT EE	BLn1	SBT	SBR
Capacity (veh/h)	1630	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	0	-	0	-	-
HCM Lane LOS	Α	-	Α	-	-
HCM 95th %tile Q(veh)	0	-	-	-	-

Mov Cap-2 Maneuver

Stage 1

Stage 2

995

1023

1007

Intersection						
Int Delay, s/veh	5					
		===			05=	055
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	₽	
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	25	33	0	0	0	33
Mymt Flow	4	6	16	11	3	1
IVIVIII( I IOVV		U	10		J	
Major/Minor	Minor2	<u> </u>	//ajor1	N	/lajor2	
Conflicting Flow All	47	4	4	0	-	0
Stage 1	4	-	-	-	-	-
Stage 2	43	-	-	-	-	-
Critical Hdwy	6.65	6.53	4.1	-	_	-
Critical Hdwy Stg 1	5.65	-		_	_	_
Critical Hdwy Stg 2	5.65	_	_	_	_	_
Follow-up Hdwy	3.725		2.2	_	_	_
Pot Cap-1 Maneuver	908	996	1631	_	_	_
Stage 1	962	-	1001	_	_	_
Stage 2	924	-	_	_		_
	924	_	_	-		
Platoon blocked, %	000	000	4004	-	-	-
Mov Cap-1 Maneuver	899	996	1631	-	-	-
Mov Cap-2 Maneuver	899	-	-	-	-	-
Stage 1	952	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	EB		NB		SB	
			4.3			
HCM Control Delay, s	8.8		4.3		0	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1631		955		
HCM Lane V/C Ratio		0.01	_	0.01	_	_
HCM Control Delay (s)	\	7.2	0	8.8	_	_
HCM Lane LOS			-			
	.\	A	Α	A	-	-
HCM 95th %tile Q(veh	)	0	-	0	-	-

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	110	145	1	1	11	4	1	1	0	4	2	14
Future Vol, veh/h	110	145	1	1	11	4	1	1	0	4	2	14
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	18	100	0	0	0	0	0	0	25	0	0
Mvmt Flow	110	145	1	1	11	4	1	1	0	4	2	14
Major/Minor	Major1			Major2			Minor1			Minor2		
	Major1	^			0			202			204	40
Conflicting Flow All	15	0	0	146	0	0	389	383	146	381	381	13
Stage 1	-	-	-	-	-	-	366	366	-	15 366	15	-
Stage 2	4 04	-	-	11	-	-	23	17	6.0		366	- 6.0
Critical Hdwy	4.21	-	-	4.1	-	-	7.1 6.1	6.5 5.5	6.2	7.35 6.35	6.5 5.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1		-	6.35	5.5	-
Critical Hdwy Stg 2	2.299	-	-	2.2	-	-	3.5	5.5	3.3	3.725	5.5	3.3
Follow-up Hdwy	1546	-	-	1448	-	-		553	906	537	•	1073
Pot Cap-1 Maneuver	1340	-	-	1440	-	-	574 657	626	906	949	555 887	
Stage 1	-	-	-	-	-	-	1000	885		609	626	-
Stage 2 Platoon blocked, %		-	-		-	-	1000	000	-	009	020	-
Mov Cap-1 Maneuver	1546	-	_	1448	-	-	531	510	906	504	512	1073
Mov Cap-1 Maneuver	1540	-	-	1440	-	-	531	510	900	504	512	1073
Stage 1	-	-	-	-	-	-	606	578	-	876	886	
Stage 2	_	-	-	-	-	-	984	884	-	561	578	-
Staye 2	_	_	<u>-</u>	-	_	<u>-</u>	JU4	004	_	JU I	310	<u>-</u>
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.2			0.5			12			9.6		
HCM LOS							В			Α		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBI n1			
Capacity (veh/h)	. 1	520	1546	-	-	1448	-	-	804			
HCM Lane V/C Ratio		0.004		_		0.001	-		0.025			
HCM Control Delay (s)		12	7.5	0	_	7.5	0	-	9.6			
HCM Lane LOS		B	7.5 A	A	_	7.5 A	A	-	9.0 A			
HCM 95th %tile Q(veh	1	0	0.2	- -		0	- -		0.1			
HOW JOHN JOHN W(VEH	1	U	U.Z			U	_		0.1			

Intersection						
Int Delay, s/veh	3.1					
Movement W	VBL	WBR	NBT	NBR	SBL	SBT
		WDK		NDK	ODL	
	Y	40	<b>♣</b>	^	<b>5</b> 4	<u>ન</u>
Traffic Vol, veh/h	5	48	140	8	54	38
Future Vol, veh/h	5	48	140	8	54	38
Conflicting Peds, #/hr	0	0	0	0	0	0
	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor 1	100	100	100	100	100	100
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	5	48	140	8	54	38
NA ' (NA) NA	4					
Major/Minor Mino			//ajor1		Major2	
	290	144	0	0	148	0
	144	-	-	-	-	-
	146	-	-	-	-	-
•	6.6	6.2	-	-	4.12	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy 3	3.68	3.3	-	-	2.218	-
Pot Cap-1 Maneuver 6	664	909	-	-	1434	-
Stage 1 8	841	-	-	-	-	-
	839	-	-	-	-	-
Platoon blocked, %			_	-		_
	639	909	-	_	1434	_
	639	-	_	_	-	_
	841	_	_	_	_	_
9	807	_	_	_	_	_
Olago Z	JU 1	_		_		
	WB		NB		SB	
HCM Control Delay, s	9.4		0		4.5	
HCM LOS	Α					
NA' I /NA - ' NA I		NDT	NDDV	MDL . 4	ODI	ODT
Minor Lane/Major Mvmt		NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-			-
HCM Lane V/C Ratio		-	-	0.061		-
HCM Control Delay (s)		-	-	9.4	7.6	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Movement	Intersection						
Movement		0					
Lane Configurations							
Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h  Conflicting Peds, #/hr  O O O O O O O O O O O O O O O O O O	Movement		EBR	WBL	WBT		NBR
Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h Future Vol, veh/h  Conflicting Peds, #/hr  O O O O O O O O O O O O O O O O O O	Lane Configurations	ĵ.			र्स	W	
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Traffic Vol, veh/h		0	0			0
Sign Control         Free Row Free Free Free Free Free Stop Stop RT Channelized         Free RT Channelized         Free RT Channelized         None RT Channelized	Future Vol, veh/h	4	0	0	3	0	0
Sign Control         Free Row Free Row Free Row RT Channelized         Free Row RT Channelized         Free Row RT Channelized         Free Row RT Channelized         None RT Channelized         None RT Channelized         None RT None RT Row RT	Conflicting Peds, #/hr	0	0	0	0	0	0
RT Channelized		Free	Free	Free	Free	Stop	Stop
Storage Length	RT Channelized					•	None
Veh in Median Storage, #         0         -         -         0         0           Grade, %         0         -         -         0         0           Peak Hour Factor         100         100         100         100         100         100           Heavy Vehicles, %         2		-	_	-		0	-
Grade, %         0         -         -         0         0           Peak Hour Factor         100         1		# 0	_	_	0		_
Peak Hour Factor         100							_
Heavy Vehicles, %							
Mount Flow         4         0         0         3         0         0           Major/Minor         Major1         Major2         Minor1           Conflicting Flow All         0         0         4         0         7         4           Stage 1         -         -         -         -         4         Stage 2         -         -         -         4         6.22         -         -         -         4         6.22         -         -         -         4         5.42         -							2
Major/Minor         Major1         Major2         Minor1           Conflicting Flow All         0         0         4         0         7         4           Stage 1         -         -         -         4         Stage 2         -         -         -         4           Stage 2         - <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>0</td>							0
Conflicting Flow All 0 0 4 0 7 A Stage 1 4 Stage 2 3 Critical Hdwy 4.12 - 6.42 6.22 Critical Hdwy Stg 1 5.42 Critical Hdwy Stg 2 5.42 Follow-up Hdwy 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1618 - 1014 1080 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1014 Stage 1 1014 Stage 1 1014 Stage 2 1019 Stage 2 1019 Stage 2 1019 Stage 2 1014 Stage 1 1014 Stage 1 1019 Stage 2 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2	IVIVIIIL FIOW	4	U	U	3	U	U
Conflicting Flow All 0 0 4 0 7 A Stage 1 4 Stage 2 3 Critical Hdwy 4.12 - 6.42 6.22 Critical Hdwy Stg 1 5.42 Critical Hdwy Stg 2 5.42 Follow-up Hdwy 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1618 - 1014 1080 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1014 Stage 1 1014 Stage 1 1014 Stage 2 1019 Stage 2 1019 Stage 2 1019 Stage 2 1014 Stage 1 1014 Stage 1 1019 Stage 2 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2							
Conflicting Flow All 0 0 4 0 7 A Stage 1 4 Stage 2 3 Critical Hdwy 4.12 - 6.42 6.22 Critical Hdwy Stg 1 5.42 Critical Hdwy Stg 2 5.42 Follow-up Hdwy 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1618 - 1014 1080 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1014 Stage 1 1014 Stage 1 1014 Stage 2 1019 Stage 2 1019 Stage 2 1019 Stage 2 1014 Stage 1 1014 Stage 1 1019 Stage 2 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1014 Stage 1 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2 1019 Stage 2 1020 Platoon blocked, % 1019 Stage 2	Major/Minor Ma	ajor1	N	Major2		Minor1	
Stage 1       -       -       -       4         Stage 2       -       -       -       3         Critical Hdwy       -       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42         Critical Hdwy Stg 2       -       -       -       5.42         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1019       1080         Stage 2       -       -       -       1020         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0       0							4
Stage 2       -       -       -       3         Critical Hdwy       -       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42         Critical Hdwy Stg 2       -       -       -       5.42         Follow-up Hdwy       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020         Platoon blocked, %       -       -       -       -       1020         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       1080         Stage 1       -       -       -       1014       1080         Approach       EB       WB       NB         HCM Control Delay, s       0       0       0         HCM Lane V/C Ratio       -       -       -       1618         HCM Lane LOS       A       -       -       -			-				_
Critical Hdwy       -       -       4.12       -       6.42       6.22         Critical Hdwy Stg 1       -       -       -       5.42         Critical Hdwy Stg 2       -       -       -       5.42         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020         Platoon blocked, %       -       -       -       -       -       1020         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1019       1019         Stage 1       -       -       -       1019       1020         Approach       EB       WB       NB       NB         HCM Control Delay, s       0       0       0       0         A Capacity (veh/h)       -       -       -       -         A Capacity (veh/h)       -	•		_				_
Critical Hdwy Stg 1       -       -       -       5.42         Critical Hdwy Stg 2       -       -       -       5.42         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020         Platoon blocked, %       -       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1019       109       109       109       109       109       109       109       100       <			<u>-</u>				
Critical Hdwy Stg 2       -       -       -       5.42         Follow-up Hdwy       -       -       2.218       -       3.518       3.318         Pot Cap-1 Maneuver       -       -       1618       -       1014       1080         Stage 1       -       -       -       -       1020         Platoon blocked, %       -       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       -       1019       1014       1080         Stage 1       -       -       -       -       1019       1019       1019       1020         Approach       EB       WB       NB       NB       NB       NB       HCM Control Delay, s       0       0       0       0       0       0       A       HCM Lane V/C Ratio       -       -       -       1618       HCM Lane LOS       A       -       -       0       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -			-				
Follow-up Hdwy - 2.218 - 3.518 3.318  Pot Cap-1 Maneuver - 1618 - 1014 1080  Stage 1 1019  Stage 2 1020  Platoon blocked, %  Mov Cap-1 Maneuver - 1618 - 1014 1080  Mov Cap-2 Maneuver - 1618 - 1014 1080  Mov Cap-2 Maneuver 1019  Stage 1 1019  Stage 2 1020  Approach EB WB NB  HCM Control Delay, s 0 0 0  HCM LOS A  Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT  Capacity (veh/h) - 1618  HCM Lane V/C Ratio			-				-
Pot Cap-1 Maneuver         -         -         1618         -         1014         1080           Stage 1         -         -         -         1019         1020         102			-				- 240
Stage 1         -         -         -         1019           Stage 2         -         -         -         1020           Platoon blocked, %         -         -         -         -           Mov Cap-1 Maneuver         -         -         1618         -         1014         1080           Mov Cap-2 Maneuver         -         -         -         1014         1080         1014         1080         108         108         108         109 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Stage 2       -       -       -       1020         Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       -       -       1618       -       1014       1080         Mov Cap-2 Maneuver       -       -       -       1014       1019       1019       1019       1019       1020       1020       1020         Approach       EB       WB       NB       NB       NB       NB       HCM Control Delay, s       0       0       0       0       0       A       HCM Lane/Major Mvmt       NBLn1       EBT       EBR       WBL       WBT       WBT       WBT       Capacity (veh/h)       -       -       1618       HCM Lane V/C Ratio       - <td< td=""><td>•</td><td></td><td>-</td><td></td><td></td><td></td><td></td></td<>	•		-				
Platoon blocked, %         -         -         -           Mov Cap-1 Maneuver         -         -         1618         -         1014         1080           Mov Cap-2 Maneuver         -         -         -         -         1014         1080           Stage 1         -         -         -         -         1019         1019         1020           Approach         EB         WB         NB         NB         NB         NB         HCM Control Delay, s         0         0         0         0         0         A         A         -         -         1618         WB         NB			-				-
Mov Cap-1 Maneuver         -         -         1618         -         1014         1080           Mov Cap-2 Maneuver         -         -         -         -         1014         1080           Stage 1         -         -         -         -         1019         1019         1020           Approach         EB         WB         NB         NB <t< td=""><td></td><td>-</td><td>-</td><td>-</td><td>-</td><td>1020</td><td>-</td></t<>		-	-	-	-	1020	-
Mov Cap-2 Maneuver         -         -         -         1014           Stage 1         -         -         -         1019           Stage 2         -         -         -         1020             Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618           HCM Lane V/C Ratio         -         -         -         0           HCM Control Delay (s)         0         -         -         0           HCM Lane LOS         A         -         -         A		-	-		-		
Stage 1         -         -         -         -         1019           Stage 2         -         -         -         -         1020           Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A    Minor Lane/Major Mvmt  NBLn1  EBT  EBR  WBL  WBT  Capacity (veh/h)  1618  HCM Lane V/C Ratio		-	-	1618	-		1080
Stage 2         -         -         -         -         1020           Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618           HCM Lane V/C Ratio         -         -         -         -           HCM Control Delay (s)         0         -         -         0           HCM Lane LOS         A         -         -         A	Mov Cap-2 Maneuver	-	-	-	-	1014	-
Stage 2         -         -         -         -         1020           Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618           HCM Lane V/C Ratio         -         -         -         0           HCM Control Delay (s)         0         -         -         0           HCM Lane LOS         A         -         -         A	Stage 1	-	-	-	-	1019	-
Approach         EB         WB         NB           HCM Control Delay, s         0         0         0           HCM LOS         A         A             Minor Lane/Major Mvmt         NBLn1         EBT         EBR         WBL         WBT           Capacity (veh/h)         -         -         -         1618           HCM Lane V/C Ratio         -         -         -         -           HCM Control Delay (s)         0         -         -         0           HCM Lane LOS         A         -         -         A	•	-	_	-	-		-
HCM Control Delay, s	U =						
HCM Control Delay, s							
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 1618 HCM Lane V/C Ratio HCM Control Delay (s) 0 0 HCM Lane LOS A - A	Approach	EB		WB			
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 1618 HCM Lane V/C Ratio HCM Control Delay (s) 0 0 HCM Lane LOS A - A	HCM Control Delay, s	0		0			
Capacity (veh/h)       -       -       -       1618         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0         HCM Lane LOS       A       -       -       A	HCM LOS					Α	
Capacity (veh/h)       -       -       -       1618         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0         HCM Lane LOS       A       -       -       A							
Capacity (veh/h)       -       -       -       1618         HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0         HCM Lane LOS       A       -       -       A	Minor Lane/Major Mymt	N	VIRI n1	EDT	EPD	\//DI	\\/DT
HCM Lane V/C Ratio       -       -       -       -         HCM Control Delay (s)       0       -       -       0         HCM Lane LOS       A       -       -       A		ľ	NDLIII	EDI			
HCM Control Delay (s) 0 0 HCM Lane LOS A A			-	-			-
HCM Lane LOS A A				-	-		-
				-	-		-
LION OF the 0/4:1- O/h)			Α	-	-		-
HCM 95th %tile Q(ven) U	HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection	
Intersection Delay, s/veh	9.5
Intersection LOS	Α

Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્ન	ĵ»		, M		
Traffic Vol, veh/h	7	66	15	6	185	202	
Future Vol, veh/h	7	66	15	6	185	202	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	7	66	15	6	185	202	
Number of Lanes	0	1	1	0	1	0	
Approach	EB		WB		SB		
Opposing Approach	WB		EB				
Opposing Lanes	1		1		0		
Conflicting Approach Left	SB				WB		
Conflicting Lanes Left	1		0		1		
Conflicting Approach Right			SB		EB		
Conflicting Lanes Right	0		1		1		
HCM Control Delay	8.3		7.8		9.8		
HCM LOS	Α		Α		Α		

	EDI 4	MDI 4	001 4
Lane	EBLn1	WBLn1	SBLn1
Vol Left, %	10%	0%	48%
Vol Thru, %	90%	71%	0%
Vol Right, %	0%	29%	52%
Sign Control	Stop	Stop	Stop
Traffic Vol by Lane	73	21	387
LT Vol	7	0	185
Through Vol	66	15	0
RT Vol	0	6	202
Lane Flow Rate	73	21	387
Geometry Grp	1	1	1
Degree of Util (X)	0.097	0.027	0.417
Departure Headway (Hd)	4.802	4.679	3.879
Convergence, Y/N	Yes	Yes	Yes
Сар	750	769	917
Service Time	2.803	2.682	1.956
HCM Lane V/C Ratio	0.097	0.027	0.422
HCM Control Delay	8.3	7.8	9.8
HCM Lane LOS	Α	Α	Α
HCM 95th-tile Q	0.3	0.1	2.1

Intersection	
Intersection Delay, s/veh	7
Intersection LOS	A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			4			4			4		
Traffic Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0	
Future Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0	
Mvmt Flow	1	0	0	0	0	3	0	1	1	3	2	0	
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0	
Approach	EB				WB			NB		SB			
Opposing Approach	WB				EB			SB		NB			
Opposing Lanes	1				1			1		1			
Conflicting Approach Lo	eft SB				NB			EB		WB			
Conflicting Lanes Left	1				1			1		1			
Conflicting Approach R	igh <b>N</b> B				SB			WB		EB			
Conflicting Lanes Right	: 1				1			1		1			
HCM Control Delay	7.1				6.3			6.6		7.6			
HCM LOS	Α				Α			Α		Α			

Lane	NBLn1	EBLn1\	NBLn1	SBLn1
Vol Left, %	0%	100%	0%	60%
Vol Thru, %	50%	0%	0%	40%
Vol Right, %	50%	0%	100%	0%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	2	1	3	5
LT Vol	0	1	0	3
Through Vol	1	0	0	2
RT Vol	1	0	3	0
Lane Flow Rate	2	1	3	5
Geometry Grp	1	1	1	1
Degree of Util (X)	0.002	0.001	0.003	0.006
Departure Headway (Hd)	3.611	4.114	3.313	4.591
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	996	874	1086	784
Service Time	1.613	2.118	1.316	2.591
HCM Lane V/C Ratio	0.002	0.001	0.003	0.006
HCM Control Delay	6.6	7.1	6.3	7.6
HCM Lane LOS	Α	Α	Α	Α
HCM 95th-tile Q	0	0	0	0

Intersection						
Int Delay, s/veh	4.2					
<u> </u>	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				NDK
Lane Configurations	<b>}</b>	101	<u>ች</u>	<b>†</b>	120	0
Traffic Vol, veh/h	93	104	6	23	138	8
Future Vol, veh/h	93	104	6	23	138	8
Conflicting Peds, #/hr	_ 0	_ 0	0	_ 0	0	0
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	18	0	18	8	0
Mvmt Flow	93	104	6	23	138	8
Major/Minor	nior1		Ania-O		Mine -1	
	ajor1		Major2		Minor1	4.45
Conflicting Flow All	0	0	197	0	180	145
Stage 1	-	-	-	-	145	-
Stage 2	-	-	-	-	35	-
Critical Hdwy	-	-	4.1	-	6.48	6.2
Critical Hdwy Stg 1	-	-	-	-	5.48	-
Critical Hdwy Stg 2	-	-	-	-	5.48	-
Follow-up Hdwy	-	-	2.2	-	3.572	3.3
Pot Cap-1 Maneuver	-	-	1388	-	796	908
Stage 1	-	-	-	-	868	-
Stage 2	-	-	-	-	972	-
Platoon blocked, %	-	-		_		
Mov Cap-1 Maneuver	-	_	1388	_	793	908
Mov Cap-2 Maneuver	_	_	-	_	793	-
Stage 1	_	_	_	_	868	_
Stage 2		_	_	_	968	_
Glage Z		-	-	_	300	_
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		10.5	
HCM LOS					В	
Miner Lene/Major Mymt		UDI p1	ГОТ	EDD	WDI	WDT
Minor Lane/Major Mvmt	ľ	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		799	-	-	1388	-
HCM Lane V/C Ratio		0.183	-	-	0.004	-
HCM Control Delay (s)		10.5	-	-	7.6	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		0.7	-	-	0	-

Intersection						
Int Delay, s/veh	2.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Movement		LDK				NDK
Lane Configurations	<b>}</b>	400	7	450	<b>**</b> **	۲0
Traffic Vol, veh/h	145	189	5	156	84	52
Future Vol, veh/h	145	189	5	156	84	52
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage,	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	13	9	0	10	31	4
Mvmt Flow	145	189	5	156	84	52
	/lajor1		Major2		Minor1	
Conflicting Flow All	0	0	334	0	406	240
Stage 1	-	-	-	-	240	-
Stage 2	-	-	-	-	166	-
Critical Hdwy	-	-	4.1	-	6.71	6.24
Critical Hdwy Stg 1	-	-	-	-	5.71	-
Critical Hdwy Stg 2	_	_	-	_	5.71	-
Follow-up Hdwy	_	-	2.2	_		3.336
Pot Cap-1 Maneuver	_	_	1237	_	549	794
Stage 1	_	_		_	736	-
Stage 2	_			_	798	_
Platoon blocked, %	_	_	_	_	130	_
		-	1237		547	794
Mov Cap-1 Maneuver	-	-		-		
Mov Cap-2 Maneuver	-	-	-	-	547	-
Stage 1	-	-	-	-	736	-
Stage 2	-	-	-	-	795	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		12.4	
HCM LOS	U		0.2		12.4 B	
I IOIVI LOS					D	
Minor Lane/Major Mvm	t N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		621			1237	_
HCM Lane V/C Ratio		0.219	_		0.004	_
HCM Control Delay (s)		12.4	_	_	7.9	
HCM Lane LOS		В	_	_	Α.5	_
		0.8				
HCM 95th %tile Q(veh)		۵.8	-	-	0	-

Intersection												
Int Delay, s/veh	5.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ች	ĵ.			4	7		4	
Traffic Vol, veh/h	1	135	31	114	122	4	21	1	198	1	1	0
Future Vol, veh/h	1	135	31	114	122	4	21	1	198	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	135	31	114	122	4	21	1	198	1	1	0
Major/Minor N	/lajor1		ا	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	126	0	0	166	0	0	506	507	151	604	520	124
Stage 1	-	-	-	-	-	-	153	153	-	352	352	-
Stage 2	-	-	-	-	-	-	353	354	-	252	168	-
Critical Hdwy	4.1	-	-	4.4	-	-	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.25	5.5	-	6.1	6.5	-
Follow-up Hdwy	2.2	-	-	2.47	-	-	3.635	4	3.39	3.5	4.9	3.3
Pot Cap-1 Maneuver	1473	-	-	1259	-	-	457	471	875	413	347	932
Stage 1	-	-	-	-	-	-	820	775	-	669	490	-
Stage 2	-	-	_	-	-	-	638	634	-	757	607	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1473	-	-	1259	-	-	424	428	875	297	315	932
Mov Cap-2 Maneuver	-	-	-	-	-	-	424	428	-	297	315	-
Stage 1	-	-	-	-	-	-	819	774	-	668	445	-
Stage 2	-	-	-	-	-	-	579	576	-	585	606	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			3.9			10.7			16.8		
HCM LOS							В			С		
Minor Lane/Major Mvm		NBLn1 I	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBL <sub>n1</sub>		
Capacity (veh/h)		424	875	1473	-	-	1259	-	-	306		
HCM Lane V/C Ratio		0.052	0.226	0.001	-	-	0.091	-	-	0.007		
HCM Control Delay (s)		14	10.3	7.4	-	-	8.1	-	-	16.8		
HCM Lane LOS		В	В	Α	-	-	Α	-	-	С		
HCM 95th %tile Q(veh)		0.2	0.9	0	-	-	0.3	-	-	0		
,												

Intersection												
Int Delay, s/veh	5.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	f)		ሻ	<del>(</del> î	
Traffic Vol, veh/h	20	8	13	10	9	66	8	126	14	93	49	4
Future Vol, veh/h	20	8	13	10	9	66	8	126	14	93	49	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	20	8	13	10	9	66	8	126	14	93	49	4
Major/Minor N	/linor2			Minor1		- 1	Major1			Major2		
Conflicting Flow All	424	393	51	397	388	133	53	0	0	140	0	0
Stage 1	237	237	-	149	149	-	-	-	-	-	-	-
Stage 2	187	156	_	248	239	_	<u>-</u>	<u>-</u>	_	_	_	_
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1		_	4.46	_	_
Critical Hdwy Stg 1	6.1	5.5	- 0.20	6.1	5.5	- 0.70	-T. I	<u> </u>	_		_	_
Critical Hdwy Stg 2	6.1	5.5	_	6.1	5.5	_	_		_		_	_
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	<u> </u>	_	2.524	_	_
Pot Cap-1 Maneuver	544	546	1000	567	550	863	1566	_		1259		
Stage 1	771	713	-	858	778	- 000	1000	<u> </u>	_	1200	_	_
Stage 2	819	772	-	760	711	-	<u>-</u>	<u>-</u>	-	-	-	-
Platoon blocked, %	013	112	_	700	7 1 1		_	_	_	_	_	_
Mov Cap-1 Maneuver	466	503	1000	520	507	863	1566	_	-	1259	_	
Mov Cap-1 Maneuver	466	503	-	520	507	- 000	1500	_	_	1209	_	_
Stage 1	767	660		854	774	_	-	_	_	-	-	_
Stage 2	744	768	-	686	658	-	_	-	_	_	_	_
Olaye Z	144	7 00	-	000	000	-	<u>-</u>	<u>-</u>	-	-	-	_
A				MA			ND			0.5		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.8			10.4			0.4			5.2		
HCM LOS	В			В								
Minor Lane/Major Mvmt	t	NBL	NBT	NBR I	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1566	-	-	571	749	1259	-	-			
HCM Lane V/C Ratio		0.005	-	-		0.113		-	-			
HCM Control Delay (s)		7.3	-	-	11.8	10.4	8.1	-	-			
HCM Lane LOS		Α	-	-	В	В	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	0.2	0.4	0.2	-	-			

Intersection						
Int Delay, s/veh	0.1					
		WDD	NET	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ.			र्स
Traffic Vol, veh/h	2	0	26	0	0	227
Future Vol, veh/h	2	0	26	0	0	227
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	40	0	0	6
Mvmt Flow	2	0	26	0	0	227
WWW.CT IOW	_	•	20	•		
Major/Minor N	/linor1		//ajor1	N	Major2	
Conflicting Flow All	253	26	0	0	26	0
Stage 1	26	-	-	-	-	-
Stage 2	227	-	-	-	-	-
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	_	-	-	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	740	1056	_	_	1601	_
Stage 1	1002	-	_	_	-	_
Stage 2	815	_			_	
Platoon blocked, %	013	-	-	_	_	-
	740	1056	-	-	1601	-
Mov Cap-1 Maneuver	740	1056	-	-	1601	-
Mov Cap-2 Maneuver	740	-	-	-	-	-
Stage 1	1002	-	-	-	-	-
Stage 2	815	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.9		0		0	
HCM LOS	9.9 A		U		U	
TICIVI LOG						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)			-	740	1601	_
HCM Lane V/C Ratio		_	_	0.003	-	_
HCM Control Delay (s)		_	_		0	_
HCM Lane LOS		_	_	Α	A	_
HCM 95th %tile Q(veh)		_		0	0	_
How som whe Q(ven)		_	-	U	U	-

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	40	0	0	184	0
Future Vol, veh/h	0	0	0	0	0	0	0	40	0	0	184	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	40	0	0	184	0
Major/Minor N	1inor2		N	Minor1		N	Major1		N	Major2		
Conflicting Flow All	224	224	184	224	224	40	184	0	0	40	0	0
Stage 1	184	184	-	40	40	-	-	-	-	-	-	-
Stage 2	40	40	-	184	184	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	_	-	-	_	_
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	736	678	864	736	678	1037	1403	-	-	1583	-	-
Stage 1	822	751	-	980	866	-	-	-	-	_	-	-
Stage 2	980	866	-	822	751	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	736	678	864	736	678	1037	1403	-	-	1583	-	-
Mov Cap-2 Maneuver	736	678	-	736	678	-	-	-	-	-	-	-
Stage 1	822	751	-	980	866	-	-	-	-	-	-	-
Stage 2	980	866	-	822	751	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A			•					
	, ,			, ,								
Minor Lane/Major Mvmt		NBL	NBT	NRR	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1403	-	- 12111			1583		-			
HCM Lane V/C Ratio		1405	-	-	-	_	-	_	<u>-</u>			
HCM Control Delay (s)		0	<u>-</u>		0	0	0	<u>-</u>	<u>-</u>			
HCM Lane LOS		A	-	-	A	A	A	_	<u>-</u>			
HCM 95th %tile Q(veh)		0		_	-	-	0	_	_			
HOW JOHN JOHN Q (VEH)		U					U					

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	34	0	9	0	24	9	38	155	0
Future Vol, veh/h	0	0	0	34	0	9	0	24	9	38	155	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	6	0	44	0	39	33	16	13	0
Mvmt Flow	0	0	0	34	0	9	0	24	9	38	155	0
Major/Minor N	/linor2			Minor1		ľ	Major1		ľ	Major2		
Conflicting Flow All	264	264	155	260	260	29	155	0	0	33	0	0
Stage 1	231	231	-	29	29	-	-	-	-	-	_	_
Stage 2	33	33	-	231	231	_	-	-	_	_	-	-
Critical Hdwy	7.1	6.5	6.2	7.16	6.5	6.64	4.1	-	-	4.26	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.16	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.554	4	3.696	2.2	-	-	2.344	-	-
Pot Cap-1 Maneuver	693	645	896	685	648	937	1438	-	-	1493	-	-
Stage 1	776	717	-	978	875	-	-	-	-	-	-	-
Stage 2	988	872	-	763	717	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	672	627	896	671	630	937	1438	-	-	1493	-	-
Mov Cap-2 Maneuver	672	627	-	671	630	-	-	-	-	-	-	-
Stage 1	776	697	-	978	875	-	-	-	-	-	-	-
Stage 2	979	872	-	742	697	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			10.4			0			1.5		
HCM LOS	A			В								
Minor Lane/Major Mvmt	t	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1438				713	1493		_			
HCM Lane V/C Ratio		-	_	_	_		0.025	-	_			
HCM Control Delay (s)		0	_	_	0	10.4	7.5	0	_			
HCM Lane LOS		A	_	_	A	В	A	A	_			
HCM 95th %tile Q(veh)		0	-	-	-	0.2	0.1	-	_			
						7.2	<b>J</b> .,					

Intersection Int Delay, s/veh Movement						
	0.1					
iviovement		EDD	NDI	NDT	CDT	CDD
	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M	•		- 4	ĵ.	_
Traffic Vol, veh/h	0	0	1	13	44	7
Future Vol, veh/h	0	0	1	13	44	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	13	44	7
NA : /NA:	\d: 0				4 : 0	
	Minor2		//ajor1		/lajor2	
Conflicting Flow All	63	48	51	0	-	0
Stage 1	48	-	-	-	-	-
Stage 2	15	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	_	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	948	1027	1568	-	-	-
Stage 1	980	-	-	-	-	-
Stage 2	1013	_	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	947	1027	1568	-	_	-
Mov Cap-2 Maneuver	947	-	-	_	_	_
Stage 1	979	_	_	_	_	_
Stage 2	1013	_	_	_	_	_
Olage 2	1010					
	EB		NB		SB	
Approach	0		0.5		0	
Approach HCM Control Delay, s						
	Α					
HCM Control Delay, s	Α					
HCM Control Delay, s HCM LOS		NRI	NRT	FRI n1	SRT	SRR
HCM Control Delay, s HCM LOS Minor Lane/Major Mvm		NBL 1569	NBT	EBLn1	SBT	SBR
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h)		1568	-	-	-	-
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	nt	1568 0.001	-	-	- -	-
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	nt	1568 0.001 7.3	- - 0	- - 0	- - -	- - -
HCM Control Delay, s HCM LOS  Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	nt	1568 0.001	-	-	- -	-

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Future Vol, veh/h	1	0	0	0	0	0	0	13	0	1	43	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow	1	0	0	0	0	0	0	13	0	1	43	0
Major/Minor I	Minor2			Minor1		N	Major1		N	Major2		
Conflicting Flow All	58	58	43	58	58	13	43	0	0	13	0	0
		45		13	13	13	43	U	U	13		
Stage 1	45 13	13	-	45	45	-	-	-	-	-	-	-
Stage 2	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy	6.1			6.1	5.5	0.Z -		-	-		-	-
Critical Hdwy Stg 1		5.5	-	6.1			-	-	<del>-</del>	-	-	-
Critical Hdwy Stg 2	6.1	5.5	2.2		5.5	2.2	-	-	-	2.2	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	944	837	1033	944	837	1073	1579	-	-	1619	-	-
Stage 1	974	861	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	974	861	-	-	-	-	-	-	-
Platoon blocked, %	0.40	000	1000	040	000	1072	1570	-	-	1010	-	-
Mov Cap-1 Maneuver	943	836	1033	943	836	1073	1579	-	-	1619	-	-
Mov Cap-2 Maneuver	943	836	-	943	836	-	-	-	-	-	-	-
Stage 1	974	860	-	1013	889	-	-	-	-	-	-	-
Stage 2	1013	889	-	973	860	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	8.8			0			0			0.2		
HCM LOS	Α			A								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR F	EBLn1V	VRI n1	SBL	SBT	SBR			
	n e	1579	1101	-	0.40			001	OBIT			
Capacity (veh/h) HCM Lane V/C Ratio			-		0.001	-	0.001	-	=			
		_	-					-	-			
HCM Lang LOS		0	-	-	8.8	0	7.2	0	-			
HCM Ceth % tile O(vah)	١	A	-	-	A	Α	A	Α	-			
HCM 95th %tile Q(veh)	)	0	-	-	0	-	0	-	-			

Intersection						
Int Delay, s/veh	5.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		<b>1</b>			4
Traffic Vol, veh/h	35	22	9	3	2	24
Future Vol, veh/h	35	22	9	3	2	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	_	- 13113	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	3	0	22	0	22	0
Mvmt Flow	35	22	9	3	2	24
IVIVIII TIOW	00		J	0		27
	Minor1		Major1		Major2	
Conflicting Flow All	39	11	0	0	12	0
Stage 1	11	-	-	-	-	-
Stage 2	28	-	-	-	-	-
Critical Hdwy	6.43	6.2	-	-	4.32	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	-	-	2.398	-
Pot Cap-1 Maneuver	970	1076	-	-	1486	-
Stage 1	1009	-	-	-	-	-
Stage 2	992	-	-	-	_	_
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	969	1076	-	-	1486	-
Mov Cap-2 Maneuver	969	-	-	-	-	-
Stage 1	1009	_	_	-	-	_
Stage 2	991	_	_	_	_	_
olago L	001					
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		0.6	
HCM LOS	Α					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	1008	1486	_
HCM Lane V/C Ratio		_	_	0.057		-
HCM Control Delay (s)		_	_	8.8	7.4	0
HCM Lane LOS		_	-	A	A	A
HCM 95th %tile Q(veh)		-	_	0.2	0	-
7000 4(1011)				V. <u>~</u>		

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4	7		4			4	
Traffic Vol, veh/h	12	8	0	1	98	138	0	1	0	54	0	30
Future Vol, veh/h	12	8	0	1	98	138	0	1	0	54	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	58	0	0	0	8	10	0	0	0	19	0	10
Mvmt Flow	12	8	0	1	98	138	0	1	0	54	0	30
Major/Minor	Major1		1	Major2		N	Minor1		1	Minor2		
Conflicting Flow All	236	0	0	8	0	0	216	270	8	133	132	98
Stage 1	-	-	-	-	-	-	32	32	-	100	100	-
Stage 2	-	-	-	-	-	-	184	238	-	33	32	-
Critical Hdwy	4.68	-	-	4.1	-	-	7.1	6.5	6.2	7.29	6.5	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.29	5.5	-
Follow-up Hdwy	2.722	-	-	2.2	-	-	3.5	4	3.3	3.671	4	3.39
Pot Cap-1 Maneuver	1063	-	-	1625	-	-	745	640	1080	801	762	936
Stage 1	-	-	-	-	-	-	990	872	-	866	816	-
Stage 2	-	-	-	-	-	-	822	712	-	941	872	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1063	-	-	1625	-	-	714	632	1080	793	753	936
Mov Cap-2 Maneuver	-	-	-	-	-	-	714	632	-	793	753	-
Stage 1	-	-	-	-	-	-	979	862	-	856	815	-
Stage 2	-	-	-	-	-	-	795	711	-	930	862	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	5.1			0			10.7			9.8		
HCM LOS							В			Α		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		632	1063	-	-	1625	-	_	839			
HCM Lane V/C Ratio		0.002		-	-	0.001	-	-	0.1			
HCM Control Delay (s)		10.7	8.4	0	-	7.2	0	-	9.8			
HCM Lane LOS		В	Α	Α	-	Α	Α	-	Α			
HCM 95th %tile Q(veh	)	0	0	-	-	0	-	-	0.3			

Intersection												
Int Delay, s/veh	2.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	1>		ች	₽			4			4	
Traffic Vol, veh/h	1	36	1	11	345	31	4	0	9	71	2	7
Future Vol, veh/h	1	36	1	11	345	31	4	0	9	71	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	36	1	11	345	31	4	0	9	71	2	7
Major/Minor M	lajor1		ı	Major2		N	Minor1			Minor2		
Conflicting Flow All	376	0	0	37	0	0	426	437	37	426	422	361
Stage 1	-	-	-	-	-	-	39	39	-	383	383	-
Stage 2	-	-	-	-	-	-	387	398	-	43	39	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4	3.3	3.581	4	3.561
Pot Cap-1 Maneuver	798	-	-	1587	-	-	542	516	1041	527	526	627
Stage 1	-	-	-	-	-	-	981	866	-	626	616	-
Stage 2	-	-	-	-	-	-	641	606	-	954	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	798	-	-	1587	-	-	531	512	1041	519	522	627
Mov Cap-2 Maneuver	-	-	-	-	-	-	531	512	-	519	522	-
Stage 1	-	-	-	-	-	-	980	865	-	625	612	-
Stage 2	-	-	-	-	-	-	627	602	-	945	865	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			9.6			13.1		
HCM LOS							Α			В		
Minor Lane/Major Mvmt	1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		804	798	_	_	1587	_		527			
HCM Lane V/C Ratio		0.016		_	_	0.007	_	_	0.152			
HCM Control Delay (s)		9.6	9.5	-	-	7.3	-	-	13.1			
HCM Lane LOS		A	A	_	_	A	_	_	В			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0.5			

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.			4						4	
Traffic Vol, veh/h	0	10	88	8	305	0	0	0	0	2	2	99
Future Vol, veh/h	0	10	88	8	305	0	0	0	0	2	2	99
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	10	88	8	305	0	0	0	0	2	2	99
Major/Minor N	/lajor1			Major2					N	/linor2		
Conflicting Flow All	-	0	0	98	0	0				375	419	305
Stage 1	_	-	-	-	-	-				321	321	-
Stage 2	_	_	_	_	_	-				54	98	_
Critical Hdwy	_	_	-	4.85	_	-				6.4	7	6.34
Critical Hdwy Stg 1	_	_	-		_	_				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1141	-	0				630	459	708
Stage 1	0	-	-	-	-	0				740	574	-
Stage 2	0	-	-	-	-	0				974	730	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1141	-	-				625	0	708
Mov Cap-2 Maneuver	-	-	-	-	-	-				625	0	-
Stage 1	-	-	-	-	-	-				740	0	-
Stage 2	-	-	-	-	-	-				966	0	-
Ŭ												
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						11		
HCM LOS				0.2						В		
Minor Lane/Major Mvmt	f	EBT	EBR	WBL	WRT	SBLn1						
Capacity (veh/h)		LDI	- EBR		- 1000	706						
HCM Lane V/C Ratio		-		0.007		0.146						
		-	-	8.2		11						
HCM Control Delay (s) HCM Lane LOS		-			0 A	В						
HCM 95th %tile Q(veh)		-	-	A 0	- -	0.5						
		-	-	U		0.5						

Int Delay, siveh   10.4	Intersection												
Lane Configurations		10.4											
Traffic Vol, veh/h	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	Lane Configurations		सी			1₃			43-				
Future Vol, veh/h		9		0	0		1	309		24	0	0	0
Sign Control         Free RTCANNOR         Free RTCANNOR         Free RTCANNOR         Free RTCANNOR         Free RTCANNOR         Free RTCANNOR         Stop RTCANNOR         Stop None         None         - None	Future Vol, veh/h	9	3	0	0	4	1	309	5	24	0	0	0
Sign Control         Free RTCE RTCE AND NOTE         Free RTC Annelized         Free RTCA Annelized         F	Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
RT Channelized		Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Veh in Median Storage, #         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         0         0         -         0         0         -         0         0         -         0 <td>RT Channelized</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td>	RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Grade, %	Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Peak Hour Factor         100         0         26         26         2         2         2         2         2         2         2         2         2         3         3         4         3         4	Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, %	Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Mynt Flow         9         3         0         0         4         1         309         5         24         0         0         0           Major/Minor         Major1         Major2         Minor1         Minor2	Peak Hour Factor	100	100	100	100		100						
Major/Minor   Major1   Major2   Minor1	Heavy Vehicles, %	14		0	0							2	2
Conflicting Flow All	Mvmt Flow	9	3	0	0	4	1	309	5	24	0	0	0
Conflicting Flow All													
Stage 1       -       -       -       -       21       21       -         Stage 2       -       -       -       -       5       5       -         Critical Hdwy       4.24       -       -       -       6.5       7.3       6.46         Critical Hdwy Stg 1       -       -       -       -       5.5       6.3       -         Critical Hdwy Stg 2       -       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       3.59       4.72       3.534         Pot Cap-1 Maneuver       1541       -       0       0       -       969       736       1015         Stage 1       -       0       0       -       988       759       -         Platoon blocked, %       -       -       -       -       -       -         Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       998       0       -         Stage 2       -       -       -       -       998	Major/Minor I	Major1		<u> </u>	Major2		<u> </u>	Minor1					
Stage 2       -       -       -       -       5       5       -         Critical Hdwy       4.24       -       -       -       6.5       7.3       6.46         Critical Hdwy Stg 1       -       -       -       5.5       6.3       -         Critical Hdwy Stg 2       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       969       736       1015         Stage 1       -       0       0       -       981       745       -         Stage 2       -       0       0       -       981       759       -         Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       975       0       -         Stage 1       -       -       -       -       975       0       - </td <td>Conflicting Flow All</td> <td>5</td> <td>0</td> <td>-</td> <td>-</td> <td>-</td> <td>0</td> <td></td> <td></td> <td>3</td> <td></td> <td></td> <td></td>	Conflicting Flow All	5	0	-	-	-	0			3			
Critical Hdwy       4.24       -       -       -       -       6.5       7.3       6.46         Critical Hdwy Stg 1       -       -       -       -       5.5       6.3       -         Critical Hdwy Stg 2       -       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       3.59       4.72       3.534         Pot Cap-1 Maneuver       1541       -       0       0       -       969       736       1015         Stage 1       -       -       0       0       -       981       745       -         Stage 2       -       -       0       0       -       983       759       -         Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       963       0       -         Stage 1       -       -       -       -       975       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB	Stage 1	-	-	-	-	-	-	21	21	-			
Critical Hdwy Stg 1       -       -       -       -       5.5       6.3       -         Critical Hdwy Stg 2       -       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       -       3.59       4.72       3.534         Pot Cap-1 Maneuver       1541       -       0       0       -       981       745       -         Stage 2       -       -       0       0       -       988       759       -         Platoon blocked, %       -       -       -       998       759       -         Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       963       0       -         Stage 1       -       -       -       -       975       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7       -       -         HCM	Stage 2	-	-	-	-	-	-						
Critical Hdwy Stg 2       -       -       -       -       5.5       6.3       -         Follow-up Hdwy       2.326       -       -       -       3.59       4.72       3.534         Pot Cap-1 Maneuver       1541       -       0       0       -       969       736       1015         Stage 1       -       -       0       0       -       998       759       -         Platoon blocked, %       -       -       -       998       759       -         Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       963       0       -         Stage 1       -       -       -       -       995       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7         HCM Lane/Major Mvmt       NBLn1       EBL       EBT       WBT       WBR         Capacity (veh/h)       967       1541		4.24	-	-	-	-	-			6.46			
Follow-up Hdwy 2.326 3.59 4.72 3.534  Pot Cap-1 Maneuver 1541 - 0 0 - 969 736 1015  Stage 1 - 0 0 0 - 981 745 - Stage 2 - 0 0 0 - 998 759 -  Platoon blocked, % 963 0 1015  Mov Cap-1 Maneuver 1541 963 0 1015  Mov Cap-2 Maneuver 963 0 - Stage 1 963 0 - Stage 2 975 0 - Stage 2 998 0 -   Approach EB WB NB  HCM Control Delay, s 5.5 0 10.7  HCM LOS B  Minor Lane/Major Mvmt NBLn1 EBL EBT WBT WBR  Capacity (veh/h) 967 1541  HCM Lane V/C Ratio 0.35 0.006  HCM Control Delay (s) 10.7 7.3 0  HCM Lane LOS B A A	Critical Hdwy Stg 1	-	-	-	-	-	-			-			
Pot Cap-1 Maneuver       1541       -       0       0       -       -       969       736       1015         Stage 1       -       -       0       0       -       -       981       745       -         Stage 2       -       -       0       0       -       -       998       759       -         Plation blocked, %       -	Critical Hdwy Stg 2		-	-	-	-	-						
Stage 1       -       -       0       0       -       -       981       745       -         Stage 2       -       -       0       0       -       -       998       759       -         Platoon blocked, %       -<			-	-	-	-	-						
Stage 2       -       0       0       -       998       759       -         Platoon blocked, %       -       -       -       -       -       -         Mov Cap-1 Maneuver       1541       -       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       -       963       0       -         Stage 1       -       -       -       -       -       975       0       -         Stage 2       -       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7         HCM Lane/Major Mvmt       NBLn1       EBL       EBT       WBT       WBR         Minor Lane/Major Mvmt       NBLn1       EBL       EBT       WBT       WBR         Capacity (veh/h)       967       1541       -       -       -         HCM Lane V/C Ratio       0.35       0.006       -       -       -         HCM Control Delay (s)       10.7       7.3       0       -	Pot Cap-1 Maneuver	1541	-		0	-	-			1015			
Platoon blocked, %		-	-	0		-	-			-			
Mov Cap-1 Maneuver       1541       -       -       -       963       0       1015         Mov Cap-2 Maneuver       -       -       -       -       975       0       -         Stage 1       -       -       -       -       975       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7       HCM LOS       B     Minor Lane/Major Mvmt  NBLn1  EBL  EBT  WBT  WBR  Capacity (veh/h)  967  1541   HCM Lane V/C Ratio  0.35  0.006   - HCM Control Delay (s)  10.7  7.3  0  - HCM Control Delay (s)  B  A  A        -		-	-	0	0	-	-	998	759	-			
Mov Cap-2 Maneuver       -       -       -       -       963       0       -         Stage 1       -       -       -       -       975       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7       HCM LOS       B         Minor Lane/Major Mvmt       NBLn1       EBL       EBT       WBT       WBR         Capacity (veh/h)       967       1541       -       -       -         HCM Lane V/C Ratio       0.35       0.006       -       -       -         HCM Control Delay (s)       10.7       7.3       0       -       -         HCM Lane LOS       B       A       A       -       -			-			-	-						
Stage 1       -       -       -       -       975       0       -         Stage 2       -       -       -       -       998       0       -         Approach       EB       WB       NB         HCM Control Delay, s       5.5       0       10.7         HCM Lane //Major Mvmt       NBLn1       EBL       EBT       WBT       WBR         Capacity (veh/h)       967       1541       -       -       -         HCM Lane V/C Ratio       0.35       0.006       -       -       -         HCM Control Delay (s)       10.7       7.3       0       -       -         HCM Lane LOS       B       A       A       -       -		1541	-	-	-	-	-			1015			
Stage 2         -         -         -         -         998         0         -           Approach         EB         WB         NB           HCM Control Delay, s         5.5         0         10.7           HCM LOS         B    Minor Lane/Major Mvmt  NBLn1  EBL  EBT  WBT  WBR  Capacity (veh/h)  967  1541  HCM Lane V/C Ratio  0.35  0.006  HCM Control Delay (s)  10.7  7.3  0  HCM Lane LOS  B  A  A		-	-	-	-	-	-			-			
Approach         EB         WB         NB           HCM Control Delay, s         5.5         0         10.7           HCM LOS         B           Minor Lane/Major Mvmt         NBLn1         EBL         EBT         WBT         WBR           Capacity (veh/h)         967         1541         -         -         -           HCM Lane V/C Ratio         0.35         0.006         -         -         -           HCM Control Delay (s)         10.7         7.3         0         -         -           HCM Lane LOS         B         A         A         -         -		-	-	-	-	-	-			-			
HCM Control Delay, s   5.5   0   10.7     HCM LOS	Stage 2	-	-	-	-	-	-	998	0	-			
HCM Control Delay, s   5.5   0   10.7     HCM LOS													
Minor Lane/Major Mvmt         NBLn1         EBL         EBT         WBT         WBR           Capacity (veh/h)         967         1541         -         -         -           HCM Lane V/C Ratio         0.35         0.006         -         -         -           HCM Control Delay (s)         10.7         7.3         0         -         -           HCM Lane LOS         B         A         A         -         -	Approach	EB			WB			NB					
Minor Lane/Major Mvmt         NBLn1         EBL         EBT         WBT         WBR           Capacity (veh/h)         967         1541         -         -         -           HCM Lane V/C Ratio         0.35         0.006         -         -         -           HCM Control Delay (s)         10.7         7.3         0         -         -           HCM Lane LOS         B         A         A         -         -	HCM Control Delay, s	5.5			0			10.7					
Capacity (veh/h) 967 1541 HCM Lane V/C Ratio 0.35 0.006 HCM Control Delay (s) 10.7 7.3 0 HCM Lane LOS B A A	HCM LOS							В					
Capacity (veh/h) 967 1541  HCM Lane V/C Ratio 0.35 0.006  HCM Control Delay (s) 10.7 7.3 0  HCM Lane LOS B A A													
HCM Lane V/C Ratio 0.35 0.006 HCM Control Delay (s) 10.7 7.3 0 HCM Lane LOS B A A	Minor Lane/Major Mvm	nt N	NBL <sub>n1</sub>	EBL	EBT	WBT	WBR						
HCM Control Delay (s) 10.7 7.3 0 HCM Lane LOS B A A					-	-	-						
HCM Lane LOS B A A	HCM Lane V/C Ratio		0.35	0.006	-	-	-						
HCM Lane LOS B A A	HCM Control Delay (s)		10.7	7.3	0	-	-						
HCM 95th %tile Q(veh) 1.6 0	HCM Lane LOS			Α	Α	-	-						
	HCM 95th %tile Q(veh)	)	1.6	0	-	-	-						

## 15: I-82 Southbound On-Ramp/I-82 Southbound Off-Ramp & Coffin Road Weekday Morning Peak Hour

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĥ			4						4	
Traffic Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Future Vol, veh/h	0	5	2	3	11	0	0	0	0	10	6	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	_	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	_	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	0	0	50	0	2	2	2	10	83	0
Mvmt Flow	0	5	2	3	11	0	0	0	0	10	6	3
Major/Minor N	/lajor1		<u> </u>	Major2					<u> </u>	/linor2		
Conflicting Flow All	-	0	0	7	0	0				23	24	11
Stage 1	-	-	-	-	-	-				17	17	-
Stage 2	-	-	-	-	-	-				6	7	-
Critical Hdwy	-	-	-	4.1	-	-				6.5	7.33	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.5	6.33	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.5	6.33	-
Follow-up Hdwy	-	-	-	2.2	-	-				3.59	4.747	3.3
Pot Cap-1 Maneuver	0	-	-	1627	-	0				973	734	1076
Stage 1	0	-	-	-	-	0				985	744	-
Stage 2	0	-	-	-	-	0				997	753	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1627	-	-				971	0	1076
Mov Cap-2 Maneuver	-	-	-	-	-	-				971	0	-
Stage 1	-	-	-	-	-	-				985	0	-
Stage 2	-	-	-	-	-	-				995	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			1.5						8.7		
HCM LOS										Α		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-	-	1627	-	993						
HCM Lane V/C Ratio		-		0.002	-	0.019						
HCM Control Delay (s)		-	-		0	8.7						
HCM Lane LOS		-	-	Α	A	Α						
HCM 95th %tile Q(veh)		-	-	0	-	0.1						
,												

Intersection													
Int Delay, s/veh	2.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स			ĵ.			4					
Traffic Vol, veh/h	0	15	0	0	14	4	0	5	9	0	0	0	
Future Vol, veh/h	0	15	0	0	14	4	0	5	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
_	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	_	-	0	_	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2	
Mvmt Flow	0	15	0	0	14	4	0	5	9	0	0	0	
Major/Minor Major/Minor	ajor1		N	Major2		N	Minor1						
Conflicting Flow All	18	0	-	-	-	0	31	33	15				
Stage 1	-	-	-	-	-	-	15	15	-				
Stage 2	-	-	-	-	-	-	16	18	-				
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.3	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-				
Follow-up Hdwy	2.2	-	-	-	-	-	3.5	4.72	3.408				
Pot Cap-1 Maneuver	1612	-	0	0	-	-	988	729	1036				
Stage 1	-	-	0	0	-	-	1013	750	-				
Stage 2	-	-	0	0	-	-	1012	748	_				
Platoon blocked, %		-			-	-							
•	1612	-	-	-	-	-	988	0	1036				
Mov Cap-2 Maneuver	-	-	-	-	-	-	988	0	-				
Stage 1	-	-	-	-	-	-	1013	0	-				
Stage 2	-	-	-	-	-	-	1012	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	0			0			8.5						
HCM LOS							Α						
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		1036	1612	-	-	-							
HCM Lane V/C Ratio		0.014	-	-	-	-							
HCM Control Delay (s)		8.5	0	_	_	-							
		A	A	_	_	_							
HCM Lane LOS		, ,											

| Int Delay, s/veh   0.8   NBC   SBC   |--|
| Movement         EBL         EBT         EBR         WBL         WBR         WBL         NBT         NBR         SBL         SBT         SBR           Lane Configurations         4         4         4         0         2         0         0         0         0         0         2           Traffic Vol, veh/h         0         22         2         0         14         0         2         0         0         0         0         0         0         2           Future Vol, veh/h         0         22         2         0         14         0         2         0  |
| Lane Configurations         Image: Configuration of the proof of                               |
| Traffic Vol, veh/h         0         22         2         0         14         0         2         0         0         0         0         2           Future Vol, veh/h         0         22         2         0         14         0         2         0   |
| Future Vol, veh/h         0         22         2         0         14         0         2         0         0         0         0         2           Conflicting Peds, #/hr         0   |
| Conflicting Peds, #/hr         0   |
| Sign Control         Free         Free         Free         Free         Free         Free         Free         Free         Free         Stop  |
| RT Channelized         -         -         None         -         -         None         -         -         None           Storage Length         -   |
| Storage Length         -   |
| Veh in Median Storage, # -       0       -       -       0       -       -       0       -       -       0       -         Grade, %       -       0       -       -       0       -       -       0       -       -       0       -         Peak Hour Factor       100   |
| Grade, % - 0 0 0 0 0 Peak Hour Factor 100 100 100 100 100 100 100 100 100 10   |
| Peak Hour Factor         100   |
| Heavy Vehicles, % 0 13 0 0 21 0 0 0 0 0 50   |
|  |
| MVMt Flow 0 22 2 0 14 0 2 0 0 0 2  |
|  |
|  |
| Major/Minor Major1 Major2 Minor1 Minor2  |
| Conflicting Flow All 14 0 0 24 0 0 38 37 23 37 38 14   |
| Stage 1 23 23 - 14 14 -  |
| Stage 2 15 14 - 23 24 -  |
| Critical Hdwy 4.1 4.1 7.1 6.5 6.2 7.1 6.5 6.7  |
| Critical Hdwy Stg 1 6.1 5.5 - 6.1 5.5 -  |
| Critical Hdwy Stg 2 6.1 5.5 - 6.1 5.5 -  |
| Follow-up Hdwy 2.2 2.2 3.5 4 3.3 3.5 4 3.75  |
| Pot Cap-1 Maneuver 1617 1604 972 859 1060 973 858 942  |
| Stage 1 1000 880 - 1011 888 -  |
| Stage 2 1010 888 - 1000 879 -  |
| Platoon blocked, %   |
| Mov Cap-1 Maneuver 1617 1604 970 859 1060 973 858 942  |
| Mov Cap-2 Maneuver 970 859 - 973 858 -   |
| Stage 1 1000 880 - 1011 888 -  |
| Stage 2 1008 888 - 1000 879 -  |
|  |
| Approach EB WB NB SB   |
| HCM Control Delay, s 0 0 8.7 8.8   |
| HCM LOS A A  |
| TIOM LOO   |
|  |
| Minor Lane/Major Mvmt NBLn1 EBL EBT EBR WBL WBT WBR SBLn1  |
| Capacity (veh/h) 970 1617 1604 942   |
| HCM Lane V/C Ratio 0.002 0.002   |
| HCM Control Delay (s) 8.7 0 0 8.8  |
| HCM Lane LOS A A A   |
| HCM 95th %tile Q(veh) 0 0 0 - 0  |

Movement   EBL   EBT   EBR   WBL   WBT   WBR   NBL   NBT   NBR   SBL   SBT   SBR	Intersection												
Lane Configurations	Int Delay, s/veh	7.4											
Traffic Vol, veh/h	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h	Lane Configurations		ĵ.			सी						43-	
Conflicting Peds, #/hr		0		5	118		0	0	0	0	67		36
Conflicting Peds, #/hr	Future Vol. veh/h	0	30	5	118	10	0	0	0	0	67	1	36
Sign Control         Free RTCE RTCE AND AND AND AND AND AND AND AND AND AND	<u> </u>	0	0	0		0	0	0	0	0	0		0
RT Channelized	•		Free	Free	Free	Free	Free		Stop	Stop			Stop
Storage Length													
Veh in Median Storage, #         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         0         -         0         0         -         0         0         0         0         100		_	_		_	_		-	_		_	-	-
Grade, % - 0 0 0 0 0 - 0 0 0 0 0 0 0 0		.# -	0	_	_	0	-	-	0	_	_	0	_
Peak Hour Factor         100         0         2         3         3         3			0	-	-	0	-	-	0	-	_		-
Heavy Vehicles, %		100		100	100		100	100		100	100		100
Mynt Flow         0         30         5         118         10         0         0         0         67         1         36           Major/Minor         Major1         Major2         Minor2         Minor3         Minor													
Major/Minor         Major1         Major2         Minor2           Conflicting Flow All         -         0         0         35         0         0         279         281         10           Stage 1         -         -         -         -         -         246         246         -           Stage 2         -         -         -         -         -         33         35         -           Critical Hdwy         Stg 1         -         -         -         -         5.43         6.5         6.2           Critical Hdwy Stg 1         -         -         -         -         5.43         5.5         -           Critical Hdwy Stg 2         -         -         -         -         5.43         5.5         -           Critical Hdwy Stg 2         -         -         -         -         3.527         4         3.3           Follow-up Hdwy         -         -         -         2.254         -         -         3.527         4         3.3           Pot Cap-1 Maneuver         0         -         -         0         987         870         -           Stage 1         -         -	•												
Conflicting Flow All													
Conflicting Flow All	Major/Minor N	/laior1			Maior2					N	/linor2		
Stage 1       -       -       -       -       -       33       35       -         Critical Hdwy       -       -       4.16       -       -       6.43       6.5       6.2         Critical Hdwy Stg 1       -       -       -       -       -       5.43       5.5       -         Critical Hdwy Stg 2       -       -       -       -       -       5.43       5.5       -         Follow-up Hdwy       -       -       2.254       -       -       3.527       4       3.3         Pollow-up Hdwy       -       -       2.254       -       -       3.527       4       3.3         Pollow-up Hdwy       -       -       2.254       -       -       0       793       10.77         Stage 1       0       -       -       -       0       793       706       -       870       -			٥			٥	٥					281	10
Stage 2       -				U									
Critical Hdwy       -       -       4.16       -       -       6.43       6.5       6.2         Critical Hdwy Stg 1       -       -       -       -       -       5.43       5.5       -         Critical Hdwy Stg 2       -       -       -       -       -       5.43       5.5       -         Follow-up Hdwy       -       -       2.254       -       3.527       4       3.3         Pot Cap-1 Maneuver       0       -       1551       -       0       709       631       1077         Stage 1       0       -       -       -       0       987       870       -         Platon blocked, %       -       -       -       0       987       870       -         Mov Cap-1 Maneuver       -       -       1551       -       -       654       0       1077         Mov Cap-2 Maneuver       -       -       1551       -       -       654       0       -         Stage 1       -       -       -       -       -       -       793       0       -         Stage 2       -       -       -       -       -       -	O .			-									
Critical Hdwy Stg 1       -       -       -       -       -       5.43       5.5       -         Critical Hdwy Stg 2       -       -       -       -       -       5.43       5.5       -         Follow-up Hdwy       -       -       -       2.254       -       -       3.527       4       3.3         Pot Cap-1 Maneuver       0       -       -       -       0       709       631       1077         Stage 1       0       -       -       -       0       987       870       -         Platoon blocked, %       -       -       -       0       987       870       -         Mov Cap-1 Maneuver       -		-	-	-			-						
Critical Hdwy Stg 2       -		_	-	-	4.10		-						
Follow-up Hdwy 2.254 3.527 4 3.3  Pot Cap-1 Maneuver 0 - 1551 - 0 709 631 1077  Stage 1 0 1551 - 0 793 706 - 793	, ,	-	-	-	-								
Pot Cap-1 Maneuver       0       -       -       1551       -       0       709       631       1077         Stage 1       0       -       -       -       0       987       870       -         Stage 2       0       -       -       -       0       987       870       -         Platoon blocked, %       -		-	_	_									
Stage 1       0       -       -       -       0       793       706       -         Stage 2       0       -       -       -       0       987       870       -         Platoon blocked, %       -<													
Stage 2       0       -       -       -       0       987       870       -         Platoon blocked, %       -       -       -       -       -       -       654       0       1077         Mov Cap-1 Maneuver       -       -       -       -       -       -       654       0       -         Stage 1       -       -       -       -       -       -       793       0       -         Stage 2       -       -       -       -       -       911       0       -         Approach       EB       WB       WB       SB         HCM Control Delay, s       0       6.9       10.5       - <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>													
Platoon blocked, %       -       -       -         Mov Cap-1 Maneuver       -       -       1551       -       -       654       0       1077         Mov Cap-2 Maneuver       -       -       -       -       -       654       0       -         Stage 1       -       -       -       -       -       793       0       -         Stage 2       -       -       -       -       911       0       -         Approach       EB       WB       WB       WB       SB         HCM Control Delay, s       0       6.9       10.5       B         Minor Lane/Major Mvmt       EBT       EBR       WBL       WBT SBLn1         Capacity (veh/h)       -       -       1551       -       758         HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B			-	_	_								
Mov Cap-1 Maneuver       -       -       1551       -       -       654       0       1077         Mov Cap-2 Maneuver       -       -       -       -       -       654       0       -         Stage 1       -       -       -       -       -       -       793       0       -         Stage 2       -       -       -       -       -       911       0       -         Approach       EB       WB       WB       SB         HCM Control Delay, s       0       6.9       10.5       B         Minor Lane/Major Mvmt       EBT       EBR       WBL       WBT SBLn1         Capacity (veh/h)       -       -       1551       -       758         HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B		U	-	-	-		U				901	0/0	-
Mov Cap-2 Maneuver         -         -         -         -         -         -         793         0         -         Stage 1         -         -         -         -         -         -         -         -         -         -         -         -         -         -         911         0         -           Approach         EB         WB         WB         SB         WB			-		1551						GE A	٥	1077
Stage 1       -       -       -       -       -       -       911       0       -         Stage 2       -       -       -       -       -       911       0       -         Approach       EB       WB       SB         HCM Control Delay, s       0       6.9       10.5         HCM LOS       B             Minor Lane/Major Mvmt       EBT       EBR       WBL       WBT SBLn1         Capacity (veh/h)       -       -       1551       -       758         HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B			-										
Stage 2         -         -         -         -         -         -         911         0         -           Approach         EB         WB         SB           HCM Control Delay, s         0         6.9         10.5           HCM LOS         B           Minor Lane/Major Mvmt         EBT         EBR         WBL         WBT SBLn1           Capacity (veh/h)         -         -         1551         -         758           HCM Lane V/C Ratio         -         -         0.076         -         0.137           HCM Control Delay (s)         -         -         7.5         0         10.5           HCM Lane LOS         -         -         A         A         B				-		-	-						
Approach EB WB SB  HCM Control Delay, s 0 6.9 10.5  HCM LOS B  Minor Lane/Major Mvmt EBT EBR WBL WBT SBLn1  Capacity (veh/h) - 1551 - 758  HCM Lane V/C Ratio - 0.076 - 0.137  HCM Control Delay (s) - 7.5 0 10.5  HCM Lane LOS - A A B				-		-	-						
HCM Control Delay, s	Stage 2	-	-	-	-	-	-				911	U	-
HCM Control Delay, s													
Minor Lane/Major Mvmt         EBT         EBR         WBL         WBT SBLn1           Capacity (veh/h)         -         -         1551         -         758           HCM Lane V/C Ratio         -         -         0.076         -         0.137           HCM Control Delay (s)         -         -         7.5         0         10.5           HCM Lane LOS         -         -         A         A         B	Approach	EB			WB						SB		
Minor Lane/Major Mvmt         EBT         EBR         WBL         WBT SBLn1           Capacity (veh/h)         -         -         1551         -         758           HCM Lane V/C Ratio         -         -         0.076         -         0.137           HCM Control Delay (s)         -         -         7.5         0         10.5           HCM Lane LOS         -         -         A         A         B	HCM Control Delay, s	0			6.9						10.5		
Minor Lane/Major Mvmt         EBT         EBR         WBL         WBT SBLn1           Capacity (veh/h)         -         -         1551         -         758           HCM Lane V/C Ratio         -         -         0.076         -         0.137           HCM Control Delay (s)         -         -         7.5         0         10.5           HCM Lane LOS         -         -         A         A         B	HCM LOS												
Capacity (veh/h)       -       -       1551       -       758         HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B													
Capacity (veh/h)       -       -       1551       -       758         HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B	Minor Lane/Maior Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
HCM Lane V/C Ratio       -       -       0.076       -       0.137         HCM Control Delay (s)       -       -       7.5       0       10.5         HCM Lane LOS       -       -       A       A       B													
HCM Control Delay (s) 7.5 0 10.5 HCM Lane LOS A A B													
HCM Lane LOS A A B			_	_									
			_	_									
1.0.11 00a1 70a10 a(1011)			_										
	. 13111 3341 70410 34(1011)				J. <u>L</u>		3.0						

Intersection													
Int Delay, s/veh	1.4												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स			ĵ.			4					
Traffic Vol, veh/h	14	83	0	0	106	86	22	2	9	0	0	0	
Future Vol, veh/h	14	83	0	0	106	86	22	2	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2	
Mvmt Flow	14	83	0	0	106	86	22	2	9	0	0	0	
Major/Minor N	//ajor1		N	Major2		N	Minor1						
Conflicting Flow All	192	0	_	-	_	0	260	303	83				
Stage 1	-	-	-	-	_	-	111	111	-				
Stage 2	-	-	-	-	-	-	149	192	-				
Critical Hdwy	4.67	-	-	-	-	-	6.4	7.5	6.53				
Critical Hdwy Stg 1	_	-	-	_	-	-	5.4	6.5	_				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.5	-				
Follow-up Hdwy	2.713	-	-	-	-	-	3.5	4.9	3.597				
Pot Cap-1 Maneuver	1111	-	0	0	-	-	733	477	897				
Stage 1	-	-	0	0	-	-	919	648	-				
Stage 2	-	-	0	0	-	-	884	590	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1111	-	-	-	-	-	723	0	897				
Mov Cap-2 Maneuver	-	-	-	-	-	-	723	0	-				
Stage 1	-	-	-	-	-	-	907	0	-				
Stage 2	-	-	-	-	-	-	884	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	1.2			0			9.9						
HCM LOS							A						
							, ,						
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)			1111										
HCM Lane V/C Ratio				_	_	_							
HCM Control Delay (s)		9.9	8.3	0	_	_							
HCM Lane LOS		3.5 A	Α	A	_	<u>-</u>							
HCM 95th %tile Q(veh)		0.1	0	-	_	_							
		J. 1											

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	87	2	1	188	0	1	0	0	1	0	3
Future Vol, veh/h	3	87	2	1	188	0	1	0	0	1	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	38	50	0	21	0	0	0	0	0	0	67
Mvmt Flow	3	87	2	1	188	0	1	0	0	1	0	3
Major/Minor N	/lajor1		ľ	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	188	0	0	89	0	0	286	284	88	284	285	188
Stage 1	-	_	_	_	_	_	94	94	_	190	190	-
Stage 2	_	-	_	-	-	_	192	190	_	94	95	_
Critical Hdwy	4.43	-	_	4.1	-	_	7.1	6.5	6.2	7.1	6.5	6.87
Critical Hdwy Stg 1	_	_	_	_	_	_	6.1	5.5	_	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.903
Pot Cap-1 Maneuver	1220	-	-	1519	-	-	670	628	976	672	628	712
Stage 1	-	-	-	-	-	-	918	821	-	816	747	-
Stage 2	-	-	-	-	-	-	814	747	-	918	820	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1220	-	-	1519	-	-	665	625	976	670	625	712
Mov Cap-2 Maneuver	-	-	-	-	-	-	665	625	-	670	625	-
Stage 1	-	-	-	-	-	-	915	819	-	814	746	-
Stage 2	-	-	-	-	-	-	810	746	-	915	818	-
_												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0			10.4			10.2		
HCM LOS	J.•						В			В		
										_		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SRI n1			
Capacity (veh/h)		665	1220	-	-	1519	-	-	701			
HCM Lane V/C Ratio		0.002		-		0.001	-		0.006			
HCM Control Delay (s)		10.4	0.002	0	-	7.4	0	-	10.2			
HCM Lane LOS		10.4 B	A	A	-	7.4 A	A	-	10.2 B			
HCM 95th %tile Q(veh)		0	0	- -	_	0	- -	-	0			
		U	U		_	U			U			

Intersection												
Int Delay, s/veh	4.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		*	ĵ.			4			4	
Traffic Vol, veh/h	15	36	7	1	25	1	1	1	1	4	11	32
Future Vol, veh/h	15	36	7	1	25	1	1	1	1	4	11	32
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	13	31	0	100	33	0	0	100	100	0	0	16
Mvmt Flow	15	36	7	1	25	1	1	1	1	4	11	32
Major/Minor I	Major1		ı	Major2		N	Minor1		N	/linor2		
Conflicting Flow All	26	0	0	43	0	0	119	98	40	98	101	26
Stage 1	-	-	-	-	-	-	70	70	-	28	28	-
Stage 2	-	-	-	-	-	-	49	28	-	70	73	-
Critical Hdwy	4.23	-	-	5.1	-	-	7.1	7.5	7.2	7.1	6.5	6.36
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Follow-up Hdwy	2.317	-	-	3.1	-	-	3.5	4.9	4.2	3.5	4	3.444
Pot Cap-1 Maneuver	1520	-	-	1113	-	-	861	640	810	889	793	1011
Stage 1	-	-	-	-	-	-	945	679	-	994	876	-
Stage 2	-	-	-	-	-	-	969	712	-	945	838	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1520	-	-	1113	-	-	818	633	810	879	784	1011
Mov Cap-2 Maneuver	-	-	-	-	-	-	818	633	-	879	784	-
Stage 1	-	-	-	-	-	-	936	672	-	984	875	-
Stage 2	-	-	-	-	-	-	926	711	-	933	830	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.9			0.3			8.4			9		
HCM LOS							Α			A		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		1071	1520	-	-	1113	-	-	936			
HCM Lane V/C Ratio		0.003	0.01	-	-	0.001	-	-	0.05			
HCM Control Delay (s)		8.4	7.4	_	_	8.2	_	-	9			
HCM Lane LOS		A	Α	_	_	A	_	_	A			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.2			
72.00												

Intersection						
Int Delay, s/veh	0.8					
Movement I	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>		ነ ነ	<u> </u>	¥	
Traffic Vol, veh/h	37	3	8	25	0	0
Future Vol, veh/h	37	3	8	25	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- Olop	None
Storage Length	_	-	150	-	0	-
Veh in Median Storage, #		_	-	0	0	_
Grade, %	0	<u>-</u>	_	0	0	<u>-</u>
	100	100	100	100	100	100
Heavy Vehicles, %	8	33	0	13	0	0
Mvmt Flow	37	3	8	25	0	0
MINITE FIOW	31	<b>ა</b>	0	25	U	U
Major/Minor Ma	ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	40	0	80	39
Stage 1	-	-	-	-	39	-
Stage 2	-	-	-	-	41	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	_	5.4	-
Critical Hdwy Stg 2	_	_	-	-	5.4	-
Follow-up Hdwy	-	_	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	-	_	1583	-	927	1038
Stage 1	_	_	-	_	989	-
Stage 2	_	_	_	_	987	_
Platoon blocked, %	_	_		_	001	
Mov Cap-1 Maneuver	_	_	1583	_	922	1038
Mov Cap-1 Maneuver	_	<u>-</u>	-	<u>-</u>	922	-
Stage 1	_		_	_	989	_
Stage 2		_	_	_	982	_
Stage 2	-	-	-	-	302	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.8		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
		NDLIII	LDI	LDK		וטייי
Capacity (veh/h)		-	-	-	1583	-
HCM Lana VIIC Datia		-	-	-	0.005	-
HCM Central Dalay (a)		0			7 2	
HCM Control Delay (s)		0	-	-	7.3	-
		0 A	- -	-	7.3 A 0	-

L. ( C						
Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	N/		f)			सी
Traffic Vol, veh/h	0	0	0	1	0	14
Future Vol, veh/h	0	0	0	1	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	14
Mvmt Flow	0	0	0	1	0	14
IVIVIII( I IOW	U	U	U		U	17
Major/Minor N	/linor1	N	Major1	N	Major2	
Conflicting Flow All	15	1	0	0	1	0
Stage 1	1	-	-	-	-	-
Stage 2	14	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	_	_	_
Follow-up Hdwy	3.5	3.3	_	-	2.2	_
Pot Cap-1 Maneuver	1009	1090	-	_	1635	_
Stage 1	1028	-	_	_	-	_
Stage 2	1014	_	_	_	_	_
Platoon blocked, %	1011		_	_		_
Mov Cap-1 Maneuver	1009	1090	_	_	1635	_
Mov Cap-1 Maneuver	1009	1030		_	1000	_
Stage 1	1009	_	_	_		-
•	1026	_	_	_		-
Stage 2	1014	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Long/Major Manual		NDT	NDD	MDI 1	CDI	CDT
Minor Lane/Major Mvmt		NBT	NRKA	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	1635	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		-	-	0	0	-
HCM Lane LOS		-	-	Α	Α	-
HCM 95th %tile Q(veh)		_	_	_	0	_

Intersection						
Int Delay, s/veh	0.4					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	^	^	4	<b>∱</b>	^
Traffic Vol, veh/h	1	0	0	0	19	0
Future Vol, veh/h	1	0	0	0	19	0
Conflicting Peds, #/hr	0	0	0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	11	0
Mvmt Flow	1	0	0	0	19	0
NA - ' /NA'	4 0		1.1.1		4 - ' 0	
	Minor2		//ajor1		/lajor2	
Conflicting Flow All	19	19	19	0	-	0
Stage 1	19	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	1004	1065	1611	-	-	-
Stage 1	1009	-	-	-	-	-
Stage 2	-	-	_	-	-	-
Platoon blocked, %				_	_	-
Mov Cap-1 Maneuver	1004	1065	1611	_	_	_
Mov Cap-2 Maneuver	1004	-	-	_	_	_
Stage 1	1009	_	_	_	_	_
Stage 2	1003	_	_	_	_	_
Stage 2	_	_	-	_	-	_
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
NA: 1 /NA: NA		NDI	NDT	EDL 4	ODT	000
Minor Lane/Major Mvm	IT	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1611		1004	-	-
HCM Lane V/C Ratio		-	-	0.001	-	-
HCM Control Delay (s)		0	-	8.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)	)	0	-	0	-	-

Intersection						
Int Delay, s/veh	4.4					
<u> </u>		EDD	NDI	NDT	CDT	SBR
Movement	EBL	EBR	NBL	NBT	SBT	SBK
Lane Configurations	¥	40	-	4	<b>₽</b>	7
Traffic Vol, veh/h	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	12	5	1	9	7
Major/Minor N	Minor2		Major1		Major2	
Conflicting Flow All	24	13	16	0	-	0
Stage 1	13	-	-	-	-	-
Stage 2	11	-	-	-	-	-
Critical Hdwy	6.4	6.28	4.5	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.372	2.56	-	-	-
Pot Cap-1 Maneuver	997	1050	1386	-	-	-
Stage 1	1015	-	-	-	-	-
Stage 2	1017	_	-	-	-	-
Platoon blocked, %				-	_	-
Mov Cap-1 Maneuver	993	1050	1386	_	-	-
Mov Cap-2 Maneuver	993	-	-	_	_	_
Stage 1	1011	_	_	_	_	_
Stage 2	1017	<u>-</u>	_	_	_	_
Olage 2	1017					
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
			NDT	ΓDI1	CDT	CDD
Minor Long/Mair - M	.1	NID1		EBLn1	SBT	SBR
Minor Lane/Major Mvm	nt	NBL				
Capacity (veh/h)	nt	1386	-	1041	-	-
Capacity (veh/h) HCM Lane V/C Ratio		1386 0.004	-	1041 0.013	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1386 0.004 7.6	- - 0	1041 0.013 8.5		- -
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS		1386 0.004 7.6 A	-	1041 0.013 8.5 A	-	
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		1386 0.004 7.6	- - 0	1041 0.013 8.5	-	-

Intersection												
Int Delay, s/veh	4.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	6	12	1	0	27	2	2	2	1	1	0	30
Future Vol, veh/h	6	12	1	0	27	2	2	2	1	1	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	6	12	1	0	27	2	2	2	1	1	0	30
Major/Minor I	Major1		1	Major2		ľ	Minor1		N	Minor2		
Conflicting Flow All	29	0	0	13	0	0	68	54	13	54	53	28
Stage 1	_	-	_	-	-	-	25	25	-	28	28	-
Stage 2	-	-	-	-	-	-	43	29	-	26	25	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.516
Pot Cap-1 Maneuver	1405	-	-	1619	-	-	930	841	1073	949	842	987
Stage 1	-	-	-	-	-	-	998	878	-	994	876	-
Stage 2	-	-	-	-	-	-	976	875	-	997	878	-
Platoon blocked, %	440=	-	-	1010	-	-	000	000	4070	0.40	000	00-
Mov Cap-1 Maneuver	1405	-	-	1619	-	-	899	838	1073	943	839	987
Mov Cap-2 Maneuver	-	-	-	-	-	-	899	838	-	943	839	-
Stage 1	-	-	-	-	-	-	994	874	-	990	876	-
Stage 2	-	-	-	-	-	-	946	875	-	990	874	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.4			0			9			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBL <sub>n1</sub>			
Capacity (veh/h)		902	1405	-	-	1619	-	-	986			
HCM Lane V/C Ratio		0.006		-	-	-	-	-	0.031			
HCM Control Delay (s)		9	7.6	0	-	0	-	-	8.8			
HCM Lane LOS		Α	Α	Α	-	Α	-	-	Α			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.1			

-						
Intersection						
Int Delay, s/veh	2.8					
		WED	NET	NES	05:	007
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		₽			र्स
Traffic Vol, veh/h	5	19	30	2	15	42
Future Vol, veh/h	5	19	30	2	15	42
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storag	e,# 0	_	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	6	21	0	12	13
Mvmt Flow	5	19	30	2	15	42
				_	.0	
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	103	31	0	0	32	0
Stage 1	31	-	-	-	-	-
Stage 2	72	-	-	-	-	-
Critical Hdwy	6.6	6.26	-	_	4.22	-
Critical Hdwy Stg 1	5.6	_	_	_	_	_
Critical Hdwy Stg 2	5.6	_	_	_	_	_
Follow-up Hdwy	3.68	3.354	_	_	2.308	_
Pot Cap-1 Maneuver	853	1032	_	_	1518	_
Stage 1	947	-	_	_	-	_
Stage 2	907	_	_	_	_	
Platoon blocked, %	907	_	_	_	_	_
	044	4000	-	_	1510	-
Mov Cap-1 Maneuver		1032	-	-	1518	-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	947	-	-	-	-	-
Stage 2	898	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		1.9	
HCM LOS	0.7 A		U		1.9	
HOW LOS	A					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	_	986	1518	_
HCM Lane V/C Ratio		_	_	0.024	0.01	_
HCM Control Delay (s	:)	_	_	8.7	7.4	0
HCM Lane LOS	7	<u>-</u>		Α	Α	A
HCM 95th %tile Q(vel	2)	-	-	0.1	0	-
HOW SOUL WILLE CONTROL	I)	-	-	U. I	U	-

Intersection						
Int Delay, s/veh	0					
	EBT	EDD	WDI	WDT	NBL	NBR
		EBR	WBL	WBT		NBK
Lane Configurations	<b>-</b>	۸	٥	<u>र्</u> च	<b>Y</b>	٨
Traffic Vol, veh/h	4	0	0	4	0	0
Future Vol, veh/h	4	0	0	4	0	0
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	0	0	4	0	0
NA=:==/NA:===	-:1		A-:0		M: 1	
	ajor1		Major2		Minor1	
Conflicting Flow All	0	0	4	0	8	4
Stage 1	-	-	-	-	4	-
Stage 2	-	-	-	-	4	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	_	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1618	-	1013	1080
Stage 1	-	-	-	-	1019	-
Stage 2	-	-	-	-	1019	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1618	-	1013	1080
Mov Cap-2 Maneuver	-	_	-	_	1013	-
Stage 1	-	-	_	-	1019	-
Stage 2	_	_	_	_	1019	_
Olago Z					1010	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		0	
HCM LOS					Α	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
	<u> </u>	NDLIII	EDI	LDK		WDI
Capacity (veh/h)		-	-	-	1618	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	-	0	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection						
Int Delay, s/veh	0					
		EST	MOT	14/55	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	Þ		Y	
Traffic Vol, veh/h	0	5	57	0	0	0
Future Vol, veh/h	0	5	57	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	5	57	0	0	0
Mainu/Minau	1-11		4-:0		\ 4:O	
	/lajor1		Major2		Minor2	
Conflicting Flow All	57	0	-	0	62	57
Stage 1	-	-	-	-	57	-
Stage 2	-	-	-	-	5	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1547	-	-	-	944	1009
Stage 1	-	-	-	-	966	-
Stage 2	-	-	-	-	1018	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1547	-	-	-	944	1009
Mov Cap-2 Maneuver	-	-	-	-	944	-
Stage 1	_	_	_	-	966	-
Stage 2	_	_	_	_	1018	_
Olago L					1010	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS					Α	
Minor Lane/Major Mvm	t	EBL	EBT	WBT	WBR :	SRI n1
			LUI	VVDI	VVDIX	ODLIII
Capacity (veh/h) HCM Lane V/C Ratio		1547		-		_
		-	-	-	-	-
		Λ				
HCM Control Delay (s)		0	-	-	-	0
		0 A 0	- -	-	-	0 A

ntersection	
ntersection Delay, s/veh	8.1
ntersection LOS	Α

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Future Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	1	0	3	0	0	0	4	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		8.2				0		8		
HCM LOS		-		Α				-		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	25%	100%	
Vol Thru, %	100%	100%	0%	0%	
Vol Right, %	0%	0%	75%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	0	0	4	4	
LT Vol	0	0	1	4	
Through Vol	0	0	0	0	
RT Vol	0	0	3	0	
Lane Flow Rate	0	0	4	4	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0	0	0.006	0.006	
Departure Headway (Hd)	3.911	3.911	5.208	4.958	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	0	0	691	726	
Service Time	1.917	1.917	3.212	2.962	
HCM Lane V/C Ratio	0	0	0.006	0.006	
HCM Control Delay	6.9	6.9	8.2	8	
HCM Lane LOS	N	N	Α	Α	
HCM 95th-tile Q	0	0	0	0	

Intersection						
Int Delay, s/veh	6.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>		*	<b>↑</b>	¥	
Traffic Vol, veh/h	21	178	35	64	256	4
Future Vol, veh/h	21	178	35	64	256	4
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storag	je,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	21	178	35	64	256	4
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	199	0	244	110
Stage 1	-	-	-	-	110	-
Stage 2	-	-	-	-	134	-
Critical Hdwy	-	-	4.16	-	6.45	6.7
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.254	-	3.545	3.75
Pot Cap-1 Maneuver	_	-	1350	-	738	828
Stage 1	-	-	-	-	907	-
Stage 2	-	-	-	-	885	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	r -	_	1350	-	719	828
Mov Cap-2 Maneuver		-	-	-	719	-
Stage 1	-	-	-	-	907	-
Stage 2	-	-	-	-	862	-
3 11 9						
A I			\A4D		NE	
Approach	EB		WB		NB	
HCM Control Delay, s	s 0		2.7		12.8	
HCM LOS					В	
Minor Lane/Major Mv	mt l	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		720	_	-		-
HCM Lane V/C Ratio		0.361	_		0.026	-
HCM Control Delay (s		12.8	_	_	7.7	_
HCM Lane LOS	7	В	_	_	A	-
HCM 95th %tile Q(vel	h)	1.6	-	-	0.1	-
	,					

Intersection						
Int Delay, s/veh	2.5					
		EDE	MAID	WE	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		ች	<u></u>	À	
Traffic Vol, veh/h	184	233	14	306	105	15
Future Vol, veh/h	184	233	14	306	105	15
Conflicting Peds, #/hr	0	0	0	0	0	0
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	150	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	8	8	5	25	21
Mvmt Flow	184	233	14	306	105	15
Majar/Minor Ma	-:1		Maia#0		Minard	
	ajor1		Major2		Minor1	004
Conflicting Flow All	0	0	417	0	635	301
Stage 1	-	-	-	-	301	-
Stage 2	-	-	-	-	334	-
Critical Hdwy	-	-	4.18	-	6.65	6.41
Critical Hdwy Stg 1	-	-	-	-	5.65	-
Critical Hdwy Stg 2	-	-	-	-	5.65	-
Follow-up Hdwy	-	-	2.272	-	••	3.489
Pot Cap-1 Maneuver	-	-	1110	-	408	696
Stage 1	-	_	-	-	701	-
Stage 2	-	-	-	-	677	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1110	-	403	696
Mov Cap-2 Maneuver	-	-	-	-	403	-
Stage 1	_	_	_	_	701	_
Stage 2	_	_	_	_	668	_
- W.go _						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		16.8	
HCM LOS					С	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
	<u> </u>					
Capacity (veh/h)		425	-		1110	-
HCM Cartes   Dalay (a)		0.282	-		0.013	-
HCM Control Delay (s)		16.8	-	-	8.3	-
HCM Lane LOS		С	-	-	A	-
HCM 95th %tile Q(veh)		1.1	-	-	0	-

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.		ሻ	1>			र्स	7		4	
Traffic Vol, veh/h	0	180	48	217	189	5	84	2	232	5	0	2
Future Vol, veh/h	0	180	48	217	189	5	84	2	232	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	180	48	217	189	5	84	2	232	5	0	2
Major/Minor M	1ajor1			Major2			Minor1		N	Minor2		
Conflicting Flow All	194	0	0	228	0	0	831	832	204	947	854	192
Stage 1	-	-	-	-	-	-	204	204	-	626	626	-
Stage 2	_	_	_	_	_	_	627	628	_	321	228	_
Critical Hdwy	4.1	-	-	4.26	_	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1391	-	-	1262	-	-	277	307	791	243	298	855
Stage 1	-	-	-	-	-	-	773	737	-	475	480	-
Stage 2	-	_	-	-	-	-	453	479	-	695	719	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1391	-	-	1262	-	-	240	254	791	148	247	855
Mov Cap-2 Maneuver	-	-	-	-	-	-	240	254	-	148	247	-
Stage 1	-	-	-	-	-	-	773	737	-	475	397	-
Stage 2	-	-	-	-	-	-	374	397	-	490	719	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.5			15.9			24.3		
HCM LOS							С			С		
Minor Lane/Major Mvmt		NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR 9	SBLn1		
Capacity (veh/h)		240	791	1391	-	-	1262	_	-	194		
HCM Lane V/C Ratio		0.358		-	-	-	0.172	-	_	0.036		
HCM Control Delay (s)		28.1	11.4	0	-	-	8.4	-	-	24.3		
HCM Lane LOS		D	В	A	-	-	Α	-	_	С		
HCM 95th %tile Q(veh)		1.6	1.2	0	-	-	0.6	-	-	0.1		

Intersection												
Int Delay, s/veh	7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
	LDL		LDK	VVDL		NDL	NDL		NOR			אמט
Lane Configurations Traffic Vol. veh/h	18	<b>4</b>	12	30	<b>♣</b>	176		<b>}</b>	20	126	124	28
Future Vol, veh/h	18	21	12	30	26 26	176 176	12 12	88 88	20	126 126	124 124	28
Conflicting Peds, #/hr	0	0	0	0	0	0	0	00	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	Stop -	Stop -	None	Stop -	Stop -	None	riee -	riee -	None	-	- FIEE	None
Storage Length	_	-	NOHE	-	_	-	150	-	-	230	-	NOHE
Veh in Median Storage	e.# -	0		_	0	_	-	0	_	230	0	
Grade, %	ν, π -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	6	0	0	7	0	16	0	15	32	29	5	0
Mymt Flow	18	21	12	30	26	176	12	88	20	126	124	28
IVIVIII I IOVV	10	<b>L</b> I	12	- 00	20	170	12	00	20	120	144	20
	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	613	522	138	529	526	98	152	0	0	108	0	0
Stage 1	390	390	-	122	122	-	-	-	-	-	-	-
Stage 2	223	132	-	407	404	-	-	-	-	-	-	-
Critical Hdwy	7.16	6.5	6.2	7.17	6.5	6.36	4.1	-	-	4.39	-	-
Critical Hdwy Stg 1	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.554	4		3.563	4	3.444	2.2	-	-	2.461	-	-
Pot Cap-1 Maneuver	399	462	916	453	460	921	1441	-	-	1330	-	-
Stage 1	626	611	-	870	799	-	-	-	-	-	-	-
Stage 2	771	791	-	611	603	-	-	-	-	-	-	-
Platoon blocked, %	000	445	040	200	440	004	1111	-	-	4000	-	-
Mov Cap-1 Maneuver	283	415	916	396	413	921	1441	-	-	1330	-	-
Mov Cap-2 Maneuver	283	415	-	396	413	-	-	-	-	-	-	-
Stage 1	621	553	-	863	793	-	-	-	-	-	-	-
Stage 2	598	785	-	525	546	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.3			12.6			0.8			3.6		
HCM LOS	С			В								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1441	_	_	401	703	1330	_	_			
HCM Lane V/C Ratio		0.008	_	_	0.127		0.095	_	_			
HCM Control Delay (s)		7.5	_	_	15.3	12.6	8	-	-			
HCM Lane LOS		A	_	_	C	В	A	_	_			
HCM 95th %tile Q(veh	)	0	_	-	0.4	1.4	0.3	-	-			
7041. 70410 4(1011	1				<b>J</b> .1		3.0					

Intersection           Int Delay, s/veh         0.3           Movement         WBL         WBR         NBT         NBR         SBL         SBT           Lane Configurations         ↑         ↓
Movement         WBL         WBR         NBT         NBR         SBL         SBT           Lane Configurations         Y         Image: Configuration of the conf
Lane Configurations         ▼         ↓         ↓           Traffic Vol, veh/h         3         5         216         8         3         112           Future Vol, veh/h         3         5         216         8         3         112           Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free         Free         Free         Free         Free           RT Channelized         -         None         -         None         -         None           Storage Length         0         -         -         -         -         -         -           Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         0
Traffic Vol, veh/h         3         5         216         8         3         112           Future Vol, veh/h         3         5         216         8         3         112           Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free
Future Vol, veh/h         3         5         216         8         3         112           Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free         Free         Free         Free         Free         Free         Free         Free         Ree         None         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         - <td< td=""></td<>
Conflicting Peds, #/hr         0         0         0         0         0         0           Sign Control         Stop         Stop         Free         Free         Free         Free           RT Channelized         -         None         -         None         -         None           Storage Length         0         -         -         -         -         -         -           Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         0
Sign ControlStopStopFreeFreeFreeFreeFreeRT Channelized-None-None-NoneStorage Length0Veh in Median Storage, #0-00Grade, %0-00
RT Channelized         - None         - None         - None         - None           Storage Length         0
Storage Length         0         -         -         -         -         -         -         -         0           Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         0
Veh in Median Storage, #         0         -         0         -         0           Grade, %         0         -         0         -         -         0
Grade, % 0 - 0 0
Peak Hour Factor 100 100 100 100 100 100
Heavy Vehicles, % 0 20 19 0 0 34
Mvmt Flow 3 5 216 8 3 112
Major/Minor Minor1 Major1 Major2
Conflicting Flow All 338 220 0 0 224 0
Stage 1 220
Stage 2 118
Critical Hdwy 6.4 6.4 4.1 -
Critical Hdwy Stg 1 5.4
Critical Hdwy Stg 2 5.4
Follow-up Hdwy 3.5 3.48 2.2 -
Pot Cap-1 Maneuver 662 777 1357 -
Stage 1 821
Stage 2 912
Platoon blocked, %
Mov Cap-1 Maneuver 661 777 1357 -
Mov Cap-1 Maneuver 661
•
Stage 2 910
Approach WB NB SB
HCM Control Delay, s 10 0 0.2
HCM LOS B
Minor Lane/Major Mymt NRT NRRWRI n1 SRI SRT
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT
Capacity (veh/h) 729 1357 -
Capacity (veh/h) 729 1357 - HCM Lane V/C Ratio - 0.011 0.002 -
Capacity (veh/h) 729 1357 - HCM Lane V/C Ratio - 0.011 0.002 - HCM Control Delay (s) - 10 7.7 0
Capacity (veh/h) 729 1357 - HCM Lane V/C Ratio - 0.011 0.002 -

Intersection												
Int Delay, s/veh	0											
					=							
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	193	0	0	102	0
Future Vol, veh/h	1	0	0	0	0	0	0	193	0	0	102	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	193	0	0	102	0
Major/Minor N	Minor2		N	Minor1		N	Major1		N	Major2		
Conflicting Flow All	295	295	102	295	295	193	102	0	0	193	0	0
Stage 1	102	102		193	193	193	102	U	U	193		
	193	193	-	102	102	-	-	-	=	-	-	-
Stage 2	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	<del>-</del>	4.1	-	<del>-</del>
Critical Hdwy				6.1	5.5	0.Z -		-	-			-
Critical Hdwy Stg 1	7.1	5.5	-				-	-	<del>-</del>	-	-	-
Critical Hdwy Stg 2	7.1	5.5	- 2 2	6.1	5.5	2.2	-	-	-	-	-	-
Follow-up Hdwy	4.4	620	3.3	3.5	620	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	502	620	959	661	620	854	1503	-	-	1392	-	-
Stage 1	712	815	-	813	745	-	-	-	-	-	-	-
Stage 2	628	745	-	909	815	-	-	-	-	-	-	-
Platoon blocked, %	F00	000	050	004	000	054	4500	-	-	4000	-	-
Mov Cap-1 Maneuver	502	620	959	661	620	854	1503	-	-	1392	-	-
Mov Cap-2 Maneuver	502	620	-	661	620	-	-	-	-	-	-	-
Stage 1	712	815	-	813	745	-	-	-	-	-	-	-
Stage 2	628	745	-	909	815	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.2			0			0			0		
HCM LOS	В			A								
				,,								
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1503		-		-	1392		_			
HCM Lane V/C Ratio		1000	_		0.002	_	1002	_	<u>-</u>			
HCM Control Delay (s)		0	_	_		0	0	_	_			
HCM Lane LOS		A	_	_	12.2 B	A	A	_	<u>-</u>			
HCM 95th %tile Q(veh)		0	_	_	0	-	0	_	_			
How both wile Q(Ven)		U	_	_	U	_	U	-	_			

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4		1,02	4	71 DIX	,,,,,,,	4	TIDIT	UDL	4	OBIN
Traffic Vol, veh/h	0	4	0	13	1	60	1	147	43	40	57	2
Future Vol, veh/h	0	4	0	13	1	60	1	147	43	40	57	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	_	_	None	-	-	None
Storage Length	_	_	-	-	_	-	-	-	-	-	_	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mvmt Flow	0	4	0	13	1	60	1	147	43	40	57	2
Major/Minor N	/linor2			Minor1			Major1			Major2		
Conflicting Flow All	339	330	58	311	310	169	59	0	0	190	0	0
Stage 1	138	138	-	171	171	-	-	-	-	-	-	-
Stage 2	201	192	-	140	139	-	-	-	_	-	-	_
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.18	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.572	4	3.426	2.2	-	-	2.452	-	-
Pot Cap-1 Maneuver	619	592	1014	630	608	845	1558	-	-	1242	-	_
Stage 1	870	786	-	817	761	-	-	-	-	-	-	-
Stage 2	805	745	-	849	785	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	560	572	1014	610	587	845	1558	-	-	1242	-	-
Mov Cap-2 Maneuver	560	572	-	610	587	-	-	-	-	-	-	-
Stage 1	869	760	-	816	760	-	-	-	-	-	-	-
Stage 2	746	744	-	817	759	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.3			10			0			3.2		
HCM LOS	В			В								
Minor Lane/Major Mvmt	ŀ	NBL	NBT	NBR I	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1558		-		787		-				
HCM Lane V/C Ratio		0.001	_			0.094		_	_			
HCM Control Delay (s)		7.3	0	_		10	8	0	_			
HCM Lane LOS		Α.	A	_	В	В	A	A	<u>-</u>			
HCM 95th %tile Q(veh)		0	-	_	0	0.3	0.1	-	_			
		J			J	3.0	J. 1					

Intersection						
Int Delay, s/veh	1.1					
		EDD	ND	NET	ODT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	Դ	
Traffic Vol, veh/h	9	2	3	64	22	12
Future Vol, veh/h	9	2	3	64	22	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	14	0
Mvmt Flow	9	2	3	64	22	12
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	98	28	34	0	-	0
Stage 1	28	-	-	-	-	-
Stage 2	70	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	906	1053	1591	_	-	-
Stage 1	1000	-	-	-	-	-
Stage 2	958	_	_	-	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	904	1053	1591	_	_	_
Mov Cap-1 Maneuver	904	1000	1001	_	_	
Stage 1	998	-	-	_	-	_
	958	-	-	_	-	-
Stage 2	900	<del>-</del>	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.9		0.3		0	
HCM LOS	A					
	, ,					
				<b>-</b> D. (	05-	055
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1591	-		-	-
HCM Lane V/C Ratio		0.002	-	0.012	-	-
HCM Control Delay (s)		7.3	0	8.9	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-
.,,						

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	1	65	0	0	25	1
Future Vol, veh/h	0	0	0	0	0	0	1	65	0	0	25	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	1	65	0	0	25	1
Major/Minor M	linor2		1	Minor1		N	/lajor1		N	Major2		
Conflicting Flow All	93	93	26	93	93	65	26	0	0	65	0	0
Stage 1	26	26	-	67	67	-		-	-	-	-	-
Stage 2	67	67	-	26	26	-	-	-	_	-	_	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	895	801	1056	895	801	1005	1601	-	-	1550	-	-
Stage 1	997	878	-	948	843	-	-	-	-	-	-	-
Stage 2	948	843	-	997	878	-	-	-		-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	894	800	1056	894	800	1005	1601	-	-	1550	-	-
Mov Cap-2 Maneuver	894	800	-	894	800	-	-	-	-	-	-	-
Stage 1	996	878	-	947	842	-	-	-	-	-	-	-
Stage 2	947	842	-	997	878	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0.1			0		
HCM LOS	A			A								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBL <sub>n1</sub>	SBL	SBT	SBR			
Capacity (veh/h)		1601	-	-	-	-	1550	-	-			
HCM Lane V/C Ratio		0.001	-	-	-	-	-	-	-			
HCM Control Delay (s)		7.2	0	-	0	0	0	-	-			
HCM Lane LOS		Α	Α	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		4			4
Traffic Vol, veh/h	12	3	52	41	26	32
Future Vol, veh/h	12	3	52	41	26	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	0	14	23	12	26
Mymt Flow	12	3	52	41	26	32
IVIVIII I IOW	12	J	JZ	41	20	32
Major/Minor I	Minor1	N	Major1	I	Major2	
Conflicting Flow All	157	73	0	0	93	0
Stage 1	73	-	-	-	-	-
Stage 2	84	-	-	-	-	-
Critical Hdwy	6.57	6.2	_	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	-	-	-	-	-
Follow-up Hdwy	3.653	3.3	-	-	2.308	-
Pot Cap-1 Maneuver	800	995	-	-	1441	-
Stage 1	913	_	_	-	_	_
Stage 2	903	-	_	_	_	-
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	786	995	_	_	1441	_
Mov Cap-2 Maneuver	786	-	_	_	-	_
Stage 1	913	_	_	_	_	_
Stage 2	887	_	_	_	_	_
Olage 2	001					
Approach	WB		NB		SB	
HCM Control Delay, s	9.5		0		3.4	
HCM LOS	Α					
Minor Lanc/Major Mun	nt	NBT	NIPDV	VBLn1	SBL	SBT
Minor Lane/Major Mvm	IL	INDI	INDKV			اقد
Capacity (veh/h)		-	-	820	1441	-
HCM Lane V/C Ratio		-	-	0.018		-
HCM Control Delay (s)		-	-	9.5	7.5	0
HCM Lane LOS		-	-	0.1	0.1	A -
HCM 95th %tile Q(veh)						

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			स	7		4			4	
Traffic Vol, veh/h	54	66	3	0	23	54	1	0	2	136	1	12
Future Vol, veh/h	54	66	3	0	23	54	1	0	2	136	1	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	54	66	3	0	23	54	1	0	2	136	1	12
Major/Minor N	Major1			Major2		<u> </u>	Minor1			Minor2		
Conflicting Flow All	77	0	0	69	0	0	233	253	68	200	200	23
Stage 1	-	-	-	-	-	-	176	176	-	23	23	-
Stage 2	-	-	-	-	-	-	57	77	-	177	177	-
Critical Hdwy	4.39	-	-	4.1	-	-	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Follow-up Hdwy	2.461	-	-	2.2	-	-	3.5	4	3.3	3.689		3.975
Pot Cap-1 Maneuver	1367	-	-	1545	-	-	726	654	1001	719	699	877
Stage 1	-	-	-	-	-	-	831	757	-	948	880	-
Stage 2	-	-	-	-	-	-	960	835	-	782	756	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1367	-	-	1545	-	-	693	627	1001	695	670	877
Mov Cap-2 Maneuver	-	-	-	-	-	-	693	627	-	695	670	-
Stage 1	-	-	-	-	-	-	797	726	-	909	880	-
Stage 2	-	-	-	-	-	-	946	835	-	748	725	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.1			11.4		
HCM LOS							A			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBI n1			
Capacity (veh/h)		872	1367			1545	-		707			
HCM Lane V/C Ratio		0.003	0.04	_	_	-	_		0.211			
HCM Control Delay (s)		9.1	7.7	0	_	0	_	_	11.4			
HCM Lane LOS		A	A	A	_	A	_	_	В			
HCM 95th %tile Q(veh)	)	0	0.1	-	_	0	_	_	0.8			
			<b>7.</b> 1						5.5			

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	f)		*	f)			4			4	
Traffic Vol, veh/h	9	288	8	29	101	102	1	5	28	72	7	7
Future Vol, veh/h	9	288	8	29	101	102	1	5	28	72	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	9	288	8	29	101	102	1	5	28	72	7	7
Major/Minor N	/lajor1		_ [	Major2		ı	Minor1		- 1	Minor2		
Conflicting Flow All	203	0	0	296	0	0	527	571	292	537	524	152
Stage 1	-	-	-		-	-	310	310	-	210	210	-
Stage 2	_	_	_	<u>-</u>	_	_	217	261	<u>-</u>	327	314	<u>-</u>
Critical Hdwy	4.32	_	_	4.17	_	_	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	02	_	_	-	_	_	6.1	5.5	- 0.2	6.19	5.64	- 0.0
Critical Hdwy Stg 2	_	_	_	_	_	_	6.1	5.5	_	6.19	5.64	_
	2.398	_	_	2.263	_	_	3.5	4	3.3	3.581	4.126	3.426
Pot Cap-1 Maneuver	1258	_	_	1237	_	_	465	434	752	444	442	864
Stage 1	.200	_	_	-	_	_	705	663	-	776	706	-
Stage 2	_	_	_	_	_	_	790	696	_	671	635	_
Platoon blocked, %		_	_		_	_	, 50	000		J1 1	300	
Mov Cap-1 Maneuver	1258	_	_	1237	_	_	445	421	752	414	429	864
Mov Cap-2 Maneuver	-	_	_	-	_	_	445	421	-	414	429	-
Stage 1	_	_	_	_	_	_	700	658	_	771	690	_
Stage 2	_	_	_	<u>-</u>	_	_	757	680	<u>-</u>	637	631	<u>-</u>
owyo z								000		001	001	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			10.7			15.3		
HCM LOS	U.Z						В			13.3 C		
I IOWI LOO							ט			U		
Minor Lane/Major Mvm	, N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	2DI 51			
	. 1											
Capacity (veh/h) HCM Lane V/C Ratio		662		-	-	1237	-	-	434			
		0.051		-	-	0.023	-		0.198			
HCM Long LOS		10.7	7.9	-	-	8	-	-	15.3			
HCM 05th % file O(vah)		В	A	-	-	Α	-	-	C			
HCM 95th %tile Q(veh)		0.2	0	-	-	0.1	-	-	0.7			

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	97	269	13	169	0	0	0	0	1	4	55
Future Vol, veh/h	0	97	269	13	169	0	0	0	0	1	4	55
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mvmt Flow	0	97	269	13	169	0	0	0	0	1	4	55
Major/Minor N	//ajor1		ı	Major2					N	/linor2		
Conflicting Flow All	- -	0	0	366	0	0				427	561	169
Stage 1	_	-	-	-	-	-				195	195	-
Stage 2	_	_	_	_	_	-				232	366	-
Critical Hdwy	_		<u>-</u>	4.25	-	-				6.4	6.75	6.5
Critical Hdwy Stg 1	-	-	_	4.20	-	-				5.4	5.75	0.5
Critical Hdwy Stg 2	-		<u>-</u>	<u>-</u>		-				5.4	5.75	
Follow-up Hdwy	-	-	_	2.335	<u>-</u>	-				3.5	4.225	3.57
Pot Cap-1 Maneuver	0	-	-	1124	-	0				588	4.225	807
Stage 1	0	-	-	1124	-	0				843	698	- 007
Stage 1 Stage 2	0		-	_		0				811	584	
Platoon blocked, %	U	-	-	=	-	U				011	304	-
Mov Cap-1 Maneuver		-	_	1124	-	_				580	0	807
Mov Cap-1 Maneuver	-		-	1124		-				580	0	007
Stage 1	-	-	-	-	-	-				843	0	-
	-	-	-	=		-				800	0	-
Stage 2	-	-	-	-	-	-				000	U	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.6						9.9		
HCM LOS										Α		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)			-		-	801						
HCM Lane V/C Ratio		_		0.012		0.075						
HCM Control Delay (s)		_	_	8.2	0	9.9						
HCM Lane LOS		_	_	Α	A	3.5 A						
HCM 95th %tile Q(veh)		_	_	0	-	0.2						
				- 0		J.L						

Intersection												
Int Delay, s/veh	10											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LDIT	*****	<b>1</b>	TTDIT.	HDL	4	TIDIT.	ODL	051	OBIT
Traffic Vol, veh/h	94	4	0	0	6	5	176	10	6	0	0	0
Future Vol, veh/h	94	4	0	0	6	5	176	10	6	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	_	None	-	_	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	7	25	0	0	50	0	24	50	50	2	2	2
Mvmt Flow	94	4	0	0	6	5	176	10	6	0	0	0
Major/Minor N	Major1		ľ	Major2			Minor1					
Conflicting Flow All	11	0	-		-	0	201	203	4			
Stage 1	-	-	-	-	-	-	192	192	_			
Stage 2	-	-	-	-	-	-	9	11	-			
Critical Hdwy	4.17	-	-	-	-	-	6.64	7	6.7			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.64	6	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.64	6	-			
Follow-up Hdwy	2.263	-	-	-	-	-	3.716	4.45	3.75			
Pot Cap-1 Maneuver	1576	-	0	0	-	-	740	616	955			
Stage 1	-	-	0	0	-	-	791	660	-			
Stage 2	-	-	0	0	-	-	960	800	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1576	-	-	-	-	-	696	0	955			
Mov Cap-2 Maneuver	-	-	-	-	-	-	696	0	-			
Stage 1	-	-	-	-	-	-	744	0	-			
Stage 2	-	-	-	-	-	-	960	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	7.1			0			12					
HCM LOS							В					
Minor Lane/Major Mvm	it 1	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		702	1576	-	-	_						
HCM Lane V/C Ratio		0.274	0.06	-	-	-						
HCM Control Delay (s)		12	7.4	0	-	-						
HCM Lane LOS		В	Α	Α	-	-						
HCM 95th %tile Q(veh)		1.1	0.2	-	-	-						

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		î,			र्स						4	
Traffic Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	2	7	7	2	0	0	0	0	6	11	6
Major/Minor N	/lajor1		ı	Major2					N	/linor2		
Conflicting Flow All	-	0	0	9	0	0				22	25	2
Stage 1	-	-	-	-	-	-				16	16	-
Stage 2	-	-	-	-	-	-				6	9	-
Critical Hdwy	-	-	-	4.27	-	-				6.57	7.23	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.57	6.23	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.57	6.23	-
Follow-up Hdwy	-	-	-	2.353	-	-				3.653	4.657	3.3
Pot Cap-1 Maneuver	0	-	-	1518	-	0				957	747	1088
Stage 1	0	-	-	-	-	0				969	760	-
Stage 2	0	-	-	-	-	0				979	766	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1518	-	-				952	0	1088
Mov Cap-2 Maneuver	-	-	-	-	-	-				952	0	-
Stage 1	-	-	-	-	-	-				969	0	-
Stage 2	-	-	-	-	-	-				974	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.7						8.6		
HCM LOS										Α		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-	-	1518	-	1015						
HCM Lane V/C Ratio		_		0.005		0.023						
HCM Control Delay (s)		-	-	7.4	0	8.6						
HCM Lane LOS		-	-	Α	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	_	0.1						

Intersection								
Int Delay, s/veh 3.1								
Movement EBL EBT EBR	WBL WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations 4	f)			4				
Traffic Vol, veh/h 5 3 0	0 8		1	3	9	0	0	0
Future Vol, veh/h 5 3 0	0 8	19	1	3	9	0	0	0
Conflicting Peds, #/hr 0 0 0	0 0	0	0	0	0	0	0	0
Sign Control Free Free Free	Free Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized None		None	-	-	None	-	-	None
Storage Length		-	-	-	-	-	-	-
Veh in Median Storage, # - 0 -	- 0	-	-	0	_	-	0	-
Grade, % - 0 -	- 0	-	-	0	-	-	0	-
Peak Hour Factor 100 100 100	100 100	100	100	100	100	100	100	100
Heavy Vehicles, % 0 0 0	0 13	11	100	33	0	2	2	2
Mvmt Flow 5 3 0	0 8	19	1	3	9	0	0	0
Major/Minor Major1 Ma	ajor2		Minor1					
Conflicting Flow All 27 0 -			31	40	3			
Stage 1			13	13	-			
Stage 2			18	27	_			
Critical Hdwy 4.1		-	7.4	6.83	6.2			
Critical Hdwy Stg 1			6.4	5.83	0.2			
Critical Hdwy Stg 2			6.4	5.83	<u>-</u>			
Follow-up Hdwy 2.2			4.4	4.297	3.3			
Pot Cap-1 Maneuver 1600 - 0	0 -		782	795	1087			
Stage 1 - 0	0 -		806	827	-			
Stage 2 0	0 -		801	815	<u>-</u>			
Platoon blocked, %	-		001	013	_			
Mov Cap-1 Maneuver 1600		-	780	0	1087			
Mov Cap-1 Maneuver			780	0	1007			
Stage 1		-	804	0				
•			801	0	-			
Stage 2	-	_	001	U	-			
Approach EB	WB		NB					
HCM Control Delay, s 4.5	0		8.5					
HCM LOS			Α					
Minor Lane/Major Mvmt NBLn1 EBL	EBT WBT	WBR						
Capacity (veh/h) 1046 1600		-						
HCM Lane V/C Ratio 0.012 0.003		. <u>-</u>						
HCM Control Delay (s) 8.5 7.3	0 -							
HCM Lane LOS A A	A -							
HCM 95th %tile Q(veh) 0 0								
20 /3 2(1.51.)								

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	7	4	0	15	0	12	2	0	0	0	0
Future Vol, veh/h	1	7	4	0	15	0	12	2	0	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	25	0	0	0	18	0	0	0	0	0
Mvmt Flow	1	7	4	0	15	0	12	2	0	0	0	0
Major/Minor N	/lajor1			Major2			Minor1		<u> </u>	Minor2		
Conflicting Flow All	15	0	0	11	0	0	26	26	9	27	28	15
Stage 1	-	-	-	-	-	-	11	11	-	15	15	-
Stage 2	-	-	-	-	-	-	15	15	-	12	13	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.28	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.28	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.662	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1616	-	-	1621	-	-	945	871	1079	988	869	1070
Stage 1	-	-	-	-	-	-	970	890	-	1010	887	-
Stage 2	-	-	-	-	-	-	965	887	-	1014	889	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1616	-	-	1621	-	-	944	870	1079	985	868	1070
Mov Cap-2 Maneuver	-	-	-	-	-	-	944	870	-	985	868	-
Stage 1	-	-	-	-	-	-	969	889	-	1009	887	-
Stage 2	-	-	-	-	-	-	965	887	-	1011	888	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			8.9			0		
HCM LOS							Α			A		
Minor Lane/Major Mvmt	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)			1616	-		1621	_	_	_			
HCM Lane V/C Ratio		0.015		_	_	-	_	_	_			
HCM Control Delay (s)		8.9	7.2	0	_	0	_	_	0			
HCM Lane LOS		A	A	A	_	A	-	-	A			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	-			

Intersection												
Int Delay, s/veh	6.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			4						4	
Traffic Vol, veh/h	0	66	0	33	7	0	0	0	0	109	1	5
Future Vol, veh/h	0	66	0	33	7	0	0	0	0	109	1	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
_	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	66	0	33	7	0	0	0	0	109	1	5
Major/Minor M	ajor1		N	Major2					N	/linor2		
Conflicting Flow All	-	0	0	66	0	0				139	139	7
Stage 1	-	-	-	-	-	-				73	73	-
Stage 2	-	-	-	-	-	-				66	66	-
Critical Hdwy	-	-	_	4.23	_	-				6.43	7.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	6.5	-
Follow-up Hdwy	-	-	-	2.317	-	-				3.527	4.9	3.48
Pot Cap-1 Maneuver	0	-	-	1469	-	0				852	604	1025
Stage 1	0	-	-	-	_	0				947	676	-
Stage 2	0	-	-	-	-	0				954	682	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1469	-	-				832	0	1025
Mov Cap-2 Maneuver	-	-	-	-	-	-				832	0	-
Stage 1	-	-	-	-	-	-				947	0	-
Stage 2	-	-	-	-	-	-				932	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.2						10		
HCM LOS										В		
Minor Lane/Major Mvmt		EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-	-	1469	-							
HCM Lane V/C Ratio		-		0.022	_	0.137						
HCM Control Delay (s)		-	-		0	10						
HCM Lane LOS		_	-	A	A	В						
HCM 95th %tile Q(veh)		-	-	0.1	-	0.5						

Int Delay alveh
Int Delay, s/veh 4
Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
Lane Configurations 4 4
Traffic Vol, veh/h 42 133 0 0 31 78 9 4 123 0 0 0
Future Vol, veh/h 42 133 0 0 31 78 9 4 123 0 0 0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0
Sign Control Free Free Free Free Free Stop Stop Stop Stop Stop Stop
RT Channelized None None None
Storage Length
Veh in Median Storage, # - 0 0 0 -
Grade, % - 0 0 0 -
Peak Hour Factor 100 100 100 100 100 100 100 100 100 10
Heavy Vehicles, % 17 7 2 0 12 4 0 25 10 2 2 2
Mvmt Flow 42 133 0 0 31 78 9 4 123 0 0 0
Major/Minor Major1 Major2 Minor1
Conflicting Flow All 109 0 0 287 326 133
Stage 1 217 217 -
Stage 2 70 109 -
Critical Hdwy 4.27 6.4 6.75 6.3
Critical Hdwy Stg 1 5.4 5.75 -
Critical Hdwy Stg 2 5.4 5.75 -
Follow-up Hdwy 2.353 3.5 4.225 3.39
Pot Cap-1 Maneuver 1393 - 0 0 - 708 556 895
Stage 1 - 0 0 - 824 682 -
Stage 2 - 0 0 - 958 763 -
Platoon blocked, %
Mov Cap-1 Maneuver 1393 685 0 895
Mov Cap-2 Maneuver 685 0 -
Stage 1 797 0 -
Stage 2 958 0 -
Approach EB WB NB
-11
•
HCM LOS A
M. I. M. I. M. I. NOL A EDI. EDT. MOT. MOD.
Minor Lane/Major Mvmt NBLn1 EBL EBT WBT WBR
Capacity (veh/h) 877 1393
HCM Lane V/C Ratio 0.155 0.03
HCM Control Delay (s) 9.9 7.7 0
HCM Lane LOS A A A
HCM 95th %tile Q(veh) 0.5 0.1

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	253	0	0	102	3	1	1	4	1	0	6
Future Vol, veh/h	3	253	0	0	102	3	1	1	4	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	253	0	0	102	3	1	1	4	1	0	6
Major/Minor N	/lajor1		N	Major2		ľ	Minor1		N	/linor2		
Conflicting Flow All	105	0	0	253	0	0	366	364	253	366	363	104
Stage 1	-	-	-	-	-	-	259	259	-	104	104	-
Stage 2	-	-	-	-	-	-	107	105	-	262	259	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.453
Pot Cap-1 Maneuver	1499	-	-	1324	-	-	594	567	791	594	568	911
Stage 1	-	-	-	-	-	-	750	697	-	907	813	-
Stage 2	-	-	-	-	-	-	903	812	-	747	697	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1499	-	-	1324	-	-	589	566	791	589	567	911
Mov Cap-2 Maneuver	-	-	-	-	-	-	589	566	-	589	567	-
Stage 1	-	-	-	-	-	-	749	696	-	905	813	-
Stage 2	-	-	-	-	-	-	897	812	-	741	696	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0			10.2			9.3		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)			1499	-		1324	-	-				
HCM Lane V/C Ratio		0.009		-	_	_	_		0.008			
HCM Control Delay (s)		10.2	7.4	0	-	0	-	_				
HCM Lane LOS		В	Α	A	-	A	-	-	Α			
HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	0			
		-										

Intersection							
Int Delay, s/veh	5.5						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	CDL Š	<u> </u>		MOR	ODL 1	JDK 7	
Traffic Vol, veh/h	171	<b>T</b> 57	<b>1</b> → 58	29	<b>1</b> 25	69	
Future Vol, veh/h	171	57	58	29	25	69	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-		-	None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage	e, # -	0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	171	57	58	29	25	69	
Major/Minor I	Major1	N	Major2	ı	Minor2		
Conflicting Flow All	87	0	- -	0	472	73	
Stage 1	-	-	-	-	73	-	
Stage 2	-	_	_	-	399	-	
Critical Hdwy	4.15	-	-	-	6.4	6.26	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.245	-	-	-	3.5	3.354	
Pot Cap-1 Maneuver	1490	-	-	-	554	978	
Stage 1	-	-	-	-	955	-	
Stage 2	-	-	-	-	682	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1490	-	-	-	490	978	
Mov Cap-2 Maneuver	-	-	-	-	490	-	
Stage 1	-	-	-	-	845	-	
Stage 2	-	-	-	-	682	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.8		0		10		
HCM LOS					В		
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WRR	SBLn1 S	SRI n2
Capacity (veh/h)		1490		-	-		978
HCM Lane V/C Ratio		0.115		-		0.051	
HCM Control Delay (s)		7.7	_	_		12.7	9
HCM Lane LOS		Α	<u>-</u>	_	<u>-</u>	В	A
HCM 95th %tile Q(veh)	)	0.4	-	_	-	0.2	0.2

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ሻ	<del>(</del>			4			4	
Traffic Vol, veh/h	47	33	4	1	46	4	8	16	7	0	2	22
Future Vol, veh/h	47	33	4	1	46	4	8	16	7	0	2	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	-	_	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	_	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	47	33	4	1	46	4	8	16	7	0	2	22
Major/Minor N	Major1		ı	Major2		ı	Minor1		N	Minor2		
Conflicting Flow All	50	0	0	37	0	0	191	181	35	187	181	48
Stage 1	-	-	-	-	-	-	129	129	-	50	50	-
Stage 2	-	_	_	_	_	_	62	52	_	137	131	-
Critical Hdwy	4.21	_	-	4.1	_	-	7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1	-	_	_	- "-	_	_	6.1	5.56	-	6.1	5.5	-
Critical Hdwy Stg 2	_	_	-	-	_	-	6.1	5.56	_	6.1	5.5	_
Follow-up Hdwy	2.299	_	_	2.2	_	_	3.5	4.054	3.426	3.5	4	3.39
Pot Cap-1 Maneuver	1501	_	-	1587	_	-	773	706	1005	778	717	999
Stage 1	-	_	_	-	_	_	880	782	-	968	857	-
Stage 2	_	_	-	-	_	-	954	844	-	871	792	_
Platoon blocked, %		_	_		_	_	- 00 r	J 1 7		- VI I	, 02	
Mov Cap-1 Maneuver	1501	_	-	1587	_	-	736	683	1005	741	694	999
Mov Cap-2 Maneuver	-	_	_	-	_	_	736	683	-	741	694	-
Stage 1	_	_	-	-	_	-	853	758	-	938	856	_
Stage 2	_	_	_	_	_	_	930	843	_	820	767	_
2.6.50 2							300	3.0		320		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4.2			0.1			9.1			8.8		
HCM LOS	1.2			J. 1			Α			Α		
							, \			,,		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		904	1501	-	-	1587	-	-	964			
HCM Lane V/C Ratio		0.034		-		0.001	_		0.025			
HCM Control Delay (s)		9.1	7.5	<u>-</u>	-	7.3	-	_	8.8			
HCM Lane LOS		9.1 A	7.5 A	<u>-</u>	_	7.3 A	-	_	Α			
HCM 95th %tile Q(veh)		0.1	0.1	_	_	0	_	_	0.1			
HOW JOHN JOHN Q(VEII)		0.1	0.1			U			0.1			

Intersection						
Int Delay, s/veh	2.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDIX	YVDL	<u>₩</u>	NDL Y	אטא
Traffic Vol, veh/h	40	1	<b>1</b> 5	<b>T</b> 42	10	14
Future Vol, veh/h	40	1	5	42	10	14
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- -	None
Storage Length	_	-	150	-	0	INUITE
Veh in Median Storage,		_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	26	0	0	15	0	7
Mvmt Flow	40	1	5	42	10	14
INIVITIL FIOW	40	- 1	ິນ	42	10	14
Major/Minor Ma	ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	41	0	93	41
Stage 1	-	-	-	-	41	-
Stage 2	-	-	-	-	52	-
Critical Hdwy	-	-	4.1	-	6.4	6.27
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.363
Pot Cap-1 Maneuver	-	-	1581	-	912	1016
Stage 1	-	-	-	-	987	-
Stage 2	-	-	-	-	976	-
Platoon blocked, %	-	_		-		
Mov Cap-1 Maneuver	-	-	1581	-	909	1016
Mov Cap-2 Maneuver	_	-	-	_	909	-
Stage 1	_	_	_	_	987	_
Stage 2	_	_	_	_	973	_
Olago 2					010	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.8	
HCM LOS					Α	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		968	-	_	1581	_
HCM Lane V/C Ratio		0.025	_	_	0.003	_
HCM Control Delay (s)		8.8	_	_	7.3	_
HCM Lane LOS		A	-	-	Α.	_
HCM 95th %tile Q(veh)		0.1	_	_	0	_
1.5.11 55th 75th Q(1511)		J. 1			J	

Intersection   Int Delay, s/veh   0.7
Movement
Lane Configurations         Y         Image: Configuration of the processing o
Traffic Vol, veh/h         2         0         16         0         0         6           Future Vol, veh/h         2         0         16         0         0         6           Conflicting Peds, #/hr         0         0         0         0         0         0           Sign Control         Stop         Stop         Free
Future Vol, veh/h 2 0 16 0 0 6 Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 Sign Control Stop Stop Free Free Free Free RT Channelized - None - None - None Storage Length 0 Veh in Median Storage, # 0 - 0 0 Grade, % 0 - 0 - 0 0 Peak Hour Factor 100 100 100 100 100 100 100 Heavy Vehicles, % 0 0 0 0 0 0 17 Mvmt Flow 2 0 16 0 0 6  Major/Minor Minor1 Major1 Major2  Conflicting Flow All 22 16 0 0 16 0 Stage 1 16
Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free         Color of the color of the color of the color of the color of the color of the color of the color of the color of the color of the
Sign Control         Stop         Stop         Free         Free         Free         Free         Free         Free         Free         RT Ceannelized         - None
RT Channelized         - None         - None         - None           Storage Length         0
Storage Length         0         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         -         0         -         -         -         0         0         -         -         -         0         0         0         -         -         0         0         100         100         100         100         100         100         100         100         100         100         100         100         100         100         17         Mwriter         Mwriter         2         0         16         0         0         0         0         17         Mwriter         100         10
Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         -         0           Peak Hour Factor         100         100         100         100         100         100           Heavy Vehicles, %         0         0         0         0         0         0         17           Mvmt Flow         2         0         16         0         0         6           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -
Grade, %         0         -         0         -         -         0           Peak Hour Factor         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         17         Mwmt Flow         2         0         16         0         0         16         0         6         6         4         0         0         16         0         0         6         6         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         16         0         0         14         1         -         -         -<
Peak Hour Factor         100         17         Mwmt Flow         2         0         16         0         0         6         6           Major/Minor         Minor         Minor         Major
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         22         16         0         0         16           Stage 1         16         -         -         -         -         -           Stage 2         6         -         -         -         -         -         -           Critical Hdwy         6.4         6.2         -         4.1         -
Mount Flow         2         0         16         0         0         6           Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -<
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -
Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -         -         -         -         -         -           Stage 2         6         -         -         -         -         -         -         -           Critical Hdwy         6.4         6.2         -         -         4.1         -           Critical Hdwy Stg 1         5.4         -         -         -         -         -           Critical Hdwy Stg 2         5.4         -         -         -         -         -           Follow-up Hdwy         3.5         3.3         -         -         2.2         -           Pot Cap-1 Maneuver         1000         1069         -         -         1615         -           Stage 1         1012         -         -         -         -         -           Stage 2         1022         -         -         -         -         -           Platoon blocked, %         -         -         -         -         -         -
Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -         -         -         -         -         -           Stage 2         6         -         -         -         -         -         -         -           Critical Hdwy         6.4         6.2         -         -         4.1         -           Critical Hdwy Stg 1         5.4         -         -         -         -         -           Critical Hdwy Stg 2         5.4         -         -         -         -         -           Follow-up Hdwy         3.5         3.3         -         -         2.2         -           Pot Cap-1 Maneuver         1000         1069         -         -         1615         -           Stage 1         1012         -         -         -         -         -           Stage 2         1022         -         -         -         -         -           Platoon blocked, %         -         -         -         -         -         -
Conflicting Flow All         22         16         0         0         16         0           Stage 1         16         -         -         -         -         -         -           Stage 2         6         -         -         -         -         -         -         -           Critical Hdwy         6.4         6.2         -         -         4.1         -           Critical Hdwy Stg 1         5.4         -         -         -         -         -           Critical Hdwy Stg 2         5.4         -         -         -         -         -           Follow-up Hdwy         3.5         3.3         -         -         2.2         -           Pot Cap-1 Maneuver         1000         1069         -         -         1615         -           Stage 1         1012         -         -         -         -         -           Stage 2         1022         -         -         -         -         -           Platoon blocked, %         -         -         -         -         -         -
Stage 1       16       -       -       -       -         Stage 2       6       -       -       -       -         Critical Hdwy       6.4       6.2       -       -       4.1       -         Critical Hdwy Stg 1       5.4       -       -       -       -       -         Critical Hdwy Stg 2       5.4       -       -       -       -       -         Follow-up Hdwy       3.5       3.3       -       -       2.2       -         Pot Cap-1 Maneuver       1000       1069       -       1615       -         Stage 1       1012       -       -       -       -         Stage 2       1022       -       -       -       -         Platoon blocked, %       -       -       -       -       -
Stage 2       6       -       -       -       -         Critical Hdwy       6.4       6.2       -       4.1       -         Critical Hdwy Stg 1       5.4       -       -       -       -         Critical Hdwy Stg 2       5.4       -       -       -       -         Follow-up Hdwy       3.5       3.3       -       -       2.2       -         Pot Cap-1 Maneuver       1000       1069       -       -       1615       -         Stage 1       1012       -       -       -       -       -         Stage 2       1022       -       -       -       -       -         Platoon blocked, %       -       -       -       -       -       -
Critical Hdwy 6.4 6.2 - 4.1 - Critical Hdwy Stg 1 5.4 Critical Hdwy Stg 2 5.4 Follow-up Hdwy 3.5 3.3 - 2.2 - Pot Cap-1 Maneuver 1000 1069 - 1615 - Stage 1 1012 Stage 2 1022 Platoon blocked, %
Critical Hdwy Stg 1       5.4       -       -       -       -         Critical Hdwy Stg 2       5.4       -       -       -       -       -         Follow-up Hdwy       3.5       3.3       -       -       2.2       -         Pot Cap-1 Maneuver       1000       1069       -       -       1615       -         Stage 1       1012       -       -       -       -       -         Stage 2       1022       -       -       -       -         Platoon blocked, %       -       -       -       -       -
Critical Hdwy Stg 2       5.4       -
Follow-up Hdwy 3.5 3.3 - 2.2 - Pot Cap-1 Maneuver 1000 1069 - 1615 - Stage 1 1012 Stage 2 1022 Platoon blocked, %
Pot Cap-1 Maneuver 1000 1069 1615 - Stage 1 1012
Stage 1       1012       -       -       -       -       -         Stage 2       1022       -       -       -       -       -         Platoon blocked, %       -       -       -       -       -
Stage 1       1012       -       -       -       -       -         Stage 2       1022       -       -       -       -       -         Platoon blocked, %       -       -       -       -       -
Stage 2 1022 Platoon blocked, %
Platoon blocked, %
Mov Cap-2 Maneuver 1000
Stage 1 1012
Stage 2 1022
Stage 2 1022
Approach WB NB SB
HCM Control Delay, s 8.6 0 0
HCM LOS A
Minor Long/Major Myrot NDT NDDW/DLgd CDL CDT
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT
Capacity (veh/h) 1000 1615 -
HCM Lane V/C Ratio 0.002
HCM Control Delay (s) 8.6 0 -
HCM Lane LOS A A - HCM 95th %tile Q(veh) 0 0 -
HCM 95th %tile Q(veh) 0 0 -

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	74	LDIX	NDL	4	1 <u>uc</u>	אופט
Traffic Vol, veh/h	0	0	0	22	5	0
Future Vol, veh/h	0	0	0	22	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-		-	None
Storage Length	0	-	_	-		INUITE
Veh in Median Storage			_	0	0	_
Grade, %	, # 0	<u> </u>	_	0	0	_
Peak Hour Factor	100	100	100	100		100
					100	
Heavy Vehicles, %	0	0	0	5	20	0
Mvmt Flow	0	0	0	22	5	0
Major/Minor N	/linor2	N	Major1	N	/lajor2	
Conflicting Flow All	27	5	5	0		0
Stage 1	5	_	_	_	_	_
Stage 2	22	_	_	_	_	_
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-	_	_	_	_
Critical Hdwy Stg 2	5.4	-	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	993	1084	1630	_	_	_
Stage 1	1023	1004	1000	_	_	_
Stage 2	1006	-	-	_	_	_
Platoon blocked, %	1000	_	-	_	_	_
	993	1084	1630	<del>-</del>	-	_
Mov Cap-1 Maneuver		1004	1030	-	-	-
Mov Cap-2 Maneuver	993	-	-	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1006	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A					
	, ,					
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1630	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
		0	-	0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	-	-	-
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)			-		-	

Intersection						
Int Delay, s/veh	5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	7	<b></b>
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Otop	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	<u>-</u>
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	25	33	0	0	0	33
Mymt Flow	4	6	16	11	3	1
IVIVIIIL FIOW	4	O	10	11	J	I
Major/Minor N	Minor2	N	Major1	N	//ajor2	
Conflicting Flow All	47	4	4	0	-	0
Stage 1	4	-	-	-	-	-
Stage 2	43	-	-	-	-	-
Critical Hdwy	6.65	6.53	4.1	-	-	_
Critical Hdwy Stg 1	5.65	-	-	_	_	_
Critical Hdwy Stg 2	5.65	_	_	_	_	_
Follow-up Hdwy	3.725		2.2	_	_	_
Pot Cap-1 Maneuver	908	996	1631	_	_	_
Stage 1	962	-	1001	_	_	_
Stage 2	924	_	_	_	_	_
Platoon blocked, %	324	-	-	_		-
	900	996	1631	<u>-</u>	-	-
Mov Cap-1 Maneuver	899		1031	-	-	-
Mov Cap-2 Maneuver	899	-	-	-	-	-
Stage 1	952	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		4.3		0	
HCM LOS	A		1.0		V	
TIOW EGO	71					
Minor Lane/Major Mvm	<u>it</u>	NBL	NBTI	EBLn1	SBT	SBR
Capacity (veh/h)		1631	-	955	-	-
HCM Lane V/C Ratio		0.01	-	0.01	-	-
HCM Control Delay (s)		7.2	0	8.8	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Intersection												
Int Delay, s/veh	3.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	29	41	1	1	8	4	1	1	0	4	2	11
Future Vol, veh/h	29	41	1	1	8	4	1	1	0	4	2	11
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	18	100	0	0	0	0	0	0	25	0	0
Mvmt Flow	29	41	1	1	8	4	1	1	0	4	2	11
Major/Minor N	Major1			Major2		<u> </u>	Minor1			Minor2		
Conflicting Flow All	12	0	0	42	0	0	119	114	42	112	112	10
Stage 1	-	-	-	-	-	-	100	100	-	12	12	-
Stage 2	-	-	-	-	-	-	19	14	-	100	100	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.5	6.2	7.35	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4	3.3	3.725	4	3.3
Pot Cap-1 Maneuver	1550	-	-	1580	-	-	861	780	1034	814	782	1077
Stage 1	-	-	-	-	-	-	911	816	-	952	890	-
Stage 2	-	-	-	-	-	-	1005	888	-	853	816	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1550	-	-	1580	-	-	838	764	1034	801	766	1077
Mov Cap-2 Maneuver	-	-	-	-	-	-	838	764	-	801	766	-
Stage 1	-	-	-	-	-	-	894	800	-	934	889	-
Stage 2	-	-	-	-	-	-	992	887	-	836	800	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3			0.6			9.5			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)			1550	-		1580	-	-	954			
HCM Lane V/C Ratio		0.003		_		0.001	_	_	0.018			
HCM Control Delay (s)		9.5	7.4	0	_	7.3	0	_	8.8			
HCM Lane LOS		A	Α	A	_	A	A	-	A			
HCM 95th %tile Q(veh)		0	0.1	-	-	0	-	-	0.1			
((*******************************												

Intersection						
Int Delay, s/veh	3.9					
		WED	NET	NDD	ODI	OPT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		Þ			4
Traffic Vol, veh/h	5	48	75	8	55	36
Future Vol, veh/h	5	48	75	8	55	36
Conflicting Peds, #/hr	0	0	_ 0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	5	48	75	8	55	36
	1inor1		//ajor1		Major2	
Conflicting Flow All	225	79	0	0	83	0
Stage 1	79	-	-	-	-	-
Stage 2	146	-	-	-	-	-
Critical Hdwy	6.6	6.2	-	-	4.12	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	3.68	3.3	-	-	2.218	-
Pot Cap-1 Maneuver	725	987	-	-	1514	_
Stage 1	901	-	-	_	_	-
Stage 2	839	_	_	_	_	_
Platoon blocked, %	000		_	_		_
Mov Cap-1 Maneuver	698	987	_	_	1514	_
Mov Cap-1 Maneuver	698	-	_	_	-	_
	901	-	-	_		_
Stage 1			-	-		-
Stage 2	808	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9		0		4.5	
HCM LOS	A				1.0	
	,,					
Minor Lane/Major Mvmt		NBT	NIPDV	VBLn1	SBL	SBT
		INDI	INDE			SDI
Capacity (veh/h)		-	-	950	1514	-
HCM Lane V/C Ratio		-	-	0.056		-
HCM Control Delay (s)		-	-	9	7.5	0
HCM Lane LOS		-	-	Α	Α	Α
HCM 95th %tile Q(veh)		-	-	0.2	0.1	-

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7.	LDIX	******	4	¥	HOIL
Traffic Vol, veh/h	4	0	0	3	0	0
Future Vol, veh/h	4	0	0	3	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	0	0	3	0	0
Major/Minor Ma	oior1		Majara		Minor4	
	ajor1		Major2		Minor1	1
Conflicting Flow All	0	0	4	0	7	4
Stage 1	-	-	-	-	4	-
Stage 2	-	-	- 4.40	-	3	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1618	-	1014	1080
Stage 1	-	-	-	-	1019	-
Stage 2	-	-	-	-	1020	-
Platoon blocked, %	-	-	1010	-	1011	1000
Mov Cap-1 Maneuver	-	-	1618	-	1014	1080
Mov Cap-2 Maneuver	-	-	-	-	1014	-
Stage 1	-	-	-	-	1019	-
Stage 2	-	-	-	-	1020	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		0	
	•		<u> </u>		A	
HCM LOS						
HCM LOS						
		IDL A	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Minor Lane/Major Mvmt Capacity (veh/h)	1	NBLn1 -	EBT -	-	WBL 1618	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	1	-	-	-	1618 -	-
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	1	- - 0	- - -	- - -	1618 - 0	- - -
Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	١	-	-	-	1618 -	-

Intersection						
Int Delay, s/veh	0					
	EBL	EDT	WPT	WBR	CDI	SBR
Movement	ERF	EBT	WBT	WBK	SBL	SBK
Lane Configurations	0	<del>ન</del>	₽	0	À	^
Traffic Vol, veh/h	0	67	15	0	0	0
Future Vol, veh/h	0	67	15	0	0	0
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	е,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	67	15	0	0	0
N.A'/N.A'	NA - 1		4.1.0		A' O	
	Major1		Major2		Minor2	
Conflicting Flow All	15	0	-	0	82	15
Stage 1	-	-	-	-	15	-
Stage 2	-	-	-	-	67	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1603	-	-	-	920	1065
Stage 1	-	-	-	-	1008	-
Stage 2	-	-	-	-	956	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1603	_	-	_	920	1065
Mov Cap-2 Maneuver	-	_	_	_	920	-
Stage 1	_	_	_	_	1008	_
Stage 2	_	_	_	_	956	_
Olage 2					300	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS					Α	
Mineral and Maria Af	-1	EDI	CDT	MOT	MPD	2DL 4
Minor Lane/Major Mvn	nt	EBL	EBT	WBT	WBR :	SRFUI
Capacity (veh/h)		1603	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s	)	0	-	-	-	0
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh	1)	0	-	-	-	-

Intersection			
Intersection Delay, s/veh	7		
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Future Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0
Mvmt Flow	1	0	0	0	0	3	0	1	1	3	2	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB				WB			NB		SB		
Opposing Approach	WB				EB			SB		NB		
Opposing Lanes	1				1			1		1		
Conflicting Approach Left	SB				NB			EB		WB		
Conflicting Lanes Left	1				1			1		1		
Conflicting Approach Right	NB				SB			WB		EB		
Conflicting Lanes Right	1				1			1		1		
HCM Control Delay	7.1				6.3			6.6		7.6		
HCM LOS	Α				Α			Α		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	100%	0%	60%	
Vol Thru, %	50%	0%	0%	40%	
Vol Right, %	50%	0%	100%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	2	1	3	5	
LT Vol	0	1	0	3	
Through Vol	1	0	0	2	
RT Vol	1	0	3	0	
Lane Flow Rate	2	1	3	5	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.002	0.001	0.003	0.006	
Departure Headway (Hd)	3.611	4.114	3.313	4.591	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	996	874	1086	784	
Service Time	1.613	2.118	1.316	2.591	
HCM Lane V/C Ratio	0.002	0.001	0.003	0.006	
HCM Control Delay	6.6	7.1	6.3	7.6	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0	0	0	0	

Intersection						
Int Delay, s/veh	4.2					
		EDD	MDI	MOT	ND	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		• ኝ		W	
Traffic Vol, veh/h	93	105	6	23	138	8
Future Vol, veh/h	93	105	6	23	138	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	_	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	18	0	18	8	0
Mymt Flow	93	105	6	23	138	8
IVIVIIIL I IUW	30	100	U	23	100	U
Major/Minor Ma	ajor1	N	//ajor2		Minor1	
Conflicting Flow All	0	0	198	0	181	146
Stage 1	-	-	-	-	146	-
Stage 2	_	_	_	_	35	_
Critical Hdwy		_	4.1	_	6.48	6.2
Critical Hdwy Stg 1		_	7.1	_	5.48	0.2
	-	-	-		5.48	
Critical Hdwy Stg 2	-	-	-	-		-
Follow-up Hdwy	-	-	2.2	-	3.572	3.3
Pot Cap-1 Maneuver	-	-	1387	-	795	906
Stage 1	-	-	-	-	867	-
Stage 2	-	-	-	-	972	-
Platoon blocked, %	-			-		
Mov Cap-1 Maneuver	-	-	1387	-	792	906
Mov Cap-2 Maneuver	-	-	-	-	792	-
Stage 1	-	_	_	-	867	-
Stage 2	_	_	_	_	968	_
Jugo L					300	
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		10.5	
HCM LOS					В	
= • •						
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		797	-	-	1387	-
HCM Lane V/C Ratio		0.183	-		0.004	-
HCM Control Delay (s)		10.5	_	-	7.6	_
HCM Lane LOS		В	_	_	Α	_
HCM 95th %tile Q(veh)		0.7	_	_	0	_
HOW JOHN JOHNE Q(VEII)		0.1			U	_

3.8					
EBT	EBR	WBL	WBT	NBL	NBR
	189				52
146	189			134	52
0	0	0	0	0	0
Free	Free	Free	Free	Stop	Stop
-	None	-	None	-	None
-	-	150	-	0	-
, # 0	-	-	0	0	-
0	-	-	0	0	-
100	100	100	100	100	100
13	9	0	10	31	4
146	189	5	156	134	52
Agior1	N	Jaior?	N	/linor1	
					241
					241
-	-				_
-	-				6.24
					0.24
-	-		_		
-	-		_		3.336
-	-		_		793
-	-		_		195
	-				
	_	-		130	_
		1236		5/16	793
					195
					_
	_				_
				730	
0		0.2			
				В	
	EBT  146 146 0 Free ,# 0 0 100 13	EBT EBR  146 189 146 189 0 0 Free Free - None ,# 0 100 100 13 9 146 189  Major1 N 0 0	EBT EBR WBL  146 189 5 146 189 5 0 0 0 0 Free Free Free - None 150 ,# 0 100 100 100 13 9 0 146 189 5  Major1 Major2 0 0 335 4.1 4.1 1236 1236 1236 1236 1236 1236	EBT EBR WBL WBT  146 189 5 156 146 189 5 156 0 0 0 0 0 Free Free Free Free - None - None - 150 150 150 0 100 100 100 100 13 9 0 10 146 189 5 156  Major1 Major2 N 0 0 335 0 4.1 1236 1236 1236 1236 1236 1236 1236	EBT         EBR         WBL         WBT         NBL           146         189         5         156         134           146         189         5         156         134           0         0         0         0         0           Free         Free         Free         Free         Stop           - None         - None         -         0                     - None         - None         -         0         0         0           - 150         - 0         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100         100 </td

Minor Lane/Major Mvmt

HCM Lane V/C Ratio

HCM Control Delay (s)

HCM 95th %tile Q(veh)

Capacity (veh/h)

HCM Lane LOS

NBLn1

0.311

13.7

В

1.3

598

WBL

1236

7.9

Α

0

- 0.004

WBT

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ሻ	£			र्स	7		4	
Traffic Vol, veh/h	1	135	31	164	122	4	21	1	199	1	1	0
Future Vol, veh/h	1	135	31	164	122	4	21	1	199	1	1	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	_	None	-	_	None	-	-	None	-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	10	30	7	0	15	0	10	0	100	0
Mvmt Flow	1	135	31	164	122	4	21	1	199	1	1	0
Major/Minor N	/lajor1			Major2			Minor1		N	Minor2		
Conflicting Flow All	126	0	0	166	0	0	606	607	151	705	620	124
Stage 1	-	-	-	-	-	-	153	153	-	452	452	-
Stage 2	_	<u>-</u>	_	_	_	_	453	454	<u>-</u>	253	168	_
Critical Hdwy	4.1			4.4	_	_	7.25	6.5	6.3	7.1	7.5	6.2
Critical Hdwy Stg 1	-T. I	<u> </u>	_	-7.7	_	_	6.25	5.5	0.5	6.1	6.5	0.2
Critical Hdwy Stg 2	_	_	_	_	_	_	6.25	5.5	-	6.1	6.5	_
Follow-up Hdwy	2.2	<u>-</u>	_	2.47	_	_		4	3.39	3.5	4.9	3.3
Pot Cap-1 Maneuver	1473			1259	_	_	391	414	875	354	299	932
Stage 1	-	<u>-</u>	_	1200	_	_	820	775	-	591	435	-
Stage 2	_				_	_	562	573	_	756	607	_
Platoon blocked, %		_	-		_	_	JUZ	313		100	001	
Mov Cap-1 Maneuver	1473			1259	_	_	351	360	875	246	260	932
Mov Cap-1 Maneuver	-	<u> </u>	_	1200	_	_	351	360	-	246	260	-
Stage 1	_					_	819	774	-	590	378	_
Stage 2	_	_	_	_	_	_	487	499	-	583	606	_
Olaye Z	_	_				_	701	733		500	000	
A				\A/D			ND			00		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.7			10.9			19.3		
HCM LOS							В			С		
Minor Lane/Major Mvm	t	NBLn1 I	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1		
Capacity (veh/h)		351	875	1473	-	-	1259	-	-	253		
HCM Lane V/C Ratio		0.063	0.227	0.001	-	-	0.13	-	-	800.0		
HCM Control Delay (s)		15.9	10.3	7.4	-	-	8.3	-	-	19.3		
HCM Lane LOS		С	В	Α	-	-	Α	-	-	С		
HCM 95th %tile Q(veh)		0.2	0.9	0	-	-	0.4	-	-	0		

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ች	ĵ.		*	ĵ.	
Traffic Vol, veh/h	20	8	13	11	9	67	8	126	28	143	49	4
Future Vol, veh/h	20	8	13	11	9	67	8	126	28	143	49	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	8	0	0	23	0	4	7	36	23	0
Mvmt Flow	20	8	13	11	9	67	8	126	28	143	49	4
Major/Minor M	linor2		ľ	Minor1			Major1		ı	Major2		
Conflicting Flow All	531	507	51	504	495	140	53	0	0	154	0	0
Stage 1	337	337	-	156	156	-	-	-	-	-	-	-
Stage 2	194	170	-	348	339	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.43	4.1	-	-	4.46	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.507	2.2	-	-	2.524	-	-
Pot Cap-1 Maneuver	462	471	1000	482	479	855	1566	-	-	1243	-	-
Stage 1	681	645	-	851	772	-	-	-	-	-	-	-
Stage 2	812	762	-	672	643	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	381	415	1000	426	422	855	1566	-	-	1243	-	-
Mov Cap-2 Maneuver	381	415	-	426	422	-	-	-	-	-	-	-
Stage 1	678	571	-	847	768	-	-	-	-	-	-	-
Stage 2	736	758	-	579	569	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.1			10.9			0.4			6		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1566			484	693	1243					
HCM Lane V/C Ratio		0.005	_	_		0.126		_	<u>-</u>			
HCM Control Delay (s)		7.3	_	_	13.1	10.9	8.3	_	_			
HCM Lane LOS		Α.	_	_	В	В	Α	_	_			
HCM 95th %tile Q(veh)		0	_	_	0.3	0.4	0.4	_	_			
TOW JOHN JOHN Q(VOII)		- 0			0.0	0.4	0.7					

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		<b>1</b>			4
Traffic Vol. veh/h	2	0	28	0	0	291
Future Vol, veh/h	2	0	28	0	0	291
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	<u>-</u>	0	_	_	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	40	0	0	6
Mymt Flow	2	0	28	0	0	291
IVIVIIIL FIOW		U	20	U	U	291
Major/Minor N	Minor1	<u> </u>	//ajor1	<u> </u>	Major2	
Conflicting Flow All	319	28	0	0	28	0
Stage 1	28	-	-	-	-	-
Stage 2	291	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	-	_	_	_	-
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	678	1053	_	_	1599	_
Stage 1	1000	-	_	_	1000	_
Stage 2	763			<u>-</u>	-	_
Platoon blocked, %	103	_		-		
	670	1053	-	-	1599	-
Mov Cap-1 Maneuver	678		-	-		-
Mov Cap-2 Maneuver	678	-	-	-	-	-
Stage 1	1000	-	-	-	_	-
Stage 2	763	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.3		0		0	
HCM LOS	В		- 0		- 0	
TIOWI LOO	U					
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	678	1599	-
HCM Lane V/C Ratio		-	-	0.003	-	-
HCM Control Delay (s)		-	-	10.3	0	-
HCM Lane LOS		-	-	В	A	-
HCM 95th %tile Q(veh)		-	_	0	0	-

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	0	42	0	0	248	0
Future Vol, veh/h	0	0	0	0	0	0	0	42	0	0	248	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	_	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	49	0	0	10	0
Mvmt Flow	0	0	0	0	0	0	0	42	0	0	248	0
Major/Minor N	/linor2			Minor1		N	Major1		I	Major2		
Conflicting Flow All	290	290	248	290	290	42	248	0	0	42	0	0
Stage 1	248	248		42	42	-		-	-	-	-	-
Stage 2	42	42	-	248	248	-	-	-	_	-	_	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	666	624	796	666	624	1034	1330	-	-	1580	-	-
Stage 1	760	705	-	978	864	-	-	-	-	-	-	-
Stage 2	978	864	-	760	705	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	666	624	796	666	624	1034	1330	-	-	1580	-	-
Mov Cap-2 Maneuver	666	624	-	666	624	-	-	-	-	-	-	-
Stage 1	760	705	-	978	864	-	-	-	-	-	-	-
Stage 2	978	864	-	760	705	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0			0		
HCM LOS	A			A			•					
	, ,			, ,								
Minor Lane/Major Mvm	t	NBL	NBT	NRR	EBLn1V	VBI n1	SBL	SBT	SBR			
Capacity (veh/h)		1330					1580		-			
HCM Lane V/C Ratio		1330	_	<u>-</u>	<u> </u>	-	1300	_	_			
HCM Control Delay (s)		0	_		0	0	0	_				
HCM Lane LOS		A	_	<u>-</u>	A	A	A	_	<u> </u>			
HCM 95th %tile Q(veh)		0	_	_	-	-	0	_	_			
How Jour Joure Q(Veri)		U			_		U	_				

Intersection
Int Delay, s/veh 4
Movement EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR
Lane Configurations 💠 💠
Traffic Vol, veh/h 0 4 0 34 0 11 0 24 9 102 155 0
Future Vol, veh/h 0 4 0 34 0 11 0 24 9 102 155 0
Conflicting Peds, #/hr 0 0 0 0 0 0 0 0 0 0 0 0 0
Sign Control Stop Stop Stop Stop Stop Free Free Free Free Free Free Free Fre
RT Channelized None None None
Storage Length
Veh in Median Storage, # - 0 0 0 -
Grade, % - 0 0 0 0 -
Peak Hour Factor 100 100 100 100 100 100 100 100 100 10
Heavy Vehicles, % 0 0 0 6 0 44 0 39 33 16 13 0
Mvmt Flow 0 4 0 34 0 11 0 24 9 102 155 0
Major/Minor Minor2 Minor1 Major1 Major2
Conflicting Flow All 393 392 155 390 388 29 155 0 0 33 0 0
Stage 1 359 359 - 29 29
Stage 2 34 33 - 361 359
Critical Hdwy 7.1 6.5 6.2 7.16 6.5 6.64 4.1 4.26 -
Critical Hdwy Stg 1 6.1 5.5 - 6.16 5.5
Critical Hdwy Stg 2 6.1 5.5 - 6.16 5.5
Follow-up Hdwy 3.5 4 3.3 3.554 4 3.696 2.2 2.344
Pot Cap-1 Maneuver 570 547 896 562 550 937 1438 1493 -
Stage 1 663 631 - 978 875
Stage 2 987 872 - 649 631
Platoon blocked, %
Mov Cap-1 Maneuver 531 506 896 527 509 937 1438 1493 -
Mov Cap-2 Maneuver 531 506 - 527 509
Stage 1 663 584 - 978 875
Stage 2 975 872 - 596 584
Approach EB WB NB SB
HCM Control Delay, s 12.2 11.6 0 3
HCM LOS B B
TIOW LOO
NI I (II I II I I I I I I I I I I I I I
Minor Lane/Major Mvmt NBL NBT NBR EBLn1WBLn1 SBL SBT SBR
Capacity (veh/h) 1438 506 590 1493
HCM Lane V/C Ratio 0.008 0.076 0.068
HCM Control Delay (s) 0 12.2 11.6 7.6 0 -
HCM Lane LOS A B B A A -
HCM 95th %tile Q(veh) 0 0 0.2 0.2

W۵	ekday	Mornir	na Pea	ak Hour
***	citady	IVIOITIII	ig i cc	ait i ioui

Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	<u>₽</u>	
Traffic Vol, veh/h	0	0	1	15	103	7
Future Vol, veh/h	0	0	1	15	103	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop	None		None	Free -	None
			-		-	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	46	5	0
Mvmt Flow	0	0	1	15	103	7
Major/Minor N	/linor2	N	Major1		//ajor2	
		107			_	^
Conflicting Flow All	124		110	0	-	0
Stage 1	107	-	-	-	-	-
Stage 2	17	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	876	953	1493	-	-	-
Stage 1	922	-	-	-	-	-
Stage 2	1011	-	-	-	-	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	875	953	1493	_	_	_
Mov Cap-2 Maneuver	875	-	- 100	_	_	_
Stage 1	921	_	-	_	_	_
		-	-	-	-	-
Stage 2	1011	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0.5		0	
HCM LOS	A		0.0		U	
I IOWI LOG	Α					
Minor Lane/Major Mvm	t	NBL	NBTI	EBLn1	SBT	SBR
Capacity (veh/h)		1493	_	_	_	-
HCM Lane V/C Ratio		0.001	_	_	_	_
HCM Control Delay (s)		7.4	0	0	_	_
HCM Lane LOS		Α	A	A	<u>-</u>	_
HCM 95th %tile Q(veh)		0	-	-	_	_
How John Johne Q(Ven)		U	_	_	-	

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	15	0	1	102	0
Future Vol, veh/h	1	0	0	0	0	0	0	15	0	1	102	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	46	0	0	7	0
Mvmt Flow	1	0	0	0	0	0	0	15	0	1	102	0
Major/Minor N	Minor2		1	Minor1		N	//ajor1		N	Major2		
Conflicting Flow All	119	119	102	119	119	15	102	0	0	15	0	0
Stage 1	104	104	-	15	15	-	-	-	-	-	-	-
Stage 2	15	15	-	104	104	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	861	775	959	861	775	1070	1503	-	-	1616	-	-
Stage 1	907	813	-	1010	887	-	-	-	-	-	-	-
Stage 2	1010	887	-	907	813	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	860	774	959	860	774	1070	1503	-	-	1616	-	-
Mov Cap-2 Maneuver	860	774	-	860	774	-	-	-	-	-	-	-
Stage 1	907	812	-	1010	887	-	-	-	-	-	-	-
Stage 2	1010	887	-	906	812	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.2			0			0			0.1		
HCM LOS	Α			A								
Minor Lane/Major Mvm	it	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1503	_	-	860	-	1616	_	_			
HCM Lane V/C Ratio		-	-	-	0.001	_	0.001	-	_			
HCM Control Delay (s)		0	-	-	9.2	0	7.2	0	-			
HCM Lane LOS		A	-	-	Α	A	Α	A	-			
HCM 95th %tile Q(veh)		0	-	-	0	-	0	-	-			

Intersection						
Int Delay, s/veh	5.8					
		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<b>Y</b>	00	_ ∱	00	400	<del>વ</del>
Traffic Vol, veh/h	37	26	9	60	129	24
Future Vol, veh/h	37	26	9	60	129	24
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	3	0	22	0	22	0
Mvmt Flow	37	26	9	60	129	24
NA=:==/NA:===	\ 4:4		1-11		M-:0	
	Minor1		//ajor1		Major2	
Conflicting Flow All	321	39	0	0	69	0
Stage 1	39	-	-	-	-	-
Stage 2	282	-	-	-	-	-
Critical Hdwy	6.43	6.2	-	-	4.32	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.3	-	-	2.398	-
Pot Cap-1 Maneuver	671	1038	-	-	1414	-
Stage 1	981	-	-	-	-	-
Stage 2	763	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	609	1038	-	-	1414	-
Mov Cap-2 Maneuver	609	_	-	_	_	_
Stage 1	981	-	_	-	-	-
Stage 2	693	_	_	_	_	_
ougo 2	000					
Approach	WB		NB		SB	
HCM Control Delay, s	10.4		0		6.6	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)				734	1414	
HCM Lane V/C Ratio		_		0.086		_
HCM Control Delay (s)		_	_	10.4	7.8	0
HCM Lane LOS		_	-	10.4 B	7.0 A	A
HCM 95th %tile Q(veh)	\	-	-	0.3	0.3	
		-	-	0.5	0.3	-

Intersection												
Int Delay, s/veh	2.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4	7		4			4	
Traffic Vol, veh/h	12	8	0	1	98	138	0	1	0	54	0	30
Future Vol, veh/h	12	8	0	1	98	138	0	1	0	54	0	30
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	_	-	None	_	_	None	_	_	None	-	_	None
Storage Length	_	_	_	_	_	200	-	-	-	-	-	_
Veh in Median Storage	.# -	0	-	_	0	-	_	0	-	-	0	-
Grade, %	_	0	_	_	0	-	_	0	-	-	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	58	0	0	0	8	10	0	0	0	19	0	10
Mvmt Flow	12	8	0	1	98	138	0	1	0	54	0	30
Major/Minor	Major1		ı	Major2		ı	Minor1		ı	Minor2		
Conflicting Flow All	236	0	0	8	0	0	216	270	8	133	132	98
Stage 1	-	-	-	-	-	-	32	32	-	100	100	-
Stage 2	_	_	-	_	_	_	184	238	_	33	32	_
Critical Hdwy	4.68	-	-	4.1	-	_	7.1	6.5	6.2	7.29	6.5	6.3
Critical Hdwy Stg 1		_	-	-	_	_	6.1	5.5	-	6.29	5.5	-
Critical Hdwy Stg 2	-	_	-	_	-	_	6.1	5.5	_	6.29	5.5	_
Follow-up Hdwy	2.722	_	-	2.2	_	_	3.5	4		3.671	4	3.39
Pot Cap-1 Maneuver	1063	-	-	1625	-	_	745	640	1080	801	762	936
Stage 1	-	_	_	-	_	_	990	872	-	866	816	-
Stage 2	_	_	-	-	-	_	822	712	_	941	872	-
Platoon blocked, %		_	_		_	_						
Mov Cap-1 Maneuver	1063	_	_	1625	-	_	714	632	1080	793	753	936
Mov Cap-2 Maneuver	-	_	_	-	_	_	714	632	-	793	753	-
Stage 1	-	_	-	-	-	_	979	862	_	856	815	_
Stage 2	_	_	_	_	_	_	795	711	_	930	862	_
0 -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	5.1			0			10.7			9.8		
HCM LOS	<b>U.</b> 1						В			A		
										, ,		
Minor Lane/Major Mvm	nt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		632	1063			1625			839			
HCM Lane V/C Ratio		0.002		_	_	0.001	-	-	0.1			
HCM Control Delay (s)		10.7	8.4	0	_	7.2	0	_	9.8			
HCM Lane LOS		В	Α	A	_	Α.Σ	A	_	3.0 A			
HCM 95th %tile Q(veh)	1	0	0	-	_	0		_	0.3			
. Tom Cour /out ox (Vol)						- 0			3.0			

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		î,		ሻ	1>			4			4	
Traffic Vol, veh/h	1	36	1	11	345	88	4	0	9	73	2	7
Future Vol, veh/h	1	36	1	11	345	88	4	0	9	73	2	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	23	0	0	6	13	0	0	0	9	0	29
Mvmt Flow	1	36	1	11	345	88	4	0	9	73	2	7
Major/Minor N	1ajor1		N	Major2		N	Minor1			Minor2		
Conflicting Flow All	433	0	0	37	0	0	455	494	37	454	450	389
Stage 1	-	-	-	-	-	-	39	39	-	411	411	-
Stage 2	-	-	-	-	-	-	416	455	-	43	39	-
Critical Hdwy	5.1	-	-	4.1	-	-	7.1	6.5	6.2	7.19	6.5	6.49
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.19	5.5	-
Follow-up Hdwy	3.1	-	-	2.2	-	-	3.5	4		3.581	4	3.561
Pot Cap-1 Maneuver	753	-	-	1587	-	-	519	479	1041	505	508	604
Stage 1	-	-	-	-	-	-	981	866	-	604	598	-
Stage 2	-	-	-	-	-	-	618	572	-	954	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	753	-	-	1587	-	-	508	475	1041	497	504	604
Mov Cap-2 Maneuver	-	-	-	-	-	-	508	475	-	497	504	-
Stage 1	-	-	-	-	-	-	980	865	-	603	594	-
Stage 2	-	-	-	-	-	-	605	568	-	944	865	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0.2			9.7			13.5		
HCM LOS							Α			В		
Minor Lane/Major Mvmt		NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		787	753	-	-	1587	-	-	505			
HCM Lane V/C Ratio		0.017	0.001	-	-	0.007	-	-	0.162			
HCM Control Delay (s)		9.7	9.8	-	-	7.3	-	-	13.5			
HCM Lane LOS		Α	Α	-	-	Α	-	-	В			
HCM 95th %tile Q(veh)		0.1	0	-	-	0	-	-	0.6			

Weekday Morning Peak Hour

- 1139

- 0.007

8.2

Α

0

655

11.5

В

0.6

- 0.157

Α

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		₽			4						4	
Traffic Vol, veh/h	0	10	90	8	362	0	0	0	0	2	2	99
Future Vol, veh/h	0	10	90	8	362	0	0	0	0	2	2	99
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storag	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	20	21	75	12	0	2	2	2	0	50	14
Mvmt Flow	0	10	90	8	362	0	0	0	0	2	2	99
Major/Minor	Major1		ı	Major2					N	/linor2		
Conflicting Flow All	-	0	0	100	0	0				433	478	362
Stage 1	-	-	-	-	-	-				378	378	-
Stage 2	-	-	-	-	-	-				55	100	-
Critical Hdwy	-	-	-	4.85	-	-				6.4	7	6.34
Critical Hdwy Stg 1	-	-	-	-	-	-				5.4	6	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	6	-
Follow-up Hdwy	-	-	-	2.875	-	-				3.5	4.45	3.426
Pot Cap-1 Maneuver	0	-	-	1139	-	0				584	423	657
Stage 1	0	-	-	-	-	0				697	539	-
Stage 2	0	-	-	-	-	0				973	728	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1139	-	-				579	0	657
Mov Cap-2 Maneuver	-	-	-	-	-	-				579	0	-
Stage 1	-	-	-	-	-	-				697	0	-
Stage 2	-	-	-	-	-	-				964	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			0.2						11.5		
HCM LOS										В		
Minor Lane/Major Mvi	mt	EBT	EBR	WBL	WBT	SBLn1						
				4400								

Capacity (veh/h)

HCM Lane LOS

HCM Lane V/C Ratio

HCM Control Delay (s)

HCM 95th %tile Q(veh)

Intersection												
Int Delay, s/veh	11											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स			ĵ.			4				
Traffic Vol, veh/h	9	3	0	0	4	1	366	5	24	0	0	0
Future Vol, veh/h	9	3	0	0	4	1	366	5	24	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	. # -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	-, -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	14	0	0	0	75	0	10	80	26	2	2	2
Mymt Flow	9	3	0	0	4	1	366	5	24	0	0	0
Major/Minor	Major1		1	Major2		N	/linor1					
Conflicting Flow All	5	0	-	-	_	0	26	26	3			
Stage 1	-	-	-	_	-	-	21	21	-			
Stage 2	-	_	_	_	_	-	5	5	_			
Critical Hdwy	4.24	-	_	_	-	-	6.5	7.3	6.46			
Critical Hdwy Stg 1	-	_	-	_	_	-	5.5	6.3	-			
Critical Hdwy Stg 2	_	-	_	_	-	-	5.5	6.3	-			
Follow-up Hdwy	2.326	_	-	-	_	-	3.59		3.534			
Pot Cap-1 Maneuver	1541	-	0	0	-	-	969	736	1015			
Stage 1	-	-	0	0	-	-	981	745	-			
Stage 2	-	-	0	0	-	-	998	759	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1541	-	-	-	-	-	963	0	1015			
Mov Cap-2 Maneuver	-	-	-	-	-	-	963	0	-			
Stage 1	-	-	-	-	-	-	975	0	-			
Stage 2	-	-	-	-	-	-	998	0	-			
ŭ.												
Approach	EB			WB			NB					
HCM Control Delay, s	5.5			0			11.3					
HCM LOS							В					
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)		966	1541	-	-	-						
HCM Lane V/C Ratio			0.006	-	-	-						
HCM Control Delay (s)		11.3	7.3	0	-	-						
HCM Lane LOS		В	Α	Α	-	-						
HCM 95th %tile Q(veh)	)	2	0	-	-	-						

Α

0

Α

Α

0.1

**HCM Lane LOS** 

HCM 95th %tile Q(veh)

Intersection													
Int Delay, s/veh	2.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4			ĵ.			4					
Traffic Vol, veh/h	0	15	0	0	14	4	0	5	9	0	0	0	
Future Vol, veh/h	0	15	0	0	14	4	0	5	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,	# -	0	-	-	0	_	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	0	11	0	0	29	0	0	80	12	2	2	2	
Mvmt Flow	0	15	0	0	14	4	0	5	9	0	0	0	
Major/Minor N	1ajor1			Major2		N	Minor1						
Conflicting Flow All	18	0	-	-	-	0	31	33	15				
Stage 1	-	-	-	-	-	-	15	15	-				
Stage 2	-	-	-	-	-	-	16	18	-				
Critical Hdwy	4.1	-	-	-	-	-	6.4	7.3	6.32				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.3	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.3	-				
Follow-up Hdwy	2.2	-	-	-	-	-	3.5	4.72	3.408				
Pot Cap-1 Maneuver	1612	-	0	0	-	-	988	729	1036				
Stage 1	-	-	0	0	-	-	1013	750	-				
Stage 2	-	-	0	0	-	-	1012	748	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1612	-	-	-	-	-	988	0	1036				
Mov Cap-2 Maneuver	-	-	-	-	-	-	988	0	-				
Stage 1	-	-	-	-	-	-	1013	0	-				
Stage 2	-	-	-	-	-	-	1012	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	0			0			8.5						
HCM LOS							Α						
Minor Lane/Major Mvmt	: 1	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		1036	1612	-	-	-							
HCM Lane V/C Ratio		0.014	-	-	_	-							
HCM Control Delay (s)		8.5	0	-	-	-							
HCM Lane LOS		Α	A	-	-	-							
HCM 95th %tile Q(veh)		0	0	-	-	-							

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	22	2	0	14	0	2	0	0	0	0	2
Future Vol, veh/h	0	22	2	0	14	0	2	0	0	0	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	13	0	0	21	0	0	0	0	0	0	50
Mvmt Flow	0	22	2	0	14	0	2	0	0	0	0	2
Major/Minor N	//ajor1		ľ	Major2		ı	Minor1		N	/linor2		
Conflicting Flow All	14	0	0	24	0	0	38	37	23	37	38	14
Stage 1	-	-	-	-	-	-	23	23	-	14	14	-
Stage 2	-	-	-	-	-	-	15	14	-	23	24	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.7
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.75
Pot Cap-1 Maneuver	1617	-	-	1604	-	-	972	859	1060	973	858	942
Stage 1	_	-	_	-	-	-	1000	880	-	1011	888	-
Stage 2	-	-	-	-	-	-	1010	888	-	1000	879	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1617	-	-	1604	-	-	970	859	1060	973	858	942
Mov Cap-2 Maneuver	-	-	-	-	-	-	970	859	-	973	858	-
Stage 1	-	-	-	-	-	-	1000	880	-	1011	888	-
Stage 2	-	-	-	-	-	-	1008	888	-	4000	879	-
J												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			8.7			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	t 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		970	1617	-	-	1604	-	_	942			
HCM Lane V/C Ratio		0.002	-	-	-	-	-	-	0.002			
HCM Control Delay (s)		8.7	0	-	-	0	-	-	8.8			
HCM Lane LOS		A	A	-	-	A	-	-	A			
HCM 95th %tile Q(veh)		0	0	_	-	0	-	_	0			

Intersection												
Int Delay, s/veh	7.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			ની						4	
Traffic Vol, veh/h	0	32	5	118	25	0	0	0	0	67	1	117
Future Vol, veh/h	0	32	5	118	25	0	0	0	0	67	1	117
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	25	0	6	0	0	2	2	2	3	0	0
Mvmt Flow	0	32	5	118	25	0	0	0	0	67	1	117
Major/Minor N	Major1		<u> </u>	Major2					<u> </u>	/linor2		
Conflicting Flow All	-	0	0	37	0	0				296	298	25
Stage 1	-	-	-	-	-	-				261	261	-
Stage 2	-	-	-	-	-	-				35	37	-
Critical Hdwy	-	-	-	4.16	-	-				6.43	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	5.5	-
Follow-up Hdwy	-	-	-	2.254	-	-				3.527	4	3.3
Pot Cap-1 Maneuver	0	-	-	1548	-	0				693	617	1057
Stage 1	0	-	-	-	-	0				780	696	-
Stage 2	0	-	-	-	-	0				985	868	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1548	-	-				640	0	1057
Mov Cap-2 Maneuver	-	-	-	-	-	-				640	0	-
Stage 1	-	-	-	-	-	-				780	0	-
Stage 2	-	-	-	-	-	-				909	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.2						10.4		
HCM LOS										В		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-		1548	-							
HCM Lane V/C Ratio		_		0.076	_	0.217						
HCM Control Delay (s)		_	_		0	10.4						
HCM Lane LOS		-	-	A	A	В						
HCM 95th %tile Q(veh)		-	-	0.2	-	0.8						

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		स			ĵ.			4				
Traffic Vol, veh/h	16	83	0	0	121	86	22	2	9	0	0	0
Future Vol, veh/h	16	83	0	0	121	86	22	2	9	0	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	_	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	57	41	0	0	18	23	0	100	33	2	2	2
Mvmt Flow	16	83	0	0	121	86	22	2	9	0	0	0
Major/Minor N	Major1		ľ	Major2		N	Minor1					
Conflicting Flow All	207	0	-		-	0	279	322	83			
Stage 1	-	-	-	-	-	-	115	115	-			
Stage 2	-	-	-	-	-	-	164	207	-			
Critical Hdwy	4.67	-	-	-	-	-	6.4	7.5	6.53			
Critical Hdwy Stg 1	-	-	-	-	-	-	5.4	6.5	-			
Critical Hdwy Stg 2	-	-	-	-	-	-	5.4	6.5	-			
Follow-up Hdwy	2.713	-	-	-	-	-	3.5	4.9	3.597			
Pot Cap-1 Maneuver	1096	-	0	0	-	-	715	464	897			
Stage 1	-	-	0	0	-	-	915	645	-			
Stage 2	-	-	0	0	-	-	870	580	-			
Platoon blocked, %		-			-	-						
Mov Cap-1 Maneuver	1096	-	-	-	-	-	704	0	897			
Mov Cap-2 Maneuver	-	-	-	-	-	-	704	0	-			
Stage 1	-	-	-	-	-	-	901	0	-			
Stage 2	-	-	-	-	-	-	870	0	-			
Approach	EB			WB			NB					
HCM Control Delay, s	1.3			0			10					
HCM LOS				-			В					
Minor Lane/Major Mvm	ıt N	NBLn1	EBL	EBT	WBT	WBR						
Capacity (veh/h)	· '	751	1096	-								
HCM Lane V/C Ratio		0.044		_		_						
HCM Control Delay (s)		10	8.3	0	_							
HCM Lane LOS		В	Α	A	_	_						
HCM 95th %tile Q(veh)		0.1	0	-	_	_						
TION JOHN JUHIC Q(VEII)		J. 1										

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	87	2	1	203	0	1	0	0	1	0	3
Future Vol, veh/h	3	87	2	1	203	0	1	0	0	1	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	_	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	38	50	0	21	0	0	0	0	0	0	67
Mvmt Flow	3	87	2	1	203	0	1	0	0	1	0	3
Major/Minor N	Major1		ľ	Major2		ľ	Minor1		N	/linor2		
Conflicting Flow All	203	0	0	89	0	0	301	299	88	299	300	203
Stage 1	-	-	-	-	-	-	94	94	-	205	205	-
Stage 2	-	-	-	-	-	-	207	205	-	94	95	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.87
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.903
Pot Cap-1 Maneuver	1204	-	-	1519	-	-	655	616	976	657	616	698
Stage 1	-	-	-	-	-	-	918	821	-	802	736	-
Stage 2	-	-	-	-	-	-	800	736	-	918	820	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1204	-	-	1519	-	-	650	614	976	655	614	698
Mov Cap-2 Maneuver	-	-	-	-	-	-	650	614	-	655	614	-
Stage 1	-	-	-	-	-	-	915	819	-	800	735	-
Stage 2	-	-	-	-	-	-	796	735	-	915	818	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.3			0			10.5			10.3		
HCM LOS							В			В		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)			1204	-		1519	-	-				
HCM Lane V/C Ratio		0.002		_		0.001	_		0.006			
HCM Control Delay (s)		10.5	8	0	-	7.4	0	-				
HCM Lane LOS		В	A	A	-	Α	A	-	В			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0			

Intersection							
Int Delay, s/veh	5.7						
<u> </u>		EDT	WDT	WDD	CDI	CDD	
Movement Configurations	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	<b>ች</b>	<b>†</b>	<b>1</b> → 49	15	<u>ሻ</u>	120	
Traffic Vol. veh/h	6	20 20	49	15 15	11 11	129 129	
Future Vol, veh/h	6	20	49	15	0	129	
Conflicting Peds, #/hr Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-ree	None	Free -	None			
Storage Length	180	None -	-	None -	100	0	
Veh in Median Storage,		0	0	-	0	-	
Grade, %	# - -	0	0	_	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	0	5	7	0	0	2	
Mvmt Flow	6	20	49	15	11	129	
IVIVIIIL I IUW	U	20	43	10	11	123	
	lajor1		Major2		/linor2		
Conflicting Flow All	64	0	-	0	89	57	
Stage 1	-	-	-	-	57	-	
Stage 2	-	-	-	-	32	-	
Critical Hdwy	4.1	-	-	-	6.4	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.2	-	-	-		3.318	
	1551	-	-	-	917	1009	
Stage 1	-	-	-	-	971	-	
Stage 2	-	-	-	-	996	-	
Platoon blocked, %	4==4	-	-	-	0.10	1000	
•	1551	-	-	-	913	1009	
Mov Cap-2 Maneuver	-	-	-	-	913	-	
Stage 1	-	-	-	-	967	-	
Stage 2	-	-	-	-	996	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.7		0		9.1		
HCM LOS					Α		
Minard ana/Maria Maria		EDI	EDT	MPT	MES		
Minor Lane/Major Mvmt		EBL	EBT	WBT		SBLn1	
Capacity (veh/h)		1551	-	-	-		1009
HCM Lane V/C Ratio		0.004	-	-		0.012	
HCM Control Delay (s)		7.3	-	-	-	9	9.1
HCM Lane LOS		A	-	-	-	A	A
HCM 95th %tile Q(veh)		0	-	-	-	0	0.4

Intersection												
Int Delay, s/veh	4.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	ĵ,		7	f)			4			4	
Traffic Vol, veh/h	15	36	7	1	29	1	1	1	1	4	11	32
Future Vol, veh/h	15	36	7	1	29	1	1	1	1	4	11	32
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	13	31	0	100	33	0	0	100	100	0	0	16
Mvmt Flow	15	36	7	1	29	1	1	1	1	4	11	32
Major/Minor I	Major1		ľ	Major2		1	Minor1		N	/linor2		
Conflicting Flow All	30	0	0	43	0	0	123	102	40	102	105	30
Stage 1	-	-	-	-	-	-	70	70	-	32	32	-
Stage 2	-	-	-	-	-	-	53	32	-	70	73	-
Critical Hdwy	4.23	-	-	5.1	-	-	7.1	7.5	7.2	7.1	6.5	6.36
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	6.5	-	6.1	5.5	-
Follow-up Hdwy	2.317	-	-	3.1	-	-	3.5	4.9	4.2	3.5	4	3.444
Pot Cap-1 Maneuver	1514	-	-	1113	-	-	856	636	810	884	789	1006
Stage 1	-	-	-	-	-	-	945	679	-	990	872	-
Stage 2	-	-	-	-	-	-	965	709	-	945	838	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1514	-	-	1113	-	-	813	629	810	874	780	1006
Mov Cap-2 Maneuver	-	-	-	-	-	-	813	629	-	874	780	-
Stage 1	-	-	-	-	-	-	936	672	-	980	871	-
Stage 2	-	-	-	-	-	-	922	708	-	933	830	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.9			0.3			8.4			9.1		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR S	SBLn1			
Capacity (veh/h)		1064	1514	_	_	1113	_	_	931			
HCM Lane V/C Ratio		0.003	0.01	_	_	0.001	-	_	0.05			
HCM Control Delay (s)		8.4	7.4	-	-	8.2	-	-	9.1			
HCM Lane LOS		A	A	_	_	A	-	_	A			
HCM 95th %tile Q(veh)	)	0	0	_	_	0	-	-	0.2			
2 22 70 2(1011)												

Intersection						
Int Delay, s/veh	0.8					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDIX	VVDL	<u>₩</u>	NDL W	אטוז
Traffic Vol, veh/h	37	3	8	<b>T</b> 29	0	0
Future Vol, veh/h	37	3	8	29	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	riee -	None	Stop -	None
Storage Length	-	NOITE	150	-	0	None -
			150			
Veh in Median Storage,		-		0	0	-
Grade, %	0	400	400	0	0	400
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	8	33	0	13	0	0
Mvmt Flow	37	3	8	29	0	0
Major/Minor Ma	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	40	0	84	39
Stage 1	-	_	-	-	39	-
Stage 2	_	_	_	_	45	_
Critical Hdwy	_	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	<u>-</u>	T. I	_	5.4	- 0.2
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	_	<u>-</u>	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	_	_	1583	_	923	1038
Stage 1	_	_	1303	_	989	-
Stage 2			-		983	-
	-	-	-	-	900	-
Platoon blocked, %	-	-	4500	-	040	4000
Mov Cap-1 Maneuver	-	-	1583	-	918	1038
Mov Cap-2 Maneuver	-	-	-	-	918	-
Stage 1	-	-	-	-	989	-
Stage 2	-	-	-	-	978	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.6		0	
HCM LOS	U		1.0		A	
I IOW LOS						
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1583	-
HCM Lane V/C Ratio		-	-	-	0.005	-
HCM Control Delay (s)		0	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection						
Int Delay, s/veh	0					
		WED	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	^	<b>₽</b>	4	^	4
Traffic Vol, veh/h	0	0	0	1	0	14
Future Vol, veh/h	0	0	0	1	0	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	400	0	400	400	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	14
Mvmt Flow	0	0	0	1	0	14
Major/Minor N	Minor1	N	Major1	N	Major2	
Conflicting Flow All	15	1	0	0	1	0
Stage 1	1	-	-	-	-	-
Stage 2	14	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	1009	1090	-	-	1635	-
Stage 1	1028	-	-	-	-	-
Stage 2	1014	_	_	_	_	_
Platoon blocked, %			-	-		_
Mov Cap-1 Maneuver	1009	1090	_	_	1635	-
Mov Cap-2 Maneuver	1009	-	_	_	-	_
Stage 1	1028	_	_	_	_	_
Stage 2	1014	<u>-</u>	_	_	_	_
Olage 2	1017					
Approach	WB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvmt	t	NBT	NRRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	-	1635	-
HCM Lane V/C Ratio		<u>-</u>	_	_	-	<u>-</u>
HCM Control Delay (s)		_	_	0	0	_
HCM Lane LOS		<u>-</u>	_	A	A	<u>-</u>
HCM 95th %tile Q(veh)		_	_	-	0	_
HOW JOHN JOHN Q(VEII)					U	

Intersection						
Int Delay, s/veh	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W			4	₽	
Traffic Vol, veh/h	1	0	0	0	19	0
Future Vol. veh/h	1	0	0	0	19	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_	None	_	None
Storage Length	0	-	-	-	_	-
Veh in Median Storage,		_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	11	0
Mymt Flow	1	0	0	0	19	0
IVIVIIIL I IOW	ļ	U	U	U	13	U
Major/Minor N	1inor2		Major1		/lajor2	
Conflicting Flow All	19	19	19	0	-	0
Stage 1	19	-	-	-	-	-
Stage 2	0	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	_	-	_	-	-
Critical Hdwy Stg 2	5.4	_	-	-	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	1004	1065	1611	_	_	_
Stage 1	1009	-	-	_	_	_
Stage 2	-	_	_	_	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	1004	1065	1611	_	_	_
Mov Cap-1 Maneuver	1004	1005	-	_	_	_
	1004			-		
Stage 1			-			
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
N. 1 (24 ) 1 2		NE	NET	EDL 4	057	000
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1611	-		-	-
HCM Lane V/C Ratio		-	-	0.001	-	-
HCM Control Delay (s)		0	-	8.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	f)	
Traffic Vol, veh/h	2	12	5	1	9	7
Future Vol, veh/h	2	12	5	1	9	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	-	-
Veh in Median Storage		-	_	0	0	_
Grade, %	0	_	-	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	8	40	0	10	14
Mvmt Flow	2	12	5	1	9	7
WWW.CT IOW	_			•		•
	Minor2		Major1		/lajor2	
Conflicting Flow All	24	13	16	0	-	0
Stage 1	13	-	-	-	-	-
Stage 2	11	-	-	-	-	-
Critical Hdwy	6.4	6.28	4.5	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.372	2.56	-	-	-
Pot Cap-1 Maneuver	997	1050	1386	-	-	-
Stage 1	1015	-	-	-	-	-
Stage 2	1017	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	993	1050	1386	-	-	-
Mov Cap-2 Maneuver	993	-	-	-	-	-
Stage 1	1011	-	-	-	-	-
Stage 2	1017	_	-	_	_	_
3 11 9						
Approach	EB		NB		SB	
HCM Control Delay, s	8.5		6.3		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1386		1041	-	
HCM Lane V/C Ratio		0.004		0.013	_	_
HCM Control Delay (s)		7.6	0	8.5	_	_
HCM Lane LOS		Α.	A	Α	_	_
HCM 95th %tile Q(veh	)	0		0	_	_
Sivi ootii 70tiio Q(Voii	1	- 0				

Intersection												
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	8	14	1	0	123	2	2	2	1	1	0	104
Future Vol, veh/h	8	14	1	0	123	2	2	2	1	1	0	104
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	_	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	_	-	0	-
Grade, %	_	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	33	50	0	0	15	0	0	0	0	0	0	24
Mvmt Flow	8	14	1	0	123	2	2	2	1	1	0	104
Major/Minor	Major1		ľ	Major2		N	/linor1		N	/linor2		
Conflicting Flow All	125	0	0	15	0	0	207	156	15	156	155	124
Stage 1	-	-	-	-	-	-	31	31	-	124	124	-
Stage 2	-	-	-	-	-	-	176	125	-	32	31	-
Critical Hdwy	4.43	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.44
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.497	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.516
Pot Cap-1 Maneuver	1291	-	-	1616	-	-	755	740	1070	815	741	871
Stage 1	-	-	-	-	-	-	991	873	-	885	797	-
Stage 2	-	-	-	-	-	-	831	796	-	990	873	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1291	-	-	1616	-	-	662	736	1070	808	737	871
Mov Cap-2 Maneuver	-	-	-	-	-	-	662	736	-	808	737	-
Stage 1	-	-	-	-	-	-	985	868	-	880	797	-
Stage 2	-	-	-	-	-	-	732	796	-	981	868	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	2.7			0			9.8			9.7		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt l	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBL <sub>n1</sub>			
Capacity (veh/h)		749	1291	_	-	1616	_	_	870			
HCM Lane V/C Ratio			0.006	-	-	-	-	-	0.121			
HCM Control Delay (s)		9.8	7.8	0	-	0	-	-	9.7			
HCM Lane LOS		Α	Α	Α	-	Α	-	-	Α			
HCM 95th %tile Q(veh)	)	0	0	-	-	0	-	-	0.4			

Intersection						
Int Delay, s/veh	1.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WBL	WBK		NBK	OBL	
Lane Configurations		10	<b>}</b>	2	15	<del>વ</del>
Traffic Vol, veh/h	5	19	32	2	15	101
Future Vol, veh/h	5	19	32	2	15	101
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	6	21	0	12	13
Mvmt Flow	5	19	32	2	15	101
Major/Minor N	1inor1	N	Major1	-	Major2	
	164	33		0	34	0
Conflicting Flow All	33		0	U		
Stage 1		-	-	-	-	-
Stage 2	131	-	-	-	4.00	-
Critical Hdwy	6.6	6.26	-	-	4.22	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy		3.354	-	-	2.308	-
Pot Cap-1 Maneuver	787	1029	-	-	1515	-
Stage 1	945	-	-	-	-	-
Stage 2	853	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	779	1029	-	-	1515	-
Mov Cap-2 Maneuver	779	-	-	-	-	-
Stage 1	945	-	-	-	-	-
Stage 2	844	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.8		0		1	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1515	_
HCM Lane V/C Ratio		_		0.025	0.01	_
HCM Control Delay (s)		-	_		7.4	0
HCM Lane LOS		_	_	A	Α	A
HCM 95th %tile Q(veh)		-	_	0.1	0	-
				٠. ١	_	

0 EBT	EBR	WBL	WBT	NDI	
ĵ.	EBR	WBL	WRT	NDI	
ĵ.				NBL	NBR
			4	Y	H.SIK
4	0	0	4	0	0
4	0	0	4	0	0
0		0	0	0	0
Free	Free	Free	Free	Stop	Stop
-					None
_	-	_			-
e. # 0	_	-			-
		_			_
					100
					2
					0
7	J		<b>-</b>		
0	0	4	0		4
-	-	-	-	4	-
-	-	-	-	4	-
-	-	4.12	-	6.42	6.22
-	-	-	-	5.42	-
-	-	-	-	5.42	-
-	-	2.218	-	3.518	3.318
-	-	1618	-	1013	1080
-	-	-	-	1019	-
-	-	-	-	1019	-
_	_		_		
_		1618	_	1013	1080
		-	_		-
_		_			_
					_
				1010	_
EB		WB		NB	
0		0		0	
				Α	
<b>.</b>	NIDI -1	EDT	EDD	WDI	WDT
IL I	INDLIII				WBT
	-	-			-
	-	-	-	-	-
	0	-	-	0	-
	-				
)	Α	-	-	A 0	-
		Section   Sect	- None	- None - None 0 0 0 100 100 100 100 2 2 2 2 2 4 0 0 4  Major1 Major2 0 0 0 4 0	- None - None - O O O O O O O O O O O O O O O O O O

Laterrandon						
Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ની	ĵ.		¥	
Traffic Vol, veh/h	184	5	57	170	4	6
Future Vol, veh/h	184	5	57	170	4	6
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	_	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	_	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	184	5	57	170	4	6
	Major1		//ajor2		Minor2	
Conflicting Flow All	227	0	-	0	515	142
Stage 1	-	-	-	-	142	-
Stage 2	-	-	-	-	373	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1341	-	-	-	520	906
Stage 1	-	-	-	-	885	-
Stage 2	-	-	-	-	696	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1341	-	-	-	448	906
Mov Cap-2 Maneuver	_	_	-	-	448	-
Stage 1	-	_	-	-	763	_
Stage 2	_	_	_	_	696	_
01490 2					000	
Approach	EB		WB		SB	
HCM Control Delay, s	7.9		0		10.7	
HCM LOS					В	
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WRR	SBLn1
Capacity (veh/h)		1341	LD1	***	-	643
HCM Lane V/C Ratio		0.137	-	-		0.016
HCM Control Delay (s	١	8.1	0	-		10.7
HCM Lane LOS	)			-		
	,)	A	Α	-	-	В
HCM 95th %tile Q(veh	1)	0.5	-	-	-	0

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Future Vol, veh/h	0	0	0	1	0	3	0	0	0	4	0	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	100	0	67	0	0	0	50	0	0
Mvmt Flow	0	0	0	1	0	3	0	0	0	4	0	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach		EB		WB				NB		SB		
Opposing Approach		WB		EB				SB		NB		
Opposing Lanes		1		1				1		1		
Conflicting Approach Left		SB		NB				EB		WB		
Conflicting Lanes Left		1		1				1		1		
Conflicting Approach Right		NB		SB				WB		EB		
Conflicting Lanes Right		1		1				1		1		
HCM Control Delay		0		8.2				0		8		
HCM LOS		-		Α				-		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	0%	25%	100%	
Vol Thru, %	100%	100%	0%	0%	
Vol Right, %	0%	0%	75%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	0	0	4	4	
LT Vol	0	0	1	4	
Through Vol	0	0	0	0	
RT Vol	0	0	3	0	
Lane Flow Rate	0	0	4	4	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0	0	0.006	0.006	
Departure Headway (Hd)	3.911	3.911	5.208	4.958	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	0	0	691	726	
Service Time	1.917	1.917	3.212	2.962	
HCM Lane V/C Ratio	0	0	0.006	0.006	
HCM Control Delay	6.9	6.9	8.2	8	
HCM Lane LOS	N	N	Α	Α	
HCM 95th-tile Q	0	0	0	0	

-						
Intersection						
Int Delay, s/veh	6.1					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>₽</b>	000	<u> </u>	<u></u>	Y	
Traffic Vol, veh/h	21	228	35	64	256	4
Future Vol, veh/h	21	228	35	64	256	4
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	130	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	10	20	6	5	5	50
Mvmt Flow	21	228	35	64	256	4
	<b>4</b> 1	LLU	- 00	U-7	200	
	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	249	0	269	135
Stage 1	-	-	-	-	135	-
Stage 2	-	_	-	-	134	-
Critical Hdwy	_	_	4.16	-	6.45	6.7
Critical Hdwy Stg 1	_	_	-	_	5.45	-
Critical Hdwy Stg 2	_	_	_	_	5.45	_
Follow-up Hdwy	_	_	2.254	_	3.545	3.75
Pot Cap-1 Maneuver	_		1294	_	714	800
	_	_	1234	-	884	- 000
Stage 1	-	-	-			
Stage 2	-	-	-	-	885	-
Platoon blocked, %	-	-	1001	-		000
Mov Cap-1 Maneuver	-	-	1294	-	695	800
Mov Cap-2 Maneuver	-	-	-	-	695	-
Stage 1	-	-	-	-	884	-
Stage 2	-	-	-	-	861	-
A n n n a a a b	<b>ED</b>		\A/D		NID	
Approach	EB		WB		NB	
HCM Control Delay, s	0		2.8		13.2	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
	T					
Capacity (veh/h)		696	-		1294	-
HCM Lane V/C Ratio		0.374	-	-	0.027	-
HCM Control Delay (s)		13.2	-	-	7.9	-
HCM Lane LOS		В	-	-	Α	-
HCM 95th %tile Q(veh)		1.7	-	-	0.1	-

Intersection						
Int Delay, s/veh	2.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u> </u>		ሻ	<u> </u>	W	
Traffic Vol, veh/h	234	233	14	306	106	15
Future Vol, veh/h	234	233	14	306	106	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-	None	-	None
Storage Length	_	-	150	-	0	-
Veh in Median Storage	e,# 0	-	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	8	8	5	25	21
Mymt Flow	234	233	14	306	106	15
WWITETIOW	204	200	IT	000	100	10
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	467	0	685	351
Stage 1	-	-	-	-	351	-
Stage 2	-	-	-	-	334	-
Critical Hdwy	-	-	4.18	-	6.65	6.41
Critical Hdwy Stg 1	-	-	-	-	5.65	-
Critical Hdwy Stg 2	-	-	-	-	5.65	-
Follow-up Hdwy	-	-	2.272	-	3.725	3.489
Pot Cap-1 Maneuver	-	-	1064	-	381	652
Stage 1	-	-	-	-	664	-
Stage 2	-	-	-	_	677	-
Platoon blocked, %	_	_		-		
Mov Cap-1 Maneuver	-	_	1064	_	376	652
Mov Cap-2 Maneuver		_	-	_	376	-
Stage 1	_	_	_	_	664	_
Stage 2	_	_	_	_	668	_
Olago Z					000	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		18	
HCM LOS					С	
Minor Lane/Major Mvr	nt N	NBLn1	EBT	EBR	WBL	WBT
	nt I			LDK		WDI
Capacity (veh/h)		397	-	-	1064	-
HCM Cantral Dalay (	\	0.305	-	-	0.013	-
HCM Control Delay (s	)	18	-	-	8.4	-
HCM Lane LOS	.\	C	-	-	A	-
HCM 95th %tile Q(veh	1)	1.3	-	-	0	-

Intersection												
Int Delay, s/veh	7.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	ĵ.		ሻ	ĵ.			र्स	7		4	
Traffic Vol, veh/h	0	180	48	218	189	5	84	2	282	5	0	2
Future Vol, veh/h	0	180	48	218	189	5	84	2	282	5	0	2
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	75	-	-	300	-	-	-	-	140	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	2	6	16	3	0	13	0	21	0	0	0
Mvmt Flow	0	180	48	218	189	5	84	2	282	5	0	2
Major/Minor M	1ajor1			Major2		ı	Minor1			Minor2		
Conflicting Flow All	194	0	0	228	0	0	833	834	204	974	856	192
Stage 1	-	-	-	-	-	-	204	204	-	628	628	-
Stage 2	-	-	-	-	-	-	629	630	-	346	228	-
Critical Hdwy	4.1	-	-	4.26	-	-	7.23	6.5	6.41	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.23	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.344	-	-	3.617	4	3.489	3.5	4	3.3
Pot Cap-1 Maneuver	1391	-	-	1262	-	-	276	306	791	233	297	855
Stage 1	-	-	-	-	-	-	773	737	-	474	479	-
Stage 2	-	-	-	-	-	-	452	478	-	674	719	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1391	-	-	1262	-	-	239	253	791	129	246	855
Mov Cap-2 Maneuver	-	-	-	-	-	-	239	253	-	129	246	-
Stage 1	-	-	-	-	-	-	773	737	-	474	396	-
Stage 2	-	-	-	-	-	-	373	395	-	433	719	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			4.5			15.9			27.1		
HCM LOS							С			D		
Minor Lane/Major Mvmt	t <b>1</b>	NBLn11	NBLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1		
Capacity (veh/h)		239	791	1391	-	-	1262	-	-	170		
HCM Lane V/C Ratio			0.357	-	-	-	0.173	-	-	0.041		
HCM Control Delay (s)		28.3	12.1	0	-	-	8.4	-	-	27.1		
HCM Lane LOS		D	В	A	-	-	Α	-	-	D		
HCM 95th %tile Q(veh)		1.6	1.6	0	-	-	0.6	-	-	0.1		

Intersection												
Int Delay, s/veh	8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LDIT	1122	4	TTD.T.	ሻ	\$	III	<u> </u>	<u>\$</u>	ODIT
Traffic Vol, veh/h	18	21	12	44	26	226	12	88	21	127	124	28
Future Vol, veh/h	18	21	12	44	26	226	12	88	21	127	124	28
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	150	-	-	230	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	6	0	0	7	0	16	0	15	32	29	5	0
Mvmt Flow	18	21	12	44	26	226	12	88	21	127	124	28
Major/Minor N	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	641	525	138	532	529	99	152	0	0	109	0	0
Stage 1	392	392	-	123	123	-	-	-	-	-	-	-
Stage 2	249	133	-	409	406	-	-	-	-	-	-	-
Critical Hdwy	7.16	6.5	6.2	7.17	6.5	6.36	4.1	-	-	4.39	-	-
Critical Hdwy Stg 1	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.16	5.5	-	6.17	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.554	4	3.3	3.563	4	3.444	2.2	-	-	2.461	-	-
Pot Cap-1 Maneuver	382	460	916	450	458	920	1441	-	-	1329	-	-
Stage 1	625	610	-	869	798	-	-	-	-	-	-	-
Stage 2	746	790	-	610	601	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	253	413	916	393	411	920	1441	-	-	1329	-	-
Mov Cap-2 Maneuver	253	413	-	393	411	-	-	-	-	-	-	-
Stage 1	620	551	-	862	792	-	-	-	-	-	-	-
Stage 2	540	784	-	524	543	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	16			13.8			0.7			3.6		
HCM LOS	С			В								
Minor Lane/Major Mvm	ıt	NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1441	-	-		703	1329					
HCM Lane V/C Ratio		0.008	_			0.421		_	_			
HCM Control Delay (s)		7.5	-	-	16	13.8	8	-	-			
HCM Lane LOS		A	-	_	C	В	A	_	_			
HCM 95th %tile Q(veh)	)	0	_	-	0.5	2.1	0.3	-	-			

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	אטוע		אטוז	ODL	<u> </u>
Traffic Vol, veh/h	<b>T</b>	5	<b>₽</b> 280	8	3	<b>~ 터</b> 114
Future Vol, veh/h	3	5 5	280	8	3	114
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	20	19	0	0	34
Mvmt Flow	3	5	280	8	3	114
Major/Minor	Minor1	N	Acior1		Major?	
			Major1		Major2	
Conflicting Flow All	404	284	0	0	288	0
Stage 1	284	-	-	-	-	-
Stage 2	120	-	-	-	-	-
Critical Hdwy	6.4	6.4	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.48	-	-	2.2	-
Pot Cap-1 Maneuver	606	714	-	-	1286	-
Stage 1	769	-	-	-	-	-
Stage 2	910	-	-	-	_	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	605	714	-	_	1286	_
Mov Cap-2 Maneuver	605	_	_	_	_	_
Stage 1	769	_	_	_	_	_
Stage 2	908	_	_	_	_	<u>_</u>
Glage 2	300		_		_	
Approach	WB		NB		SB	
HCM Control Delay, s	10.4		0		0.2	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-		1286	-
HCM Lane V/C Ratio		-	-	0.012	0.002	-
HCM Control Delay (s)		-	-	10.4	7.8	0
HCM Lane LOS		_	-	В	Α	Α
HCM 95th %tile Q(veh)	)	-	-	0	0	-

Intersection												
Int Delay, s/veh	0											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	0	0	257	0	0	104	0
Future Vol, veh/h	1	0	0	0	0	0	0	257	0	0	104	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	100	0	0	0	0	0	0	21	0	0	33	0
Mvmt Flow	1	0	0	0	0	0	0	257	0	0	104	0
Major/Minor N	Minor2		ı	Minor1		N	Major1		N	/lajor2		
Conflicting Flow All	361	361	104	361	361	257	104	0	0	257	0	0
Stage 1	104	104	104	257	257	201	104	<u>_</u>	-	231	-	-
Stage 2	257	257	_	104	104	_	_	_	_	_	_	_
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	<u>-</u>	<u>-</u>	4.1		-
Critical Hdwy Stg 1	7.1	5.5	0.2	6.1	5.5	0.2	4.1	_	_	4.1	_	_
Critical Hdwy Stg 2	7.1	5.5	_	6.1	5.5			<u>-</u>	<u>-</u>	<u>-</u>		-
Follow-up Hdwy	4.4	3.5	3.3	3.5	4	3.3	2.2	_	_	2.2	_	_
Pot Cap-1 Maneuver	449	569	956	598	569	787	1500	-	-	1320	_	-
Stage 1	710	813	950	752	699	101	1000	_	_	1020	_	_
Stage 2	574	699	<u>-</u>	907	813	<u>-</u>	_	_	<del>-</del>	_	-	<u>-</u>
Platoon blocked, %	314	099	_	301	013	_	_	_	_	_	_	_
Mov Cap-1 Maneuver	449	569	956	598	569	787	1500	<del>-</del>	<del>-</del>	1320	-	<del>-</del>
Mov Cap-2 Maneuver	449	569	950	598	569	101	1000	_	_	1020	_	_
Stage 1	710	813	-	752	699	<u>-</u>	<u>-</u>	<u>-</u>	<u>-</u>	<u>-</u>	-	-
Stage 2	574	699	_	907	813	_	_	_	_	_		_
Olaye Z	314	099	-	301	010	<u>-</u>	_	<u>-</u>	_	<u>-</u>	_	<u>-</u>
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13			0			0			0		
HCM LOS	В			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR F	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1500		-	110	-	1320					
HCM Lane V/C Ratio		-	_		0.002	_	1020	_	<u>-</u>			
HCM Control Delay (s)		0	_	_	13	0	0	_	_			
HCM Lane LOS		A	_	_	В	A	A	<u>-</u>	<u>-</u>			
HCM 95th %tile Q(veh)	\	0	_	_	0	-	0	_	_			
TION JOHN JOHN W(VOII)		U			U		U					

Intersection												
Int Delay, s/veh	4.2											
	EDI	ГОТ	EDD	WDI	WDT	WDD	MDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	^	4	^	40	4	404	1	447	40	40	4	0
Traffic Vol, veh/h	0	4	0	13	5	124	1	147	43	42	57	2
Future Vol, veh/h	0	4	0	13	5	124	1 0	147	43	42 0	57	2
Conflicting Peds, #/hr	O Ctop	O Cton		O Cton				0			0	
Sign Control RT Channelized	Stop	Stop	Stop None	Stop -	Stop -	Stop None	Free	Free	Free None	Free -	Free	Free
Storage Length	_	-	None	_	-	None	-	-	NONE -	_	-	None
Veh in Median Storage		0	_	-	0	-		0	_		0	<u>-</u>
Grade, %	;, <del>#</del> - -	0	_	_	0	_	<u>-</u>	0	_	_	0	_
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	0	14	0	18	12	28	31	100
Mymt Flow	0	4	0	13	5	124	1	147	43	42	57	2
WWITELLOW	U	т.	U	10	J	127		177	70	72	01	
Major/Minor	Miner			Mine-1			Major4			Maisro		
	Minor2	20.4		Minor1	24.4		Major1			Major2		
Conflicting Flow All	377	334	58	315	314	169	59	0	0	190	0	0
Stage 1	142	142	_	171	171	-	-	-	-	-	-	-
Stage 2	235	192	6.0	144	143	6 24	- 11	-	-	4.20	-	-
Critical Hdwy	7.1	6.5	6.2	7.18	6.5	6.34	4.1	-	-	4.38	-	-
Critical Hdwy Stg 1	6.1 6.1	5.5 5.5	-	6.18	5.5 5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2 Follow-up Hdwy	3.5	5.5 4	2.2	6.18 3.572	5.5	3.426	2.2	-		2.452	-	-
Pot Cap-1 Maneuver	584	589	1014	626	605	845	1558	_	-	1242	-	_
Stage 1	866	783	1014	817	761	043	1000	-	-	1242	-	-
Stage 2	773	745	-	845	782	-	<u>-</u>	<del>-</del>	-	-	-	<del>-</del>
Platoon blocked, %	113	140		U <del>4</del> U	102		_	_	_	_	_	_
Mov Cap-1 Maneuver	481	568	1014	605	583	845	1558	_	-	1242	_	-
Mov Cap-2 Maneuver	481	568	- 101	605	583	-	-	_	_	1272	_	_
Stage 1	865	756	_	816	760	_	_	_	_	_	_	_
Stage 2	655	744	_	811	755	_	_	_	_	_	_	_
Jugo 2	300	, 77		311	, 00							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.4			10.4			0			3.3		
HCM LOS	11.4 B			10.4 B			U			٥.٥		
TIOWI LOO	ט			D								
Minor Long/Maior M.	.4	NDI	NDT	NDD I	TDL 41	MDL 4	CDI	CDT	CDD			
Minor Lane/Major Mvm	IL	NBL	NBT		EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		1558	-	-	568	803	1242	-	-			
HCM Cantrol Polocy (a)		0.001	-				0.034	-	-			
HCM Control Delay (s)		7.3	0	-		10.4	8	0	-			
HCM Lane LOS	\	A	Α	-	В	В	Α	Α	-			
HCM 95th %tile Q(veh)	)	0	-	-	0	0.6	0.1	-	-			

Intersection						
Int Delay, s/veh	0.7					
Movement	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	_		4	- ♣	40
Traffic Vol, veh/h	9	2	3	123	24	12
Future Vol, veh/h	9	2	3	123	24	12
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	8	14	0
Mvmt Flow	9	2	3	123	24	12
		_		120	<b>_</b>	16
Major/Minor N	/linor2	١	//ajor1	N	/lajor2	
Conflicting Flow All	159	30	36	0	-	0
Stage 1	30	-	-	-	-	-
Stage 2	129	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-		_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	-
	837	1050	1588	-	-	-
Pot Cap-1 Maneuver		1050	1000			
Stage 1	998	-	-	-	-	-
Stage 2	902	-	-	-	-	-
Platoon blocked, %		10	4	-	-	-
Mov Cap-1 Maneuver	835	1050	1588	-	-	-
Mov Cap-2 Maneuver	835		-	-	-	-
Stage 1	996	-	-	-	-	-
Stage 2	902	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	9.2		0.2		0	
HCM LOS	Α					
Minor Long/Major Mare	+	NDI	NDT	EDI 51	CDT	CDD
Minor Lane/Major Mvm		NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1588	-	•••	-	-
HCM Lane V/C Ratio		0.002		0.013	-	-
HCM Control Delay (s)		7.3	0	9.2	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-
-						

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	0	0	0	0	0	0	1	124	0	0	27	1
Future Vol, veh/h	0	0	0	0	0	0	1	124	0	0	27	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	0	0	13	0	0	8	0
Mvmt Flow	0	0	0	0	0	0	1	124	0	0	27	1
Major/Minor M	/linor2			Minor1		<u> </u>	/lajor1		<u> </u>	Major2		
Conflicting Flow All	154	154	28	154	154	124	28	0	0	124	0	0
Stage 1	28	28	-	126	126	-	-	-	-	-	-	-
Stage 2	126	126	-	28	28	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	817	741	1053	817	741	932	1599	-	-	1475	-	-
Stage 1	994	876	-	883	796	-	-	-	-	-	-	-
Stage 2	883	796	-	994	876	-	-	-		-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	816	740	1053	816	740	932	1599	-	-	1475	-	-
Mov Cap-2 Maneuver	816	740	-	816	740	-	-	-	-	-	-	-
Stage 1	993	876	-	882	795	-	-	-	-	-	-	-
Stage 2	882	795	-	994	876	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0			0.1			0		
HCM LOS	Α			Α								
Minor Lane/Major Mvmt	t	NBL	NBT	NBR I	EBLn1V	VBL <sub>n1</sub>	SBL	SBT	SBR			
Capacity (veh/h)		1599	-	-	-	-	1475	-	-			
HCM Lane V/C Ratio		0.001	-	-	-	-	-	-	-			
HCM Control Delay (s)		7.3	0	-	0	0	0	-	-			
HCM Lane LOS		Α	Α	-	Α	Α	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-	0	-	-			

Intersection						
Int Delay, s/veh	6.3					
		MDD	NDT	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		₽			र्स
Traffic Vol, veh/h	69	130	52	43	30	32
Future Vol, veh/h	69	130	52	43	30	32
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	17	0	14	23	12	26
Mvmt Flow	69	130	52	43	30	32
			<b>V</b> _	.0		<b>V</b> _
	Minor1		//ajor1		Major2	
Conflicting Flow All	166	74	0	0	95	0
Stage 1	74	-	-	-	-	-
Stage 2	92	-	-	-	-	-
Critical Hdwy	6.57	6.2	-	-	4.22	-
Critical Hdwy Stg 1	5.57	-	-	-	-	-
Critical Hdwy Stg 2	5.57	_	-	-	-	-
Follow-up Hdwy	3.653	3.3	_	_	2.308	-
Pot Cap-1 Maneuver	791	993	-	-	1438	-
Stage 1	912	-	_	_	-	_
Stage 2	895	_	_	_	_	_
Platoon blocked, %	000					_
Mov Cap-1 Maneuver	774	993	_	-	1438	-
•	774		-	-	1430	•
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	912	-	-	-	-	-
Stage 2	876	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.1		0		3.7	
HCM LOS	В		- 0		0.1	
1 IOIVI LOO	D					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	904	1438	-
HCM Lane V/C Ratio		-	-	0.22	0.021	-
HCM Control Delay (s	)	-	-		7.6	0
HCM Lane LOS		_	-	В	Α	Ā
HCM 95th %tile Q(veh	1)	-	_	0.8	0.1	-
	,			3.0	V. 1	

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	7		4			4	
Traffic Vol, veh/h	54	66	3	0	23	54	1	0	2	136	1	12
Future Vol, veh/h	54	66	3	0	23	54	1	0	2	136	1	12
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	200	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	29	14	0	0	18	46	0	0	0	21	0	75
Mvmt Flow	54	66	3	0	23	54	1	0	2	136	1	12
Major/Minor I	Major1		ľ	Major2		ľ	Minor1		1	Minor2		
Conflicting Flow All	77	0	0	69	0	0	233	253	68	200	200	23
Stage 1	-	-	-	-	-	-	176	176	-	23	23	
Stage 2	_	-	-	-	-	-	57	77	-	177	177	_
Critical Hdwy	4.39	-	-	4.1	-	-	7.1	6.5	6.2	7.31	6.5	6.95
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.31	5.5	-
Follow-up Hdwy	2.461	-	-	2.2	-	-	3.5	4	3.3	3.689	4	3.975
Pot Cap-1 Maneuver	1367	-	-	1545	-	-	726	654	1001	719	699	877
Stage 1	-	-	-	-	-	-	831	757	-	948	880	-
Stage 2	-	-	-	-	-	-	960	835	-	782	756	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1367	-	-	1545	-	-	693	627	1001	695	670	877
Mov Cap-2 Maneuver	-	-	-	-	-	-	693	627	-	695	670	-
Stage 1	-	_	-	-	-	-	797	726	-	909	880	-
Stage 2	-	-	-	-	-	-	946	835	-	748	725	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.4			0			9.1			11.4		
HCM LOS							Α			В		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		872	1367			1545			707			
HCM Lane V/C Ratio		0.003	0.04	-	_	-	_	_	0.211			
HCM Control Delay (s)		9.1	7.7	0	-	0	_	_	11.4			
HCM Lane LOS		A	A	A	_	A	_	_	В			
HCM 95th %tile Q(veh	)	0	0.1	-	-	0	-	-	0.8			
	,											

Intersection												
Int Delay, s/veh	4.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ĵ.		*	1>			4			4	
Traffic Vol, veh/h	9	288	8	29	101	104	1	5	28	129	7	7
Future Vol, veh/h	9	288	8	29	101	104	1	5	28	129	7	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-		-	-	None
Storage Length	75	-	-	75	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	_	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	22	12	0	7	41	11	0	0	0	9	14	14
Mvmt Flow	9	288	8	29	101	104	1	5	28	129	7	7
Major/Minor I	Major1			Major2		I	Minor1			Minor2		
Conflicting Flow All	205	0	0	296	0	0	528	573	292	538	525	153
Stage 1	-	-	-	-	-	-	310	310	-	211	211	-
Stage 2	_	_	_	_	_	_	218	263	_	327	314	_
Critical Hdwy	4.32	-	-	4.17	-	_	7.1	6.5	6.2	7.19	6.64	6.34
Critical Hdwy Stg 1	-	-	-	-	_	_	6.1	5.5	-	6.19	5.64	-
Critical Hdwy Stg 2	_	-	-	_	_	-	6.1	5.5	-	6.19	5.64	_
Follow-up Hdwy	2.398	-	-	2.263	_	_	3.5	4	3.3	3.581	4.126	3.426
Pot Cap-1 Maneuver	1256	-	-	1237	-	-	464	432	752	443	441	862
Stage 1	-	-	-	-	-	-	705	663	-	775	706	-
Stage 2	-	-	-	-	-	-	789	694	-	671	635	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1256	-	-	1237	-	-	444	419	752	413	428	862
Mov Cap-2 Maneuver	-	-	-	-	-	-	444	419	-	413	428	-
Stage 1	-	-	_	-	-	-	700	658	-	770	690	-
Stage 2	-	-	-	-	-	-	756	678	-	637	631	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			1			10.7			17.7		
HCM LOS							В			С		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBLn1			
Capacity (veh/h)		661	1256			1237			425			
HCM Lane V/C Ratio		0.051	0.007	_	_	0.023	_	_	0.336			
HCM Control Delay (s)		10.7	7.9	-	-	8	-	-	17.7			
HCM Lane LOS		В	Α	_	_	A	_	_	C			
HCM 95th %tile Q(veh)	)	0.2	0	_	_	0.1	_	-	1.5			

Intersection												
Int Delay, s/veh	1.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		f)			र्स						4	
Traffic Vol, veh/h	0	97	326	13	171	0	0	0	0	1	4	55
Future Vol, veh/h	0	97	326	13	171	0	0	0	0	1	4	55
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	8	19	15	24	0	2	2	2	0	25	30
Mvmt Flow	0	97	326	13	171	0	0	0	0	1	4	55
Major/Minor N	/lajor1		ı	Major2					N	/linor2		
Conflicting Flow All	- -	0	0	423	0	0			•	457	620	171
Stage 1	_	-	-	-	-	-				197	197	- 17.1
Stage 2	_	_	_	_	_	<u>-</u>				260	423	_
Critical Hdwy	_	_	_	4.25	_	_				6.4	6.75	6.5
Critical Hdwy Stg 1	_	_	_	20	_	_				5.4	5.75	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.4	5.75	-
Follow-up Hdwy	_	-	-	2.335	_	-				3.5	4.225	3.57
Pot Cap-1 Maneuver	0	-	-	1070	-	0				565	375	805
Stage 1	0	_	_	_	_	0				841	697	-
Stage 2	0	-	-	-	-	0				788	550	-
Platoon blocked, %		_	_		_						300	
Mov Cap-1 Maneuver	_	-	_	1070	_	_				558	0	805
Mov Cap-2 Maneuver	-	_	-		_	_				558	0	-
Stage 1	-	-	-	-	-	-				841	0	-
Stage 2	-	-	-	-	-	-				778	0	-
Approach	EB			WB						SB		
	0			0.6						9.9		
HCM Control Delay, s HCM LOS	U			0.0								
TION LOS										A		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-	-		-	799						
HCM Lane V/C Ratio		-	-	0.012		0.075						
HCM Control Delay (s)		-	-	8.4	0	9.9						
HCM Lane LOS		-	-	Α	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	-	0.2						

Intersection													
Int Delay, s/veh	10												
	EBL	CDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	0.4	ર્ન	۸	^	<b>€</b>	_	470	4	^	^	0	٥	
Traffic Vol, veh/h	94	4	0	0	6	5	178	10	6	0	0	0	
Future Vol, veh/h	94	4	0	0	6	5	178	10	6	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length		-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage,		0	-	-	0	-	-	0	-	-	0	-	
Grade, %	400	0	400	400	0	400	400	0	400	400	0	400	
Peak Hour Factor	100	100	100	100	100	100	100 24	100 50	100 50	100	100	100	
Heavy Vehicles, %	7 94	25 4	0	0	50 6	0 5	178	10	6	0	0	0	
Mvmt Flow	94	4	U	U	b	5	1/0	10	Ö	U	U	U	
Major/Minor N	/lajor1		<u> </u>	Major2			Minor1						
Conflicting Flow All	11	0	-	-	-	0	201	203	4				
Stage 1	-	-	-	-	-	-	192	192	-				
Stage 2	-	-	-	-	-	-	9	11	-				
Critical Hdwy	4.17	-	-	-	-	-	6.64	7	6.7				
Critical Hdwy Stg 1	-	-	-	-	-	-	5.64	6	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	5.64	6	-				
Follow-up Hdwy	2.263	-	-	-	-	-	3.716	4.45	3.75				
Pot Cap-1 Maneuver	1576	-	0	0	-	-	740	616	955				
Stage 1	-	-	0	0	-	-	791	660	-				
Stage 2	_	-	0	0	-	-	960	800	_				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver	1576	-	-	-	-	-	696	0	955				
Mov Cap-2 Maneuver	-	-	-	-	-	-	696	0	-				
Stage 1	-	-	-	-	-	-	744	0	-				
Stage 2	-	-	-	-	-	-	960	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	7.1			0			12.1						
HCM LOS							В						
110111 200													
NA:		IDL 4	ED!	EDT	MET	MES							
Minor Lane/Major Mvmt	t 1	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		702	1576	-	-	-							
HCM Lane V/C Ratio		0.276	0.06	-	-	-							
HCM Control Delay (s)		12.1	7.4	0	-	-							
HCM Lane LOS		В	A	Α	-	-							
HCM 95th %tile Q(veh)		1.1	0.2	-	-	-							

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		î,			र्स						4	
Traffic Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Future Vol, veh/h	0	2	7	7	2	0	0	0	0	6	11	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	0	14	17	0	0	2	2	2	17	73	0
Mvmt Flow	0	2	7	7	2	0	0	0	0	6	11	6
Major/Minor N	/lajor1		ı	Major2					N	/linor2		
Conflicting Flow All	-	0	0	9	0	0				22	25	2
Stage 1	-	-	-	-	-	-				16	16	-
Stage 2	-	-	-	-	-	-				6	9	-
Critical Hdwy	-	-	-	4.27	-	-				6.57	7.23	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-				5.57	6.23	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.57	6.23	-
Follow-up Hdwy	-	-	-	2.353	-	-				3.653	4.657	3.3
Pot Cap-1 Maneuver	0	-	-	1518	-	0				957	747	1088
Stage 1	0	-	-	-	-	0				969	760	-
Stage 2	0	-	-	-	-	0				979	766	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1518	-	-				952	0	1088
Mov Cap-2 Maneuver	-	-	-	-	-	-				952	0	-
Stage 1	-	-	-	-	-	-				969	0	-
Stage 2	-	-	-	-	-	-				974	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			5.7						8.6		
HCM LOS										Α		
Minor Lane/Major Mvmt	t	EBT	EBR	WBL	WBT:	SBLn1						
Capacity (veh/h)		-	-	1518	-	1015						
HCM Lane V/C Ratio		_		0.005		0.023						
HCM Control Delay (s)		-	-	7.4	0	8.6						
HCM Lane LOS		-	-	Α	Α	Α						
HCM 95th %tile Q(veh)		-	-	0	_	0.1						

Intersection													
Int Delay, s/veh	3.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स			ĵ.			4					
Traffic Vol, veh/h	5	3	0	0	8	19	1	3	9	0	0	0	
Future Vol, veh/h	5	3	0	0	8	19	1	3	9	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-	
Veh in Median Storage, #	# -	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	0	0	0	0	13	11	100	33	0	2	2	2	
Mvmt Flow	5	3	0	0	8	19	1	3	9	0	0	0	
Major/Minor Ma	ajor1		N	/lajor2		N	/linor1						
Conflicting Flow All	27	0	-	-	-	0	31	40	3				
Stage 1	-	-	-	-	-	-	13	13	-				
Stage 2	-	-	-	-	-	-	18	27	-				
Critical Hdwy	4.1	-	-	-	-	-	7.4	6.83	6.2				
Critical Hdwy Stg 1	-	-	-	-	-	-	6.4	5.83	-				
Critical Hdwy Stg 2	-	-	-	-	-	-	6.4	5.83	_				
Follow-up Hdwy	2.2	-	-	-	-	-	4.4	4.297	3.3				
Pot Cap-1 Maneuver 1	1600	-	0	0	-	-	782	795	1087				
Stage 1	-	-	0	0	-	-	806	827	-				
Stage 2	-	-	0	0	-	-	801	815	-				
Platoon blocked, %		-			-	-							
Mov Cap-1 Maneuver 1	1600	-	-	-	-	-	780	0	1087				
Mov Cap-2 Maneuver	-	-	-	-	-	-	780	0	-				
Stage 1	-	-	-	-	-	-	804	0	-				
Stage 2	-	-	-	-	-	-	801	0	-				
Approach	EB			WB			NB						
HCM Control Delay, s	4.5			0			8.5						
HCM LOS							Α						
Minor Lane/Major Mvmt	N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		1046	1600	-	-	-							
HCM Lane V/C Ratio		0.012		-	-	-							
HCM Control Delay (s)		8.5	7.3	0	_	-							
HCM Lane LOS		Α	Α	Α	-	-							

Storage Length	Intersection												
Lane Configurations	Int Delay, s/veh	3.2											
Lane Configurations	Movement	EBI	EBT	EBR	WBI	WBT	WBR	NBI	NBT	NBR	SBL	SBT	SBR
Traffic Vol, veh/h  Future Vol,					1,02		1,51	,,,,,,		, , j	UDL		UDIT
Future Vol, veh/h Conflicting Peds, #/hr Sign Control Free Free Free Free Free Free Free Free		1		4	0		0	12		0	0		0
Conflicting Peds, #/hr	The second secon	-	•	•						-		~	
Sign Control         Free RTCE         Free RTCE         Free RTCE         Free RTCE None         Free RTCE None         Free RTCE None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         Stop None         None         - None		0	0	0			0			0			
RT Channelized		Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop		Stop
Veh in Median Storage, #         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         -         -         0         0         -         -         0 <td>RT Channelized</td> <td>-</td> <td>-</td> <td></td> <td>-</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td>•</td> <td>•</td> <td></td>	RT Channelized	-	-		-	-					•	•	
Grade, %	Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Peak Hour Factor	Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, %	Grade, %	-	0	-	-	0	-	-	0	-	-		-
Mynt Flow         1         7         4         0         15         0         12         2         0         0         0         0           Major/Minor         Major1         Major2         Minor1         Minor2           Conflicting Flow All         15         0         0         11         0         0         26         26         9         27         28         15           Stage 1         -         -         -         -         -         11         11         -         15         15         -         12         13         -           Critical Hdwy Stg 2         -         -         -         -         -         6.28         5.5         -         6.1         5.5         -           Critical Hdwy Stg 2         -         -         -         -         6.28         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         6.1         5.5         -         <	Peak Hour Factor	100	100	100	100	100	100		100	100	100	100	100
Major/Minor   Major1	Heavy Vehicles, %						0						
Conflicting Flow All 15 0 0 11 0 0 26 26 9 27 28 15  Stage 1	Mvmt Flow	1	7	4	0	15	0	12	2	0	0	0	0
Conflicting Flow All 15 0 0 11 0 0 26 26 9 27 28 15  Stage 1 11 11 - 15 15 - Stage 2 15 15 15 - 12 13 - Critical Hdwy 4.1 4.1 7.28 6.5 6.2 7.1 6.5 6.2  Critical Hdwy Stg 1 6.28 5.5 - 6.1 5.5 - Critical Hdwy Stg 2 6.28 5.5 - 6.1 5.5 - Critical Hdwy Stg 2 6.28 5.5 - 6.1 5.5 - Critical Hdwy Stg 2 6.28 5.5 - 6.1 5.5 - Critical Hdwy Stg 2 2.2 - 3.662 4 3.3 3.5 4 3.3  Pot Cap-1 Maneuver 1616 1621 945 871 1079 988 869 1070  Stage 1 945 887 1079 988 869 1070  Stage 1 965 887 - 1010 887 - Stage 2 965 887 - 1014 889 - Platoon blocked, % 965 887 - 1014 889 - Platoon blocked, % 986 887 - 1014 889 - Stage 1 969 889 - 1009 887 - Stage 2 969 889 - 1009 887 - Stage 2 969 889 - 1009 887 - Stage 2 969 889 - 1009 887 - Stage 2 965 887 - 1011 888 MINOR Cap-2 Maneuver 969 889 - 1009 887 - Stage 2 965 887 - 1011 888 MINOR Cap-2 Maneuver 969 889 - 1009 887 - Stage 2 965 887 - 1011 888 MINOR Cap-2 Maneuver													
Stage 1	Major/Minor M	1ajor1		ľ	Major2			Minor1		N	Minor2		
Stage 1	Conflicting Flow All	15	0			0	0	26	26	9	27	28	15
Critical Hdwy       4.1       -       -       4.1       -       -       7.28       6.5       6.2       7.1       6.5       6.2         Critical Hdwy Stg 1       -       -       -       -       -       -       6.28       5.5       -       6.1       5.5       -         Critical Hdwy Stg 2       -       -       -       -       -       6.28       5.5       -       6.1       5.5       -         Follow-up Hdwy       2.2       -       -       2.2       -       -       3.662       4       3.3       3.5       4       3.3         Pot Cap-1 Maneuver       1616       -       -       1621       -       -       945       871       1079       988       869       1070         Stage 1       -       -       -       -       -       -       965       887       -       1014       889       -         Platoon blocked, %       -       -       -       1621       -       944       870       1079       985       868       1070         Mov Cap-2 Maneuver       -       -       -       -       -       944       870       1079	Stage 1	-	-	-	-	-	-	11	11	-			-
Critical Hdwy Stg 1       -       -       -       -       -       6.28       5.5       -       6.1       5.5       -         Critical Hdwy Stg 2       -       -       -       -       -       6.28       5.5       -       6.1       5.5       -         Follow-up Hdwy       2.2       -       -       2.2       -       -       3.662       4       3.3       3.5       4       3.3         Pot Cap-1 Maneuver       1616       -       -       1621       -       -       945       871       1079       988       869       1070         Stage 1       -       -       -       -       -       965       887       -       1014       889       -         Platoon blocked, %       -<			-	-	-	-	-						
Critical Hdwy Stg 2         -         -         -         -         6.28         5.5         -         6.1         5.5         -           Follow-up Hdwy         2.2         -         -         2.2         -         -         3.662         4         3.3         3.5         4         3.3           Pot Cap-1 Maneuver         1616         -         -         1621         -         945         871         1079         988         869         1070           Stage 1         -         -         -         -         965         887         -         1010         887         -           Stage 2         -         -         -         -         -         965         887         -         1014         889         -           Platoon blocked, %         -         -         -         -         -         965         887         -         1014         889         -           Mov Cap-1 Maneuver         1616         -         1621         -         944         870         1079         985         868         1070           Mov Cap-2 Maneuver         -         -         -         -         969         889	Critical Hdwy	4.1	-	-	4.1	-	-			6.2			6.2
Follow-up Hdwy 2.2 - 2.2 - 3.662 4 3.3 3.5 4 3.3  Pot Cap-1 Maneuver 1616 - 1621 - 945 871 1079 988 869 1070  Stage 1 970 890 - 1010 887 - 945 872 1014 889 - 945 875 1014 889 875 1014 889 875 1014 889 875 1014 889 875 1014 889 875 1014 875 875 875 875 875 875 875 875 875 875	Critical Hdwy Stg 1	-	-	-	-	-	-			-			-
Pot Cap-1 Maneuver	Critical Hdwy Stg 2		-	-		-	-						
Stage 1         -         -         -         -         970         890         -         1010         887         -           Stage 2         -         -         -         -         -         965         887         -         1014         889         -           Platoon blocked, %         -			-	-		-	-						
Stage 2         -         -         -         -         965         887         -         1014         889         -           Platoon blocked, %         -	•	1616	-	-	1621		-						
Platoon blocked, %         -         944         870         -         985         868         -         -         -         -         -         -         -         944         870         -         985         868         -         -         -         969         889         -         1009         887         -         -         -         -         -         965         887         -         1011         888         -           Approach         EB         WB         WB         WB         NB         SB           HCM Control Delay, s         0.6         0         8.9         0         0         A         A         A         A         A         A         A         A         A         A         A         A         A         A         A		-	-	-	-	-	-						-
Mov Cap-1 Maneuver         1616         -         -         1621         -         -         944         870         1079         985         868         1070           Mov Cap-2 Maneuver         -         -         -         -         -         944         870         -         985         868         -           Stage 1         -         -         -         -         -         969         889         -         1009         887         -           Stage 2         -         -         -         -         965         887         -         1011         888         -           Approach         EB         WB         NB         SB         SB         -	•	-		-	-			965	887	-	1014	889	-
Mov Cap-2 Maneuver         -         -         -         -         944         870         -         985         868         -           Stage 1         -         -         -         -         -         969         889         -         1009         887         -           Stage 2         -         -         -         -         965         887         -         1011         888         -           Approach         EB         WB         NB         NB         SB           HCM Control Delay, s         0.6         0         8.9         0           HCM Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         933         1616         -         -         1621         -         -           HCM Lane V/C Ratio         0.015         0.001         -<		1010		-	1004			044	070	4070	005	000	4070
Stage 1         -         -         -         -         969         889         -         1009         887         -           Stage 2         -         -         -         -         -         965         887         -         1011         888         -           Approach         EB         WB         NB         NB         SB           HCM Control Delay, s         0.6         0         8.9         0           HCM LOS         A         A         A         A           Minor Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         933         1616         -         -         1621         -         -           HCM Lane V/C Ratio         0.015         0.001         -	•			-	1621								
Stage 2         -         -         -         -         965         887         -         1011         888         -           Approach         EB         WB         NB         SB           HCM Control Delay, s         0.6         0         8.9         0           HCM LOS         A         A         A    Minor Lane/Major Mvmt  NBLn1  EBL  EBT  EBR  WBL  WBT  WBR SBLn1  Capacity (veh/h)  933  1616  1621   HCM Lane V/C Ratio  0.015  0.001  0  HCM Control Delay (s)  8.9  7.2  0  - 0  - 0  - 0  HCM Lane LOS  A  A  A  A  - A  - A  - A  - A  - A			-	-	-					<del>-</del>			
Approach         EB         WB         NB         SB           HCM Control Delay, s         0.6         0         8.9         0           HCM LOS         A         A         A           Minor Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         933         1616         -         -         1621         -         -           HCM Lane V/C Ratio         0.015         0.001         -         -         -         -         -           HCM Control Delay (s)         8.9         7.2         0         -         0         -         0           HCM Lane LOS         A         A         A         A         -         A         -         A		-	-	-	-	-	-			-			
HCM Control Delay, s   0.6   0   8.9   0	Staye 2	_	-	_	-	-	-	900	007	_	1011	000	-
HCM Control Delay, s   0.6   0   8.9   0													
Minor Lane/Major Mvmt         NBLn1         EBL         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         933         1616         -         -         1621         -         -           HCM Lane V/C Ratio         0.015         0.001         -         -         -         -         -           HCM Control Delay (s)         8.9         7.2         0         -         0         -         0           HCM Lane LOS         A         A         A         -         A         -         A													
Minor Lane/Major Mvmt         NBLn1         EBL         EBT         EBR         WBL         WBT         WBR SBLn1           Capacity (veh/h)         933         1616         -         -         1621         -         -           HCM Lane V/C Ratio         0.015         0.001         -         -         -         -         -           HCM Control Delay (s)         8.9         7.2         0         -         0         -         -         0           HCM Lane LOS         A         A         A         -         A         -         A	<b>3</b> ·	0.6			0								
Capacity (veh/h) 933 1616 1621 HCM Lane V/C Ratio 0.015 0.001 HCM Control Delay (s) 8.9 7.2 0 - 0 0 HCM Lane LOS A A A - A - A - A	HCM LOS							Α			Α		
Capacity (veh/h) 933 1616 1621 HCM Lane V/C Ratio 0.015 0.001 HCM Control Delay (s) 8.9 7.2 0 - 0 0 HCM Lane LOS A A A - A - A - A													
HCM Lane V/C Ratio 0.015 0.001 HCM Control Delay (s) 8.9 7.2 0 - 0 0 HCM Lane LOS A A A - A - A - A	Minor Lane/Major Mvmt	<u> </u>	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
HCM Control Delay (s) 8.9 7.2 0 - 0 - 0 HCM Lane LOS A A A - A - A - A	Capacity (veh/h)		933	1616	-	-	1621	-	-	-			
HCM Lane LOS A A A - A A	HCM Lane V/C Ratio			0.001	-	-	-	-	-	-			
	HCM Control Delay (s)					-		-	-				
HCM 95th %tile Q(veh) 0 0 0					Α	-		-	-	Α			
	HCM 95th %tile Q(veh)		0	0	-	-	0	-	-	-			

Intersection												
Int Delay, s/veh	4.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ß			- 4						4	
Traffic Vol, veh/h	0	162	0	33	7	0	0	0	0	109	1	7
Future Vol, veh/h	0	162	0	33	7	0	0	0	0	109	1	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	_	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	19	0	13	0	0	2	2	2	3	100	20
Mvmt Flow	0	162	0	33	7	0	0	0	0	109	1	7
Major/Minor N	//ajor1		N	Major2					<u> </u>	Minor2		
Conflicting Flow All	-	0	0	162	0	0				235	235	7
Stage 1	-	-	-	-	-	-				73	73	-
Stage 2	-	-	-	-	-	-				162	162	-
Critical Hdwy		-	-	4.23	-	-				6.43	7.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-				5.43	6.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-				5.43	6.5	-
Follow-up Hdwy	-	-	-	2.317	-	-				3.527	4.9	3.48
Pot Cap-1 Maneuver	0	-	-	1353	-	0				751	526	1025
Stage 1	0	-	-	-	-	0				947	676	-
Stage 2	0	-	-	-	-	0				865	611	-
Platoon blocked, %		-	-		-							
Mov Cap-1 Maneuver	-	-	-	1353	-	-				733	0	1025
Mov Cap-2 Maneuver	-	-	-	-	-	-				733	0	-
Stage 1	-	-	-	-	-	-				947	0	-
Stage 2	-	-	-	-	-	-				844	0	-
Approach	EB			WB						SB		
HCM Control Delay, s	0			6.4						10.7		
HCM LOS										В		
Minor Lane/Major Mvm	t	EBT	EBR	WBL	WBT	SBLn1						
Capacity (veh/h)		-		1353		746						
HCM Lane V/C Ratio		-		0.024	_	0.157						
HCM Control Delay (s)		-	-		0	10.7						
HCM Lane LOS		-	-	Α	A	В						
HCM 95th %tile Q(veh)		-	-	0.1	-	0.6						

Intersection													
Int Delay, s/veh	4.6												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		<u></u>			<b>1</b>			4					
Traffic Vol, veh/h	123	148	0	0	31	78	9	4	123	0	0	0	
Future Vol, veh/h	123	148	0	0	31	78	9	4	123	0	0	0	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	-	_	_	-	-	-	-	-	-	-	
Veh in Median Storage	,# -	0	_	_	0	_	_	0	-	-	0	-	
Grade, %	, -	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100	
Heavy Vehicles, %	17	7	2	0	12	4	0	25	10	2	2	2	
Mvmt Flow	123	148	0	0	31	78	9	4	123	0	0	0	
Major/Minor N	Major1		1	Major2		N	Minor1						
Conflicting Flow All	109	0	_	-	_	0	464	503	148				
Stage 1	-	-	-	-	-	-	394	394	-				
Stage 2	_	_	_	_	_	_	70	109	_				
Critical Hdwy	4.27	-	_	_	-	-	6.4	6.75	6.3				
Critical Hdwy Stg 1	-	_	_	_	_	_	5.4	5.75	-				
Critical Hdwy Stg 2	-	-	-	_	_	_	5.4	5.75	-				
Follow-up Hdwy	2.353	_	-	-	-	-	3.5	4.225	3.39				
Pot Cap-1 Maneuver	1393	-	0	0	-	-	560	439	878				
Stage 1	_	_	0	0	-	-	686	567	-				
Stage 2	-	-	0	0	-	-	958	763	-				
Platoon blocked, %		_			-	-							
Mov Cap-1 Maneuver	1393	-	-	-	-	-	506	0	878				
Mov Cap-2 Maneuver	-	-	-	-	-	-	506	0	-				
Stage 1	-	-	-	-	-	-	620	0	-				
Stage 2	-	-	-	-	-	-	958	0	-				
S Comments													
Approach	EB			WB			NB						
HCM Control Delay, s	3.6			0			10.1						
HCM LOS							В						
Minor Lane/Major Mvm	it N	NBLn1	EBL	EBT	WBT	WBR							
Capacity (veh/h)		836	1393	-									
HCM Lane V/C Ratio		0.163		_	_	<u>-</u>							
HCM Control Delay (s)		10.1	7.8	0	_	_							
HCM Lane LOS		В	Α	A	_	_							
HCM 95th %tile Q(veh)		0.6	0.3	-	-	-							
		5.5	J.5										

Intersection												
Int Delay, s/veh	0.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	268	0	0	102	3	1	1	4	1	0	6
Future Vol, veh/h	3	268	0	0	102	3	1	1	4	1	0	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	0	7	0	0	4	67	0	0	0	0	0	17
Mvmt Flow	3	268	0	0	102	3	1	1	4	1	0	6
Major/Minor N	/lajor1		ľ	Major2		1	Minor1		N	Minor2		
Conflicting Flow All	105	0	0	268	0	0	381	379	268	381	378	104
Stage 1	-	-	-	-	-	-	274	274	-	104	104	-
Stage 2	-	-	-	-	-	-	107	105	-	277	274	_
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.37
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.453
Pot Cap-1 Maneuver	1499	-	-	1307	-	-	581	556	776	581	557	911
Stage 1	-	-	-	-	-	-	736	687	-	907	813	-
Stage 2	-	-	-	-	-	-	903	812	-	734	687	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1499	-	-	1307	-	-	576	555	776	576	556	911
Mov Cap-2 Maneuver	-	-	-	-	-	-	576	555	-	576	556	-
Stage 1	-	-	-	-	-	-	735	686	-	905	813	-
Stage 2	_	-	-	-	-	-	897	812	-	728	686	-
-												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			0			10.3			9.3		
HCM LOS							В			Α		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		690	1499	-	_	1307	_	_	841			
HCM Lane V/C Ratio		0.009		_	_	_	_		0.008			
HCM Control Delay (s)		10.3	7.4	0	_	0	_	-	9.3			
HCM Lane LOS		В	A	A	_	A	-	-	A			
HCM 95th %tile Q(veh)		0	0	-	_	0	_	_	0			
( · · · · · · · · · · · · · · · · · · ·						_			_			

Intersection							
Int Delay, s/veh	5.6						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	T T	<u></u>	₩ <b>(</b>	וטייי	JDL Š	JUIN 7	
Traffic Vol, veh/h	182	61	58	29	25	69	
Future Vol, veh/h	182	61	58	29	25	69	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-		-	None		None	
Storage Length	180	-	-	-	100	0	
Veh in Median Storage		0	0	-	0	-	
Grade, %	-,	0	0	-	0	-	
Peak Hour Factor	100	100	100	100	100	100	
Heavy Vehicles, %	5	9	11	7	0	6	
Mvmt Flow	182	61	58	29	25	69	
Major/Minor I	Major1	N	//ajor2	N	Minor2		
Conflicting Flow All	87	0	-	0	498	73	
Stage 1	-	-	-	-	73	-	
Stage 2	-	_	_	-	425	-	
Critical Hdwy	4.15	-	_	-	6.4	6.26	
Critical Hdwy Stg 1	-	-	-	-	5.4	-	
Critical Hdwy Stg 2	-	-	-	-	5.4	-	
Follow-up Hdwy	2.245	-	-	-		3.354	
Pot Cap-1 Maneuver	1490	-	-	-	535	978	
Stage 1	-	-	-	-	955	-	
Stage 2	-	-	-	-	664	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver	1490	-	-	-	470	978	
Mov Cap-2 Maneuver	-	-	-	-	470	-	
Stage 1	-	-	-	-	838	-	
Stage 2	-	-	-	-	664	-	
Approach	EB		WB		SB		
HCM Control Delay, s	5.8		0		10.1		
HCM LOS			•		В		
Minor Lane/Major Mvm	n#	EBL	EBT	WBT	WDD (	SBLn1 S	PDI n2
	IL		LDI	VVDI			
Capacity (veh/h) HCM Lane V/C Ratio		1490	-	-	-	470 0.053	978
HCM Control Delay (s)		0.122 7.8	-	-	-	13.1	0.071
HCM Lane LOS		7.0 A		_	_	13.1 B	A
HCM 95th %tile Q(veh	)	0.4		_	_	0.2	0.2
HOW JOHN JOHNE WIVELL	1	U. <del>1</del>		_		0.2	U.Z

Intersection												
	4.4											
Int Delay, s/veh	4.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	- ኝ	ĵ.		7	f)			4			4	
Traffic Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	22
Future Vol, veh/h	47	37	4	1	46	4	8	16	7	0	2	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	Stop	-	-	None
Storage Length	100	-	-	150	-	-	-	-	-	-	-	-
Veh in Median Storage	e, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	28	50	0	11	25	0	6	14	0	0	10
Mvmt Flow	47	37	4	1	46	4	8	16	7	0	2	22
Major/Minor	Major1			Major2			Minor1		N	/linor2		
Conflicting Flow All	50	0	0	41	0	0	195	185	39	191	185	48
Stage 1	-	-	-	-	-	-	133	133	-	50	50	-
Stage 2	_	_	_	_	_	_	62	52	_	141	135	_
Critical Hdwy	4.21		-	4.1	_	_	7.1	6.56	6.34	7.1	6.5	6.3
Critical Hdwy Stg 1	4.21	_	_	4.1	_	_	6.1	5.56	0.54	6.1	5.5	0.5
Critical Hdwy Stg 2	-		-	_	_	_	6.1	5.56	-	6.1	5.5	_
Follow-up Hdwy	2.299	_	_	2.2	_	_	3.5	4.054		3.5	4	3.39
Pot Cap-1 Maneuver	1501		-	1581	_	_	769	702	999	773	713	999
Stage 1	1301	_	_	-	_		875	779	-	968	857	-
Stage 2	-		-	_	_	_	954	844	_	867	789	_
Platoon blocked, %		_			_	_	JJ4	U <del>11</del>		001	100	
Mov Cap-1 Maneuver	1501	_	_	1581	_	_	732	680	999	735	690	999
Mov Cap-2 Maneuver	-	_	_	-	_	_	732	680	-	735	690	-
Stage 1	_		_		_	_	848	755	_	938	856	_
Stage 2	_	_	_	_	_	_	930	843	_	816	765	_
Olugo Z							500	U-10		010	, 00	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	4			0.1			9.1			8.8		
HCM LOS							Α			Α		
Minor Lane/Major Mvm	nt I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)		900	1501	-		1581			963			
HCM Lane V/C Ratio		0.034		_	_	0.001	-	_	0.025			
HCM Control Delay (s)		9.1	7.5	-	_	7.3		_	8.8			
HCM Lane LOS		Α	7.5 A	_	_	7.5 A	-	_	Α			
I TOWN LUNG LOO												

0.1

0.1

0.1

HCM 95th %tile Q(veh)

Intersection						
Int Delay, s/veh	2.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
		EDK				INDIX
Lane Configurations	<b>}</b>	1	ዃ	<b>†</b>	<b>\</b>	1.1
Traffic Vol, veh/h	44 44	1	5	42	10	14 14
Future Vol, veh/h		1	5	42	10	
Conflicting Peds, #/hr	0	0	0	0	0	0
<u> </u>	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	450	None	-	None
Storage Length	_	-	150	-	0	
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	26	0	0	15	0	7
Mvmt Flow	44	1	5	42	10	14
Major/Minor M	ajor1	N	Major2	N	/linor1	
Conflicting Flow All	0	0	45	0	97	45
Stage 1	-	-	-	-	45	-
Stage 2	_	_	_	_	52	_
Critical Hdwy			4.1		6.4	6.27
		_		-	5.4	0.27
Critical Hdwy Stg 1	-	_	-		5.4	
Critical Hdwy Stg 2	-	-	2.2	-		3.363
Follow-up Hdwy	-	-		-		
Pot Cap-1 Maneuver	-	-	1576	-	907	1011
Stage 1	-	-	-	-	983	-
Stage 2	-	-	-	-	976	-
Platoon blocked, %	-	-	4	-	224	1011
Mov Cap-1 Maneuver	-	-	1576	-	904	1011
Mov Cap-2 Maneuver	-	-	-	-	904	-
Stage 1	-	-	-	-	983	-
Stage 2	-	-	-	-	973	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		8.8	
HCM LOS	U		0.0		A	
TIOM EOO						
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		963	-	-	1576	-
HCM Lane V/C Ratio		0.025	-	-	0.003	-
HCM Control Delay (s)		8.8	-	-	7.3	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-
, ,						

Intersection						
Int Delay, s/veh	0.7					
		WED	NET	NDD	ODI	ODT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	**	^	<b>^}</b>	^		र्चु
Traffic Vol, veh/h	2	0	16	0	0	6
Future Vol, veh/h	2	0	16	0	0	6
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	0	0	17
Mvmt Flow	2	0	16	0	0	6
Major/Minor M	/linor1	N	/lajor1	N	Major2	
Conflicting Flow All	22	16	0	0	16	0
Stage 1	16	-	_	-	_	-
Stage 2	6	_	_	_	_	_
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_		_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	1000	1069	_	_	1615	_
Stage 1	1012	-	_	_	-	_
Stage 2	1022	_	_	_	_	_
Platoon blocked, %	1022		_	_		_
Mov Cap-1 Maneuver	1000	1069		_	1615	_
Mov Cap-1 Maneuver	1000	-		_	-	_
Stage 1	1012	-	-	_	_	_
_	1012	-	_	-	_	-
Stage 2	1022	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
	1	NBT	NDDV	VBLn1	SBL	SBT
Minor Lane/Major Mymt				VDLIII	ODL	301
Minor Lane/Major Mvmt				1000	1615	
Capacity (veh/h)		-	-	1000	1615	-
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.002	-	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		- - -	- - -	0.002 8.6	0	-
Capacity (veh/h) HCM Lane V/C Ratio		-	-	0.002	-	-

Intersection						
Int Delay, s/veh	0					
		ED.5	NE	NET	057	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	₽	
Traffic Vol, veh/h	0	0	0	22	5	0
Future Vol, veh/h	0	0	0	22	5	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	0	0	0	5	20	0
Mvmt Flow	0	0	0	22	5	0
	Minor2		//ajor1		/lajor2	
Conflicting Flow All	27	5	5	0	-	0
Stage 1	5	-	-	-	-	-
Stage 2	22	-	-	_	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	993	1084	1630	-	_	_
Stage 1	1023	-	-	_	_	_
Stage 2	1006	_	_	_	_	_
Platoon blocked, %	1000			_	_	_
Mov Cap-1 Maneuver	993	1084	1630			
	993		1030	•	_	•
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	1023	-	-	-	-	-
Stage 2	1006	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A		U		U	
TIOIVI LOO	^					
Minor Lane/Major Mvm	ıt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1630	_	-	-	-
HCM Lane V/C Ratio		-	_	-	_	-
HCM Control Delay (s)		0	_	0	-	_
HCM Lane LOS		A	_	A	_	_
HCM 95th %tile Q(veh)		0	_	-	_	_
Holvi Jour Joure Q(Veri)		U				

Intersection						
Int Delay, s/veh	5					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	7	<b></b>
Traffic Vol, veh/h	4	6	16	11	3	1
Future Vol, veh/h	4	6	16	11	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- Otop	None	-	None	-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	<u>-</u>
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	25	33	0	0	0	33
Mymt Flow	4	6	16	11	3	1
IVIVIIIL FIOW	4	O	10	11	J	I
Major/Minor N	Minor2	N	Major1	N	//ajor2	
Conflicting Flow All	47	4	4	0	-	0
Stage 1	4	-	-	-	-	-
Stage 2	43	-	-	-	-	-
Critical Hdwy	6.65	6.53	4.1	-	-	_
Critical Hdwy Stg 1	5.65	-	-	_	_	_
Critical Hdwy Stg 2	5.65	_	_	_	_	_
Follow-up Hdwy	3.725		2.2	_	_	_
Pot Cap-1 Maneuver	908	996	1631	_	_	_
Stage 1	962	-	1001	_	_	_
Stage 2	924	_	_	_	_	_
Platoon blocked, %	324	-	-	_		-
	900	996	1631	<u>-</u>	-	-
Mov Cap-1 Maneuver	899		1031	-	-	-
Mov Cap-2 Maneuver	899	-	-	-	-	-
Stage 1	952	-	-	-	-	-
Stage 2	924	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.8		4.3		0	
HCM LOS	A		1.0		V	
TIOW EGG	71					
Minor Lane/Major Mvm	<u>it</u>	NBL	NBTI	EBLn1	SBT	SBR
Capacity (veh/h)		1631	-	955	-	-
HCM Lane V/C Ratio		0.01	-	0.01	-	-
HCM Control Delay (s)		7.2	0	8.8	-	-
HCM Lane LOS		Α	Α	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

Intersection												
Int Delay, s/veh	3.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	103	137	1	1	10	4	1	1	0	4	2	13
Future Vol, veh/h	103	137	1	1	10	4	1	1	0	4	2	13
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	100	100	100	100	100	100	100	100	100	100	100	100
Heavy Vehicles, %	11	18	100	0	0	0	0	0	0	25	0	0
Mvmt Flow	103	137	1	1	10	4	1	1	0	4	2	13
Major/Minor N	Major1			Major2		1	Minor1		ı	Minor2		
Conflicting Flow All	14	0	0	138	0	0	366	360	138	358	358	12
Stage 1	-	-	-	-	-	-	344	344	-	14	14	-
Stage 2	-	-	-	-	-	-	22	16	-	344	344	-
Critical Hdwy	4.21	-	-	4.1	-	-	7.1	6.5	6.2	7.35	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.35	5.5	-
Follow-up Hdwy	2.299	-	-	2.2	-	-	3.5	4	3.3	3.725	4	3.3
Pot Cap-1 Maneuver	1547	-	-	1458	-	-	594	570	916	557	572	1074
Stage 1	-	-	-	-	-	-	676	640	-	950	888	-
Stage 2	-	-	-	-	-	-	1002	886	-	626	640	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1547	-	-	1458	-	-	552	528	916	525	530	1074
Mov Cap-2 Maneuver	-	-	-	-	-	-	552	528	-	525	530	-
Stage 1	-	-	-	-	-	-	627	594	-	882	887	-
Stage 2	-	-	-	-	-	-	987	885	-	580	594	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	3.2			0.5			11.7			9.6		
HCM LOS							В			Α		
Minor Lane/Major Mvm	nt N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		540				1458	-	-				
HCM Lane V/C Ratio			0.067	_		0.001	_		0.023			
HCM Control Delay (s)		11.7	7.5	0	_	7.5	0		9.6			
HCM Lane LOS		В	Α.	A	-	Α	A	_	Α.			
HCM 95th %tile Q(veh)	)	0	0.2	-	_	0	-	_	0.1			
			V						<b>V</b> . 1			

Interception						
Intersection Int Delay, s/veh	3.2					
iiii Delay, 5/VeII						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		ĵ»			4
Traffic Vol, veh/h	5	48	134	8	55	38
Future Vol, veh/h	5	48	134	8	55	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	20	0	10	0	2	6
Mvmt Flow	5	48	134	8	55	38
Majar/Miner	N Alima = =4		1-1-1		Mais	
	Minor1		Major1		Major2	
Conflicting Flow All	286	138	0	0	142	0
Stage 1	138	-	-	-	-	-
Stage 2	148	-	-	-	-	-
Critical Hdwy	6.6	6.2	-	-	4.12	-
Critical Hdwy Stg 1	5.6	-	-	-	-	-
Critical Hdwy Stg 2	5.6	-	-	-	-	-
Follow-up Hdwy	3.68	3.3	-	-	2.218	-
Pot Cap-1 Maneuver	668	916	-	-	1441	-
Stage 1	846	-	-	-	-	-
Stage 2	837	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	642	916	-	-	1441	
Mov Cap-2 Maneuver	642	-	-	-	-	-
Stage 1	846	-	-	-	-	-
Stage 2	804	_	_	-	_	_
Δ	14/5		ND		0.5	
Approach	WB		NB		SB	
HCM Control Delay, s	9.3		0		4.5	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		וטוו	-	881	1441	051
HCM Lane V/C Ratio			_		0.038	_
HCM Control Delay (s)	\	-	-	9.3	7.6	0
HCM Lane LOS		-	-			
	\	-	-	A	Α	Α
HCM 95th %tile Q(veh	)	-	-	0.2	0.1	-

Intersection						
Int Delay, s/veh	0					
		EDD	\\/DI	WDT	NIDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	Þ	^	0	<u>र्</u> च	<b>Y</b>	0
Traffic Vol, veh/h	4	0	0	3	0	0
Future Vol, veh/h	4	0	0	3	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	400	400	0	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	0	0	3	0	0
Major/Minor Ma	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	4	0	7	4
Stage 1	-	-	_	-	4	-
Stage 2	_	_	_	_	3	_
Critical Hdwy	_	_	4.12	_	6.42	6.22
Critical Hdwy Stg 1	_	_	- 14	_	5.42	0.22
Critical Hdwy Stg 2	_		_	_	5.42	_
Follow-up Hdwy	_		2.218	_	3.518	
Pot Cap-1 Maneuver	_		1618	_	1014	1080
Stage 1	_	_	1010	_	1014	-
Stage 2	_		_	_	1019	_
Platoon blocked, %	-	-	-	-	1020	-
		-	1618	-	1014	1080
Mov Cap-1 Maneuver	-	-	1010	-		
Mov Cap-2 Maneuver	-	<del>-</del>	-	-	1014	-
Stage 1	-	-	-	-	1019	-
Stage 2	-	-	-	-	1020	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		0	
HCM LOS	U		- 0		A	
TOW LOO					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		-	-	-	1618	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	-	0	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		-	-	-	0	-

Intersection						
Int Delay, s/veh	8.7					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EDL			WDK		SDK
Lane Configurations	•	4	ĵ,		<b>Y</b>	101
Traffic Vol, veh/h	6	67	15	4	170	184
Future Vol, veh/h	6	67	15	4	170	184
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storag	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	100	100	100	100	100	100
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	67	15	4	170	184
WWW.CT IOW	J	O1	10	•	170	101
Major/Minor	Major1	N	Major2	1	Minor2	
Conflicting Flow All	19	0	-	0	96	17
Stage 1	-	-	-	-	17	-
Stage 2	-	-	-	-	79	-
Critical Hdwy	4.12	-	_	-	6.42	6.22
Critical Hdwy Stg 1	_	_	_	_	5.42	_
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	2.218	_	_			3.318
Pot Cap-1 Maneuver	1597	_	_	_	903	1062
Stage 1	1551	_	_	_	1006	-
Stage 2		-	-	_	944	_
	_	_	-		944	-
Platoon blocked, %	4507	-	-	-	000	4000
Mov Cap-1 Maneuver		-	-	-	899	1062
Mov Cap-2 Maneuver		-	-	-	899	-
Stage 1	-	-	-	-	1002	-
Stage 2	-	-	-	-	944	-
Approach	EB		WB		SB	
HCM Control Delay, s	0.6		0		10.8	
HCM LOS					В	
Minor Lane/Major Mvi	mt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1597		,,,,,	-	
		0.004	-	-		0.362
			0	-		
HCM Control Dolay (s	•1			_	-	10.8
HCM Control Delay (s	s)	7.3				
	•	7.3 A	A	-	-	B 1.7

Intersection			
Intersection Delay, s/veh	7		
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Future Vol, veh/h	1	0	0	0	0	3	0	1	1	3	2	0
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Heavy Vehicles, %	0	0	0	0	0	33	0	0	0	33	0	0
Mvmt Flow	1	0	0	0	0	3	0	1	1	3	2	0
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB				WB			NB		SB		
Opposing Approach	WB				EB			SB		NB		
Opposing Lanes	1				1			1		1		
Conflicting Approach Left	SB				NB			EB		WB		
Conflicting Lanes Left	1				1			1		1		
Conflicting Approach Right	NB				SB			WB		EB		
Conflicting Lanes Right	1				1			1		1		
HCM Control Delay	7.1				6.3			6.6		7.6		
HCM LOS	Α				Α			Α		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	100%	0%	60%	
Vol Thru, %	50%	0%	0%	40%	
Vol Right, %	50%	0%	100%	0%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	2	1	3	5	
LT Vol	0	1	0	3	
Through Vol	1	0	0	2	
RT Vol	1	0	3	0	
Lane Flow Rate	2	1	3	5	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.002	0.001	0.003	0.006	
Departure Headway (Hd)	3.611	4.114	3.313	4.591	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	996	874	1086	784	
Service Time	1.613	2.118	1.316	2.591	
HCM Lane V/C Ratio	0.002	0.001	0.003	0.006	
HCM Control Delay	6.6	7.1	6.3	7.6	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0	0	0	0	

## **APPENDIX J: DETAILED DRAFT SAFETY PLAN**

# Draft Traffic Safety Plan

Horse Heaven Wind & Solar September 2023

### Safety Plan

The draft Traffic Safety Plan (TSP) will assist in developing the Construction Transportation Management Plan (CTMP). The TSP will supply information about the future impacts to the transportation network due to the peak construction of the Horse Heaven Wind Farm Project (the Project). These impacts are expected to occur within and around the project boundary during the construction phases of the Project. This document will provide recommendations to enhance traffic safety during the construction of the Project.

#### Objectives and Strategies

The objective of traffic safety management plan is:

- To avoid interrupting normal traffic flow. This flow varies depending on the time (rush hours, shift change) and location of the project entrances (cars, trucks, deliveries).
- For the management of traffic flow allowing project daily activities to be conducted in a safe manner.
- To protect all road users by defining the type of the required traffic safety equipment. The equipment to be
  used will be, but not limited to, truck route signage, detour route signage, barricades, traffic lights, safety
  cones, fences, closed road signs.
- To enclose and separate the walking/driving paths on site while construction work is being performed.

The Project has multiples strategy to ensure a smooth traffic flow in and out the project construction zone. These plans will be as follows:

- Ensuring construction tasks will be conducted sequentially to lessen any impacts as far as schedule or cost.
- Detailed safety project procedure will be issued with the involvement and approval of both project and safety management, taking into consideration the entrance to the project by both equipment and personnel.
- All entry and exit movements to and from traffic streams shall be in accordance with the projects core process and Environmental Safety and Health Plan (ES&H).
- Design the Project Driveways to include the proper number of traffic lanes and traffic control infrastructure to accommodate the traffic flow and provide safe exit onto the local roadway system.
- Maximize the deliveries to the site to be occur during off-peak construction worker commuting times

All traffic management works, and control devices shall be in accordance with Washington State and Federal Law for construction and improvement of roads. The safety manager and supervisors will work to implement the safety rules on site by keeping a close supervision on all personnel. They will take all reasonable measures to prevent accident or injury during the construction of the plant. All approved procedures and managements practices should be applied and closely implemented.

#### Construction Traffic Management Plan

A Construction Traffic Management Plan (CTMP) will be developed in consultation with the Washington State Department of Transportation (WSDOT) and Benton County public works staff, as appropriate, prior to construction. The CTMP will follow WSDOT Design Manual 22-01.21 Chapter 1010. A CTMP is a key element in addressing known work zone safety and mobility impacts.

The TMP may include Temporary Traffic Control (TTC) Components such as:

- · Lane closures or lane shifts
- Traffic Control devices

- Pavement markings
- · Changeable message signs
- · Temporary signals

Transportation Systems Management and Operations (TSMO) is the second component to a TMS. Key TSMO components may include Work Zone Safety Management strategies such as:

- Positive protective devices
- Speed limit reductions
- Automated flagger assistance devices
- Radar speed display signs

The final element of a TMP to be considered are Public Information and Awareness Strategies, which may include, but are not limited to, the following strategies:

- Public Awareness Strategies such as Brochures or mailers, press releases, paid advertisements, and project website (consider providing information in other languages if appropriate).
- Motorist Information Strategies such as Highway advisory radio (HAR), changeable message signs, and transportation management center (TMC).

### Impacts during Construction

During peak construction, a typical day would include the transportation of workers, transportation of materials, and movement of heavy equipment.

On-site workers would include technicians, laborers, foremen, equipment operators, and construction managers, with the majority of these positions expected to be filled by workers normally residing in Benton and Franklin Counties (Horse Heaven Wind Farm, LLC 2021).-Most of the construction worker traffic would originate from the Tri-Cities of Kennewick, Pasco, and Richland, as well as nearby communities. The workforce would use the same roads to access the Project as the equipment transporters. To be conservative with analysis, it is assumed that workers would drive alone and that the average vehicle would only have 1.25 occupants (Horse Heaven Wind Farm, LLC 2021). Private vehicles would primarily travel mornings and evenings, corresponding to the workday, and the construction truck traffic would be more uniformly distributed throughout the workday. For the LOS analysis, the more conservative 374 worker trips for the construction of the first half of the Project and 344 worker trips for the construction of the second half of the Project were used. Three Project laydown yard locations have been preliminarily identified to facilitate the delivery and assembly of materials and equipment:

- One adjacent to the eastern substation location on Beck Road
- One along Locust Grove Road, East of S. Plymouth Road
- One along Locust Grove Road, between Nicoson Road and S. Clodfelter Road

During construction, trucks are anticipated to use I-82, State Route 397, State Route 221, and local Benton County roads to bring construction equipment, turbine components, solar components, substation equipment, and transmission line equipment to the various Project construction sites.

Trucks would also be used to bring road base aggregate to improve existing roads and construct new access roads; concrete for the turbine, substation, BESS, and O&M facility foundations; and water for dust control. Some large Project components such as turbine blades, tower components, and nacelles may be delivered to remote ports, such as the Port of Vancouver or Port of Longview, and transported overland via I-84 to I-82. Other components may originate within the continental United States and be transported overland from other locations to I-84 and on to I-82 (Horse Heaven Wind Farm, LLC 2021).

Equipment such as excavators, trenching equipment, backhoe loaders, cranes, forklifts, and other material handling equipment would be brought on site by a flatbed semi-tractor trailer and would remain on site throughout construction. Equipment such as water trucks, fuel trucks, service trucks, and trucks delivering components would make frequent trips to deliver supplies. Some trucks would be required to obtain oversize/overweight permits, which allow travel on all unrestricted roads.

### Applicant Commitments and Identified Mitigation (from DEIS 4.14.2.4)

This section describes the measures that would reduce or compensate for impacts related to traffic from construction, operation, and decommissioning of the Project. These measures would be implemented in addition to compliance with the environmental permits, plans, and authorizations required for the Proposed Action.

#### **Applicant Commitments**

The Applicant has identified measures and/or best practices that are designed to prevent or minimize potential impacts on the affected environment for the Project. Measures presented by the Applicant in the ASC (Horse Heaven Wind Farm, LLC 2021), and taken into consideration in the characterization of potential impacts on traffic are discussed in Section 2.3 and summarized below.

- All road improvement and construction would be performed in conjunction with Benton County Public Works and WSDOT requirements. The Applicant would maintain new access roads to access the turbine structures during operations.
- Prior to commencement of construction, the Applicant would consult with WSDOT and Benton County on the development of a Construction Traffic Management Plan.
- The Applicant would obtain all necessary WSDOT permits to access, modify ingress and egress for, or transport regulated loads on state-managed roadways.
- The Applicant would obtain WSDOT trip permits for oversized and overweight loads.
- When slow or oversized wide loads are being hauled, appropriate vehicle and roadside signing and warning
  devices would be deployed. Pilot cars would be used as WSDOT dictates, depending on load size and weight.
- A detailed haul plan for the oversized wind material deliveries would be developed once turbines have been selected and the construction schedule developed. This haul plan would confirm source locations and routes to be used during Project construction, as well as anticipated loads and haul schedule.
- Ingress and egress points would be located and improved (if needed) to ensure adequate capacity for existing and projected traffic volumes and to provide efficient movement of traffic, including existing and anticipated agricultural traffic.
- The Applicant would coordinate with EFSEC and Benton County, to identify a qualified third-party engineer who would document road conditions prior to construction and again within 30 days after construction is complete or as weather permits.
- A service agreement between the Applicant and Benton County would ensure post-construction road restoration to conditions as good or better than preconstruction.
- The Applicant or its contractor and EFSEC staff would meet prior to final site plan approval to outline steps for minimizing construction traffic impacts, including conflicts if state-imposed roadway restrictions could affect transporter routes.
- The Applicant or its contractor would provide advance notification to adjacent landowners and farmers through
  mailing, informal meeting, open house, or other similar methods when construction would take place in the
  vicinity of their homes and farms to help minimize access disruptions.
- All construction vehicles would yield to school-related vehicles (e.g., school buses) and would lower their speed when approaching a school bus or bus stop along the transporter route.

- Temporary advanced warning and proper roadway signage would be placed on major state and Benton County roads to warn motorists of potential construction-related vehicles turning.
- Carpooling among the construction workers would be encouraged to reduce traffic volume to and from the Project site.
- Detour plans and warning signage would be provided in advance of any planned traffic disturbances.
- Flaggers would be employed as necessary to direct traffic when large equipment is exiting or entering public roads to minimize the risk of accidents. Should the Applicant or its construction contractor receive notice during Project construction of transportation events (e.g., WSDOT or Benton County transportation projects, roadway incidents, other traffic events) that give rise to a safety concern, the Project construction manager would review the CTMP in coordination with the applicable agency and address additional safety measures, including flagging, as may be appropriate for the situation.
- If a lane closure must occur, adequate signage for potential detours or possible delays would be posted.
- Advance notification would be provided to emergency providers and hospitals when public roads may be partially or completely closed.
- Emergency vehicles would be given the right-of-way as required by local, state, and federal requirements.
- Site access roads and an entrance driveway to the O&M facilities on site would be constructed to service truck movements of legal weight and provide adequate sight distance.
- Traffic control requests would be coordinated through the WSDOT traffic engineer and the Benton County Public Works Department abiding by seasonal County Road restrictions.
- A haul and approach route would be developed in coordination with the appropriate jurisdictional authorities.
- Permanent private Project access roads would be maintained by the Applicant for the life of the Project.
- Tracked vehicles and heavy trucks would be restricted to approved transporter roads to prevent damage to the surface and base of Benton County roads.
- After construction, all-weather access roads (including graveled roads), suitable to handle emergency
  equipment, would be provided to within 150 feet of any built structure or surface activity area.

### Safety Procedures

#### Personal Protective Equipment

All employees are required to wear Personal Protective Equipment (PPE) including subcontractors in charge of road modification and traffic control. PPE includes highlighted vest, protective footwear, eye protections, helmet, sun protection, respiratory devices and ear plugs if needed (etc) on the worksite.

#### Facility Equipment

All plant equipment shall meet statutory requirements and have the required registration, licenses or certifications where needed. All mobile equipment shall be fitted with suitable reversing alarms. All mobile plant and vehicles shall be fitted with a pair of flashing yellow lamps.

#### Incident/Accident Procedures

In the event of an accident or incident, whether involving traffic or road users, all work shall be ended, and traffic shall be stopped and rerouted as necessary to avoid further incidents. Safety, health, and environmental personnel, (ESH) should be immediately informed.

Broken down vehicles and vehicles involving in minor non-injury crashes shall be temporarily moved to the shoulder as soon as possible after details of the crash locations have been gathered and noted to maintain traffic flow. Details of all incidents and accidents shall be reported to the Site safety supervisor and Project Manager using the incident report form. Traffic incidents occurring due to the works that occur on public roads will be communicated to local authorities.

### Responsibilities

#### Project Management

Project Management has the ultimate responsibility to ensure that the presented plan is implemented per approved procedures and the designated personnel will be monitoring the execution of the work to prevent any kind of injury or property damage to employees, contractors, sub-contractors, road users and all members of the public.

#### Safety management and supervision

The safety management and supervision team will ensure all site personnel are fully aware of the job location and that traffic controllers are correctly trained. Personnel protective equipment (PPE) will be enforced and always worn during the work period.

#### Construction management

Construction management should ensure that the traffic plan is being followed by everyone and contractor will supply sufficient controllers to continuously supervise the road improvement work and traffic flow to avoid delay and traffic congestion.

Construction management will be responsible for coordination-with safety personnel to ensure compliance with the CTMP and applicable procedures before, during and after the work completion. It is important to remind the subcontractor and its personnel to follow the safety rules and execution process to avoid any incidents or delay. Construction management will report to the project manager on progress and will support engineering on the design.

#### Construction personal

Construction personal should be trained and execute the work per the project procedures and the project schedule.

#### Contractor

Contractor will ensure that all signs, barricades, and equipment to control the traffic are properly installed and functional before, during and after work hours.

#### Contractor Project Manager

Contractor project manager shall:

- Ensure all traffic control measures of this TMP are placed and maintained in accordance with this plan and the relevant Acts, Codes, Standards and Guidelines
- Ensure suitable communication and consultation with the owner is maintained at all times
- Ensure inspections of the Traffic Controls are undertaken in accordance with the TMP, and results recorded.
- Review feedback from construction and safety and take action to amend the traffic control measures as approved by the state authority representative.

#### Site Supervisors

Site supervisors on the jobsite will ensure that all proper documents are correctly filled out and daily safety meetings are conducted prior to start of work. Moreover, they will follow the correct work practices as required by WSDOT outside the project boundaries and project procedures inside the project area.

Site supervisors will be responsible for overseeing the day-to-day activities performed by subcontractor. Ensure traffic control measures are implemented and maintained by the subcontractor. Moreover, undertake and submit the required inspection and evaluation reports to construction representative to take appropriate action to correct unsafe conditions.

#### **WSDOT**

WSDOT representatives will support and advise on the progress of the work with site safety and supervisory personnel on work located outside the construction site limits. WSDOT may direct erection, relocation or removal of signs or devices, which, in the opinion of the WSDOT Representative, are not in accordance with the TMP and do not supply sufficient safety for road users.

#### Worksite Access

#### Pedestrian Access

There is no specific facility or service nearby that would include pedestrian on the roads neither increase normal use of the road facilities by people with disabilities and other vulnerable road users. Moreover, there are no schools in the vicinity of the worksite, and no significant numbers of children are expected.

#### Cyclists and Bikers

No cyclist will be present on work site. However, we can expect cyclists along local roadways throughout the study area.

#### Site Access for Works Vehicles

Construction vehicles entering and exiting the traffic stream shall be mindful of the conditions that may affect the safety of these movements. Vehicles shall reduce the speed and signal their intention by indicator to leave / enter the traffic stream. If the vehicle is disabled, driver should switch on the hazard lights.

#### **Emergency Vehicle Access**

At all times when employees are on job location, the Site Supervisor will take whatever action is practicable to assist emergency vehicles, tow trucks and/or service vehicles to gain access to crash or vehicle breakdown sites which are causing or have the potential to cause an obstruction to traffic flow or imperil the safety of road users.

#### **Public Transport**

There is no public transport that will be affected by the works.

#### Access to Adjoining Development/Properties

No properties will be impacted by the proposed work.

#### **School Crossings**

There are no school crossings on the worksite. No impacts expected.

#### Heavy and Oversize Vehicles and Loads

Any heavy or oversized vehicle used during construction activities will comply with the Washington state law and applicable WSDOT requirements.

#### Railroad Crossings

Railroad crossings within the vicinity of the project and along the assumed transportation routes for materials were analyzed and listed below.

- Crossing 927487A, where train tracks cross over Webber Canyon Road
- Crossing 928191E, where train tracks cross under I-82 near West Clearwater Avenue
- Crossing 928192L. where train tracks cross Dallas Road at grade
- Crossing 966466M, where train tracks cross under eastbound I-82 near the Lewis and Clark Trail Highway
- Crossing 966467U, where train tracks cross under westbound I-82 near the Lewis and Clark Trail Highway

All crossings except Crossing 928192L are located above (overpass) or under (underpass) the transport route. Crossing 928192L, where train tracks cross Dallas Road is a grade crossing, meaning that the crossing occurs at the same grade as other traffic. No Truck haul routes and limited worker journey to work routes are expected to use Dallas Road.

### Contingency Planning

#### Road Crash or Vehicle Breakdown within Site

In the event of road crash within the work area that may impact services requiring access to a crash site that area will be cleared to allow access. On-site traffic controllers/ ES&H will be equipped with mobile communications to advise and/or liaise with emergency services to ensure a prompt response is provided.

There will be accredited First Aid personnel on site to assist where required.

#### Serious Injury or Fatality

In the case of serious injury or fatality occurring within the traffic control zone, all work shall cease immediately, machinery and vehicles turned off and the area cleared of personnel as soon as possible. Traffic Controllers (and other personnel if necessary) shall be deployed immediately to ensure no traffic or other road users approach the area.

Emergency services shall be notified of the incident and all road workers and traffic management personnel shall preserve the scene leaving everything in situ, until direction is given by ES&H and law reinforcement.

All site personnel shall be briefed on control procedures covering incidents and crashes that result in serious injury or fatalities.

#### **Emergency Contacts**

In the event of an emergency the following relevant authorities must be contacted and advised of nature of works, location, type of emergency and contact details for the site supervisor.

Emergency Service	E-mail/Website	Phone (Emergency)
Kennewick Police Department	https://www.go2kennewick.com/386/Police	911
Richland Police Department	https://www.ci.richland.wa.us/departments/police- services	911
Benton County Emergency Services	https://www.bces.wa.gov/	(509) 628-2600
Benton County Fire District 1	https://www.bentonone.org/	911 or (509) 737-0911

#### **Recommended Mitigation Measures**

EFSEC has identified the following additional and modified mitigation measures that could be required by EFSEC, but may also involve the participation of other parties, for the Project to avoid and/or minimize potential impacts on transportation. EFSEC would work with the identified parties to facilitate cooperation in implementing this mitigation measure:

**TR-1**<sup>1</sup>: To ensure safe practices during the transportation of materials during construction and decommissioning, the load movement team would review the procedures to be followed if the load should become lodged at a crossing and would review the emergency contact numbers for each crossing daily—that is, before starting travel for the day.

<sup>&</sup>lt;sup>1</sup> Identifier of numbered mitigation item for Transportation

**TR-2:** To mitigate potential collisions at train crossings, the Applicant would work with WSDOT and Operation Lifesaver to provide train safety presentations to employees and contractors to increase knowledge regarding train safety, including train track crossings.

**TR-3:** To ensure that no changes have occurred since the traffic analysis originally provided prior to construction, a third-party engineer would provide a traffic analysis prior to decommissioning. The traffic analysis would evaluate all modes of transportation (e.g., waterways, rail, roads, etc.) used for the movement of people and materials during decommissioning via the haul route(s) in Washington State.

**TR-4:** To ensure that no changes have occurred since the route survey originally provided prior to construction, all railroad crossings and grade changes would be included in a route survey performed by a third-party engineer with the Washington Utilities and Transportation Commission participating to determine if current traffic control systems at crossings are appropriate or if additional mitigation is needed prior to decommissioning. The route survey would include anticipated traffic counts. Since this measure would require the participation of other agencies to be implemented, it cannot be considered fully effective mitigation for the purpose of this analysis.

**TR-5:** The analysis of impacts from decommissioning is based on existing laws and regulations at the time when the ASC was submitted to EFSEC. To ensure that no changes have occurred to laws and regulations used in this analysis, the Applicant would consult with WSDOT and Benton County on the development of a decommissioning-stage Traffic and Safety Management Plan, prior to decommissioning. The Traffic and Safety Management Plan must include a safety analysis of the WSDOT-controlled intersections (in conformance with the WSDOT Safety Analysis Guide) and recommend mitigation or countermeasures where appropriate. The analysis would review impacts from decommissioning traffic and be submitted to WSDOT for review and comment prior to decommissioning activities.